

Site Servicing and Stormwater Management Brief – Mooney's Bay -729 Ridgewood Avenue, Ottawa, ON

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Prepared for:

Brigil Homes

Prepared by:

Stantec Consulting Ltd.

Revision	Description	Author		Quality Check	
0	1 st submission	A. Paerez	09/06/2021	K. Kilborn	09/06/2021



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Prepared by _________(signature)

Ana M. Paerez, P. Eng.

for the Reviewed by ________(signature)

Kris Kilborn

Table of Contents

1.0 1.1	INTRODUCTION AND OBJECTIVE
2.0	REFERENCES
3.0	WATER DISTRIBUTION
4.0 4.1	SANITARY SEWER4.1SANITARY SEWER DESIGN CRITERIA4.2
5.0 5.1 5.2 5.3 5.4	STORMWATER MANAGEMENT5.1OBJECTIVES5.1EXISTING CONDITIONS5.1SWM CRITERIA AND CONSTRAINTS5.1STORMWATER MANAGEMENT DESIGN5.25.4.1Design Methodology5.4.2Water Quantity Control5.4.3Allowable Release Rate5.4.4Storage Requirements5.4.5Uncontrolled Area5.4.6Results5.4.7Water Quality Control
6.0	GRADING AND DRAINAGE
7.0	UTILITIES
8.0	EROSION CONTROL DURING CONSTRUCTION
9.0 9.1 9.2 9.3	GEOTECHNICAL INVESTIGATION AND ESA9.1GEOTECHNICAL INVESTIGATION9.1PHASE ONE ENVIRONMENTAL SITE ASSESSMENT9.3PHASE TWO ENVIRONMENTAL SITE ASSESSMENT9.4
10.0 10.1 10.2 10.3 10.4 10.5 10.6	CONCLUSIONS10.1WATER SERVICING10.1SANITARY SERVICING10.1STORMWATER SERVICING10.1GRADING10.1UTILITIES10.2APPROVAL / PERMITS10.2

LIST OF TABLES

Table 1: Estimated Water Demands	3.	1
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Table 2: Bound	arv Conditions	3.2
Table 3: Estimat	ed Wastewater Peak Flow	4.1
Table 4: Target	Release Rate	5.3
Table 5 – 2-Yea	r Summary of Roof Controls	5.3
Table 6 - 100-Ye	ear Summary of Roof Controls	5.4
Table 7: Peak C	Controlled (Cistern) 2-Year Release Rate	5.4
Table 8: Peak C	Controlled (Cistern) 100-Year Release Rate	5.4
Table 9: Peak U	ncontrolled (Non-tributary) 2-Year Release Rate	5.4
Table 10: Peak	Uncontrolled (Non-tributary) 100-Year Release Rate	5.5
Table 11: Estimo	ated Discharge from Site (2-Year)	5.5
Table 12: Estimo	ated Discharge from Site (100-Year)	5.5
Table 13: Recor	mmended Parking Structure – Parking Areas	9.1
Table 14: Recor	mmended Parking Structure – Local Roadways, Access Lanes and	
Heavy	Vehicle Parking	9.2
LIST OF FIGURES Figure 1: Site Lo	cation	1.2
LIST OF APPEND	ICES	
APPENDIX A	WATER CALCULATIONS	A.1
APPENDIX B	PROPOSED SITE PLAN	B.1
APPENDIX C	SANITARY SEWER CALCULATIONS	C.1
APPENDIX D	STORMWATER MANAGEMENT CALCULATIONS	D.1
APPENDIX E	GEOTECHNICAL REPORT AND ENVIRONMENTAL SITE ASSESSMENT	E.1
APPENDIX F	CITY OF OTTAWA SERVICING STUDY CHECKLIST	F.1
APPENDIX G	BACKGROUND CORRESPONDENCE	G.1



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Introduction and Objective

1.0 INTRODUCTION AND OBJECTIVE

Stantec Consulting Ltd. has been retained by Brigil Homes to prepare the following site servicing and stormwater management (SWM) brief to satisfy the City of Ottawa Site Plan Control Application process. The site is located at 729 Ridgewood Avenue, generally surrounded by residential and institutional developments in the city of Ottawa (see **Figure 1** below).

The site proposed for re-development measures 1.33 ha. The proposed re-development area was previously occupied by a commercial site and associated paved parking areas. The proposed development consists of five (5) multi-storey buildings with one of them consisting of commercial land uses on the ground floor and apartment buildings on the floors above. The five buildings will surround a common courtyard area, surface parking areas and an access road and will provide a total of 387 residential units, 856 m² of commercial area, two levels of underground parking and associated access and servicing infrastructure. The proposed buildings will include retail space within the ground floor of Building V, 254 one-bedroom apartments, 118 two-bedroom apartments, 15 three-bedroom apartments, underground parking and a bicycle storage room. The proposed site plan has been included in **Appendix B**.

Introduction and Objective



Figure 1: Site Location

1.1 OBJECTIVE

This site servicing and SWM brief has been prepared to present a servicing scheme that is free of conflicts and which utilizes the existing infrastructure as obtained from available as-built drawings and in consultation with City of Ottawa staff. Infrastructure requirements for water supply, sanitary and storm sewer services are presented in this report.

Criteria and constraints provided by the City of Ottawa have been used as a basis for the conceptual servicing design of the proposed development. Specific elements and potential development constraints to be addressed are as follows:

- Prepare a grading plan in accordance with the proposed site plan and existing grades.
- Storm Sewer Servicing
 - Define major and minor conveyance systems in conjunction with the proposed grading plan
 - Determine the stormwater management storage requirements to meet the allowable release rate for the site



Introduction and Objective

- Coordinate with mechanical engineer to convey drainage from roof tops, amenity areas, and private terrace areas to the internal cistern and discharge to the proposed storm service lateral at the allowable release rate.
- Define and size the proposed storm service lateral that will be connected to the existing 300 mm diameter storm sewer on Ridgewood Avenue.
- Wastewater Servicing
 - Define and size the sanitary service lateral which will be connected to the existing 225 mm diameter sanitary sewer on Ridgewood Avenue.
- Water Servicing
 - Estimate water demands to characterize the proposed feed for the development which will be serviced from the existing 305 mm diameter watermain on Ridgewood Avenue.
 - Watermain servicing for the development is to be able to provide average day and maximum day (including peak hour) demands (i.e., non-emergency conditions) at pressures within the acceptable range of 50 to 70 psi (350 to 480 kPa).
 - Under fire flow (emergency) conditions, the water distribution system is to maintain a minimum pressure greater than 20 psi (140 kPa).

The accompanying drawings included in the back of this report illustrate the proposed internal servicing scheme for the site.

References

2.0 **REFERENCES**

The following background studies have been referenced during the preliminary servicing design of the proposed site:

- City of Ottawa Design Guidelines Water Distribution, City of Ottawa, July 2010
- City of Ottawa Sewer Design Guidelines, City of Ottawa, October 2012
- Technical Bulletin ISDTB-2014-01, City of Ottawa, February 2014
- Technical Bulletin ISTB-2018-01, City of Ottawa, March 21, 2018
- Technical Bulletin ISTB-2018-02, City of Ottawa, March 21, 2018
- Technical Bulletin ISTB-2018-03, City of Ottawa, March 21, 2018
- Technical Bulletin PIEDTB -2016-01, City of Ottawa, September 6, 2016
- Geotechnical Investigation Proposed Multi-Storey Building 729 Ridgewood Avenue Ottawa, Paterson Group, September 15, 2020
- Sawmill Creek Subwatershed Study Update, CH2MHILL, May 2003
- Phase One Environmental Site Assessment 729 Ridgewood Avenue, Ottawa, Ontario, Lopers and Associates, July 27, 2020
- Phase Two Environmental Site Assessment 729 Ridgewood Avenue, Ottawa, Ontario, Lopers and Associates, August 14, 2020

Water Distribution

3.0 WATER DISTRIBUTION

The proposed development is located in Pressure Zone 2W2C of the City of Ottawa's Water Distribution System. The proposed development will be serviced through the existing 305 mm diameter watermain on Ridgewood Avenue as shown on the Site Servicing Plan (see **Drawing SSP-1**).

The proposed development encompasses four residential buildings, one mixed-use building with retail spaces on the ground floor and residential units on the higher floors, two levels of underground parking, landscaped amenity areas, surface parking areas and an access road. Tower I will consist of a 15-storey residential building with 73 one-bedroom apartments, 58 two-bedroom apartments and 10 three-bedroom apartments. Tower II is attached to Tower I and will consist of a 6-storey residential building with 36 one-bedroom apartments and 12 two-bedroom apartments. Tower III and Tower IV will each 4-storeys residential buildings with 40 one-bedroom apartments and 14 two-bedroom apartments. Tower V will consist of a 6-storey mixed-use building with 856 m² of commercial space on the ground floor and 65 one-bedroom apartments, 20 two-bedroom apartments and 5 three-bedroom apartments on the higher floors. The proposed site plan is included in **Appendix B**.

Water demands were calculated using the City of Ottawa Water Distribution Guidelines (July 2010) to determine the typical operating pressures to be expected at the proposed development (see detailed calculations in **Appendix A**). A daily rate of 350 L/cap/day has been applied for the population of the proposed site. The average daily (AVDY) residential demand was estimated for an occupancy of 1.4 persons per unit for a one-bedroom apartment, 2.1 persons per unit for a two-bedroom apartment and 3.1 persons per unit for a three-bedroom apartment. Water demands for the proposed retail space were estimated based on 28,000 L/ha/day. Maximum day (MXDY) demands were determined by multiplying the AVDY demands by a factor of 2.5 for residential areas and by a factor of 1.5 for commercial areas. Peak hourly (PKHR) demands were determined by multiplying the MXDY demands by a factor of 2.2 for residential areas and by a factor of 1.8 for commercial areas. The estimated demands are summarized in **Table 1**.

	Population/Area	AVDY (L/s)	MXDY (L/s)	PKHR (L/s)
Residential	650 persons	2.63	6.58	14.48
Commercial	498 m ²	0.28	0.42	0.75
Total Site:		2.91	7.00	15.23

Table	1:	Estimated	Water	Demands
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Given that the proposed site is a private site and that the proposed buildings will be serviced through direct connection to the mainline on Ridgewood Avenue, fire flow requirement



Water Distribution

calculations for each building have been completed based on the Ontario Building Code (OBC) methodology. The OBC Fire Flow Calculations are included in **Appendix A.** The minimum required fire flow for this development has been determined to be 150 L/s (9,000L/min). Based on preliminary information obtained from the architect, non-combustible construction was assumed for Tower I & II and Tower V, while combustible construction was used for Tower III and Tower IV. Sprinkler systems conforming to NFPA13 will be provided within all buildings.

Table 2 outlines the boundary conditions provided by the City of Ottawa on April 27, 2021.

	Connection to Ridgewood Av.
Min. HGL (m)	123.7
Max. HGL (m)	131.9
Max. Day + Fire Flow (117 L/s)	125.6

Table 2: Boundary Conditions

1. Fire flow requirements for the proposed site have been revised to 150 L/s and as such, revised boundary conditions will be requested to the City and included in subsequent submissions.

The desired normal operating objective pressure range as per the City of Ottawa 2010 Water Distribution Design Guidelines is 350 kPa (50 psi) to 480kPa (70 psi) and no <u>less than 275kPa (40</u> <u>psi)</u> at ground elevation. Furthermore, the maximum pressure at any point in the water distribution should not exceed 100 psi as per the Ontario Building/Plumbing Code; pressure reducing measures are required to service areas where pressures <u>greater than 552kPa (80 psi)</u> are anticipated.

The ground elevation along Ridgewood Avenue, approximately where the proposed building services are to be connected is approximately 82.43 m. With respect to the peak hour flow conditions, the resulting boundary condition HGL of 123.7 m corresponds to a peak hour pressure of 407 kPa (59 psi) at ground elevation. Since the proposed development consists of 4-storey, 6-storey and 15-storey towers, and an additional 34 kPa (5 psi) for every additional storey over two storeys is required to account for the change in elevation head and additional head loss, it is expected that booster pumps will be required for the 6-storey and 15-storey towers to maintain an acceptable level of service on the higher floors.

A maximum pressure check can be conducted using the buildings' lowest finished floor elevation (~83.45 m for Tower IV) and the maximum boundary condition HGL of 131.9 m. This results in a pressure of 476 kPa (69 psi). This value is below the limit of 80 psi for which pressure reducing valves would be required.

In regard to available fire flow, boundary conditions provided by the City confirm that a flow rate of 7,000 L/min (117 L/s) would have a residual pressure of 421 kPa (61 psi) on Ridgewood Avenue (ground elevation of 82.43 m). Based on the available boundary conditions, a fire flow rate of 7,000 L/min is achievable within the watermain at the connection location while maintaining a residual pressure above the minimum allowable pressure of 138kPa (20 psi).



Water Distribution

However, as mentioned above, fire flow requirements for the proposed development have been revised to 9,000 L/min (150 L/s) and as such, revised boundary conditions will be requested, and the resulting residual pressure will be revised in subsequent submissions.

There are two hydrants along Ridgewood Avenue in close proximity to the site: one hydrant approximately 108 m west of the site and a second hydrant approximately 65 m east of the site. It is expected that approximately 7,000 L/min can be provided from the existing hydrants on Ridgewood Avenue to Towers I & II which are located at the northern edge of the proposed development and as a result, it is proposed to install a hydrant on-site as shown on **Drawing SSP-1** to achieve the fire flow requirements.

In conclusion, based on the boundary conditions available, the 305 mm diameter watermain on Ridgewood Avenue provide adequate fire flow capacity. In order to meet the City water supply objective that limits a single feed to 50 m³/d during basic day demands, two connections are required to service the proposed development. The service connections will be capable of providing anticipated demands at acceptable pressures to the lower storeys but will require booster pumps to maintain minimum required pressures for the higher floors of the proposed 6-storey and 15-storey towers.

Sanitary Sewer

4.0 SANITARY SEWER

As illustrated on Drawing SSP-1, sanitary servicing for the proposed development will be provided through a proposed 200 mm diameter service lateral connecting to the existing 225 mm diameter sanitary sewer running east on Ridgewood Avenue.

The proposed 1.33 hare-development area will consist of five (5) multi-storey buildings with two levels of underground parking, a common courtyard area, surface parking areas and an access road. The proposed buildings will include retail space within the ground floor of Tower V (856 m²), 254 one-bedroom apartments, 118 two-bedroom apartments, 15 three-bedroom apartments, underground parking and a bicycle storage room. The anticipated wastewater peak flow generated from the proposed development is summarized in **Table 3** below while a sanitary sewer design sheet is included in **Appendix C**.

	Residential/	Commercial Pe	ak Flows			
	# of Units/Area	Population	Peak Factor	Peak Flow (L/s)	Infiltration Flow (L/s)	Total Peak Flow (L/s)
Residential	387 units	650	3.9	8.24	0.44	0.70
Commercial	0.086 ha	N/A	1.5	0.04	0.44	0.72

Table 3: Estimated	Wastewater	Peak Flow
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1. Average residential flow based on 280 L/p/day

 Peak factor for residential units calculated using Harmon's formula
 Apartment population estimated based on 1.4 persons/unit for one-bedroom apartments, 2.1 persons/unit for two-bedroom apartments and 3.1 persons/unit for three-bedroom apartments

4. Commercial peak flows estimated based on 28,000 L/ha/day

5. Infiltration flow based on 0.33 L/s/ha.

The proposed sewage peak flows were provided to City of Ottawa staff to conduct a capacity analysis of the sanitary sewer system in the vicinity of the site and confirmation was received that there are no concerns with respect to adding the proposed sanitary peak flows to the existing sewers on Ridgewood Avenue (see correspondence in **Appendix C**).

Detailed sanitary sewage calculations are included in Appendix C. A backflow preventer will be required for the proposed building in accordance with the Ottawa Sewer Design Guidelines and will be coordinated with building mechanical engineers.

All underground parking drains should be connected to the building's internal plumbing. A sump pump will be required to drain the underground parking levels to the existing sanitary sewer on Ridgewood Avenue.

Sanitary Sewer

4.1 SANITARY SEWER DESIGN CRITERIA

As outlined in the City of Ottawa Sewer Design Guidelines and the MECP's Design Guidelines for Sewage Works, the following criteria were used to calculate estimated wastewater flow rates and to size the sanitary sewer lateral:

- Minimum Velocity 0.6 m/s (0.8 m/s for upstream sections)
- Maximum Velocity 3.0 m/s
- Manning roughness coefficient for all smooth wall pipes 0.013
- 1.4 persons/one-bedroom apartment
- 2.1 persons/two-bedroom apartment
- 3.1 persons/three-bedroom apartment
- 28,000 L/ha/day for commercial areas
- Harmon's Formula for Residential Peak Factor Max = 4.0
- Commercial Peak Factor of 1.5
- Extraneous Flow Allowance 0.33 L/s/ha (conservative value)
- Manhole Spacing 120 m
- Minimum Cover 2.5 m

Stormwater Management

5.0 STORMWATER MANAGEMENT

5.1 OBJECTIVES

The objective of this stormwater management plan is to determine the measures necessary to control the quantity of stormwater released from the proposed development to the required levels and to provide sufficient detail for approval and construction.

5.2 EXISTING CONDITIONS

The proposed re-development area was previously occupied by two slab on grade commercial buildings, associated access roads and paved parking areas. A former mechanics garage which has been recently demolished was located on the east portion of the site. The previous site was serviced through the existing 300 mm diameter storm sewer on Ridgewood Avenue (see **Drawing EX-1**).

City of Ottawa staff recommended stormwater management peak flows from the proposed site be restricted to the 2-year storm with a runoff coefficient based on the Sawmill Creek Subwatershed Study (CH2MHILL, May 2003) and a minimum time of concentration (Tc) of 10 minutes (see pre-consultation meeting notes in **Appendix G**).

The Sawmill Creek Subwatershed Study (CH2MHILL, May 2003) assessed the hydrology and fluvial geomorphology of the creek under ultimate development conditions which considered the proposed site as a commercial site per the development conditions at the time of the report and as such, target peak outflows from the proposed development have been estimated using a runoff coefficient (C) of 0.80. The time of concentration of the existing development was assess assuming a storm sewer network as per the storm sewer design sheet included in **Appendix D**, which resulted in a Tc of 12.38 minutes.

The proposed 729 Ridgewood Avenue re-development encompasses approximately 1.33 ha of land, which assuming a time of concentration (Tc) of 12.38 minutes results in an allowable peak outflow of $Q = 2.78 \times C \times I \times A = 2.78 \times 0.80 \times 68.73 \times 1.33 = 203.3 \text{ L/s.}$

5.3 SWM CRITERIA AND CONSTRAINTS

The stormwater management criteria for the proposed site are based on City of Ottawa Sewer Design Guidelines (2012) and on consultation with City of Ottawa Staff. The following summarizes the criteria used in the preparation of this stormwater management plan:

• Control post development peak flows up to the 100-year storm to the 2-year runoff with a runoff coefficient (C) of 0.80 which corresponds to **203.3 L/s**.



Stormwater Management

- Size storm sewers using an inlet time of concentration (Tc) of 10 minutes
- Post-development runoff coefficient (C) value based on proposed impervious areas as per site plan drawing (see **Appendix D**)
- Provide 'Enhanced' level of quality control (i.e., 80% TSS removal)

5.4 STORMWATER MANAGEMENT DESIGN

The proposed 1.33 ha re-development area consists of five (5) multi-storey buildings with two levels of underground parking, a common courtyard area, surface parking areas, and associated access and servicing infrastructure. The imperviousness of the proposed site is 63% (C = 0.64).

The SWM strategy for the site is to use roof storage wherever possible and to provide a cistern in the underground parking to attenuate peak flows in the downstream system to the allowable release rate of 203.3 L/s. The proposed buildings will capture storm drainage through a combination of uncontrolled roof drains (Tower II), controlled roof drains (Towers I, III, IV and V), a trench drain that will capture runoff from the roundabout, amenity area drains and a trench drain at the end of the parking ramp that will direct peak flows to the cistern located in the underground parking for attenuation. Controlled peak flows from the cistern will be pumped from Tower 4 and ultimately discharged into the existing 300 mm diameter storm sewer on Ridgewood Avenue. Coordination with the mechanical consultant is on-going to size the internal plumbing system and the underground cistern.

The proposed site plan, drainage areas and proposed storm sewer infrastructure are shown on **Drawing SD-1**.

5.4.1 Design Methodology

The intent of the stormwater management plan presented herein is to mitigate any negative impact that the proposed development could have on the existing drainage and storm sewer infrastructure, while providing adequate capacity to service the proposed buildings, parking and access areas. The proposed stormwater management plan is designed to detain runoff on the roofs of buildings 1, 3, 4 and 5 and in an underground cistern to ensure that peak flows after construction from the proposed re-development area will not exceed the target release rate for the site.

A portion of the site could not be graded to enter the building's internal plumbing system and as such it will sheet drain uncontrolled. Runoff from this uncontrolled area is included in the overall site discharge calculations.

Stormwater Management

5.4.2 Water Quantity Control

The Modified Rational Method was used to assess the quantity and volume of runoff generated during post development conditions. The site was subdivided into subcatchments (subareas) tributary to storm sewer inlets, as defined by the location of catchbasins / inlet grates and used in the storm sewer design (see **Appendix D**). A summary of subareas and runoff coefficients is provided in **Appendix D**, and **Drawing SD-1** indicates the stormwater management subcatchments.

5.4.3 Allowable Release Rate

Site discharge rates up to the 100-year storm event are to be restricted to the 2-year storm event with a runoff coefficient ('C' value of 0.80) as outlined below in **Table 4**.

Rational Method 'C'	Area (ha)	Time of Concentration (min)	Q _{Target} (L/s)
0.80	1.33	12.38	203.3

Table 4: Target Release Rate

5.4.4 Storage Requirements

The site requires quantity control measures to meet the stormwater release criteria. Therefore, it is proposed to use roof storage and underground storage in a cistern located in the underground parking. Stormwater management calculations are provided in **Appendix D**.

5.4.4.1 Roof Storage

The roof of the proposed Building 2 will consist of a partial green roof with public amenity areas and as such it has been assumed to be uncontrolled (i.e., no roof storage). The following calculations assume the roofs of Buildings 1, 3, 4 and 5 will be equipped with standard Watts Modell R110 Accuflow Single Notch Roof Drains and that 80% of the roof areas are usable. **Table 5** and **Table 6** summarize the 2-year and 100-year roof release rates and storage requirements respectively.

Area ID	# of Drains	Usable Roof Area (m²)	Depth (mm)	Discharge (L/s)	Drawdown Time (h)	Storage Volume (m³)
ROOF1	4 drains – 75% open	720	93	4.2	0.7	9.1
ROOF3	4 drains – 75% open	880	97	4.3	0.9	12.1
ROOF4	4 drains – 50% open	880	100	3.8	1.1	13.0
ROOF5	6 drains – 75% open	1,280	96	6.4	0.9	17.3

Table 5 – 2-Year Summary of Roof Controls

Stormwater Management

Area ID	# of Drains	Usable Roof Area (m²)	Depth (mm)	Discharge (L/s)	Drawdown Time (h)	Storage Volume (m³)
ROOF1	4 drains – 75% open	720	141	6.0	1.7	30.8
ROOF3	4 drains – 75% open	880	145	6.1	2.3	40.4
ROOF4	4 drains – 50% open	880	149	5.0	2.8	43.6
ROOF5	6 drains – 75% open	1,280	145	9.2	2.2	58.2

Table 6 – 100-Year Summary of Roof Controls

5.4.4.2 Subsurface Storage

It is proposed to detain stormwater within a 52 m³ cistern below grade with a maximum controlled release rate of 177.3 L/s to the gravity service provided. The modified rational method was used to determine the peak volume requirement for the cistern. Site drainage areas are captured into the building plumbing directed to the cistern for additional control.

 Table 7 and Table 8 summarize the flow rates and storage from the cistern for the 2 and 100-year events, respectively.

Table 7: Peak Controlled (Cistern) 2-Year Release Rate

Area ID	Area (ha)	Runoff 'C'	Q _{release} (L/s)	V _{stored} (m ³)
ROOF1, ROOF2, ROOF2B, ROOF3, ROOF4, ROOF5, TANK1, TANK2, TANK3, TANK4, RAMP	1.18	0.69	100.51	0.00

Table 8: Peak Controlled (Cistern) 100-Year Release Rate

Area ID	Area (ha)	Runoff 'C'	Q _{release} (L/s)	V _{stored} (m ³)
ROOF1, ROOF2, ROOF2B, ROOF3, ROOF4, ROOF5, TANK1, TANK2, TANK3, TANK4, RAMP	1.18	0.86	177.25	49.8

5.4.5 Uncontrolled Area

A portion of the site around the buildings (see **Drawing SD-1**) could not be graded to enter the building's internal plumbing system and as such it will sheet drain uncontrolled. **Table 9** and

Table 10 summarize the 2 and 100-year uncontrolled release rates from the proposeddevelopment.

Area ID	Area (ha)	Runoff 'C'	Tc (min)	Qrelease (L/s)
UNC-1	0.15	0.28	10	9.0

Table 9: Peak Uncontrolled (Non-tributary) 2-Year Release Rate

Stormwater Management

		(,,	
Area ID	Area (ha)	Runoff 'C'	Tc (min)	Q _{release} (L/s)
UNC-1	0.15	0.35	10	26.1

Table 10: Peak Uncontrolled (Non-tributary) 100-Year Release Rate

5.4.6 Results

 Table 11 and Table 12 demonstrate that the proposed stormwater management plan provides adequate attenuation storage to meet the target peak outflow for the site.

Area Type	Q _{release} (L/s)	Target (L/s)
Controlled Cistern Discharge	100.5	
Uncontrolled Sheet Flow	9.0	203.3
Total	109.5	

Table 11: Estimated Discharge from Site (2-Year)

Table 12: Estimated Discharge from Site (100-Year)

Area Type	Q _{release} (L/s)	Target (L/s)
Controlled Cistern Discharge	177.3	
Uncontrolled Sheet Flow	26.1	203.3
Total	203.4	

5.4.7 Water Quality Control

The storm sewers on Ridgewood Avenue ultimately discharge into Sawmill Creek less than 1 km downstream of the proposed development. The Sawmill Creek watershed is within the Rideau Valley Conservation Authority (RVCA) that confirmed through correspondence that given that the proposed development consists of more than 6 surface parking spaces, onsite water quality treatment will be required to provide 'Enhanced' level of quality control, which is equivalent to 80% Total Suspended Solids (TSS) removal.

The online sotware available for Stormceptor sizing was used to size a unit capable of providing the required TSS removal. A drainage area of 0.72 ha (i.e., surface parking and access areas) with an imperviousness of 50% resulted in a Stormceptor EF4 which can provide 80% TSS removal. It should be noted that the Stormceptor Unit has been provided as an example and that an approved equivalent unit can be used during construction subject to approval. Stormceptor sizing information has been provided in **Appendix D**.

Stormwater Management

Grading and Drainage

6.0 GRADING AND DRAINAGE

The proposed re-development site measures approximately 1.33 ha in area. A detailed grading plan (see **Drawing GP-1**) has been provided to satisfy stormwater management requirements and coordinated to accommodate architectural constraints.

The subject site maintains emergency overland flow routes to the back and to Ridgewood Avenue as depicted on **Drawings GP-1** and **SD-1**.

Utilities

7.0 UTILITIES

All utilities (Hydro Ottawa, Bell Canada, Rogers Ottawa, and Enbridge Gas) have existing plants in the area. The site will be serviced through connection to these existing services. Detailed design of the required utility services will be further investigated as part of the composite utility planning process following design circulation.

Erosion Control During Construction

8.0 **EROSION CONTROL DURING CONSTRUCTION**

Erosion and sediment controls must be in place during construction. The following recommendations to the contractor will be included in contract documents.

- 1. Implement best management practices to provide appropriate protection of the proposed drainage system and the receiving water course(s).
- 2. Limit extent of exposed soils at any given time.
- 3. Re-vegetate exposed areas as soon as possible.
- 4. Minimize the area to be cleared and grubbed.
- 5. Protect exposed slopes with plastic or synthetic mulches.
- 6. Provide sediment traps and basins during dewatering.
- 7. Install sediment traps (such as SiltSack® by Terrafix) between catch basins and frames.
- 8. Plan construction at proper time to avoid flooding.
- 9. Installation of a mud matt to prevent mud and debris from being transported off site.
- 10. Installation of a silt fence to prevent sediment runoff.

The contractor will, at every rainfall, complete inspections and guarantee proper performance. The inspection is to include:

- 1. Verification that water is not flowing under silt barriers.
- 2. Clean and change silt traps at catch basins.

Refer to **Drawing EC/DS-1** for the proposed location of silt fences, and other erosion control structures.

Geotechnical Investigation and ESA

9.0 GEOTECHNICAL INVESTIGATION AND ESA

9.1 GEOTECHNICAL INVESTIGATION

A geotechnical report for the site was prepared by Paterson Group in September 2020 (see **Appendix E**). As stated in the geotechnical report, the subsurface profile across the site generally consists of asphaltic concrete overlying a fill layer consisting of crushed stone and silty sand. The fill material is underlain by a stiff to hard layer of brown silty clay with sand seams. Glacial till was encountered below the above noted layers consisting of a compact to a very dense silty sand with clay, gravel, cobbles, and boulders. Seams of coarse sand where encountered in the glacial till layer at some test hole locations

Bedrock was cored at one borehole location to confirm refusal. Limestone bedrock was encountered at a depth of 9.7 m below the existing ground surface at BH6. Refusal was encountered in the other boreholes between a depth of 4.8 to 8.7 m. It should be noted that boulders are to be expected.

Groundwater levels were measured in in July 2020 and were found to range between 1.9 m and 4.7 m below ground surface elevation. Long-term groundwater levels can also be determined based on observations of the recovered soil samples, such as moisture levels, colouring and consistency. Based on these observations, the long-term groundwater level is expected at a 5 to 6 m depth.

Bedrock removal can be accomplished by hoe ramming where only small quantity of the bedrock needs to be removed. Sound bedrock may be removed by line drilling and controlled blasting and/or hoe ramming.

Prior to considering blasting operations, the blasting effects on the existing services, buildings and other structures should be addressed. A pre-blast or pre-construction survey of the existing structures located in proximity of the blasting operations should be completed prior to commencing site activities. The extent of the survey should be determined by the blasting consultant and should be sufficient to respond to any inquiries/claims related to the blasting operations.

For design purposes, the flexible pavement structure presented in the following tables could be used for the design of car only parking areas in the lower level of the parking garage.

Thickness (mm)	Material Description
50	Wear Course - HL 3 or Superpave 12.5 Asphaltic Concrete

Table 13: Recommended Parking Structure – Parking Areas



Geotechnical Investigation and ESA

Thickness (mm)	Material Description	
150	BASE - OPSS Granular A Crushed Stone	
300	SUBBASE - OPSS Granular B Type II	
SUBGRADE - Eithe	er fill, in situ silty clay or sand or crushed stone material placed	
over in situ soil		

Table 14: Recommended Parking Structure – Local Roadways, Access Lanes and Heavy Vehicle Parking

Thickness (mm)	Material Description	
40	Wear Course - Superpave 12.5 Asphaltic Concrete	
50	Binder Course - Superpave 19.0 Asphaltic Concrete	
150	BASE - OPSS Granular A Crushed Stone	
400	SUBBASE - OPSS Granular B Type II	
SUBGRADE - Either fill, in situ silty clay or sand or crushed stone material placed		
over in situ soil.		

It is expected that the building foundation walls will be placed in close proximity to all the boundaries. It is expected that the foundation wall will be blind poured against a drainage system and waterproofing system fastened against the shoring system.

A waterproofing membrane will be required to lessen the effect of water infiltration for the lower P-2 basement level. The waterproofing membrane can be placed and fastened to the shoring system (soldier pile and timber lagging) and should extend to the bottom of the excavation at the founding level of the raft foundation.

It is recommended that the composite drainage system, such as Delta Drain 6000 or equivalent, extend from the exterior finished grade to the founding elevation (underside of raft slab). The purpose of the composite drainage system is to direct any water infiltration resulting from a breach of the waterproofing membrane to the building sump pit. It is recommended that 150 mm diameter sleeves at 3 m centres be cast in the foundation wall at the raft slab interface to allow the infiltration of water to flow to an interior perimeter underfloor drainage pipe. The perimeter drainage pipe should direct water to sump pit(s) within the lower basement area.

Underfloor drainage will be required to control water infiltration due to groundwater infiltration at the proposed founding elevation. For design purposes, we recommend that 150 mm in diameter perforated pipes be placed along the interior perimeter of the foundation wall and one drainage line within each bay. The spacing of the underfloor drainage system should be confirmed at the time of backfilling the floor completing the excavation when water infiltration can be better assessed.

Geotechnical Investigation and ESA

It is anticipated that groundwater infiltration into the excavations should be controllable using open sumps. The contractor should be prepared to direct water away from all bearing surfaces and subgrades, regardless of the source, to prevent disturbance to the founding medium.

A temporary Ministry of Environment, Conservation and Parks (MECP) Category 3 Permit to Take Water (PTTW) may be required if more than 400,000 L/day are to be pumped during the construction phase. At least 4 to 5 months should be allowed for completion of the application and issuance of the permit by the MECP.

For typical ground or surface water volumes being pumped during the construction phase, typically between 50,000 to 400,000 L/day, it is required to register on the Environmental Activity and Sector Registry (EASR). A minimum of two to four weeks should be allotted for completion of the EASR registration and the Water Taking and Discharge Plan to be prepared by a Qualified Person as stipulated under O.Reg. 63/16. If a project qualifies for a PTTW based upon anticipated conditions, an EASR will not be allowed as a temporary dewatering measure while awaiting the MECP review of the PTTW application.

9.2 PHASE ONE ENVIRONMENTAL SITE ASSESSMENT

Lopers and Associates completed a Phase One Environmental Site Assessment of the existing commercial property in July 2020 (see **Appendix E**).

The Phase One Property was undeveloped prior to the 1950's when initial development was interpreted to have been for residential purposes. A commercial lease was registered at the Phase One Property in 1965 and it is inferred that commercial redevelopment of the Property occurred at this time. Demolition of the former residential building was completed prior to 1991. A retail fuel outlet and automotive service garage were present as part of the original commercial development at the Phase One Property and operated on the southeast portion of the Property until 2002 and 2018, respectively. The automotive garage moved to the south unit of the south commercial building in 2018 and has operated at that location on the Phase One Property since that time.

The presence of a former retail fuel outlet and automotive service garage on the southeast portion of the Phase One Property are a significant potentially contaminating activities (PCAs) which represent areas of potential environmental concern (APECs) for the Property. Given that previous reports were provided which document remnant petroleum hydrocarbon (PHC) and benzene, toluene, ethylbenzene and xylenes (BTEXs) soil contamination and that groundwater quality was not confirmed following the completion of a remediation program, further investigation is warranted. The contaminants of potential concern associated with retail fuelling are generally PHCs and BTEXs, and metals as this was an older facility and lead was historically present in gasoline. Based on historical soil analysis in this area of the Property, polycyclic aromatic hydrocarbons (PAH) and volatile organic compounds (VOCs) are also considered contaminants of potential concern associated with the former automotive garage operations.



Geotechnical Investigation and ESA

The practice of backfilling following demolition activities at the Phase One Property is also a significant PCA which represents an APEC for the Property. Given that no reports were provided with analytical data to support the environmental quality of the backfill used to fill the former residential building footprint on the central-south portion of the Property, this area warrants further investigation. The contaminants of potential concern commonly found in poor environmental quality backfill are PHCs/BTEXs, PAHs and metals.

The presence of an active automotive service garage was observed during the site walk over on the central portion of the Phase One Property at the time of the Site Investigation. Although this garage has only been operating for a short time period (2018 to present), these operations are a PCA which represents an APEC for the Property. Based on the observations at this automotive garage, that contaminants of potential concern are considered to be PHCs and BTEXs.

Three active and/or historical fuel storage tank locations at neighbouring properties in the Phase One Study Area constitute PCAs. The PCAs at neighbouring properties in the Phase One Study Area are located significant distances and at down- or cross-gradient orientations with respect to the Phase One Property and are not considered to represent APECs for the Phase One Property.

Based on the identification of PCAs and APECs at the Phase One Property, it was recommended that a Phase Two Environmental Site Assessment be completed to assess the soil and groundwater quality in the vicinity of the four APECs.

9.3 PHASE TWO ENVIRONMENTAL SITE ASSESSMENT

Lopers & Associates (Lopers) completed a Phase Two Environmental Site Assessment (Phase Two ESA) of the existing commercial property in August 2020.

The scope of work for the Phase Two ESA included drilling seven boreholes at the Phase Two Property. Three of the boreholes were instrumented with groundwater monitoring wells with screens installed in the overburden.

Six soil samples, including one duplicate sample, were submitted for laboratory analysis for a combination of PHCs, BTEXs, volatile organic compounds (VOCs), PAHs, metals and inorganics. One sample was also submitted for toxicity leaching characteristic procedure (TCLP) for waste characterization purposes.

Groundwater sampling was completed of the newly installed monitoring wells and two existing groundwater monitoring wells at the Phase Two Property, which were installed as part of historical investigations. A total of seven groundwater samples, including a duplicate sample and a trip blank, were submitted for laboratory analysis for a combination of PHCs, BTEXs, VOCs, PAHs, metals and inorganics.

Geotechnical Investigation and ESA

The applicable sites standard was determined to be the full depth generic site condition standard, in a non-potable groundwater condition, with course textured soil, for residential property use, as specified in Table 3 of the MECP Soil, Ground Water and Sediment Standards for Use Under Part XV.1 of the Environmental Protection Act, April 15, 2011.

At APEC #3 (placement of fill of unknown quality) the soil samples BH1-20-SS5 and BH11-20-SS5 (Duplicate of BH1-20-SS5), collected from a depth of approximately 3.1-3.7 m BGS, had reported concentrations of PHC F2 range (909 μ g/g and 306 μ g/g vs. 98 μ g/g), Methylnaphthalene (7.61 μ g/g and 2.26 μ g/g vs. 0.99 μ g/g) and reported concentrations of vanadium (101 μ g/g and 104 μ g/g vs. 86 μ g/g). These samples also had respective cobalt concentrations of 20.1 μ g/g and 22.5 μ g/g compared to the site condition standard of 22 μ g/g; since the average concentration of cobalt in these samples is less than the site condition standard, the marginal exceedance in the duplicate standard is not considered to exceed the site condition standard.

At APEC #1 (former retail fuel outlet) the soil sample BH3-20-SS6, collected from a depth of approximately 3.8-4.4 m BGS, had reported concentrations of PHC F1 range (117 μ g/g vs. 55 μ g/g), PHC F2 range (110 μ g/g vs. 98 μ g/g), benzene (3.02 μ g/g vs. 0.21 μ g/g), ethylbenzene (59 μ g/g vs. 2 μ g/g), toluene (73.5 μ g/g vs. 2.3 μ g/g) and xylenes (276 μ g/g vs. 3.1 μ g/g). Additionally, PAH exceedances from the same soil sample included Methylnaphthalene (1.95 μ g/g vs. 0.99 μ g/g) and Naphthalene (1.69 μ g/g vs. 0.6 μ g/g).

At APEC #1 (former retail fuel outlet), the groundwater samples BH3-20 and BH13-20 (Duplicate of BH3-20), collected from a screen depth of approximately 2.5-5.5 m BGS, had reported concentrations of PHC F1 range (3,600 µg/g and 3,790 µg/g vs. 750 µg/g), PHC F2 range (52,400 µg/g and 2,260 µg/g vs. 150 µg/g), PHC F3 range (3,940 µg/g vs. 500 µg/g), benzene (19,300 µg/g and 19,700 µg/g vs. 44 µg/g), ethylbenzene (3,800 µg/g and 3,700 µg/g vs. µg/g), toluene (65,200 µg/g and 60,900 µg/g vs. 18,000 µg/g) and xylenes (27,600 µg/g and 26,600 µg/g vs. 4,200 µg/g). Lead was also reported at concentrations of 51.6 µg/g and 54.6 µg/g vs. 25 µg/g.

All of the other soil and groundwater results for the Phase Two Property are in compliance with the applicable site condition standards. The Phase Two Property is not in compliance with the Table 3 site condition standards as of the certification date of June 30, 2020.

An environmental remediation program, including the bulk removal and off-site disposal of soil and groundwater in excess of the site condition standards is recommended for the Phase Two Property. Given the scope and timeline for the proposed redevelopment and the requirements for specialized construction techniques to complete remediation of the Phase Two Property to meet the site condition standards, it is recommended that remediation be completed in conjunction with redevelopment of the property. It should be noted that the proposed redevelopment includes excavation for at least two to three levels of underground parking, which is expected to be sufficient for remediation of the aforementioned environmental contamination at the Phase Two Property.

Geotechnical Investigation and ESA

Further delineation and confirmation of remediation sampling will be required prior to the completion of an environmental remediation program and confirmation of compliance with the site condition standards; however, these tasks can be completed at the time decommissioning and demolition of existing structures at the Phase Two Property. The submission of a record of site condition would be required in the event of a change of zoning of the Phase Two Property; however, these tasks can be completed at the time decommissioning and demolition of existing structures at the Phase Two Property. The Phase Two Property at the Phase Two Property; however, these tasks can be completed at the time decommissioning and demolition of existing structures at the Phase Two Property. The Phase Two ESA could be then updated at that time to show compliance with site condition standards.

Preparation of a soil management plan in accordance with O.Reg. 406/19 will be required as part of management of excess soil generated as part of construction activities. It was recommended that a remedial action plan be prepared to develop a strategy for remediation, including soil and groundwater management, during redevelopment.

Conclusions

10.0 CONCLUSIONS

10.1 WATER SERVICING

The existing 305 mm diameter watermain on Ridgewood Avenue will provide adequate fire flow capacity. In order to meet the City water supply objective that limits a single feed to 50 m³/d during basic day demands, two connections are required to service the proposed development. The service connections will be capable of providing anticipated demands at acceptable pressures to the lower storeys but will require booster pumps to maintain minimum required pressures for the higher floors of the proposed 6-storey and 15-storey towers.

10.2 SANITARY SERVICING

The proposed sanitary sewer lateral is sufficiently sized to provide gravity drainage for the site. The proposed site will be serviced by a 200 mm diameter service lateral directing wastewater flows to the existing 225 mm diameter sanitary sewer on Ridgewood Avenue. A backflow preventer will be required for the proposed building in accordance with the Ottawa sewer design guide and will be coordinated with building mechanical engineers.

All underground parking drains should be connected to the building's internal plumbing. A sump pump will be required to drain the underground parking levels to the existing sanitary sewer on Ridgewood Avenue.

10.3 STORMWATER SERVICING

The proposed stormwater management plan is in compliance with the goals specified through consultation with the City of Ottawa, as well as local standards. A combination of roof storage and underground storage within a cistern located in the underground parking will be provided to attenuate post development peak flows. Post development peak flows from the overall site up to the 100-year storm will be restricted to the target release rate. A sump pump will be required to direct flows from the internal building plumbing system to the proposed gravity service connected to the existing 300 mm diameter storm sewer running on Ridgewood Avenue.

10.4 GRADING

Erosion and sediment control measures will be implemented during construction to reduce the impact on existing infrastructure. The subject site will maintain emergency overland flow routes to the back and to Ridgewood Avenue.



Conclusions

10.5 UTILITIES

All utilities (Hydro Ottawa, Bell Canada, Rogers Ottawa, and Enbridge Gas) have existing plants in the subject area. Exact size, location and routing of utilities will be finalized after design circulation.

10.6 APPROVAL / PERMITS

Ministry of the Environment Conservation and Parks (MECP) Environmental Compliance Approvals (ECA) are not expected to be required for the subject site as the site is private and will remain under singular ownership. A Permit to Take Water may be required for pumping requirements for construction of underground parking level. No other approval requirements from other regulatory agencies are anticipated.

APPENDICES

Appendix A Water Calculations

Appendix A WATER CALCULATIONS



From:	Rasool, Rubina
To:	Mott, Peter
Cc:	Kilborn, Kris
Subject:	RE: 729 Ridgewood Avenue - Boundary Conditions Request
Date:	Tuesday, April 27, 2021 3:59:58 PM
Attachments:	729 Ridgewood April 2021.pdf

Good afternoon,

The following are boundary conditions, HGL, for hydraulic analysis at 729 Ridgewood Ave (zone 2W2C) assumed to be connected to the 305 mm on Ridgewood Ave (see attached PDF for location).

Both Connections

Minimum HGL = 123.7 m

Maximum HGL = 131.9 m

Max Day + Fire Flow (117 L/s) = 125.6 m

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

Rubina

Rubina Rasool, E.I.T. Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review – East Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue Laurier Ouest. Ottawa (Ontario) K1P 1J1 rubina.rasool@ottawa.ca

From: Mott, Peter <Peter.Mott@stantec.com>
Sent: April 13, 2021 9:33 AM
To: Rasool, Rubina <Rubina.Rasool@ottawa.ca>
Cc: Kilborn, Kris <kris.kilborn@stantec.com>
Subject: 729 Ridgewood Avenue - Boundary Conditions Request

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Hello Ms. Rasool,

I would like to request the hydraulic boundary conditions for the proposed 729 Ridgewood Avenue Development. Please find attached the concept plan, the key map showing the location of the proposed development, domestic water demand calculations, and fire flow calculations.

A summary of the proposed site is provided below:

We anticipate a minimum of two (2) connections to the existing watermain will be required to service the site. The following connections are expected for servicing:

≻Connections to the existing 305 mm (CI) watermain on Ridgewood Avenue.

*Existing hydrants on Ridgewood Avenue.

For the purpose of the boundary conditions request, may you please provide us with the boundary conditions for the following servicing options:

- i. Watermain connections to the existing 305 mm (CI) watermain on Ridgewood Avenue; assuming a fire flow requirement of **7,000 L/min** for the site in addition to the domestic water demands provided below.
- The intended land use is a combination of commercial and residential, per the summary provided in the Domestic Demands spreadsheet. (See attached Concept Plan with project stats)
- Estimated fire flow demand per the FUS methodology: 7000 L/min (117 L/s) for the worst-case scenario (Tower I & Building II)
- Domestic water demands for the entire development:
 - Average day: 172.5 L/min (2.87 L/s)
 - Maximum day: 414.3 L/min (6.91 L/s)
 - Peak hour: 901.3 L/min (15.02 L/s)

Thank you for your time and please contact me at your earliest convenience if any additional information or clarification is required.

Best regards,

Peter Mott EIT Engineering Intern, Community Development

Mobile: 613-897-0445 <u>Peter.Mott@stantec.com</u> Stantec 400 - 1331 Clyde Avenue Ottawa ON K2C 3G4



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729 Ridegwood Avenue (Brigil Development) - Domestic Water Demand Estimates Based on conceptual development plans from Neuf Architect(e)s (2021-05-14) Last updated on May 21, 2021

Ottawa Design Guidelines - Water Distribution Table 4.1 Per Unit Populations

Average Apt.	1.8	ppu
Bachelor	1.4	ppu
1 Bedroom	1.4	ppu
2 Bedroom	2.1	ppu
3 Bedroom	3.1	ppu

Development Block/Area ID	Commercial Area	Number of	Population	Daily Demand Rate	Avg. Da	y Demand ^{1,2}	Max. Day D	emand ^{1,2}	Peak Hour De	emand ^{1,2}
Development Block/Area ID	(m²)	Residential Units	Fopulation	(L/cap/day or L/ha/d)	(L/min)	(L/s)	(L/min)	(L/s)	(L/min)	(L/s)
Tower I (15 Storeys)										
Bachelor	-	20	28	350	6.8	0.11	17.0	0.28	37.4	0.62
1 Bedroom	-	53	74	350	18.0	0.30	45.1	0.75	99.2	1.65
2 Bedroom	-	58	122	350	29.6	0.49	74.0	1.23	162.8	2.71
3 Bedroom	-	10	31	350	7.5	0.13	18.8	0.31	41.4	0.69
Building II (6 Storeys)										
Bachelor	-	1	1	350	0.3	0.01	0.9	0.01	1.9	0.03
1 Bedroom	-	35	49	350	11.9	0.20	29.8	0.50	65.5	1.09
2 Bedroom	-	12	25	350	6.1	0.10	15.3	0.26	33.7	0.56
Buliding III (4 Storeys)										
1 Bedroom	-	40	56	350	13.6	0.23	34.0	0.57	74.9	1.25
2 Bedroom	-	14	29	350	7.1	0.12	17.9	0.30	39.3	0.66
Building IV (4 Storeys)										
1 Bedroom	-	40	56	350	13.6	0.23	34.0	0.57	74.9	1.25
2 Bedroom	-	14	29	350	7.1	0.1	17.9	0.3	39.3	0.66
Building V (6 Storeys)										
Bachelor	-	2	3	350	0.7	0.01	1.7	0.03	3.7	0.06
1 Bedroom	-	63	88	350	21.4	0.36	53.6	0.89	117.9	1.97
2 Bedroom	-	20	42	350	10.2	0.17	25.5	0.43	56.1	0.94
3 Bedroom	-	5	16	350	3.8	0.06	9.4	0.16	20.7	0.35
Commercial	856			28000	16.6	0.28	25.0	0.42	44.9	0.75
Total Site :	856	387	650	-	174.6	2.91	419.9	7.00	913.7	15.23

maximum daily demand rate = 2.5 x average day demand rate

peak hour demand rate = 2.2 x maximum day demand rate

1

Water demand criteria used to estimate peak demand rates for commercial/amenity/tobby areas are as follows: maximum daily demand rate = 1.5 x average day demand rate peak hour demand rate = 1.8 x maximum day demand rate 2

Population density for all residential units based on an population densities provided in Table 4.1 - Per Unit Populations of the City of Ottawa Water Distribution Design Guidelines (July 2010). 3

Project # 160401536 (729 Ridgewood Avenue) Date 2021-05-28

$$Q = KVS_{tot}$$

Designed by: Checked by: Description: AMP

15-Storey (Tower I) High-rise Residential Building and 6-Storey (Building II) Mid-rise Residential Apartment Building

- Q = Volume of water required (L)
- V = Total building volume (m3)
- K = Water supply coefficient from Table 1

Sotal of spatial coefficeint values from property line exposures on all sides as obtained from the formula

 $S_{tot} = 1.0 + [S_{side1} + S_{side2} + S_{side3} + S_{side4}]$

Type of construction	Building Classification		Water Supply Coefficient	
Non-Combustible with Fire- Resistance Ratings	A-2, B-1, B-2, B-3, C, D		10	
•		I.	1	1
Area of one floor	number of floors	height of ceiling	Total Building	
(m ²)		(m)	Volume (m ³)	
1058	17	3.05	54,857	Tower I
647.12	8	3.05	15,790	Tower I
			70,647	Total
Side	Exposure		Total Spatial	
	Distance (m)	Spatial Coefficient	Coeffiecient	
North	25	0		
East	9	0.1	11	
South	36	0		
West	18	0		
Established Fire	Reduction in		Total Volume	
Safety Plan?	Volume (%)		Reduction	
no	0%		0%	
			Total Volume 'Q' (L)	
			777,117	-
			Minimum Required Fire Flow (L/min)	
			9,000	1
	Type of construction Non-Combustible with Fire- Resistance Ratings Area of one floor (m ²) 1058 647.12 647.12 Side Side Side South East South West Established Fire Safety Plan? no no	Type of constructionBuilding ClassificationNon-Combustible with Fire- Resistance RatingsA-2, B-1, B-2, B-3, C, DArea of one floor (m²)number of floorsArea of one floor (m²)number of floors105817647.128SideExposure Distance (m)North25East9South36West18Established Fire Safety Plan?Reduction in Volume (%)no0%	Type of constructionBuilding ClassificationNon-Combustible with Fire- Resistance RatingsA-2, B-1, B-2, B-3, C, DArea of one floor (m²)number of floors (m)Area of one floor (m²)13.05647.1283.05647.1283.05647.1283.05O90.1SideExposure Distance (m)Spatial CoefficientNorth250East90.1South360West180North0%Partial CoefficientNorthSafety Plan?Volume (%)No0%	Type of constructionBuilding ClassificationWater Supply CoefficientNon-Combustible with Fire- Resistance RatingsA-2, B-1, B-2, B-3, C, D10Non-Combustible with Fire- Resistance RatingsA-2, B-1, B-2, B-3, C, D10Area of one floor (m²)number of floors (m)height of ceiling (m)Total Building Volume (m³)Area of one floor (m²)number of floors (m)height of ceiling (m)Total Building Volume (m³)(m²)number of floors (m)105854,857647.12A3.0515,790647.12A3.0515,790647.12Spatial Coefficient Spatial CoefficientCoefficient CoefficientNorth2501.1South3.601.1South3.601.1South3.601.1South3.600West100%Established Fire Safety Plan?Reduction in Volume (%)Total Volume Reductionno0%0%0%Total Volume 'Q' (L)Unimum Required Fire Flow (L/min)

1. Tower I: Fifteen storey building with two levels of underground parking.

2. Tower II: Six storey building with two levels of underground parking.

 Project #
 160401536 (729 Ridgewood Avenue)

 Date
 2021-05-28

Designed by: Checked by:

Description:

AMP

Building III: 4-storey Residential apartment building. Building information from conceptual architectural drawings by Neuf Architects.

Q = Volume of water required (L)

 $Q = KVS_{tot}$

- V = Total building volume (m3)
- K = Water supply coefficient from Table 1

Sotal of spatial coefficeint values from property line exposures on all sides as obtained from the formula

 $S_{tot} = 1.0 + [S_{side1} + S_{side2} + S_{side3} + S_{side4}]$

1	Type of construction	Building Classification		Water Supply Coefficient
	combustible without Fire- Resistance Ratings	A-2, B-1, B-2, B-3, C, D		23
2	Area of one floor	number of floors	height of ceiling	Total Building
	(m ²)		(m)	Volume (m ³)
	1089	6	3.05	19,929
3	Side	Exposure		Total Spatial
		Distance (m)	Spatial Coefficient	Coeffiecient
	North	52	0	
	East	16	0	1 1
	South	15	0	1.1
	West	9	0.1	
4	Established Fire	Reduction in		Total Volume
	Safety Plan?	Volume (%)		Reduction
	no	0%		0%
5				Total Volume 'Q' (L)
				504,204
				Minimum Required
				Fire Flow (L/min)
				9,000

1. Tower III: Four storey building with two levels of underground parking.

 Project #
 160401536 (729 Ridgewood Avenue)

 Date
 2021-05-28

Designed by: Checked by: AMP

Description:

Building IV: Residential apartment building. Building information from conceptual architectural drawings by Neuf Architects.

 $Q = KVS_{tot}$

- Q = Volume of water required (L)
- V = Total building volume (m3)
- K = Water supply coefficient from Table 1

Sotal of spatial coefficeint values from property line exposures on all sides as obtained from the formula

 $S_{tot} = 1.0 + [S_{side1} + S_{side2} + S_{side3} + S_{side4}]$

1	Type of construction	Building Classification		Water Supply Coefficient
	combustible without Fire- Resistance Ratings	A-2, B-1, B-2, B-3, C, D		23
2	Area of one floor	number of floors	height of ceiling	Total Building
	(m ²)		(m)	Volume (m ³)
	1074	6	3.05	19,654
3	Side	Exposure		Total Spatial
		Distance (m)	Spatial Coefficient	Coeffiecient
	North	15	0	
	East	16	0	1
	South	52	0	1
	West	20	0	
4	Established Fire	Reduction in		Total Volume
	Safety Plan?	Volume (%)		Reduction
	no	0%		0%
5				Total Volume 'Q' (L)
				452,042
				Minimum Required
				Fire Flow (L/min)
				9,000

1. Tower IV: Four storey building with two levels of underground parking.

 Project #
 160401536 (729 Ridgewood Avenue)

 Date
 2021-05-28

Designed by: Checked by: AMP

Description:

Building V: Commercial Areas on ground floor and apartments on higher floors. Building information from conceptual architectural drawings by Neuf Architects.

Q = Volume of water required (L)

 $Q = KVS_{tot}$

- V = Total building volume (m3)
- K = Water supply coefficient from Table 1

Stot = Sotal of spatial coefficeint values from property line exposures on all sides as obtained from the formula

 $S_{tot} = 1.0 + [S_{side1} + S_{side2} + S_{side3} + S_{side4}]$

1	Type of construction	Building Classification		Water Supply Coefficient
	Non-Combustible without Fire-Resistance Ratings	A-2, B-1, B-2, B-3, C, D		16
2	Area of one floor	number of floors	height of ceiling	Total Building
	(m ²)		(m)	Volume (m ³)
	1432	8	3.05	34,941
3	Side	Exposure		Total Spatial
		Distance (m)	Spatial Coefficient	Coeffiecient
	North	36	0	
	East	20	0	1
	South	52	0	-
	West	28	0	
4	Established Fire	Reduction in		Total Volume
	Safety Plan?	Volume (%)		Reduction
	no	0%		0%
5				Total Volume 'Q' (L)
				559,056
				Minimum Required
				Fire Flow (L/min)
				9,000

1. Tower V: Six storey building with two levels of underground parking.

SITE SERVICING AND STORMWATER MANAGEMENT BRIEF – MOONEY'S BAY - 729 RIDGEWOOD AVENUE, OTTAWA, ON

Appendix B Proposed Site Plan

Appendix B PROPOSED SITE PLAN





		r		d pin	KE n/a	ΥP	LAN		1 (A-101)
	IN 12382 Province /Province Ontario Zonage / Zoning City of Otta Supeficie du Lot / Property Area	FORMATIO	N SUR LE PI w No. 2008-250 13 238,	ROJET - PROJE 729 Ridge 4 m²/sq.m.	ECTINFORM ewood Avenu	ATION Ie	142 498 pi ² / sq.	202105-25 ft.	
	STATISTIQUES SUR LE PROJECT STATS1 Hauteur du Bâtiment / Building I STATISTIQUES DE SUNIT STATISTIQUES DE SUNIT STATISTICS Studio / Bechter 1 Chamber / 1 Bedroom	PROJET / ICS Height (m) ÉS / UNIT	B âtiment 1 / Building 1 50m (15 étages/storeys) B âtiment 1 / Building 1 20 9	Bâtiment 2 / Building 2 22m (6 étages/storeys) Bâtiment 2 / Building 2 1	Bâtiment 3/ Building 3 16m (4 étages/storeys) Bâtiment 3/ Building 3 0 12	B âtiment 4 / Building 4 16m (4 étages/storeys) B âtiment 4 / Building 4	Bâtiment 5 / Building 5 22m (6 étages/storeys) Bâtiment 5 / Building 5 2 20	TOTAL 64	
	1 Chambre + Den / 1 Bedroom + 2 Chambres + Den / 2 Bedrooms 2 Chambres + Den / 2 Bedrooms 3 Chambres / 3 Bedrooms TOTAL STATIONNE MENT RÉSIDEN	Den + Den TIEL / RESIDEN	44 51 7 10 141 <i>ITIAL PARKING</i>	24 12 0 48	12 28 14 0 54	28 14 0 54	20 43 20 5 90	64 167 111 7 15 387	
	PHASE / PHASE Bàtiment 1 à 6-Résidentiel Buil Résidential Exterieur / Interieur / I	ding 1 to 5- Exterior nterior		E) #/Uni 1.2 perdwa	KIGÉ / REQUIRE té / hit elling unit	0	Total 464	FOURNIS / PROVIDEI #/Unité / Total 460 7 440	
	Batiment Ta b - Accessible (inclus dans complet) Building 1 to 5 - Accessible (inclu- total) Exteneur / Reduced parking stalls (Sec. 10 Exteneur / Interieur / 1	uded at the Exterior Interior S up to 50% Exterior Interior	13-100 = 4% TOTAL 200 = 1+3% TOTAL 2+2% TOTAL (50%	(50 %A+50 %B). (50%A+50%B) A+50%B) TOTAL (50	ia.+δ0%B)	101- 201-1000 = 1001 =11+1%	13	13	-
	TOTAL RESIDENTS Batiment 1 - Visitors / Building 1 Exterieur / I TOTAL VISITEURS STATIONNE MENT COMMER	- Visiteur Exterior Interior	0.2 per d	G		1050	464 77 77	1,19 460 77 77	
	PHASE / PHASE Bàtiment 1 - Commerciale / Buil Exterieur / Interieur /	ding 1 - Exterior	Aire (n†) / A rea (m²) 856	Ratio Moy. (m²) / Avg. Ratio (m²) 3,4/100	EXIGETREQU		29	n n 4 25	
	Bàtiment 1 - Commerciale Acoe Building 1 - Accessible Commerc Exterieur / I Interieur / I	sible / sial Exterior nterior	1-1 13-100 = 4% T O 101:200 = 1+3%T 201-1000 = 2+2%G > 1001 = 11+1%T	2 = 1A, TAL (50 %A +50 %B), OTAL (50 %A +50 %B) OTAL (50 %A +50 %B) OTAL (50 %A +50 %B)			1	1 0 0 30	
	STATIONNE MENT POUR VÉ PHASE / PHASE Bâtiment 1 à 5- Résidentiel / Bu Bâtiment 1 - Commerciale / Buil Commercial	LOS <i>I BICYCLE</i>	PARKING Unités / Units 387	E) Aire (m²) / Area (m²) 855,9	KIGÉ I REQUIREI	0 R 0,5 / Un 1.25 per 250 m2	atio / Ratio it 194 of g.f.area 4	FOURNIS / <i>PROVIDEL</i> 194 4	
	TOTAL			MOONE	Y'S BAY GITO V		198	138	
	PIN - to be confirmed Bylaw 2008-250 Zonning – GM1 F (1.0) Lot area (sq.m) Gross foor area of the build in Fion	nj		729 Ridgewo Required (By-law)	13 238		P rovide o 13 238	1	
	Ratio Nav. SETBACK (m) Minimum Front Setback (m) Minimum Side Setback (m) Minimum interior Setback (m)		3 m 3 m For a non-resident a lot line abutting For residential use	tial or mixed-use building a residential zone: 5 mts e building higher than 11 r	5015 50% from a ny portion of n high: 3 m		4,5 m 6,4 m 6,4 m 6,4 m		
	Minimum Rear Vard Setback (m) BUILDING High (m)		Abutting a street: From any portion (7.5 m	3m ofa rear lot line abutting a 18 m	a residential zone:		N.A. 7.5 m Tower I-15 store Building II-6 store Building III-4 store	/s-±51 m /s-±21.5 m /s-±15.5 m	
	Maximum floor space index Minimum width of land scaped area		No more than 50% used Full floor space in rise Abutting a street, m but may be redi high opaque screet	i of the permitted floor sp dex may be used for a part abutting a residential or i used to 1 metre where a n en is provided	ace index may be ment dwelling, mid nstitutional zone : 3 ninimum 1,4 metre		Building IV-4 store Building V-6 store 39,97% In progree	ys-±15.5 m ys-±21.5 m is	-
	PARING AREA C - AS PER SCHEDULE 1 A Residential Owe Iling mid-high-rise	TOWER I BUILDING II BUILDING III	1.2 per dwelling	Units or m ²	Parking spaces 169 58 65	Basement 1 48 15 36	Basement 2 108 43 29	Parking spaces Total 156 58 65	
	Apartment Visitors	BUILDING IV BUILDING V EXTERIOR TOWER I BUILDING II BUILDING III	0.2 per dwelling	54 90 141 48 unit 54	65 108 28 10 11	38 29 8 28 10 11	27 78 0 0 0	65 107 8 28 10 11	
	Convenience Store	BUILDING V BUILDING V TOTAL EXTERIOR	3.4 per 100 m² of g.f.area	90 856	11 18 77 29 571 Parking thares	18	0	11 18 537 25 5 567 24/instructor	
	Residential Dwelling, mid-high-fise Apartment Convenience Store	TOWER I BUILDING II BUILDING III BUILDING IV BUILDING V	0,5 per dwelling 1.25 per 250 m² o g f.area	141 48 unit 54 90 f 855,90	71 24 27 27 45 4	33 17 27 27 29	38 7 16 4	71 24 27 27 45 4	
_	1004L				139			135	
	LEGEND				. . 5	200 <u>k</u> ((ING	
		RTY LINE	Ξ			> F 200 F F	R: RESIDE C: COMME REDUCED	ENTIAL (WITH ERCIAL PARKING	PHASES)
	EXISTIN	IG POWE	ER LINE	-			C: COMME REDUCED	ERCIAL PARKING W	
	5.0m X 8	5.0m TRI	ANGLE			200 j A	RESIDEN I ACCESSIE R: RESIDE C: COMME	IAL (WITH P BLE PARKING ENTIAL (WITH ERCIAL	HASES) I PHASES)
	-o EXISTIN	IG FENC	E		SIGN	S 1 S	STOP SIG	N	
	EXIST	ing Buil	_DING			<u> </u> 2 F	FIRE ROU	TE SIGN	
		REMOVA	EWALKS	6	-	3 F	PRIORITY SIGN	TO PEDESTR	RIAN
					SIGN:	S ALL THE	IWO LANI SIGNS FO	ES AHEAD	
	FIRE RO	JUIL							

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- l'entrepreneur avant le début des travaux. / All dimensions which appear on the documents must be verify by the contractor before starting the work. 3. Veuillez aviser l'architecte de toute dimension erreur et/ou divergences
- Veuillez aviser l'architecte de toute dimension erreur et/ou divergences entre ces documents et ceux des autres professionnels. / The architect must be notified of all errors, omissions and discrepancies between these documents and those of other professionnals.
 Les dimensions sur ces documents doivent être lues et non mesurées.
 / The dimensions on these documents must be read and not measured.

PLANIFICATEUR Planner **FOTENN Planning and Urban design** 396, Cooper Street, Ottawa, ON K2P 2H7 T 613 730 5709 www.fotenn.com

ARCHITECTURE DE PAYSAGE Landscape architect **LEVSTEK CONSULTANTS Inc** 5871, Hugh Crescent, Ottawa, ON K0A 2W0 T 613 826 0518 www.larocquelevstek.com

INGÉNIERIE TRANSPORT Engineering, Transportation **PARSONS** 100-1223, Michael Street Suite 100, Ottawa, ON K1J 7T2 T 613 738 4160 www. parsons.com

CIVIL Civil **STANTEC** 1331, Clyde Ave, Suite 400, Ottawa, ON K2C 3G4 T 613 722 4420 www.stantec.com

RCHITECTES Architect **NEUF architect(e)s** 30, boul. René-Lévesque O. 32e étage, Montréal QC H3B 1S6 514 847 1117 NEUFarchitectes.com

GEOTECHNIQUE Geotechnical **PATERSON GROUP** 154 Colonnade Rd S, Nepean, On. K2E 7J5 T 613 226 7381 www.me-eng.com

CONSERVATION DES ARBRES Tree Conservation BOWFIN

168 Montreal Rd., Cornwall, ON K6H 1B3 T 613 935 6139 www.bowfinenvironmental.ca ENVIRONNEMENT Environmental (ESA)

LOPERS ASSOCIATES

SCEAU Seal



EMPLACEMENT Location OTTAWA, ON

NO PROJET No. 12382



Ε	A101
RÉVISION Revision	NO. DESSIN Dwg Number

SITE SERVICING AND STORMWATER MANAGEMENT BRIEF – MOONEY'S BAY - 729 RIDGEWOOD AVENUE, OTTAWA, ON

Appendix C Sanitary Sewer Calculations

Appendix C SANITARY SEWER CALCULATIONS



			SUBDIVISION: Moonev's I	3av - 729	Ridgewood	d Avenue				SANI		SEWE	R			DESIGN PARAMETERS																					
	<i>c</i> .			,	g						City of Ot	tawa)					MAX PEAK FA	ACTOR (RES.	s.)=	4.0		AVG. DAILY	LOW / PERSO	ON	280	L/p/day			ELOCITY		0.60	m/s					
	Stan	tec	DATE:		6/2/	/2021					-						MIN PEAK FA	CTOR (RES.)	.)=	2.0		COMMERCIA	L		28,000	L/ha/day		MAXIMUM V	/ELOCITY		3.00	m/s					
			REVISION:			0											PEAKING FAC	CTOR (INDUS	STRIAL):	2.4		INDUSTRIAL	(HEAVY)		55,000	L/ha/day		MANNINGS	n		0.013						
			DESIGNED BY:		A	MP	FILE NUMBER	र:		16041053	6						PEAKING FAC	CTOR (ICI >2	20%):	1.5		INDUSTRIAL	(LIGHT)		35,000	L/ha/day		BEDDING C	LASS		В	3					
			CHECKED BY:														PERSONS / 1	BEDROOM		1.4		INSTITUTION	IAL		28,000	L/ha/day		MINIMUM C	OVER		2.50) m					
																	PERSONS / 2	BEDROOM		2.1																	
																	PERSONS / 3	BEDROOM		3.1		INFILTRATIO	N		0.33	L/s/ha		HARMON C	ORRECTION E	ACTOR	0.8						
																	PERSONS / T	OWNHOME		2.7																	
	LOCA	ATION					RESIDEN	TIAL AREA AND F	POPULATION					COMM	ERCIAL	INDUS	rrial (L)	INDUST	TRIAL (H)	INSTITU	ITIONAL	GREEN /	UNUSED	C+I+I		INFILTRATIO	N	TOTAL				PII	PE				
ŀ	AREA ID	FROM	TO	AREA	1 BEDROOM	2 BEDROOM	3 BEDROOM	TOWNHOME	POP.	CUML	LATIVE	PEAK	PEAK	AREA	ACCU.	AREA	ACCU.	AREA	ACCU.	AREA	ACCU.	AREA	ACCU.	PEAK	TOTAL	ACCU.	INFILT.	FLOW	LENGTH	DIA	MATERIAL	CLASS	SLOPE	CAP.	CAP. V	VEL.	VEL.
N	NUMBER	M.H.	M.H.		1 DEDITO OIII	2 DEDITORI	0 DEDITOOM	TOTTITIONE		AREA	POP.	FACT.	FLOW		AREA		AREA		AREA		AREA		AREA	FLOW	AREA	AREA	FLOW							(FULL)	PEAK FLOW	(FULL)	(ACT.)
				(ha)						(ha)			(L/s)	(ha)	(ha)	(ha)	(ha)	(ha)	(ha)	(ha)	(ha)	(ha)	(ha)	(L/s)	(ha)	(ha)	(L/s)	(L/s)	(m)	(mm)			(%)	(l/s)	(%)	(m/s)	(m/s)
Prop	oosed Site	BLDG	SAN10	0.428	254	118	15	0	650	0.428	650	3.91	8.24	0.086	0.086	0.000	0.00	0.000	0.00	0.000	0.00	0.810	0.81	0.04	1.324	1.324	0.44	8.72	2.3	200	PVC	SDR 35	1.00	33.4	26.08%	1.05	0.74
		SAN10	EX. SANMH2	0.000	0	0	0	0	0	0.428	650	3.91	8.24	0.000	0.086	0.000	0.00	0.000	0.00	0.000	0.00	0.000	0.81	0.04	0.000	1.324	0.44	8.72	12.3	200	PVC	SDR 35	0.50	23.6	36.88%	0.74	0.58

MINIMUM VELOCITY	0.60	m/s
MAXIMUM VELOCITY	3.00	m/s
MANNINGS n	0.013	
BEDDING CLASS	В	
MINIMUM COVER	2.50 n	n
	0.0	

From:	Rasool, Rubina
То:	Paerez, Ana
Cc:	<u>Kilborn, Kris; Sharp, Mike</u>
Subject:	RE: 729 Ridgewood Avenue Site
Date:	Wednesday, May 26, 2021 1:06:13 PM
Date.	Wednesday, May 20, 2021 1.00.15 PM

The City has not identified concerns with the proposed flows.

Rubina

Rubina Rasool, E.I.T. Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review – East Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue Laurier Ouest. Ottawa (Ontario) K1P 1J1 rubina.rasool@ottawa.ca

From: Rasool, Rubina
Sent: May 21, 2021 1:29 PM
To: Paerez, Ana <Ana.Paerez@stantec.com>
Cc: Kilborn, Kris <kris.kilborn@stantec.com>; Sharp, Mike <Mike.Sharp@stantec.com>
Subject: RE: 729 Ridgewood Avenue Site

Hi Ana,

I have forwarded your request to Asset Management and should receive a response next week.

Have a good long weekend

Rubina

Rubina Rasool, E.I.T. Project Manager Planning, Infrastructure and Economic Development Department - Services de la planification, de l'infrastructure et du développement économique Development Review – East Branch City of Ottawa | Ville d'Ottawa 110 Laurier Avenue West Ottawa, ON | 110, avenue Laurier Ouest. Ottawa (Ontario) K1P 1J1 rubina.rasool@ottawa.ca

From: Paerez, Ana <<u>Ana.Paerez@stantec.com</u>>
Sent: May 21, 2021 12:35 PM
To: Rasool, Rubina <<u>Rubina.Rasool@ottawa.ca</u>>
Cc: Kilborn, Kris <<u>kris.kilborn@stantec.com</u>>; Sharp, Mike <<u>Mike.Sharp@stantec.com</u>>
Subject: 729 Ridgewood Avenue Site

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ATTENTION : Ce courriel provient d'un expéditeur externe. Ne cliquez sur aucun lien et n'ouvrez pas de pièce jointe, excepté si vous connaissez l'expéditeur.

Good afternoon Rubina,

We are working on a mixed-use (residential/commercial site) on 729 Ridgewood Avenue that will consist of 5 multi-storey buildings with 387 apartment units and retail area on the ground floor of one of the buildings fronting on Ridgewood Avenue.

The sanitary peak flows from the proposed site are approximately 8.72 L/s. Would it be possible for the City to confirm if the downstream sanitary sewers have sufficient capacity for the proposed flows. Thank you very much for your feedback,

Ana Paerez P. Eng.

Water Resources Engineer

Direct: 506 204-5856 Fax: 506 858-8698 Ana.Paerez@stantec.com

Stantec



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SITE SERVICING AND STORMWATER MANAGEMENT BRIEF – MOONEY'S BAY - 729 RIDGEWOOD AVENUE, OTTAWA, ON

Appendix D Stormwater Management Calculations

Appendix D STORMWATER MANAGEMENT CALCULATIONS

	729 Ridge	wood Ave	nue - Brigil H	lomes		S	STORM	SEWER	ર		DESIGN	PARAME	TERS																										
() Stantec						0	DESIGN	SHEET	Г		l = a / (t+	b) ^c		(As per C	ity of Otta	wa Guideli	nes, 2012	:)																					
	DATE:		2021-05-	-28			(City of	Ottawa)				1:2 yr	1:5 yr	1:10 yr	1:100 yr																								
	REVISION:		0		Existing De	velopment	Conditions	to estimat	te time of		a =	732.951	998.071	1174.184	1735.688	MANNING'	Sn=	0.013		BEDDING (CLASS =	В																	
	DESIGNED	BY:	AMP		concentrat	ion					b =	6.199	6.053	6.014	6.014	MINIMUM	COVER:	2.00	m																				
	CHECKED	BY:	1.1]	FILE NUME	BER:	160404536	i			c =	0.810	0.814	0.816	0.820	TIME OF E	NTRY	10	min																				
LOCATION														DR	AINAGE AF	EA																1	PIPE SELEC	TION					
AREA ID	FROM	то	AREA	AREA	AREA	AREA	AREA	С	С	С	С	AxC	ACCUM	AxC	ACCUM.	AxC	ACCUM.	AxC	ACCUM.	T of C	I _{2-YEAR}	I _{5-YEAR}	I _{10-YEAR}	I100-YEAR	QCONTROL	ACCUM.	QACT	LENGTH	PIPE WIDTH	PIPE	PIPE	MATERIAL	CLASS	SLOPE	QCAP	% FULL	VEL.	VEL.	TIME OF
NUMBER	M.H.	M.H.	(2-YEAR) (5	5-YEAR)	(10-YEAR)	(100-YEAR)	(ROOF)	(2-YEAR)	(5-YEAR)	(10-YEAR)	(100-YEAR)	(2-YEAR)	AxC (2YR)	(5-YEAR)	AxC (5YR)	(10-YEAR)	AxC (10YR)	(100-YEAR)	AxC (100YR)	.)						Q _{CONTROL}	(CIA/360)		OR DIAMETE	HEIGHT	SHAPE				(FULL)		(FULL)	(ACT)	FLOW
			(ha)	(ha)	(ha)	(ha)	(ha)	(-)	(-)	(-)	(-)	(ha)	(ha)	(ha)	(ha)	(ha)	(ha)	(ha)	(ha)	(min)	(mm/h)	(mm/h)	(mm/h)	(mm/h)	(L/s)	(L/s)	(L/s)	(m)	(mm)	(mm)	(-)	(-)	(-)	%	(L/s)	(-)	(m/s)	(m/s)	(min)
																																							/
Half of existing Site	CB1	CB2	0.66	0.00	0.00	0.00	0.00	0.90	0.00	0.00	0.00	0.594	0.594	0.000	0.000	0.000	0.000	0.000	0.000	10.00	76.81	104.19	122.14	178.56	0.0	0.0	126.7	153.0	300	300	CIRCULAR	PVC	-	2.00	136.0	93.20%	1.93	1.99	1.28
Half of Existing Site	CB2	STM	0.66	0.00	0.00	0.00	0.00	0.90	0.00	0.00	0.00	0.594	1.188	0.000	0.000	0.000	0.000	0.000	0.000	11.28	72.21	97.88	114.70	167.63	0.0	0.0	238.3	86.0	300	300	CIRCULAR	PVC	-	2.00	136.0	175.24%	1.93	1.93	0.74
																				12.02																			

Ridgewood Avenue 26-May-21

UNC-1

TOTAL AREA HARD SOFT	1460.24 171.81 1288.43 C= 0.28
TANK 1	
TOTAL AREA	2871.63
HARD	546.94
SOFT	2324.69
	C= 0.33
TANK 2	
TOTAL AREA	1880.59
HARD	1444.14
SOFT	436.45
	C= 0.74
TANK 3	
TOTAL AREA	927.62
HARD	672.19
SOFT	255.43
	C= 0.71
TANK 4	
TOTAL AREA	231.09
HARD	0.00
SOFT	231.09
	C= 0.20
RAMP	
TOTAL AREA	452.25
HARD	134.60
SOFT	317.65
	C= 0.41

() Stantos	Mooney'	s Bay - 729 Ri	dgewood	Avenue		S	TORM	SEW	ER		DESIG	N PARAM	ETERS																								
Julie						D	ESIGN	I SHEI	ET		l = a / (t	+b) ^c		(As per C	ity of Otta	va Guidelir	nes, 2012)																				
	DATE:		2021	-06-09			(City of	Ottawa	a)			1:2 yr	1:5 yr	1:10 yr	1:100 yr																						
	REVISION:			0							a =	732.951	998.071	1174.184	1735.688	MANNING	G'Sn=	0.013		BEDDING	G CLASS =	В															
	DESIGNED	BY:	A	MP	FILE NU	JMBER:	1604015	536			b =	6.199	6.053	6.014	6.014	MINIMUM	I COVER:	2.00	m																		
	CHECKED I	BY:		KK							c =	0.810	0.814	0.816	0.820	TIME OF	ENTRY	10	min																		
L	OCATION														DRAINAGE	AREA																PIPE S	ELECTION	4			
AREA ID	FROM	TO	AREA	AREA	AREA	AREA	AREA	С	С	С	С	AxC	ACCUM	AxC	ACCUM.	AxC	ACCUM.	AxC	ACCUM.	T of C	I _{2-YEAR}	I _{5-YEAR}	I _{10-YEAR}	I _{100-YEAR} (Q _{CONTROL} A	CCUM.	Q _{ACT} LE	NGTH P	IPE WIDTH	PIPE	PIPE	MATERIAL	CLASS S	LOPE Q _{CA}	" % FULL	VEL. '	VEL. TIME OF
NUMBER	M.H.	M.H.	(2-YEAR) (5-YEAR) (10-YEAR	(100-YEAF	R (ROOF)	(2-YEAR)	(5-YEAR)	(10-YEAR)	(100-YEAR	(2-YEAR)	AxC (2YR)	(5-YEAR)	AxC (5YR)	(10-YEAR)	AxC (10YR)	(100-YEAR)	AxC (100YR)						G	CONTROL (C	A/360)	OF	R DIAMETE	IEIGHT	SHAPE			(FUL	L)	(FULL) (ACT) FLOW
			(ha)	(ha)	(ha)	(ha)	(ha)	(-)	(-)	(-)	(-)	(ha)	(ha)	(ha)	(ha)	(ha)	(ha)	(ha)	(ha)	(min)	(mm/h)	(mm/h)	(mm/h)	(mm/h)	(L/s)	(L/s)	(L/s)	(m)	(mm)	(mm)	(-)	(-)	(-)	% (L/s	<u>) (-)</u>	(m/s) ((m/s) (min)
ROOF1, ROOF2, ROOF2B, ROOF3,																																					
ROOF4, ROOF5, TANK1 TANK2, TANK3, TANK4,	, BLDG4	STC	0.710	0.00	0.00	0.00	0.47	0.43	0.00	0.00	0.00	0.305	0.305	0.000	0.000	0.000	0.000	0.000	0.000	10.00	76.81	104.19	122.14	178.56	26.3	26.3 9	91.3	3.8	300	300	CIRCULAR	PVC	SDR 28	1.20 105.	3 86.7%	1.50 1	1.51 0.04
RAMP																																					
	STC	STM MH	0.000	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.000	0.305	0.000	0.000	0.000	0.000	0.000	0.000	10.04	76.64	103.97	121.88	178.18	0.0	26.3 9	91.2	8.0	300	300	CIRCULAR	PVC	SDR 28	1.20 105	3 86.5%	1.50 1	1.51 0.09
	STM MH	Ex. STM MH10	1 0.000	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.000	0.305	0.000	0.000	0.000	0.000	0.000	0.000	10.13 10.22	76.31	103.51	121.34	177.37	0.0	26.3 9	90.9	7.9	300	300 300	CIRCULAR	PVC	SDR 28	1.20 105.	3 86.3%	1.50 1	1.51 0.09

File No: 160401536 Project: 729 Ridgewood Avenue - Brigil Homes Date: 31-May-21

SWM Approach: Restrict 100-year peak flows from entire site to 2-year storm as per existing development conditions

Post-Development Site Conditions:

Overall Runoff Coefficient for Site and Sub-Catchment Areas

		Runoff C	oefficient Table					
Sub-catchme	nt		Area		Runoff			Overall
Area Catchment Type	ID / Description		(na) "A"		Coefficient "C"	"A :	x C"	Coefficient
	p							
Tributary to Underground Tank	TANK1	Hard	0.054		0.9	0.05		
Landscaped Area		Soft	0.236		0.2	0.05		
	Sub	total		0.29			0.10	0.33
Controlled Roof	ROOF 1	Hard	0.090		0.9	0.08		
Tower I - Roof Storage		Soft	0.000		0.2	0.00		
	Sub	total		0.09			0.08	0.90
Tributary to Underground Tank	ROOF 2A	Hard	0.060		0.9	0.05		
Tower II - Green Roof - No Storage		Soft	0.000		0.2	0.00		
	Sub	total		0.06			0.05	0.90
Controlled Roof	ROOF 3	Hard	0.110		0.9	0.10		
Tower III - Roof Storage		Soft	0.000		0.2	0.00		
	Sub	total		0.11			0.10	0.90
Tributary to Underground Tank	TANK2	Hard	0.147		0.9	0.13		
Cortyard Area		Soft	0.043		0.2	0.01		
	Sub	total		0.19			0.14	0.74
Controlled Roof	ROOF 4	Hard	0.110		0.9	0.10		
Tower IV - Roof Storage		Soft	0.000		0.2	0.00		
	Sub	total		0.11			0.10	0.90
Tributary to Underground Tank	TANK3	Hard	0.066		0.9	0.06		
Access Road		Soft	0.024		0.2	0.00		
	Sub	total		0.09			0.06	0.71
Controlled Roof	ROOF 5	Hard	0.160		0.9	0.14		
Tower V - Roof Storage	0.1	Soft	0.000	0.40	0.2	0.00	0.44	0.00
	Sub	total		0.16			0.14	0.90
Tributary to Underground Tank	ROOF 2B	Hard	0.010		0.9	0.01		
Tower I - Balcony - No Storage		Soft	0.000		0.2	0.00		
	Sub	total		0.01			0.009	0.90
Tributary to Underground Tank	RAMP	Hard	0.015		0.9	0.01		
Underground Parking Ramp		Soft	0.035		0.2	0.01		
	Sub	total		0.05			0.02	0.41
Tributary to Underground Tank	TANK4	Hard	0.000		0.9	0.00		
Sideyard		Soft	0.020		0.2	0.00		
	Sub	total		0.02			0.00	0.20
Uncontrolled to ridgewood Ave.	UNC-1	Hard	0.017		0.9	0.02		
Front Yard	01	Soft	0.133	0.45	0.2	0.03	0.04	0.00
	Sub	lolai		0.15			0.04	0.28
Total				1.33			0.853	
Overall Runom Coefficient= C:								U.64
Total Roof Storage (to underground tank	() k (to Storm Course)		0.47	ha				
Total Surface Areas to Underground Tai	ik (to Storm Sewer)		1.180	ha				0.69

Total to Underground Tank Total Uncontrolled Area

0.15 ha



Project #160401536, 729 Ridgewood Avenue - Brigil Homes

	City of Otta	nsity iwa	i – av(t + D)	a = b =	1/35.688 6.014	t (min) 10	178.56	
				c =	0.820	20 30	119.95 91.87	
						40	75.15	
						50 60	55.89	
						70 80	49.79 44.99	
						90 100	41.11 37.90	
						110	35.20	
						120	32.09	
	100 YEAR	Modified Rat	ional Metho	d for Entire S	Site			
Subdrain		TANKA					and a sourced A sour	
Subdrair	Area (ha):	0.29				L Tributary to Unde	andscaped Area rground Tank	1
	C:	0.41						
	tc (min)	l (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)		
	10 20	178.56 119.95	59.38 39.89	59.38 39.89				
	30	91.87	30.55	30.55				
	40 50	75.15 63.95	24.99 21.27	24.99 21.27				
	60 70	55.89 49.79	18.59 16.56	18.59 16.56				
	80	44.99	14.96	14.96				
	90 100	41.11 37.90	13.67 12.60	13.67 12.60				
	110 120	35.20 32.89	11.71 10.94	11.71 10.94				
Subdrair	120	BOOE 1	10.04	10.04		0	ontrolled Roof	
Suburali	Area (ha):	0.09			Maximur	n Storage Depth:	150 r	m
	C:	1.00						
	tc (min)	l (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
	10	178.56	44.7	5.5	39.1	23.5	129.4	0.00
	30	91.87	23.0	6.0	17.0	30.6	141.1	0.00
	40 50	75.15 63.95	18.8 16.0	6.0 5.9	12.8 10.1	30.8 30.2	141.4 140.4	0.00
	60	55.89	14.0	5.9	8.1	29.2	138.7	0.00
	80	49.79	12.5	5.8 5.7	5.5	27.9	136.7	0.00
	90 100	41.11 37.90	10.3 9.5	5.6 5.5	4.7 3.9	25.1 23.7	132.1 129.7	0.00
	110	35.20	8.8	5.4	3.4	22.2	127.2	0.00
07000-	Roof Storag	02.00	0.2	0.4	2.0	20.7	12-1.1	0.00
Jaye.	roor otorag	Denth	Head	Discharge	Mag	Manail	Disabarga	
		(mm)	Head (m)	(L/s)	(cu. m)	(cu. m)	Check	
100-year V	Vater Level	141.4	0.14	6.0	30.8	36.0	0.0	
Subdrair	nage Area:	ROOF 2A				Tower II -	Green Roof - N	o Storage
	Area (ha): C:	0.06				I ributary to Unde	rground lank	
[tc	l (100 yr)	Qactual	Qrelease	Qstored	Vstored	1	
l	(min) 10	(mm/hr) 178.56	(L/s) 29.8	(L/s) 29.8	(L/s)	(m^3)	1	
	20	119.95	20.0	20.0				
	40	75.15	12.5	12.5				
	50 60	63.95 55.89	10.7 9.3	10.7 9.3				
	70 80	49.79	8.3	8.3				
	90	41.11	6.9	6.9				
	100	37.90 35.20	б.З 5.9	б.З 5.9				
	120	32.89	5.5	5.5				
Subdrair	nage Area: Area (ha):	ROOF 3 0.11			Maximur	C n Storage Depth:	ontrolled Roof 150 r	ım
	C:	1.00						
	tc (min)	l (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	Depth (mm)	
L	10	178.56	54.6 36.7	5.6	49.0	29.4	130.3	0.00
	20 30	91.87	30.7 28.1	5.9 6.1	22.0	39.6	144.1	0.00
	40 50	75.15 63.95	23.0 19.6	6.1 6.1	16.9 13.4	40.4 40.3	145.2 145.0	0.00
	60	55.89	17.1	6.1	11.0	39.6	144.1	0.00
	70 80	49.79 44.99	15.2	6.0	9.2 7.8	38.6 37.4	142.7	0.00
	90 100	41.11 37.90	12.6 11.6	5.9 5.8	6.7 5.8	36.0 34.6	139.2 137.3	0.00
		05.00	10.9	5.8	5.0	33.1	135.3	0.00
	110	35.20	10.0	57	4.4	21.0	132.0	0.00
	110 120	35.20 32.89	10.1	5.7	4.4	31.6	133.2	0.00
orage:	110 120 Roof Storag	35.20 32.89 Je	10.0	5.7	4.4	31.6	133.2	0.00
rage:	110 120 Roof Storag	35.20 32.89 Je Depth (mm)	10.0 10.1 Head	5.7 Discharge	4.4 Vreq	31.6 Vavail	133.2 Discharge	0.00

	C:	0.19				Tributary to Un	iderground Tank	
	tc	l (2 yr)	Qactual	Qrelease	Qstored	Vstored	1	
	(min) 10	(mm/hr) 76.81	(L/s) 30.0	(L/s) 30.0	(L/S)	(m^3)	4	
	20	52.03	20.3	20.3				
	30 40	40.04	15.7	15.7				
	50	28.04	11.0	11.0				
	60	24.56	9.6	9.6				
	70 80	21.91	8.6	8.6				
	90	18.14	7.1	7.1				
	100	16.75	6.5	6.5				
	110	15.57	6.1	6.1				
		14.30	5.7	5.7				
Sub	drainage Area: Area (ha): C:	ROOF 4 0.11 0.90			Maximu	um Storage Depth:	Controlled Roof 150	mm
	tc	l (2 vr)	Qactual	Orelease	Ostored	Vstored	Depth	1
	(min)	(mm/hr)	(L/s)	(L/s)	(L/s)	(m^3)	(mm)	
	10	/6.81	21.1	3.6	17.6	10.5	91.7	0.00
	20	40.04	14.5	3.8	7.2	13.0	100.0	0.00
	40	32.86	9.0	3.8	5.3	12.7	98.9	0.00
	50	28.04	7.7	3.7	4.0	12.0	96.7	0.00
	60	24.56	6.8	3.6	3.1	11.2	94.1	0.00
	70	21.91	6.0	3.6	2.5	10.4	91.1	0.00
	80 90	19.83	5.5 5.0	3.5 3.4	2.0 1.6	9.5	85.1	0.00
	100	16.75	4.6	3.3	1.3	7.6	82.1	0.00
	110	15.57	4.3	3.3	1.0	6.8	79.2	0.00
	120	14.56	4.0	3.2	0.8	5.9	76.3	0.00
itorage:	Roof Storage							
	j	Depth	Head	Discharge	Vreq	Vavail	Discharge	1
	oor Water Level	(mm)	(m)	(L/s)	(cu. m)	(cu. m)	Check	ł
2-y	oei vvatei Level	100.0	0.10	3.0	13.0	44.U	0.0	1
Sub	drainage Area: Area (ha):	TANK3 0.09				Tributary to Un	Access Road derground Tank	
	C:	0.71						
	C:	l (2 yr)	Qactual	Qrelease	Qstored	Vstored]	
	C: tc (min) 10	I (2 yr) (mm/hr) 76.81	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)]	
	C: (min) 10 20	I (2 yr) (mm/hr) 76.81 52.03	Qactual (L/s) 13.6 9.2	Qrelease (L/s) 13.6 9.2	Qstored (L/s)	Vstored (m^3)]	
	C: (min) 10 20 30	0.71 I (2 yr) (mm/hr) 76.81 52.03 40.04	Qactual (L/s) 13.6 9.2 7.1	Qrelease (L/s) 13.6 9.2 7.1	Qstored (L/s)	Vstored (m [*] 3)]	
	C: (min) 10 20 30 40 50	0.71 I (2 yr) (mm/hr) 76.81 52.03 40.04 32.86 28.04	Qactual (L/s) 13.6 9.2 7.1 5.8 5.0	Qrelease (L/s) 13.6 9.2 7.1 5.8 5.0	Qstored (L/s)	Vstored (m [*] 3)]	
	C: (min) 10 20 30 40 50 60	0.71 1 (2 yr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56	Qactual (L/s) 13.6 9.2 7.1 5.8 5.0 4.4	Qrelease (L/s) 13.6 9.2 7.1 5.8 5.0 4.4	Qstored (L/s)	Vstored (m [*] 3)]	
	C: tc (min) 10 20 30 40 50 60 70	0.71 (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91	Qactual (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.9	Qrelease (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.9	Qstored (L/s)	Vstored (m^3)]	
	C: (min) 10 20 30 40 50 60 70 80	1 (2 yr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91 19.83	Qactual (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.9 3.5	Qrelease (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.9 3.5	Qstored (L/s)	Vstored (m^3)]	
	C: (min) 10 20 30 40 50 60 70 80 90 90	1 (2 vr) (mm/hr) 52.03 40.04 32.86 28.04 24.56 21.91 19.83 18.14 18.14	Qactual (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.9 3.5 3.2 3.2	Qrelease (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.9 3.5 3.2 2.0	Qstored (L/s)	Vstored (m^3)]	
	C: (min) 10 20 30 40 50 60 70 80 90 100 110	i (2 yr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91 19.83 18.14 16.75 15.57	Qactual (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.9 3.5 3.2 3.0 2.8	Qrelease (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.9 3.5 3.2 3.0 2.8	Qstored (L/s)	Vstored (m^3)]	
	C: tc (min) 10 20 30 40 50 60 70 80 90 100 110 120	0.71 (2 yr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91 19.83 18.14 16.75 15.57 14.56	Qactual (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.9 3.5 3.2 3.0 2.8 2.6	Qrelease (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.9 3.5 3.2 3.0 2.8 2.6	Qstored (L/s)	Vstored (m^3)]	
Sub	C: tc (min) 10 20 30 40 50 60 70 80 90 100 110 120 trainage Area: Area (ha): C:	0.71 1 (2 vr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91 19.83 18.14 16.75 14.56 ROOF 5 0.16 0.90	Cactual (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.9 3.5 3.2 3.0 2.8 2.6	Crelease (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.5 3.5 3.2 3.0 2.8 2.6	Qstored (L/s) Maximu	Vstored (m*3)	Controlled Roof	mm
Sub	C: tc (min) 10 20 30 40 50 60 70 90 90 100 120 trainage Area: Area (ha): C: tc (min)	0.71 1 (2 vr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91 19.83 18.14 16.75 15.57 14.56 ROOF 5 0.16 0.90 1 (2 vr)	Qactual [L/s] 13.6 9.2 7.1 5.8 5.0 4.4 3.5 3.2 3.0 2.8 2.6	Orelease (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.9 3.5 3.2 3.0 2.8 2.6	Qstored (L/s) Maximu Qstored (L/s)	Vstored (m^3) um Storage Depth: Vstored (m^3)	Controlled Roof 150 Depth fmm	mm
Sub	C: tc (min) 10 20 30 40 50 50 50 60 70 80 90 90 90 90 90 90 100 110 120 traite t	U.71 I (2 vr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91 19.83 18.14 16.75 15.57 14.56 ROOF 5 0.16 0.90 I (2 vr) (mm/hr) 76.81	Qactual (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.9 3.5 3.2 3.0 2.8 2.6 Qactual (L/s) 30.7 30.7	Orelease (L/s) 13.6 9.2 7.1 5.0 4.4 3.9 3.5 3.2 3.0 2.8 2.6	Qstored (L/s) Maximu Qstored (L/s) 24.7	Vstored (m^3) um Storage Depth: Vstored (m^3) 14.8	Controlled Roof 150 Depth (mm) 90.5	mm 0.00
Sub	C: tc (min) 10 20 30 40 50 60 70 80 60 70 80 60 70 80 100 110 110 110 110 110 20 30 40 50 60 70 80 100 110 20 30 40 50 60 70 80 100 100 100 100 100 100 100	U.1 I (2 vr) (mm/h) 76.81 52.84 52.86 21.91 14.24 56 21.91 14.57 15.57 15.57 15.57 15.57 15.57 15.57 16.81 0.99 I (2 vr) (mm/h) 76.81 15.77 15.85 15.77 15.85 15.77 15.85 15.77 15.85 15.77 15.85 15.77 15.75 15.57	Qactual (L/s) 13.6 9.2 7.1 5.0 4.4 3.9 3.5 3.2 3.0 2.8 2.6 Qactual (L/s) 30.7 20.8	Orelease (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.9 3.5 3.2 3.0 2.8 2.6 Orelease (L/s) 6.1 6.4	Qstored (L/s) Maximu Qstored (L/s) 24.7 14.4	Vstored (m^3) um Storage Depth: Vstored (m^3) 14.8 17.3	Centrolled Roof 150 Depth (mm) 90.5 96.2	mm 0.00 0.00
Sub	C: tc (min) 10 20 30 40 50 60 50 60 70 80 90 100 110 20 50 60 90 90 100 110 20 50 60 60 70 80 90 90 100 120 50 60 60 100 120 50 60 60 100 120 50 60 60 100 120 50 60 60 100 120 50 60 60 100 120 50 60 60 100 120 50 60 60 100 120 50 60 100 100 120 50 60 100 100 120 50 60 100 100 120 50 60 100 100 120 50 60 100 100 120 50 60 100 100 100 100 100 100 100	U.1 I (2 vr) (mm/hr) 76.81 52.03 40.04 22.86 22.85 24.56 24.56 24.56 24.56 24.56 24.56 24.56 24.56 19.81 19.81 19.83 16.75 14.56 ROOF 5 0.16 0.99 I (2 vr) (mm/hr) 76.81 52.03 40.04 14.56 15.57 15.57 15.57 15.57 15.57 15.57 15.57 15.57 14.56 16.76 15.57 15.57 14.56 14.56 16.76 15.57	Qactual (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.9 3.5 2.8 2.6 Qactual (L/s) 30.7 20.8 16.0	Orelease (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.9 3.5 3.2 3.0 2.8 2.6 Orelease (L/s) 6.1 6.4 6.4	Qstored (L/s) Maximu Qstored (L/s) 24.7 14.4 9.6	Vstored (m*3) vstored Depth: Vstored (m*3) 14.8 17.3 17.3	Controlled Roof 150 Depth (mm) 90.5 96.2 96.2	mm 0.00 0.00 0.00
Sub	C: tc (min) 10 20 30 50 50 50 50 50 50 50 50 50 5	I (2 yr) (mm/hr) 76.81 28.03 28.04 24.04 2	Qactual (L/9) 13.6 9.2 7.1 5.8 5.0 4.4 3.9 3.5 3.2 3.0 2.8 2.6 Qactual (L/s) 30.7 20.8 16.0 13.2 13.2	Orelease (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.9 3.5 3.0 2.8 2.6 Orelease (L/s) 6.1 6.4 6.3	Qstored (L/s) Maximu Qstored (L/s) 24.7 14.4 9.6 6.9	Vstored (m*3) m Storaqe Depth: Vstored (m*3) 14.8 17.3 16.5	Controlled Roof 150 90.5 90.2 94.3 94.3	mm 0.00 0.00 0.00 0.00
Sub	C: tc (min) 10 20 30 40 50 60 60 60 60 100 120 40 80 90 90 100 120 40 40 40 40 40 40 40 40 40 4	U.7 (mn/hr) 76.81 76.81 76.81 76.81 76.83 76.83 76.83 16.75 14.56 0.90 14.57 14.56 0.90 14.57 14.56 0.90 14.57 14.56 0.90 14.57 14.56 0.90 14.57 14.56 0.90 14.57 14.56 0.90 14.57 14.55 0.90 14.57 14.55 0.90 14.57 14.55 0.90 14.57 14.55 0.90 14.57 14.55 0.90 14.57 14.55 0.90 14.57 14.55 0.90 14.57 14.55 0.90 14.57 14.55 0.90 14.57 14.55 0.90 14.57 14.55 0.90 14.57 14.55 0.90 14.57 14.55 0.90 14.57 14.55 0.90 14.57 14.55 0.90 14.57 14.55 0.90 14.57 14.55 0.90 14.57 14.55 0.90 14.57 14.55 0.90 14.55 14	Qactual (L/s) 13.6 9.2 7.1 5.5 3.5 3.0 2.8 2.6 Cactual (L/s) 30.7 20.8 16.0 13.2 11.2 9.8	Orelease (L/s) 13.6 9.2 9.2 7.1 8.5 5.0 5.0 2.8 2.6 Orelease (L/s) 6.1 6.3 6.3 6.1 6.1	Qstored (L/s) Maximu Qstored (L/s) 24.7 14.4 9.6 9.5 9.5 1 3.9	Vstored (m*3) Vstored (m*3) 17.3 17.3 17.3 16.5 13.5 13.5	Controlled Roof 150 96.2 94.3 91.5 88.6	mm 0.00 0.00 0.00 0.00
Sub	C: tc (min) 10 20 30 50 60 70 80 90 100 110 110 120 4rainage Area: (min) 10 20 30 100 110 120 50 50 50 50 50 50 50 50 50 5	I (2 yr) I (2 yr) (mmthr) 76:01 76:03 76:03 76:03 76:03 80:04 24:04 2	Qactual (L/9) 13.6 9.2 7.1 5.8 5.0 4.4 3.5 3.2 3.0 2.8 2.6 2.6 2.6 2.6 2.6 2.8 2.6 2.8 2.6 2.8 2.6 10.0 10.2 8 10.0 10.2 10.2 10.2 10.2 10.2 10.2 10.2	Crelease 13.6 13.6 9.2 7.1 5.8 5.0 4.4 3.5 3.2 2.6 Crelease (L6) 6.1 6.1 6.1 6.1 6.1 6.1 6.1 6.1 6.1 6.1	Qstored (L/s) Maximu Qstored (L/s) 24.7 14.4 9.6 6.9 5.1 3.9 3.0	Vstored (m*3) m Storage Depth: Vstored (m*3) 14.8 17.3 16.5 12.5 12.5	Controlled Roof 150 Depth (mm) 90.5 96.2 96.2 96.2 96.3 91.5 88.5 88.5	mm 0.00 0.00 0.00 0.00 0.00 0.00
Sub	C: (c) (c) (c) (c) (c) (c) (c) (c)	U.7 (mu)r) (2 yr) (mu)r) 76.81 52.03 40.04 40.04 22.05 22.05 22.03 40.04 22.05 22.03 22.05 22.03 22.05 22.05 21.91 10.57 1	Qactual (L/s) 13.6 2. 7.1 5.8 5.0 5.0 5.0 5.0 5.2 3.0 2.8 2.6 Cactual (L/s) 30.7 20.8 16.0 13.2 9.8 8.8 8.8 8.7.9	Orelease (L5) 13.6 9.2 7.1 5.8 5.0 4.4 3.9 3.5 3.5 2.8 2.6	Qstored (L/s) Maximu Qstored (L/s) 24.7 24.7 24.7 24.7 24.7 3.0 5.1 3.9 3.0 2.3	Vetored (m*3) w Storace Depth: Vetored (m*3) 14.8 17.3 16.5 15.2 13.0 12.5 11.2	Controlled Roof 150 Depth (mm) 96.2 94.3 94.5 88.5 88.5 88.5 88.5 82.2	mm 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0
Sub	C: tc (min) 10 20 30 40 50 50 60 70 80 90 90 100 110 1120 110 1120 120 20 30 100 100 100 100 100 100 100	U.1 (2 yr) (mm/hr) 76.81 52.03 40.04 28.04 29.04 2	Qactual (L(s) 13.6 9.2 7.1 5.8 5.0 4.4 3.5 3.2 3.0 2.8 2.6 2.6 2.6 2.6 2.6 2.6 2.6 2.6 2.6 2.8 2.8 2.8 2.8 2.8 2.8 2.8 2.8 2.8 2.8	Orelease 113.6 12.6 9.2 7.1 5.8 5.0 4.4 3.2 3.0 2.8 2.6 Orelease (Lb1) 6.1 6.3 6.1 6.3 5.8 5.8 5.8 5.8 5.8 5.8 5.8 5.6	Qstored (L/s) Maximu Qstored (L/s) 24,1 9,6 9,5 1,3,9 3,0 2,3 1,8	Vstored (m*3) m Storage Depth: Vstored (m*3) 16.8 17.3 16.5 12.5 12.5 12.5 12.5 9.8	Controlled Roof 150 90.5 96.2 94.3 95.5 85.5 85.5 85.5 85.2 79.2	mm 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0
Sub	C: (c) (c) (c) (c) (c) (c) (c) (c)	U.7 (mm/hr) 76.81 76.81 76.81 76.81 76.81 19.83 28.04 24.56 24.94 16.14 16.15 16.57 16	Qactual (L/8) 13.6 3.2 7.1 5.8 5.3 3.9 3.9 3.2 3.0 2.8 2.6 Qactual (L/s) 30.7 20.8 16.2 11.2 9.8 8.7.9 7.9 7.3 6.7	Orelease (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.3 3.2 3.0 2.8 2.6 Orelease (L/s) 6.1 6.4 6.3 5.6 5.6 5.6 5.4 5.3	Qstored (L/s) Maximu Qstored (L/s) 24.7 14.4 9.6 6.9 5.1 3.9 3.0 2.3 3.0 2.3 1.4	Vetored (m*3) w Storace Depth: Vetored (m*3) 14.8 17.3 16.5 13.9 12.5 11.2 9.8 8.6	Controlled Roof 150 0epth (mm) 96.2 94.3 94.5 88.5 88.5 88.5 88.5 88.5 87.2 79.2 76.3	mm 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0
Sub	C: (min) 10 20 20 20 20 30 40 50 60 90 90 100 110 120 drainage Area: Area (ha): (min) 10 10 10 10 10 10 10 10 10 10	U.1 (2 yr) (mn/hr) 70.81 52.03 40.04 28.05 28.04 28.05 28.04 28.05 28.05 28.05 28.05 28.05 28.05 28.05 28.05 28.05 28.05 29.05 2	Qactual (L/s) 13.6 13.6 5.0 7.1 5.0 3.2 3.0 2.8 0.6 13.2 11.2 9.8 7.3 6.7 6.2	Cretease (L5) 13.6 13.7 13.8 5.0 4.4 3.9 3.2 3.3 6.1 6.1 6.1 6.1 5.4 5.1	Qstored (L/s) Maximu Qstored (L/s) 24,9 24,9 24,9 24,9 24,9 24,9 24,9 24,9	Vstored (m*3) m Storage Depth: Vstored (m*3) 14.8 17.3 16.5 15.5 12.5 12.5 12.5 12.5 12.5 12.5 12	Controlled Roof 150 90.5 96.2 94.3 94.3 95.5 95.5 85.3 82.2 70.2 77.3 7.3 0	mm 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0
Sub	C: (min) 10 20 20 20 20 30 40 50 60 70 70 80 100 110 100 110 100 110 100 110 100 110 10	U.1 (2 yr) (mn/hr) 76.81 52.03 40.04 22.86 28.04 24.56 21.91 14.55 RCOF 5 0.16 0.99 14.29 14.55 14.56 16.75 14.56	Qactual (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.9 3.5 3.2 3.0 2.8 2.6 Cactual (L/a) 307 20.8 13.2 11.2 11.2 11.2 9.8 8.8 7.9 7.3 6.7 6.7 6.7 5.8	Orelease (L/s) 13.6 9.2 7.1 5.8 5.0 4.4 3.5 3.2 3.2 3.2 3.2 3.2 3.2 6.1 6.4 6.3 6.1 6.2 5.4 5.3 5.4 5.4 5.3 5.4 5.5 5.5 5.5 5.5 5.5 5.5 5.5 5.5 5.5 5.5 5.5	Qatored (L/s) Maximu 24.7 24.7 24.7 24.7 3.0 5.1 3.9 5.1 3.9 3.0 2.2 3.1 8 3.0 2.2 3.1 1.4 1.1 0.9	Vetored (m^3) vetored (m^3) 148 17.3 16.5 15.2 13.9 12.5 13.9 12.5 6.8	Controlled Roof 150 96.2 96.3 94.3 91.5 88.5 83.5 82.2 78.2 78.2 78.3 76.3 76.3 76.3 76.3 76.3 76.3 76.3 76	mm 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0
Sub	C: (min) 10 20 20 20 40 50 60 70 80 90 100 1120 120 120 120 120 120 1	U.1 (mm/hr) 76.81 76.81 76.81 76.81 76.81 76.81 76.81 76.81 76.81 76.81 19.83 19.83 19.83 14.55 ROOF 5 0.16 0.90 1(2 yr) (mm/hr) 76.81 22.04 22.84 22.84 22.84 22.84 22.84 22.85 21.91 14.55 76.81 22.84 23.84 24.95 24.94 24.95 24.94 24.95 25.97 24.95 25.97	Qactual (1/s) 13.6 9.2 7.11 5.8 5.0 4.4 3.9 3.2 3.0 2.8 2.6 207 208 2.6 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.2 3.2 3.2 3.2 3.0 2.8 2.6 2.8 2.6 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 </td <td>Orelease (L/s) 13.6 9.2 7.1 5.8 5.0 3.3 3.3 3.2 3.2 3.2 3.2 3.2 3.2 3.6 6.1 6.1 6.1 6.1 6.3 5.6 5.4 5.3 5.1 4.9</td> <td>Qstored (L/s) Maximu Qstored (L/s) 24,7 4,14 4,6 8,6 9,6 9,5,1 3,9 3,0 2,3 1,8 1,4 1,1 0,9</td> <td>vetored (m*3) m Storace Depth: Vetored (m*3) 14,3 16,5 16,5 16,5 16,5 16,5 16,5 16,5 16,5</td> <td>Controlled Roof 150 000 96.2 94.3 94.3 94.5 88.5 88.5 88.5 88.5 89.5 78.3 82.2 79.2 78.3 73.0 69.5</td> <td>mm 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0</td>	Orelease (L/s) 13.6 9.2 7.1 5.8 5.0 3.3 3.3 3.2 3.2 3.2 3.2 3.2 3.2 3.6 6.1 6.1 6.1 6.1 6.3 5.6 5.4 5.3 5.1 4.9	Qstored (L/s) Maximu Qstored (L/s) 24,7 4,14 4,6 8,6 9,6 9,5,1 3,9 3,0 2,3 1,8 1,4 1,1 0,9	vetored (m*3) m Storace Depth: Vetored (m*3) 14,3 16,5 16,5 16,5 16,5 16,5 16,5 16,5 16,5	Controlled Roof 150 000 96.2 94.3 94.3 94.5 88.5 88.5 88.5 88.5 89.5 78.3 82.2 79.2 78.3 73.0 69.5	mm 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0
Sub	C: (min) 10 20 30 40 50 50 50 60 90 100 110 120 40 40 40 50 50 50 50 50 50 50 50 50 5	U.1 (2 vr) (mn/hr) 76.81 52.03 40.04 28.05 27.95 19.55 1	Cactual (L/s) 13.6 9.2 7.1 5.8 3.5 3.2 3.0 2.8 2.6 2.8 2.8 2.8 2.8 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.1 2.8 2.8 2.8 2.8 2.8 3.0 3.12 3.2 3.2 3.2 3.2 3.2 3.2 3.2 3.2 3.2 3.2 3.2 3.2 3.2 3.2 3.2	Orelease (L/s) 1(Ls) 9.2 7.1 5.8 3.2 3.0 3.2 3.0 2.8 2.6 Orelease (L/s) 6.1 6.2 5.4 5.1 5.1 5.2 5.3 5.4 5.5 6.5 <td>Ostored (L/s) Maximu Qatored Qatored Qatored 24.7 24.7 3.0 3.0 3.0 3.1.8 1.4 0.9 Vreq</td> <td>Vstored (m*3) m Storage Depth: (m*3) 14.8 17.3 14.8 17.3 16.5 16.2 16.2 16.2 16.2 16.5 16.2 16.5 16.2 16.5 16.5 16.5 16.5 16.5 16.5 16.5 16.5</td> <td>Controlled Roof 150 00.5 96.2 96.2 96.2 96.2 96.2 96.2 96.2 96.5 85.3 85.3 85.3 85.3 85.3 85.5 85.3 85.5 85.5</td> <td>mm 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0</td>	Ostored (L/s) Maximu Qatored Qatored Qatored 24.7 24.7 3.0 3.0 3.0 3.1.8 1.4 0.9 Vreq	Vstored (m*3) m Storage Depth: (m*3) 14.8 17.3 14.8 17.3 16.5 16.2 16.2 16.2 16.2 16.5 16.2 16.5 16.2 16.5 16.5 16.5 16.5 16.5 16.5 16.5 16.5	Controlled Roof 150 00.5 96.2 96.2 96.2 96.2 96.2 96.2 96.2 96.5 85.3 85.3 85.3 85.3 85.3 85.5 85.3 85.5 85.5	mm 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0
Sub	C: (c) (c) (c) (c) (c) (c) (c) (c)	U.1 (mm/hr) 1(2 yr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 11.9 11.5	Oactual (L/s) 13.6 9.2 7.1 5.0 3.5 3.6 2.8 2.6 00.7 30.7 30.7 30.7 2.8 2.6 11.2 9.8 8.7.9 7.3 6.7 6.8 Head (m)	Orrelease (L/s) 13.6 9.2 7.1 5.8 5.0 3.5 3.2 3.2 3.2 3.2 6.1 6.4 6.3 6.1 6.0 5.6 5.6 5.7 9.8 10.9 Discharge (L/s)	Qstored (L/s) Maximu Qstored (L/s) 4.4 4.4 4.4 4.6 6.9 5.1 3.9 3.0 2.3 3.1 8 4.4 1.4 1.4 1.4 9.9 Vreq (cu, m)	Vetored (m*3) m Storage Depth: vetored (m*3) 17.3 17.3 16.5 15.2 13.9 2.5 11.2 13.9 8.6 6.8 Vavail (cu, m)	Controlled Roof 150 96.2 94.3 91.5 88.5 83.2 73.6 73.0 69.5 Discharge Check	mm 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0
Sub orage: 2-y	C: tc (min) 10 20 30 40 50 60 70 80 90 90 90 90 90 110 110 120 120 100 110 120 100 40 50 60 70 80 90 90 90 90 90 90 90 90 90 9	U.1 (mn/hr) 76.81 52.03 40.04 32.80 28.04 28.05 28.04 28.05 28.04 28.05 28.04 28.05 28.04 28.05 28.04 28.05 29.04 29	Cactual (L/s) 13.6 9.2 7.1 5.8 3.5 3.2 3.0 2.8 2.6 2.6 2.8 2.6 2.8 2.6 3.0 3.2 3.0.8 20.8 2.8 2.6 4.4 4.4 4.4 4.4 4.4 4.4 4.4 4.4 4.4 4.4 4.4 4.4 4.4 4.4 4.4 4.4 4.4 4.4 4.5 3.0 3.0 2.8 7.8 8.8 7.9 8.8 7.9 8.8 7.9 6.2 5.8 Head (m) 0.10	Qrelease (L/s) 13.6 9.2 7.1 5.8 3.2 3.0 3.2 3.0 2.8 2.6 Qrelease (L/s) 6.1 6.3 6.1 6.3 5.4 5.4 5.4 5.3 5.1 5.4 5.3 5.1 6.4 6.4	Ostored (L/s) Maximu Qstored (L/s) 24.7 14.4 9.6 9.1 3.9 3.0 2.3 1.8 1.4 1.1 0.9 Vreq (cu, m) 17.3	Vstored (m*3) m Storage Depth: Vstored (m*3) 14.8 17.3 17.3 17.3 17.5 15.2 15.2 15.2 15.2 15.2 15.2 15.2 15	Controllad Roof 150 Depth (mm) 90.5 96.2 96.2 96.2 96.2 96.2 96.2 96.2 96.5 85.3 85.3 85.3 85.3 85.3 85.3 85.5 85.5	mm 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0

Project #160401536, 729 Ridgewood Avenue - Brigil Homes Modified Rational Method Calculatons for Storage Project #160401536, 729 Ridgewood Avenue - Brigil Homes Modified Rational Method Calculatons for Storage



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Subdi	rainage Area: Area (ha): C:	0.01 0.90				Tributary to Un	derground Tank
[tc	l (2 yr)	Qactual	Qrelease	Qstored	Vstored]
ļ	(min) 10	(mm/hr) 76.81	1.9	(L/S) 1.9	(L/S)	(m^3)	L
	20	52.03	1.3	1.3			
	30	40.04	1.0	1.0			
	40	32.86	0.8	0.8			
	50	28.04	0.7	0.7			
	70	24.50	0.6	0.6			
	80	19.83	0.5	0.5			
	90	18.14	0.5	0.5			
	100	16.75	0.4	0.4			
	110	15.57	0.4	0.4			
	120	14.56	0.4	0.4			
Subdr	rainage Area:	RAMP				Tributa	ary to Underground Tank
	Area (ha):	0.05				Underground	d Parking Ramp
	U:	0.41					
Ī	tc (min)	I (2 yr)	Qactual	Qrelease	Qstored	Vstored]
L	10	76.81	4.4	4.4	(L/S)	(113)	1
	20	52.03	3.0	3.0			
	30	40.04	2.3	2.3			
	40	32.86	1.9	1.9			
	50	28.04	1.6	1.6			
	50	24.50	1.4	1.4			
	80	19.83	1.2	1.2			
	90	18.14	1.0	1.0			
	100	16.75	1.0	1.0			
	110	15.57	0.9	0.9			
	120	14.56	0.8	0.8			
Subdi	rainage Area: Area (ha):	TANK4 0.02				Tributary to Un	Sideyard derground Tank
Subdi	rainage Area: Area (ha): C:	TANK4 0.02 0.20				Tributary to Un	Sideyard derground Tank
Subdi	rainage Area: Area (ha): C: tc (min)	TANK4 0.02 0.20	Qactual	Qrelease	Qstored	Tributary to Un Vstored (m^3)	Sideyard derground Tank
Subdi	rainage Area: Area (ha): C: tc (min) 10	TANK4 0.02 0.20 I (2 yr) (mm/hr) 76.81	Qactual (L/s) 0.9	Qrelease (L/s) 0.9	Qstored (L/s)	Tributary to Un Vstored (m^3)	Sideyard derground Tank
Subdi	rainage Area: Area (ha): C: tc (min) 10 20	TANK4 0.02 0.20 I (2 yr) (mm/hr) 76.81 52.03	Qactual (L/s) 0.9 0.6	Qrelease (L/s) 0.9 0.6	Qstored (L/s)	Tributary to Un Vstored (m^3)	Sideyard derground Tank
Subdi	rainage Area: Area (ha): C: tc (min) 10 20 30	TANK4 0.02 0.20 i (2 yr) (mm/hr) 76.81 52.03 40.04	Qactual (L/s) 0.9 0.6 0.4	Qrelease (L/s) 0.9 0.6 0.4	Qstored (L/s)	Tributary to Un Vstored (m^3)	Sideyard derground Tank
Subdi	rainage Area: Area (ha): C: tc (min) 10 20 30 40 50	TANK4 0.02 0.20 i (2 yr) (mm/hr) 76.81 52.03 40.04 32.86 28.04	Qactual (L/s) 0.9 0.6 0.4 0.4	Qrelease (L/s) 0.9 0.6 0.4 0.4	Qstored (L/s)	Tributary to Un Vstored (m^3)	Sidevard derground Tank
iubdi	rainage Area: Area (ha): C: tc (min) 10 20 30 40 50 60	TANK4 0.02 0.20 I (2 yr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56	Qactual (L/s) 0.6 0.4 0.4 0.3 0.3	Qrelease (L/s) 0.6 0.4 0.4 0.3 0.3	Qstored (L/s)	Tributary to Un Vstored (m^3)	Sideyard derground Tank
Subdi	rainage Area: Area (ha): C: tc (min) 10 20 30 40 50 60 70	TANK4 0.02 0.20 1 (2 vr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91	Qactual (L/s) 0.9 0.6 0.4 0.4 0.3 0.3 0.2	Crelease (L/s) 0.9 0.6 0.4 0.4 0.3 0.3 0.2	Qstored (L/s)	Tributary to Un Vstored (m^3)	Sideyard derground Tank
Subdi	rainage Area: Area (ha): C: tc (min) 20 30 40 50 60 70 80	TANK4 0.02 0.20 I (2 yr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91 19.83	Qactual (L/s) 0.9 0.6 0.4 0.4 0.3 0.3 0.3 0.2 0.2	Crelease (L/s) 0.9 0.6 0.4 0.4 0.3 0.3 0.2 0.2	Qstored (L/s)	Tributary to Un Vstored (m^3)	Sideyard derground Tank
Subdi	rainage Area: Area (ha): C: tc (min) 10 20 30 40 50 60 70 80 90 90	TANK4 0.02 0.20 I (2 vr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91 19.83 18.14 19.83	Qactual (L/s) 0.9 0.6 0.4 0.3 0.3 0.2 0.2 0.2 0.2	Qrelease (L/s) 0.6 0.6 0.4 0.3 0.3 0.2 0.2 0.2 0.2	Qstored (L/s)	Tributary to Un Vstored (m^3)	Sidevard derground Tank
Gubdr	rainage Area: Area (ha): C: tc (min) 10 20 30 40 50 60 70 80 90 100 140	TANK4 0.02 0.20 I (2 vr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91 19.83 18.14 16.75 45.57	Qactual (L/s) 0.9 0.6 0.4 0.4 0.3 0.2 0.2 0.2 0.2 0.2 0.2 0.2	Crelease (L/s) 0.9 0.6 0.4 0.4 0.3 0.3 0.2 0.2 0.2 0.2 0.2	Qstored (L/s)	Tributary to Un Vstored (m^3)	Sidevard derground Tank
Subdi	rainage Area: Area (ha): C: tc (min) 10 20 30 40 50 60 70 80 90 100 110 120	TANK4 0.02 0.20 1 (2 vr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91 19.83 18.14 16.75 15.57 14.56	Qactual (L/s) 0.9 0.6 0.4 0.4 0.3 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2	Crelease (L/s) 0.9 0.6 0.4 0.4 0.3 0.2 0.2 0.2 0.2 0.2 0.2 0.2	Qstored (L/s)	Tributary to Un Vstored (m^3)	Sidevard derground Tank
Subdr	rainage Area: Area (ha): C: t (min) 10 20 30 40 50 60 70 80 90 100 110 120 20 20 30 40 50 60 70 80 90 100 20 30 40 50 60 70 80 90 100 20 30 40 50 60 70 80 90 100 20 30 40 50 60 70 80 90 100 20 30 40 50 60 70 80 90 100 20 30 40 50 60 70 80 90 100 20 30 70 80 80 80 90 100 20 30 80 80 80 80 80 80 80 80 80 8	TANK4 0.02 0.20 I (2 yr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91 19.83 18.14 16.75 15.57 14.56 Site Area tribut:	Qactual (L/s) 0.9 0.6 0.4 0.3 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2	Qrelease (L/s) 0.9 0.6 0.4 0.3 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2	Qstored (L/s)	Tributary to Un Vstored (m^3)	Sidevard derground Tank
Subdr	rainage Area: Area (ha): C: tc (min) 10 20 30 40 50 60 70 80 80 90 90 110 120 rainage Area: Area (ha):	TANK4 0.02 0.20 1 (2 vr) (mm/hr) 76.81 52.03 40.04 32.86 28.04 24.56 21.91 19.83 18.14 16.75 15.57 14.56 Site Area tributz 1.16	Qactual (L/s) 0.9 0.6 0.4 0.4 0.3 0.3 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2	Qrelease (L/s) 0.9 0.6 0.4 0.3 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2	Qstored (L/s)	Tributary to Un Vstored (m^3)	Sideward derground Tank
Subdr Subdr	rainage Area: Area (ha): C: tc (min) 20 30 40 50 60 60 60 60 60 60 60 70 80 80 90 00 110 110 120 120 120 120 120	TANK4 0.02 0.2 112 vr) (mm/hr) 76.81 52.03 40.04 22.86 22.90 24.56 21.91 19.83 18.14 16.75 15.57 14.56 Site Area tributs 1.16	Qactual (L/s) 0.9 0.6 0.4 0.4 0.4 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2	Qrelease (L/s) 0.9 0.6 0.4 0.3 0.2 0.2 0.2 0.2 0.2 0.2 round Tank	Qstored (L/s)	Tributary to Un Vstored (m^3)	Sideward derground Tank
Subdr	rainage Area: Area (na): C: (min) 10 20 30 40 50 60 70 80 90 100 110 120 rainage Area: Area (na): 40 50 60 70 10 10 10 10 10 10 10 10 10 1	TANK4 0.02 1 (2 vr)	Cactual (L/s) 0.6 0.4 0.4 0.3 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2	Qrelease (L/s) 0.9 0.6 0.4 0.3 0.2 <td>Qstored (L/s) Qstored (L/s)</td> <td>Tributary to Un Vstored (m^3) Vstored (m^3)</td> <td>Sideward derground Tank</td>	Qstored (L/s) Qstored (L/s)	Tributary to Un Vstored (m^3) Vstored (m^3)	Sideward derground Tank
Subdr [rainage Area: Area (ha): C: tc (min) 20 30 40 50 60 70 80 90 90 100 110 120 tanage Area: Area (ha): tr (min) 10 10 10 10 10 10 10 10 10 10	TANK4 0.02 0.20 1 (2 yr) (mm/hr) 76.81 57.83 40.04 42.86 28.04 22.86 28.04 22.86 21.91 16.75 71.55 71.55 71.455 53.55 8.56 Area tributs 1.16 12 yr) (mm/hr) 76.81	Qactual (L/s) 0.9 0.6 0.4 0.3 0.2	Qrelease (L/s) 0.9 0.6 0.4 0.3 0.2 0.4 0.5 0.6 0.7 0.8 0.9 0.10 0.2 0.2 </td <td>Qstored (L/s) Qstored (L/s) 0.00</td> <td>Tributary to Un Vstored (m*3) Vstored (m*3)</td> <td>Sideward derground Tank</td>	Qstored (L/s) Qstored (L/s) 0.00	Tributary to Un Vstored (m*3) Vstored (m*3)	Sideward derground Tank
Subdr [rainage Area: Area (na): C: tc (min) 10 20 30 40 50 60 90 90 100 120 rainage Area: Area (na): tc rainage Area: tc (min) 10 10 10 10 10 10 10 10 10 10	TANK4 0.02 0.20 1 (2 vr) (mm/hr) 76 81 52 00 28 04 0.04 40 04 40 04 40 04 40 04 28 04 28 04 28 04 28 04 28 04 28 04 28 04 28 15 15 57 14 55 55 31 15 57 14 55 55 31 1 (2 vr) (mm/hr) 76 81 52 03	Qactual (L/s) 0.9 0.9 0.4 0.3 0.2 0.3 0.4 0.5 74.70	Orelease (L/s) 0.9 0.6 0.4 0.3 0.2 0.3 0.4 0.5 0.5 0.6 0.74.70	Qstored (L/s) Qstored (L/s) 0.00	Tributary to Un Vstored (m*3) Vstored (m*3) 0.00	Sideward derground Tank
Subdr [Subdr	rainage Area: Area (n2): C: te (min) 10 10 10 10 30 40 50 60 70 80 90 90 90 90 90 90 110 120 Trainage Area: Area (na): te (min) 10 10 10 10 10 10 10 10 10 10	TANK4 0.02 0.20 0.20 11(2 yr) (mm/hr) 76 81 52.03 52.03 40.04 32.86 22.86 24.56 21.91 18.14 16.75 15.57 14.56 11(2 yr) (mm/hr) 76 81 52.03 40.04 40.04	Qactual (L/s) 0.9 0.6 0.4 0.3 0.2 0.3 0.4 0.5 100.51 74.70 61.76	Qrelease (L/S) 0.9 0.6 0.4 0.3 0.2 0.3 0.4 0.5 0.6 0.78 0.78 0.79 0.70 0.70 0.70 0.70 0.70	Qstored (L/s) Qstored (L/s) 0.00 0.00	Tributary to Un Vstored (m*3) Vstored (m*3)	Sideward deraround Tank
Subdr [rainage Area: Area (ha): C: tc (min) 10 10 10 10 10 20 30 40 50 60 60 60 60 60 60 60 60 60 6	TANK4 O.02 0.02 0.20 1 (2 vr) (mm/hr) (mm/hr) 52.03 40.04 32.86 28.04 24.56 24.59 18.33 18.33 18.15.57 14.55 55.71 1.66 12.97 (mm/hr) 76.81 52.03 32.86	Qactual (L/p) 0.9 0.6 0.4 0.3 0.2 0.3 0.4 0.5 0.7 0.7 0.7 0.7 0.7 <td>Orelease (L/s) 0.9 0.4 0.4 0.4 0.2 0.3 0.4 0.51 74.70 61.78 9.73</td> <td>Ostored (L/s) Ostored (L/s)</td> <td>Tributary to Un Vetored (m*3) 0.00 0.00 0.00</td> <td>Sideward derground Tank]</td>	Orelease (L/s) 0.9 0.4 0.4 0.4 0.2 0.3 0.4 0.51 74.70 61.78 9.73	Ostored (L/s) Ostored (L/s)	Tributary to Un Vetored (m*3) 0.00 0.00 0.00	Sideward derground Tank]
Subdi [Subdi	rainage Area: Area (na): te (min) 10 10 10 10 10 30 40 40 50 60 100 110 120 120 rainage Area: Area (na): te (min) 10 10 10 10 10 10 10 10 10 10	TANK4 0.12 0.20 1 (2 vr) (mm/hr) 76.81 52.03 40.045 22.04 24.05 22.04 24.05 22.04 24.05 22.04 24.05 22.04 24.05 15.57 14.50 53 16.75 15.57 14.50 53 16.75 15.57 14.50 53 16.27 11.27	Qactual (L/s) 0.9 0.6 0.4 0.3 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.3 100.51 74.70 51.73 51.73 51.74 0.9 43.85	Qrelease (L/s) 0.9 0.6 0.4 0.3 0.2 0.3 0.4 0.5 0.5 0.5 0.4 0.4 0.4	Qstored (L/s) (L/s) 0.00 0.00 0.00 0.00 0.00 0.00	Vstored (m^3) Vstored 000 000 000 000 000 000 000 000 000 000 000	Sideward deraround Tank
Subdi [Subdi	rainage Area: Area (hai): C: tc (min) 10 10 10 10 10 20 30 40 50 60 60 60 80 90 90 90 90 90 90 90 90 90 9	TANK4 O.02 0.02 0.20 1 (2 vr) (mm/hr) 76.81 52.86 32.86 23.96 24.50 24.50 11.57 14.55 11.57 14.55 11.55 14.55 11.2 vr) (mm/hr) 76.81 52.06 22.96 22.96 22.91 22.95	Qactual (L/s) 0.9 0.6 0.4 0.4 0.3 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.4 (L/s) (U/s) (1/s) (1/s) <td< td=""><td>Orelease (L/s) 0.9 0.6 0.4 0.3 0.2 0.3 0.4 0.5 0.5 0.5 0.5 0.5 <td>Qstored (L/s) Qstored (L/s) 0.00 0.00 0.00 0.00 0.00 0.00 0.00</td><td>Tributary to Un Vetored (m*3) 0.00 0.00 0.00 0.00 0.00 0.00</td><td>Sideward derground Tank]</td></td></td<>	Orelease (L/s) 0.9 0.6 0.4 0.3 0.2 0.3 0.4 0.5 0.5 0.5 0.5 0.5 <td>Qstored (L/s) Qstored (L/s) 0.00 0.00 0.00 0.00 0.00 0.00 0.00</td> <td>Tributary to Un Vetored (m*3) 0.00 0.00 0.00 0.00 0.00 0.00</td> <td>Sideward derground Tank]</td>	Qstored (L/s) Qstored (L/s) 0.00 0.00 0.00 0.00 0.00 0.00 0.00	Tributary to Un Vetored (m*3) 0.00 0.00 0.00 0.00 0.00 0.00	Sideward derground Tank]
Subdi [Subdi	rainage Area: Area (ha): C: tc (min) 10 20 30 40 40 40 40 40 40 40 40 40 4	TANK4 0.02 0.20 1 (2 vr) (mm/hr) 76.81 52.03 40.045 22.04 24.045 22.04 24.045 22.04 24.045 22.04 24.045 22.04 24.05 15.57 14.50 55 15.57 14.50 55 15.57 14.50 55 15.57 14.50 55 15.57 14.50 55 15.57 14.50 55 15.57 14.50 55 15.57 14.50 55 15.57 14.50 15.57 15.5	Qactual (L/s) 0.9 0.6 0.4 0.3 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.3 0.5 100.51 74.70 61.78 53.73 48.09 43.85 40.50 37.75	Orelease (L/s) 0.9 0.6 0.4 0.3 0.2 0.3 0.4 0.5 0.5 0.5 0.5 0.5 <td>Castored (L/s) (L/</td> <td>Vstored (m^3) Vstored 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00</td> <td>Sideward deraround Tank</td>	Castored (L/s) (L/	Vstored (m^3) Vstored 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00	Sideward deraround Tank
Subdr [Subdr	rainage Area: Area (ha): C: tc (min) 10 10 10 10 10 30 40 50 60 60 60 60 60 60 70 70 70 70 70 70 70 70 70 7	I I	Cactual (L/s) 0.9 0.6 0.4 0.3 0.2 0.3 0.4 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5	Orelease (L/s) 0.9 0.6 0.4 0.3 0.2 0.37.75	Qstored (L/s) Content (L/s) Content Co	Tributary to Un (m*3) (m*3) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.	Sideward derground Tank
Subdr [Subdr	rainage Area: Area (ha): C: (min) 10 20 30 40 40 40 40 40 40 40 40 40 40 40 40 40	TANK4 0.02 0.20 1 (2 vr) (mm/hr) 76.81 52.03 40.40 40.40 24.66 22.96 22.94 40.40 24.96 22.95 14.56 15.57 14.56 15.57 14.56 Site Area tribute 1.16 12 vr) (mm/hr) 76.81 15.57 14.50 55 15.57 14.50 20.00 15.57 14.50 15.57 14.50 20.00 15.57 14.50 20.00 15.57 14.50 20.00 15.57 14.50 20.00 15.57 14.50 20.00 15.57 14.50 20.00 15.57 14.50 20.00 15.57 14.50 20.00 15.57 14.50 20.00 15.57 14.50 20.00 15.57 14.50 20.00 15.57 14.50 20.00 15.57 14.50 20.00 15.57 14.50 20.00 15.57 14.50 20.00 15.57 14.50 20.00 15.57 14.50 20.00 15.57 14.50 20.00 15.57 14.50 20.00 15.57 14.50 15.57 14.50 15.57 14.50 15.57 15.57 14.50 15.57 15.5	Cactual (L/s) 0.9 0.6 0.4 0.3 0.2 0.4 0.51 100.51 100.51 102.51 103.41 104.52 105.53 107.53 107.94 103.41	Orelease (L/s) 0.9 0.6 0.4 0.3 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.3 0.4 0.5 0.74 74 70.61 74 70.61 74 70.794 43.85 40.50 37.75 33.41	Qstored (L/s) Qstored (L/s) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.	Vstored (m^3) Vstored 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00	Sideward deraround Tank
ubdi ubdi	rainage Area: Area (ha): C: tc (min) 10 10 10 10 20 30 40 50 60 70 70 70 70 70 70 70 70 70 7	TANK4 0,02 0,20 1 (2 vr) (mm/h7) 76,81 52,86 2 8,04 2 8,04 2 8,04 2 8,04 2 8,04 2 8,04 1 8,33 1 8,83 1 8,83 1 8,83 1 8,83 1 8,83 1 8,83 1 8,83 1 8,57 1 4,56 2 8,04 2 2,03 4 0,04 3 2,86 4 3,286 1 8,57 1 4,56 2 8,04 2 8,04 2 4,56 2 8,04 2 4,56 2 8,04 2 4,56 2 1,97 1 9,83 1 8,14 2 8,04 2 8,04 2 4,56 2 1,97 1 9,83 1 8,14 2 8,04 2 8,0	Cactual (L/s) 0.9 0.6 0.4 0.3 0.2 0.3 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5	Orelease (L/s) 0.9 0.6 0.4 0.3 0.2 0.3 0.4 0.51 0.51 0.53 0.53 0.53 0.63 0.63 0.63 0.63 <td>Qstored (L/s) (L/s) (00) 0.00 0.00 0.00 0.00 0.00 0.00 0.0</td> <td>Visioned (m*3) Valored (m*3) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00</td> <td>Sideward derground Tank</td>	Qstored (L/s) (L/s) (00) 0.00 0.00 0.00 0.00 0.00 0.00 0.0	Visioned (m*3) Valored (m*3) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00	Sideward derground Tank

Project #160401536, 729 Ridgewood Avenue - Brigii Homes Modified Rational Method Calculatons for Storage

Project #160401536, 729 Ridgewood Avenue - Brigil Homes Modified Rational Method Calculatons for Storage

Subdra	inage Area: Area (ha): C:	ROOF 2B 0.01 1.00				Tower I - Balcony - No Storage Tributary to Underground Tank	
	tc	l (100 yr)	Qactual	Qrelease	Qstored	Vstored	
	10 (min)	(mm/nr) 178.56	(L/S)	(L/S)	(L/S)	(m^3)	
	20	119.95	3.3	3.3			
	30	91.87	2.6	2.6			
	40	75.15	2.1	2.1			
	50	55.89	1.8	1.8			
	70	49.79	1.4	1.4			
	80	44.99	1.3	1.3			
	90	41.11	1.1	1.1			
	100	37.90	1.1	1.1			
	120	32.89	0.9	0.9			
Subdra	inage Area: Area (ha): C:	RAMP 0.050 0.51				Tributary to Underground Tank Underground Parking Ramp	(
	tc	l (100 yr)	Qactual	Qrelease	Qstored	Vstored	
	(min)	(mm/hr)	(L/s)	(L/s)	(L/s)	(m^3)	
	10	178.56	12.7	12.7			
	30	91.87	6.5	6.5			
	40	75.15	5.4	5.4			
	50	63.95	4.6	4.6			
	70	49.79	4.0	4.0			
	80	44.99	3.2	3.2			
	90	41.11	2.9	2.9			
	100	37.90	2.7	2.7			
	110	35.20	2.5	2.5			
Subdra	inage Area: Area (ha): C:	TANK4 0.02 0.25				Sideyard Tributary to Underground Tank	
Subdra	inage Area: Area (ha): C: tc	TANK4 0.02 0.25	Qactual	Qrelease	Ostored	Sidevard Tributary to Underground Tank	
Subdra	inage Area: Area (ha): C: tc (min)	TANK4 0.02 0.25 I (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Sideyard Tributary to Underground Tank Vstored (m^3)	
Subdra	inage Area: Area (ha): C: tc (min) 10	TANK4 0.02 0.25 I (100 yr) (mm/hr) 178.56	Qactual (L/s) 2.5	Qrelease (L/s) 2.5	Qstored (L/s)	Sidevard Tributary to Underground Tank Vstored (m^3)	
Subdra	inage Area: Area (ha): C: tc (min) 10 20 30	TANK4 0.02 0.25 i (100 yr) (mm/hr) 178.56 119.95 91.87	Qactual (L/s) 2.5 1.7 1.3	Qrelease (L/s) 2.5 1.7 1.3	Qstored (L/s)	Sideyard Tributary to Underground Tank Vstored (m^3)	
Subdra	inage Area: Area (ha): C: tc (min) 10 20 30 40	TANK4 0.02 0.25 I (100 yr) (mm/hr) 178.56 119.95 91.87 75.15	Qactual (L/s) 2.5 1.7 1.3 1.0	Qrelease (L/s) 2.5 1.7 1.3 1.0	Qstored (L/s)	Sideyard Tributary to Underground Tank Vstored (m*3)	
Subdra	inage Area: Area (ha): C: (min) 10 20 30 40 50	TANK4 0.02 0.25 I (100 yr) (mm/hr) 178.56 119.95 91.87 75.15 63.95	Qactual (L/s) 2.5 1.7 1.3 1.0 0.9	Qrelease (L/s) 2.5 1.7 1.3 1.0 0.9	Qstored (L/s)	Sideyard Tributary to Underground Tank Vstored (m^3)	
Subdra	inage Area: Area (ha): C: (min) 10 20 30 40 50 60 70	TANK4 0.02 0.25 I (100 yr) (mm/hr) 178.56 119.95 91.87 75.15 63.95 55.89 49.70	Qactual (L/s) 2.5 1.7 1.3 1.0 0.9 0.8 0.7	Qrelease (L/s) 2.5 1.7 1.3 1.0 0.9 0.8 0.7	Qstored (L/s)	Sideyard Tributary to Underground Tank Vetored (m^3)	
Subdra	inage Area: Area (ha): C: (min) 10 20 30 40 50 60 70 80	TANK4 0.02 0.25 1 (100 yr) (mm/hr) 178.56 119.95 91.87 75.15 63.95 55.89 49.79 44.99	Qactual (L/s) 2.5 1.7 1.3 1.0 0.9 0.8 0.7 0.6	Qrelease (L/s) 2.5 1.7 1.3 1.0 0.9 0.8 0.7 0.6	Qstored (L/s)	Sideyard Tributary to Underground Tank Vetored (m^3)	
Subdra	inage Area: Area (ha): C: (min) 10 20 30 40 50 60 70 80 90	TANK4 0.02 0.25 1(100 yr) (mm/hr) 178.56 119.95 91.87 75.15 63.95 55.89 49.79 44.99 44.11	Qactual (L/s) 2.5 1.7 1.3 1.0 0.9 0.8 0.7 0.6 0.6	Qrelease (L/s) 2.5 1.7 1.3 1.0 0.9 0.8 0.7 0.8 0.7 0.6 0.6	Qstored (L/s)	Sideyard Tributary to Underground Tank Vstored (m*3)	
Subdra	inage Area: Area (ha): C: (min) 10 20 30 40 50 60 70 80 90 100 110	TANK4 0.02 0.25 I (100 vr) (mm/hr) 178.56 119.95 91.87 75.15 63.95 55.89 49.79 44.99 44.99 44.99 41.11 37.90 25 20	Qactual (L/s) 2.5 1.7 1.3 1.0 0.9 0.8 0.7 0.6 0.6 0.6 0.5	Qrelease (L/s) 2.5 1.7 1.3 1.0 0.9 0.8 0.7 0.6 0.6 0.6 0.5	Qstored (L/s)	Sideyard Tributary to Underground Tank Vstored (m*3)	
Subdra	inage Area: Area (ha): C: tc (min) 10 20 30 40 50 60 70 80 90 100 110 100 110 20 40 50 60 70 100 100 100 101 20 30 40 50 50 50 50 60 70 70 70 70 70 70 70 70 70 7	TANK4 0.02	Qactual (L/s) 2.5 1.7 1.3 1.0 0.9 0.8 0.7 0.6 0.6 0.5 0.5 1.5 1.5	Qrelease (Us) 2.5 1.7 1.0 0.8 0.7 0.6 0.6 0.6 0.5 0.5 0.5 0.5 ground Tank	Qstored (L/s)	Sideyard Tributary to Underground Tank Vstored (m*S)	
Subdra	inage Area: Area (ha): C: (min) 10 20 30 40 50 60 70 80 90 90 90 90 90 100 110 120 inage Area: Area (ha):	TANK4 0.02 0.25 ((100 yr) (mm/hr) 178.56 119.95 91.87 75.15 63.95 63.95 63.95 63.95 63.95 63.95 63.95 63.95 63.95 55.89 44.99 44.99 44.99 44.99 44.99 44.99 55.00 22.89 Site Area tribu 1.16	Qactual (L/s) 2.5 1.7 1.3 0.9 0.8 0.7 0.6 0.6 0.5 0.5 0.5 0.5 tary to Under Qactual	Orelease (L5) 2,5 1,7 1,3 0,9 0,8 0,6 0,6 0,6 0,5 0,5 0,5 0,5 0,5 0,5 0,5 0,5 0,5 0,5 0,5 0,5 0,5	Qstored (L/s)	Sideyard Tributary to Underground Tank Vstored (m*3)	
Subdra	inage Area: Area (ha): C: (min) 10 20 30 40 40 50 60 60 70 80 90 100 110 120 100 110 120 100 10	TANK4 0.02 0.25 (1100 vr) (mm/hr) 178,56 178,56 91,87 75,15 63,995 55,89 49,79 41,11 37,90 35,20 32,29 Site Area tribu 1.16	Qactual 2.5 1.7 1.3 1.0 0.9 0.6 0.6 0.5 0.5 0.5 0.5 1.7 Under	Orrelease (Us) 2.5 1.7 1.3 1.0 0.8 0.7 0.6 0.5 0.5 0.5 ground Tank Orelease (Us)	Qstored (L/s) Qstored (L/s)	Sideyard Tributary to Underground Tank Vstored (m*3)	
Subdra	inage Area: Area (ha): C: (min) 10 20 30 40 50 60 40 50 60 70 80 90 100 110 110 100 40 50 60 110 100 20 100 100 20 100 100	TANK4 0.02 1 (100 vr) (mm/hr) 178.56 191.87 63.95 55.89 44.99 41.11 37.90 32.89 Site Area tribu 1.16 1/10.95	Qactual (L/s) 2.5 1.7 1.3 1.0 0.8 0.6 0.6 0.6 0.6 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5	Orelease (L/s) 2.5 1.7 1.3 1.0 0.9 0.8 0.7 0.6 0.5 0.5 0.5 0.5 0.5 0.5 0.7 0.6 0.7 0.6 0.5 0.5 0.5 0.5 0.7 0.8 0.7 0.6 0.5 0.5 0.5 0.5 0.7 0.8 0.7 0.8 0.7 0.8 0.5	Qstored (L/s) Qstored (L/s) 82.98 7.01	Sideyard Tributary to Underground Tank Vstored (m*3) 49.79 43 m*ha	
Subdra	inage Area: Area (ha): C: (min) 20 30 40 40 50 60 70 80 80 90 90 100 110 120 inage Area: Area (ha): tc (min) 120 100 110 120 100 100 110 120 100 100	TANK4 0.02 0.25 1(100 vr) (mm/hr) 178.56 3.95 55.89 44.79 37.515 63.95 55.89 44.79 37.50 35.20 32.20 32.20 35.20 35.20 32.20 35.20 3	Qactual (L/s) 2.5 1.7 1.3 1.0 0.8 0.7 0.6 0.5 0.5 0.5 0.5 0.5 0.5 1.0 0.2 <th>Orelease (L/s) 2.5 1.7 1.3 1.0 0.9 0.8 0.7 0.6 0.5 0.5 0.5 0.5 0.7 1.0 0.8 1.0 0.9 1.0 0.7 1.0 0.6 0.5 0.5 1.7 0.7 1.7 177.25 177.25 177.25 1.47</th> <th>Qstored (L/s) Qstored (L/s) 82.98 7.01 0.00</th> <th>Sideyard Tributary to Underground Tank Vstored (m*3) 40,0,1 6,0,1 43 m*/ha 6,0,1</th> <th></th>	Orelease (L/s) 2.5 1.7 1.3 1.0 0.9 0.8 0.7 0.6 0.5 0.5 0.5 0.5 0.7 1.0 0.8 1.0 0.9 1.0 0.7 1.0 0.6 0.5 0.5 1.7 0.7 1.7 177.25 177.25 177.25 1.47	Qstored (L/s) Qstored (L/s) 82.98 7.01 0.00	Sideyard Tributary to Underground Tank Vstored (m*3) 40,0,1 6,0,1 43 m*/ha 6,0,1	
Subdra	inage Area: Area (ha): C: (min) 20 30 40 50 60 70 80 90 90 100 110 120 100 110 120 100 120 100 10	TANK4 0.02 0.25 1(100 yr) (mm/hr) 178.56 191.87 75.15 63.95 55.89 49.79 41.11 37.90 32.89 Site Area tribu 1(100 yr) (mm/hr) 178.56 119.95 91.87 75.15	Qactual (L/s) 2.5 1.7 1.3 1.0 0.9 0.8 0.7 0.6 0.5 0.5 0.5 0.5 0.5 1.10 Qactual (L/s) 260.23 184.25 125.70	Orelease (L/S) 2.5 1.7 1.3 1.0 0.9 0.8 0.7 0.6 0.5 0.5 0.5 0.5 0.5 0.5 0.7 1.7 0.7 0.6 0.5 0.5 0.5 1.7	Qstored (L/s) Qstored (L/s) 82.98 7.01 0.00	Sideyard Tributary to Underground Tank Vetored (m*3) 49.79 43.1 m ³ /na 0.00 0.00	
Subdra	inage Area: Area (ha): C: (min) 10 20 30 40 50 60 70 80 90 100 110 120 inage Area: Area (ha): tc (min) 120 100 110 120 100 100 110 120 100 100	TANK4 0.02 0.25 1(100 yr) (mm/br) 178.56 1176.56 58.90 49.79 44.99 41.11 37.90 55.80 32.89 58 te Area tribu 1.16 178.56 118.95 178.56 1178.55 53.95 53.95	Qactual (L/s) 2.5 1.7 1.3 1.0 0.9 0.8 0.7 0.6 0.5 0.5 0.5 1.5 260.23 184.25 147.66 125.70 110.85	Orelease (L/8) 2.5 1.7 1.0 0.8 0.8 0.8 0.6 0.5 0.5 0.5 ground Tank Orelease (L/8) 177.25 147.66 147.65 10.85	Qstored (L/s) Qstored (L/s) 82-98 7.01 0.00 0.00	Sideyard Tributary to Underground Tank	
Subdra	inage Area: Area (ha): C: (min) 20 30 40 50 60 70 80 90 90 90 90 90 90 90 90 90 110 120 Area (ha): (min) 10 20 30 40 50 50 40 50 50 40 50 50 40 50 50 40 50 50 50 50 50 50 50 50 50 5	TANK4 0.02 0.25 0.25 1/100 vr) (mm/hr) 178.56 119.95 119.95 55.89 44.99 14.11 37.90 35.20 32.89 35.20 Site Area tribu 1.16 1/10.95 18.7 75.15 5.89 32.89 35.20 1.16 1.16 1.16 5.68 97.815 5.68 97.85 5.68 95.85 5.89	Qactual (L/s) 2.5 1.7 1.3 1.0 0.9 0.8 0.7 0.6 0.5 0.5 0.5 0.5 0.5 1.0 Qactual (L/s) 260.23 184.25 147.66 110.85 100.03	Orelease (Us) 2.5 1.7 1.3 1.0 0.8 0.7 0.6 0.5 0.6 0.7 10.0 0.0.03 0.0.03	Qstored (L/s) Qstored (L/s) 82.98 7.01 0.00 0.00 0.00 0.00	Sideyard Tributary to Underground Tank Vstored (m^5) 49.79 49.79 49.79 43.70 0.00 0.00 0.00 0.00	
Subdra	inage Area: Area (ha): C: (min) 10 10 20 30 40 50 60 60 70 80 90 90 110 110 120 inage Area: Area (ha): t c (min) 120 120 120 120 120 120 120 120 120 120	TANK4 0.02 0.28 0.28 0.28 0.28 0.28 0.28 0.28	Qactual (L/s) 2.5 1.7 1.0 0.0 0.8 0.7 0.6 0.5 0.5 0.5 0.5 1.7 260.23 184.25 147.66 126.65 100.03 100.03 100.72 95.10	Orelease (L/8) 2.5 1.7 1.3 1.0 0.7 0.8 0.6 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.6 0.772 177.25 100.83 100.83 00.72 9.72	Ostored (L/s) 62:98 7:01 0:00 0:00 0:00 0:00	Sideyard Tributary to Underground Tank	
Subdra	inage Area: Area (ha): C: (min) 10 10 10 10 10 10 10 10 10 10	TANK4 0.02 0.28 0.28 176.09 vr) (mm/hr) 178.56 119.95 178.56 3.95 55.89 3.95 32.89 35.20 32.89 35.20 Site Area tribut 1.16 178.56 100 vr) (mm/hr) 176.56 1.10.95 55.89 9.65 55.89 9.76 57.36 55.89 49.79 44.99 44.91	Qactual (L/s) 2.5 1.7 1.7 1.7 1.7 1.7 1.7 1.7 0.8 0.7 0.6 0.5 0.5 0.5 1.0 260.23 184.25 110.65 110.65 110.65 110.77 110.77 110.77 110.77	Orelease (L/s) 2.5 1.7 1.3 1.0 0.8 0.7 0.6 0.5 0.5 0.5 0.5 0.5 0.5 0.6 0.7 ground Tank 177.26 147.60 147.60 140.85 10.03 91.72 85.10 79.66	Qstored (L/s) Qstored (L/s) 82.98 7.01 0.00 0.00 0.00 0.00 0.00 0.00	Sideyard Tributary to Underground Tank Vstored (m*3) 49.79 49.79 43 m ³ ha 69.79 00 00 00 00 000 000 000 000	
Subdra	inage Area: Area (ha): (min) 10 20 20 20 20 20 20 20 20 20 2	Takkid 0.02 0.25 1(100 yr) (mm/hr) 178.56 119.95 91.87 75.15 65.99 44.99 44.99 44.91 35.20 32.28 Site Area tribu 1.16 178.56 119.95 Site Area tribu 1.16 55.89 56.99 44.99 41.11 178.56 55.89 44.99 41.11 178.56 55.89 44.99 41.11 178.56 55.89 44.99 44.99 41.11 178.56 55.89 55.85	Qactual (L/s) 2.5 1,7 1.3 1.0 0.8 0.7 0.6 0.5 0.5 0.5 0.5 1.0 Qactual (L/s) 260.23 184.25 110.85 100.03 91.72 95.76 75.10	Orelease (L/s) 2.5 1.7 1.3 1.0 0.8 0.6 0.5 0.5 0.5 0.5 0.5 0.7 1.728 177.25 147.60 126.705 100.03 101.72 85.10 79.66 75.10	Qstored (L/s)	Sideyard Tributary to Underground Tank Vetored (m^3) 49.79 43.79 43.79 43.79 43.79 0.00 0.00 0.00 0.00 0.00 0.00	
Subdra	inage Area: Area (ha): te (min) 10 10 10 10 10 10 10 10 10 10	Image: TANK4 0.02 0.25 0.25 176.56 119.95 178.56 119.95 178.56 3.95 55.89 44.79 44.79 9.41.11 37.90 35.20 32.89 Site Area tribut. 1.16 1.16 1.16.55 55.89 44.79 9.41.11 1.76.56 10.87 55.89 44.99 44.99 41.11 37.90 35.20 35.20 32.89	Qactual (L/s) 2.5 1.7 1.3 1.0 0.8 0.7 0.6 0.5 0.5 0.5 0.5 0.5 0.5 1.0 0.262 147 66 1260,232 110,85 100,03 91,72 85,10 79,86 75,10 71,20	Orelease (L/s) 2.5 1.7 1.3 1.0 0.8 0.7 0.8 0.7 0.8 0.7 0.8 0.7 0.8 0.7 0.8 0.7 0.8 0.7 0.8 0.7 0.8 0.7 0.8 0.7 0.8 0.9 0.5 <th>Qstored (L/s) Classifier (L/s) Classifier (L/s) Classifie</th> <th>Sideyard Tributary to Underground Tank Vstored (m*5) Vstored (m*3) 43 m³ha 8.47 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00</th> <th></th>	Qstored (L/s) Classifier (L/s) Classifier (L/s) Classifie	Sideyard Tributary to Underground Tank Vstored (m*5) Vstored (m*3) 43 m³ha 8.47 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00	

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Project #160401536, 729 Ridgewood Avenue - Brigil Homes Modified Rational Method Calculatons for Storage

Subd	rainage Area: Area (ha): C:	UNC-1 0.15 0.28				Uncontrolled	Front Yard to ridgewood Ave.
	tc (min)	l (2 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	7
	10	76.81	9.0	9.0	11		-
	20	52.03	6.1	6.1			
	30	40.04	4.7	4.7			
	40	32.86	3.8	3.8			
	50	28.04	3.3	3.3			
	60	24.56	2.9	2.9			
	70	21.91	2.6	2.6			
	80	19.83	2.3	2.3			
	90	18.14	2.1	2.1			
	100	16.75	2.0	2.0			
	110	15.57	1.8	1.8			
	120	14.56	1.7	1.7			
MARY	TO OUTLET					Roof Storage	Cirtem Storage
9	ite Area to Linde	raround Storage	1 19	ha	Available	199.0	50 m ³
0	Cieto	ro Pelezce Pate:	100.51	L/e	Available.	51.5	0.0 m ³
	Ciate	minolease ivate.	100.01	L/3	0360.	51.5	0.0 111
	U	ncontrolled Area:	0.15	ha			
	Uncontrolle	ed Release Rate:	9.0	L/s			
	Total Sit	e Release Rate:	109.5	L/s			
	Site Targ	et Release Rate:	203.3	L/s			
			00.0	1.4-			

Project #160401536, 729 Ridgewood Avenue - Brigil Homes Modified Rational Method Calculatons for Storage

Subdraina Ai	ge Area: rea (ha): C:	UNC-1 0.15 0.35				Front Yard Uncontrolled to	ridgewood Ave	B.
Γ	tc (min)	l (100 yr) (mm/hr)	Qactual (L/s)	Qrelease (L/s)	Qstored (L/s)	Vstored (m^3)	7	
	10	178.56	26.1	26.1			-	
	20	119.95	17.5	17.5				
	30	91.87	13.4	13.4				
	40	75.15	11.0	11.0				
	50	63.95	9.3	9.3				
	60	55.89	8.2	8.2				
	70	49.79	7.3	7.3				
	80	44.99	6.6	6.6				
	90	41.11	6.0	6.0				
	100	37.90	5.5	5.5				
	110	35.20	5.1	5.1				
	120	32.89	4.8	4.8				
ARY TO OU	TLET							
						Roof Storage	Cistern Sto	rade
Site Area	to Under	mound Storage	1 18	ha	Available:	188.0	52	m ³
	Cister	n Release Rate:	177.25	L/s	Used:	173.0	49.8	m ³
	Un	controlled Area	0 15	ha				
U	ncontrolle	d Release Rate:	26.1	L/s				
	Total Site	Release Rate:	203.3	L/s				
:	Site Targe	t Release Rate:	203.3	L/s				
			0.0	L/s				

Project #160401536, 729 Ridgewood Avenue - Brigil Homes Roof Drain Design Sheet, Estimated Roof Area in Block 1 (L103D-Roof) Standard Watts Model R1100 Accuflow Roof Drain

1		Detine				Valuma	atimatian		
		Rating	j Curve	-		volume E	sumation		
	Elevation	Discharge Rate	Outlet Discharge	Storage	Elevation	Area	Volume	e (cu. m)	Water Depth
	(m)	(cu.m/s)	(cu.m/s)	(cu. m)	(m)	(sq. m)	Increment	Accumulated	(m)
	0.000	0.0000	0.0000	0	0.000	0	0	0	0.000
	0.025	0.0003	0.0013	0	0.025	20	0	0	0.025
	0.050	0.0006	0.0025	1	0.050	80	1	1	0.050
	0.075	0.0009	0.0035	5	0.075	180	3	5	0.075
	0.100	0.0011	0.0044	11	0.100	320	6	11	0.100
	0.125	0.0013	0.0054	21	0.125	500	10	21	0.125
	0.150	0.0016	0.0063	36	0.150	720	15	36	0.150

	Drawdow	n Estimate	
Total	Total		
Volume	Time	Vol	Detention
(cu.m)	(sec)	(cu.m)	Time (hr)
0.0	0.0	0.0	0
1.2	462.3	1.2	0.128417
4.3	912.6	3.2	0.381915
10.5	1396.3	6.2	0.769787
20.7	1895.8	10.2	1.296405
35.8	2404.0	15.2	1.964173

Rooftop Storage Summary

Total Building Area (sq.m) Assume Available Roof Area (sq.m) Roof Imperviousness Roof Drain Requirement (sq.m/Notch) Number of Roof Notches*	80%	900 720 0.99 232 4	
Max. Allowable Depth of Roof Ponding (m) Max. Allowable Storage (cu.m) Estimated 100 Year Drawdown Time (h)		0.15 36 1.7	* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).

From Watts Drain Catalogue

Head (m)	L/s				
Open		75%	50%	25%	Closed
0.025	0.31545	0.31545	0.31545	0.31545	0.31545
0.050	0.6309	0.6309	0.6309	0.6309	0.6309
0.075	0.94635	0.86749	0.78863	0.70976	0.6309
0.100	1.2618	1.10408	0.94635	0.78863	0.6309
0.125	1.57726	1.34067	1.10408	0.86749	0.6309
0.150	1.89271	1.57726	1.2618	0.94635	0.6309

Calculation Results	5yr	100yr	Available
Qresult (cu.m/s)	0.004	0.006	-
Depth (m)	0.093	0.141	0.150
Volume (cu.m)	9.1	30.8	36.0
Draintime (hrs)	0.7	1.74	

Project #160401536, 729 Ridgewood Avenue - Brigil Homes Roof Drain Design Sheet, Estimated Roof Area in Block 2 (L103C-Roof) Standard Watts Model R1100 Accuflow Roof Drain

Rating Curve								
Elevation	Discharge Rate	Outlet Discharge	Storage	Elevation	Area	Volume	e (cu. m)	Water Depth
(m)	(cu.m/s)	(cu.m/s)	(cu. m)	(m)	(sq. m)	Increment	Accumulated	(m)
0.000	0.0000	0.0000	0	0.000	0	0	0	0.000
0.025	0.0003	0.0013	0	0.025	24	0	0	0.025
0.050	0.0006	0.0025	2	0.050	98	1	2	0.050
0.075	0.0009	0.0035	6	0.075	220	4	6	0.075
0.100	0.0011	0.0044	13	0.100	391	8	13	0.100
0.125	0.0013	0.0054	25	0.125	611	12	25	0.125
0.150	0.0016	0.0063	44	0.150	880	19	44	0.150

Drawdown Estimate							
Total	Total						
Volume	Time	Vol	Detention				
(cu.m) (sec)		(cu.m)	Time (hr)				
0.0	0.0	0.0	0				
1.4	565.0	1.4	0.1569541				
5.3	1115.4	3.9	0.4667855				
12.8	1706.6	7.5	0.940851				
25.3	2317.1	12.4	1.5844945				
43.8	2938.2	18.5	2.4006556				

Rooftop Storage Summary

Total Building Area (sq.m)		1100	
Assume Available Roof Area (sq.n	80%	880	
Roof Imperviousness		0.99	
Roof Drain Requirement (sq.m/Notch)		232	
Number of Roof Notches*		4	
Max. Allowable Depth of Roof Ponding (m)		0.15	* As
Max. Allowable Storage (cu.m)		44	
Estimated 100 Year Drawdown Time (h)		2.3	

* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).	

From Watts Drain Catalogue

Head (m) L/s						
	Open	75%	50%	25%	Closed	
0.025	0.31545	0.31545	0.31545	0.31545	0.315451	
0.050	0.6309	0.6309	0.6309	0.6309	0.630902	
0.075	0.94635	0.86749	0.78863	0.70976	0.630902	
0.100	1.2618	1.10408	0.94635	0.78863	0.630902	
0.125	1.57726	1.34067	1.10408	0.86749	0.630902	
0.150	1.89271	1.57726	1.2618	0.94635	0.630902	

Calculation Results	5yr	100yr	Available
Qresult (cu.m/s)	0.004	0.006	-
Depth (m)	0.097	0.145	0.150
Volume (cu.m)	12.1	40.4	44.0
Draintime (hrs)	0.9	2.3	

Project #160401536, 729 Ridgewood Avenue - Brigil Homes Roof Drain Design Sheet, Estimated Roof Area in Block 4 (L104B-Roof) Standard Watts Model R1100 Accuflow Roof Drain

		Rating Curve				Volume Estimation				
	Elevation	Discharge Rate	Outlet Discharge	Storage	Elevation	Area	Volume	e (cu. m)	Water Depth	
	(m)	(cu.m/s)	(cu.m/s)	(cu. m)	(m)	(sq. m)	Increment	Accumulated	(m)	
	0.000	0.0000	0.0000	0	0.000	0	0	0	0.000	
	0.025	0.0003	0.0013	0	0.025	24	0	0	0.025	
	0.050	0.0006	0.0025	2	0.050	98	1	2	0.050	
	0.075	0.0008	0.0032	6	0.075	220	4	6	0.075	
	0.100	0.0009	0.0038	13	0.100	391	8	13	0.100	
	0.125	0.0011	0.0044	25	0.125	611	12	25	0.125	
	0.150	0.0013	0.0050	44	0.150	880	19	44	0.150	

Drawdown Estimate						
Total	Total					
Volume	Time	Vol	Detention			
(cu.m) (sec)		(cu.m)	Time (hr)			
0.0	0.0	0.0	0			
1.4	565.0	1.4	0.156954			
5.3	1226.9	3.9	0.497769			
12.8	1991.1	7.5	1.050845			
25.3	2813.6	12.4	1.832412			
43.8	3672.7	18.5	2.852614			

Rooftop Storage Summary

		1100	
i otal Building Area (sq.m)		1100	
Assume Available Roof Area (sq.m)	80%	880	
Roof Imperviousness		0.99	
Roof Drain Requirement (sq.m/Notch)		232	
Number of Roof Notches*		4	
Max. Allowable Depth of Roof Ponding (m)		0.15	* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).
Max. Allowable Storage (cu.m)		44	
Estimated 100 Year Drawdown Time (h)		2.8	

From Watts Drain Catalogue

L/s				
Open	Open 75%		25%	Closed
0.31545	0.31545	0.31545	0.31545	0.31545
0.6309	0.6309	0.6309	0.6309	0.6309
0.94635	0.86749	0.78863	0.70976	0.6309
1.2618	1.10408	0.94635	0.78863	0.6309
1.57726	1.34067	1.10408	0.86749	0.6309
1.89271	1.57726	1.2618	0.94635	0.6309
	L/s Open 0.31545 0.6309 0.94635 1.2618 1.57726 1.89271	L/s Open 75% 0.31545 0.31545 0.6309 0.6309 0.94635 0.86749 1.2618 1.10408 1.57726 1.34067 1.89271 1.57726	L/s Open 75% 50% 0.31545 0.31545 0.31545 0.6309 0.6309 0.6309 0.94635 0.86749 0.78863 1.2618 1.10408 0.94635 1.57726 1.34067 1.10408 1.89271 1.57726 1.2618	L/s Open 75% 50% 25% 0.31545 0.31545 0.31545 0.6309 0.6309 0.6309 0.94635 0.86749 0.78863 0.70976 1.2618 1.10408 0.94635 0.78863 1.57726 1.34067 1.10408 0.86749 1.89271 1.57726 1.2618 0.94635

Calculation Results	5yr	100yr	Available
Qresult (cu.m/s)	0.004	0.005	-
Depth (m)	0.100	0.149	0.150
Volume (cu.m)	13.0	43.6	44.0
Draintime (hrs)	1.1	2.8	

Project #160401536, 729 Ridgewood Avenue - Brigil Homes Roof Drain Design Sheet, Estimated Roof Area in Block 6 (L103B-Roof) Standard Watts Model R1100 Accuflow Roof Drain

Rating Curve								
Elevation	Discharge Rate	Outlet Discharge	Storage	Elevation	Area	Volume	e (cu. m)	Water Depth
(m)	(cu.m/s)	(cu.m/s)	(cu. m)	(m)	(sq. m)	Increment	Accumulated	(m)
0.000	0.0000	0.0000	0	0.000	0	0	0	0.000
0.025	0.0003	0.0019	0	0.025	36	0	0	0.025
0.050	0.0006	0.0038	2	0.050	142	2	2	0.050
0.075	0.0009	0.0052	8	0.075	320	6	8	0.075
0.100	0.0011	0.0066	19	0.100	569	11	19	0.100
0.125	0.0013	0.0080	37	0.125	889	18	37	0.125
0.150	0.0016	0.0095	64	0.150	1280	27	64	0.150

	Drawdow	n Estimat	e
Total	Total		
Volume	Volume Time		Detention
(cu.m)	(sec)	(cu.m)	Time (hr)
0.0	0.0	0.0	0
2.1	547.9	2.1	0.1521979
7.7	1081.6	5.6	0.4526405
18.7	1654.9	11.0	0.9123404
36.7	2246.9	18.1	1.5364795
63.7	2849.1	27.0	2.3279085

Rooftop Storage Summary

Total Duilding Area (ag m)		1000	
rotal building Area (sq.m)		1000	
Assume Available Roof Area (sq.	80%	1280	
Roof Imperviousness		0.99	
Roof Drain Requirement (sq.m/Notch)		232	
Number of Roof Notches*		6	
Max. Allowable Depth of Roof Ponding (m)		0.15	* A
Max. Allowable Storage (cu.m)		64	
Estimated 100 Year Drawdown Time (h)		2.2	

* As per Ontario Building Code section OBC 7.4.10.4.(2)(c).	

From Watts Drain Catalogue

Head (m)	Head (m) L/s											
	Open	75%	50%	25%	Closed							
0.025	0.31545	0.31545	0.31545	0.31545	0.315451							
0.050	0.6309	0.6309	0.6309	0.6309	0.630902							
0.075	0.94635	0.86749	0.78863	0.70976	0.630902							
0.100	1.2618	1.10408	0.94635	0.78863	0.630902							
0.125	1.57726	1.34067	1.10408	0.86749	0.630902							
0.150	1.89271	1.57726	1.2618	0.94635	0.630902							

Calculation Results	5yr	100yr	Available
Qresult (cu.m/s)	0.006	0.009	-
Depth (m)	0.096	0.145	0.150
Volume (cu.m)	17.3	58.2	64.0
Draintime (hrs)	0.9	2.2	



ty: Ottawa earest Rainfall Station: OTTAWA MACDONALD-CA INT'L AP CDC Rainfall Station Id: 6000	ARTIER Project Number: Designer Name: Designer Company:	160401536 thakshika rathnaso stanter	oriya	
earest Rainfall Station: OTTAWA MACDONALD-CA INT'L AP CDC Rainfall Station Id: 6000	ARTIER Designer Name: Designer Company:	thakshika rathnaso	oriya	
INT'L AP ICDC Rainfall Station Id: 6000	Designer Company:	stantec		
ICDC Rainfall Station Id: 6000		Stantee		
	Designer Email:	thakshika.rathnaso	oriya@stantec.con	
ears of Rainfall Data: 37	Designer Phone:	613-724-4081		
ita Nama: 720 Pidgowood Avonuo	EOR Name:			
	EOR Company:			
Drainage Area (ha): 0.72	EOR Email:			
unoff Coefficient 'c': 0.55	EOR Phone:			
Required Water Quality Runoff Volume Capture (%):	90.00	Sizing Summary		
Estimated Water Quality Flow Rate (L/s):	14.31	Stormceptor Model	Stormceptor TSS Remova Model Provided (%	
Dil / Fuel Spill Risk Site?	No	EF4	80	
Jpstream Flow Control?	No	EF6	85	
veak Conveyance (maximum) Flow Rate (L/s):		EF8	89	
site Sediment Transport Rate (kg/ha/vr):		EF10	90	
		EF12	91	
	Recommende	d Stormceptor EF	Model: I	
Estimat	ted Net Annual Sediment	TSS) Load Reduct	ion (%):	
	Water Quality Ru	noff Volume Capt	ure (%): <mark>></mark>	



Forterra



THIRD-PARTY TESTING AND VERIFICATION

► Stormceptor® EF and Stormceptor® EFO are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV Procedure for Laboratory Testing of Oil-Grit Separators and performance has been third-party verified in accordance with the ISO 14034 Environmental Technology Verification (ETV) protocol.

PERFORMANCE

► Stormceptor® EF and EFO remove stormwater pollutants through gravity separation and floatation, and feature a patentpending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including highintensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterwavs.

PARTICLE SIZE DISTRIBUTION (PSD)

► The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle	Percent Less	Particle Size	Dercent
Size (µm)	Than	Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5







Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
1	51.3	51.3	1.10	66.0	55.0	92	47.2	47.2
2	8.7	60.0	2.20	132.0	110.0	86	7.5	54.7
3	5.8	65.8	3.30	198.0	165.0	80	4.7	59.3
4	4.6	70.4	4.40	264.0	220.0	74	3.4	62.7
5	4.2	74.6	5.50	330.0	275.0	70	2.9	65.7
6	3.2	77.8	6.61	396.0	330.0	64	2.1	67.7
7	2.6	80.4	7.71	462.0	385.0	60	1.6	69.3
8	2.4	82.8	8.81	528.0	440.0	58	1.4	70.7
9	1.9	84.7	9.91	594.0	495.0	57	1.1	71.7
10	1.6	86.3	11.01	661.0	550.0	57	0.9	72.6
11	1.3	87.6	12.11	727.0	605.0	56	0.7	73.4
12	1.1	88.7	13.21	793.0	661.0	56	0.6	74.0
13	1.3	90.0	14.31	859.0	716.0	55	0.7	74.7
14	1.1	91.1	15.41	925.0	771.0	55	0.6	75.3
15	0.6	91.7	16.51	991.0	826.0	55	0.3	75.6
16	0.8	92.5	17.61	1057.0	881.0	55	0.4	76.1
17	0.7	93.2	18.71	1123.0	936.0	54	0.4	76.5
18	0.5	93.7	19.82	1189.0	991.0	54	0.3	76.7
19	0.6	94.3	20.92	1255.0	1046.0	55	0.3	77.1
20	0.5	94.8	22.02	1321.0	1101.0	55	0.3	77.3
21	0.2	95.0	23.12	1387.0	1156.0	56	0.1	77.4
22	0.4	95.4	24.22	1453.0	1211.0	57	0.2	77.7
23	0.5	95.9	25.32	1519.0	1266.0	57	0.3	78.0
24	0.4	96.3	26.42	1585.0	1321.0	58	0.2	78.2
25	0.1	96.4	27.52	1651.0	1376.0	59	0.1	78.2







Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)			
26	0.3	96.7	28.62	1717.0	1431.0	58	0.2	78.4			
27	0.4	97.1	29.72	1783.0	1486.0	56	0.2	78.6			
28	0.2	97.3	30.82	1849.0	1541.0	54	0.1	78.7			
29	0.2	97.5	31.93	1916.0	1596.0	52	0.1	78.9			
30	0.2	97.7	33.03	1982.0	1651.0	50	0.1	79.0			
31	0.1	97.8	34.13	2048.0	1706.0	49	0.0	79.0			
32	0.2	98.0	35.23	2114.0	1761.0	47	0.1	79.1			
33	0.1	98.1	36.33	2180.0	1816.0	46	0.0	79.1			
34	0.1	98.2	37.43	2246.0	1871.0	44	0.0	79.2			
35	0.1	98.3	38.53	2312.0	1927.0	43	0.0	79.2			
36	0.2	98.5	39.63	2378.0	1982.0	42	0.1	79.3			
37	0.0	98.5	40.73	2444.0	2037.0	41	0.0	79.3			
38	0.1	98.6	41.83	2510.0	2092.0	40	0.0	79.3			
39	0.1	98.7	42.93	2576.0	2147.0	39	0.0	79.4			
40	0.1	98.8	44.04	2642.0	2202.0	38	0.0	79.4			
41	0.1	98.9	45.14	2708.0	2257.0	37	0.0	79.5			
42	0.1	99.0	46.24	2774.0	2312.0	36	0.0	79.5			
43	0.2	99.2	47.34	2840.0	2367.0	35	0.1	79.6			
44	0.1	99.3	48.44	2906.0	2422.0	34	0.0	79.6			
45	0.1	99.4	49.54	2972.0	2477.0	33	0.0	79.6			
46	0.0	99.4	50.64	3038.0	2532.0	33	0.0	79.6			
47	0.1	99.5	51.74	3104.0	2587.0	32	0.0	79.7			
48	0.0	99.5	52.84	3171.0	2642.0	32	0.0	79.7			
49	0.0	99.5	53.94	3237.0	2697.0	32	0.0	79.7			
50	0.0	99.5	55.04	3303.0	2752.0	31	0.0	79.7			
	Estimated Net Annual Sediment (TSS) Load Reduction =										









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Maximum Pipe Diameter / Peak Conveyance										
Stormceptor EF / EFO	EFO Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inle Diame	et Pipe eter	Max Out Diam	et Pipe eter	Peak Cor Flow	nveyance Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)	
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15	
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35	
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60	
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100	
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100	

SCOUR PREVENTION AND ONLINE CONFIGURATION

Stormceptor® EF and EFO feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV Procedure for Laboratory Testing of Oil-Grit Separators, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

DESIGN FLEXIBILITY

► Stormceptor® EF and EFO offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

OIL CAPTURE AND RETENTION

► While Stormceptor[®] EF will capture and retain oil from dry weather spills and low intensity runoff, **Stormceptor[®] EFO** has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid reentrainment testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.











45*-90* 0*-45* 0*-45* 45*-90*

INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1. For submerged conditions the applicable K value is 3.0.

Pollutant Capacity												
Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

*Increased sump depth may be added to increase sediment storage capacity

** Average density of wet packed sediment in sump = $1.6 \text{ kg/L} (100 \text{ lb/ft}^3)$

Feature	Benefit	Feature Appeals To			
Patent-pending enhanced flow treatment	Superior, verified third-party	Regulator, Specifying & Design Engineer			
and scour prevention technology	performance				
Third-party verified light liquid capture	Proven performance for fuel/oil hotspot	Regulator, Specifying & Design Engineer,			
and retention for EFO version	locations	Site Owner			
Functions as bend, junction or inlet	Design flevibility	Specifying & Design Engineer			
structure	Design nextonity	speenying & besign Engineer			
Minimal drop between inlet and outlet	Site installation ease	Contractor			
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner			

STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef





STANDARD PERFORMANCE SPECIFICATION FOR "OIL GRIT SEPARATOR" (OGS) STORMWATER QUALITY TREATMENT DEVICE

PART 1 – GENERAL

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program's **Procedure for Laboratory Testing of Oil-Grit Separators.**

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

PART 2 – PRODUCTS

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The **minimum** sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1 4 ft (1219 mm) Diameter OGS Units:

6 ft (1829 mm) Diameter OGS Units:

8 ft (2438 mm) Diameter OGS Units:

10 ft (3048 mm) Diameter OGS Units: 12 ft (3657 mm) Diameter OGS Units: $\begin{array}{l} 1.19 \ m^3 \ sediment \ / \ 265 \ L \ oil \\ 3.48 \ m^3 \ sediment \ / \ 609 \ L \ oil \\ 8.78 \ m^3 \ sediment \ / \ 1,071 \ L \ oil \\ 17.78 \ m^3 \ sediment \ / \ 1,673 \ L \ oil \\ 31.23 \ m^3 \ sediment \ / \ 2,476 \ L \ oil \\ \end{array}$

PART 3 – PERFORMANCE & DESIGN

3.1 GENERAL







The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing shall be determined using historical rainfall data and a sediment removal performance curve derived from the actual third-party verified laboratory testing data. The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators.**

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m².





NO PRODUCT SUBSTITUTIONS SHALL BE ACCEPTED UNLESS SUBMITTED 10 DAYS PRIOR TO PROJECT BID DATE, OR AS DIRECTED BY THE ENGINEER OF RECORD.

STANDARD DETAIL NOT FOR CONSTRUCTION

		The design and information shown on this drawing is provided as a service to the project owner, engineer and contractor by Imbrium Systems ("Imbrium").	Natiber this drawing, nor any part thereof, may be used, reproduced or modified in any manner without the order witten consert of imbrium. Failure to comoly.	is done at the user's own risk and imbrium expressly disciplins any liability or responsibility for such use.	If discrepancies between the supplied information upon which the drawing is based and actual field conditions are encountered as site work propresses. These	 discrepancies must be reported to imbrium immediately for re-evaluation of the design. Imbrium accepts no liability for designs based on missing, incomplete or 	inaccurate information supplied by others.
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PLAN VIEW (STANDARD)					ATES	AL RELEASE	REVISION DESCRIPTION
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_			###	###	6/8/	5/26/	DAI
			#####	#####	-	0	MARK
PLAN VIEW (INLET TOP) SE CONTACT YOUR LOCAL STORMCEPTOR REPRESENTATION OUTLET DOWNEY (INLET TOP) SE CONTACT YOUR LOCAL STORMCEPTOR REPRESENTATION ED ON THE BEST AVAILABLE INFORMATION AT THE TIME. S NOCATION OR CONNECTION PIPING MAY BE NECESSARY B IGURATION REVISIONS. ELEVATIONS SHOULD BE MAINTA STRUCTURE (IF REQUIRED).	TIVE. SOME ASED AINED						SCALE - NTS
SITE SPECIFIC DATA REQUIREMENT STORMCEPTOR MODEL EF4 STRUCTURE ID WATER QUALITY FLOW RATE (L/s) PEAK FLOW RATE (L/s) PEAK FLOW RATE (L/s) RETURN PERIOD OF PEAK FLOW (yrs) DRAINAGE AREA (HA)	NTS * * * *			imbrium	7037 RIDGE ROAD, SUITE 350, HANOVER, MD 240	USA 050-2/9-0520 UA 0UU-000-4001 INIL *1-410-9 The stromozyron eventsk is mortecings provide on the rolution of Austral Paratike, 05,144 - 1972 (s. 1988) 7040 I Australia 1984	No. 2,000,200 - 2,027,041 - 2,172,772, -2,100,201 - 2,100,200 - 2,002,000 - 2,077,050 No. 1,162/101 Command III (11) Command III (12) Command IIII (12) Command IIIII (12) Command IIIIIIIII (12) Command IIIIIIII (12) Command IIIIIIIIII (12) Command IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII
DRAINAGE AREA IMPERVIOUSNESS (%)	*	DATE: 5/26	/201	7			
PIPE DATA: I.E. MAT'L DIA SLOPE %	HGL	DESIG	NED:		DRAN	VN:	
NLET #1 * * * *	*	CHECK	(ED:		APPF	N OVED:	:
	*	BSF	CT N	a.:	SFO	IENCE	No ·
	-	EF4			*	0 .	
		SHEET	:	1	OF	1	
			_				_

INLET #2

OUTLET

SITE SERVICING AND STORMWATER MANAGEMENT BRIEF – MOONEY'S BAY - 729 RIDGEWOOD AVENUE, OTTAWA, ON

Appendix E Geotechnical Report and Environmental Site Assessment

Appendix E GEOTECHNICAL REPORT AND ENVIRONMENTAL SITE ASSESSMENT



patersongroup

Geotechnical Engineering

Environmental Engineering

Hydrogeology

Geological Engineering

Materials Testing

Building Science

Archaeological Services

Geotechnical Investigation

Proposed Multi Storey Building 729 Ridgewood Avenue Ottawa, Ontario

Prepared for

Brigil

Paterson Group Inc.

Consulting Engineers 154 Colonnade Road South Ottawa, Ontario Canada K2E 7J5

Tel: (613) 226-7381 Fax: (613) 226-6344 www.patersongroup.ca

September 15, 2020

Report PG5172-1
Table of Contents

1.0	Introduction 1
2.0	Proposed Development1
3.0	Method of Investigation3.1Field Investigation3.2Field Survey3.3Laboratory Testing3.4Analytical Testing
4.0	Observations4.1Surface Conditions44.2Subsurface Profile44.3Groundwater5
5.0	Discussion5.1Geotechnical Assessment.65.2Site Grading and Preparation65.3Foundation Design95.4Design for Earthquakes115.5Basement Slab115.6Basement Wall125.7Pavement Structure.13
6.0	Design and Construction Precautions6.1Foundation Drainage and Backfill156.2Protection of Footings Against Frost Action166.3Excavation Side Slopes176.4Pipe Bedding and Backfill206.5Groundwater Control206.6Winter Construction216.7Corrosion Potential and Sulphate22
7.0	Recommendations 23
8.0	Statement of Limitations

Appendices

- Appendix 1Soil Profile and Test Data SheetsSymbols and TermsAnalytical Testing Results
- Appendix 2Figure 1 Key PlanDrawing PG5172-1 Test Hole Location Plan

1.0 Introduction

Paterson Group (Paterson) was commissioned by Brigil to conduct a geotechnical investigation for the proposed mid-rise residential building to be located at 729 Ridgewood Avenue in the City of Ottawa, Ontario (refer to Figure 1 - Key Plan in Appendix 2 of this report).

The objectives of the current investigation were to:

- Determine the subsurface and groundwater conditions by means of boreholes and existing soils information.
- Provide geotechnical recommendations pertaining to design of the proposed development including construction considerations which may affect the design.

The following report has been prepared specifically and solely for the aforementioned project which is described herein. The report contains Paterson's findings and includes geotechnical recommendations pertaining to the design and construction of the subject development as understood at the time of writing this report.

2.0 Proposed Development

The development is understood to consist of a multi storey residential building with up to 2 levels of underground parking. It is further understood that the proposed building will encompass the majority of the subject site. Associated at-grade access lanes, car parking and landscaped areas are also anticipated. The proposed building is anticipated to be municipally serviced.

The subject property is presently occupied by a two slab on grade commercial buildings. It is expected that the existing buildings within the site will be demolished as part of the proposed project. A former mechanics garage was located on the east portion of the site. The building was recently demolished.

3.0 Method of Investigation

3.1 Field Investigation

Field Program

The field program for the current investigation was completed between June 25 and 26, 2020. At that time, 7 boreholes were advanced to a maximum depth of 9.7 m below existing grade. The borehole locations were distributed in a manner to provide general coverage of the proposed development taking into consideration existing site features and underground services. The borehole locations are shown on Drawing PG5172-1 - Test Hole Location Plan included in Appendix 2.

Sampling and In-Situ Testing

Soil samples were recovered with a 50 mm diameter split-spoon sample or from the auger flights. The split-spoon and auger samples were classified on site and placed in sealed plastic bags. All samples were transported to Paterson's laboratory. The depths at which the split-spoon and auger samples were recovered from the boreholes are presented as SS and AU, respectively, on the Soil Profile and Test Data sheets in Appendix 1.

The Standard Penetration Test (SPT) was conducted in conjunction with the recovery of the split-spoon samples. The SPT results are recorded as "N" values on the Soil Profile and Test Data sheets. The "N" value is the number of blows required to drive the split-spoon sampler 300 mm into the soil after a 150 mm initial penetration using a 63.5 kg hammer falling from a height of 760 mm.

Undrained shear strength testing was carried out in cohesive soils using a field vane apparatus.

The overburden thickness was evaluated by a dynamic cone penetration test (DCPT) completed at BH6. The DCPT consists of driving a steel drill rod, equipped with a 50 mm diameter cone at the tip, using a 63.5 kg hammer falling from a height of 760 mm. The number of blows required to drive the cone into the soil is recorded for each 300 mm increment.

Rock samples were recovered from BH7 using a core barrel and diamond drilling techniques. The bedrock samples were classified on site, placed in hard cardboard core boxes and transported to Paterson's laboratory. The depths at which rock core samples were recovered from the boreholes are presented as RC on the Soil Profile and Test Data sheets in Appendix 1.

The recovery value and a Rock Quality Designation (RQD) value were calculated for each drilled section of bedrock and are presented on the borehole logs. The recovery value is the length of the bedrock sample recovered over the length of the drilled section. The RQD value is the total length of intact rock pieces longer than 100 mm over the length of the core run. The values indicate the bedrock quality.

The subsurface conditions observed in the boreholes were recorded in detail in the field. The soil profiles are presented on the Soil Profile and Test Data sheets in Appendix 1.

Groundwater

Flexible piezometers were installed in all the boreholes to monitor the groundwater level subsequent to the completion of the sampling program. The groundwater observations are discussed in subsection 4.3 and presented in the Soil Profile and Test Data Sheets in Appendix 1.

3.2 Field Survey

The test hole locations were determined and located in the field by Paterson. All ground surface elevations reference a geodetic datum (NAD83). The locations of the boreholes and the ground surface elevations for each borehole location are presented on Drawing PG5172-1 -Test Hole Location Plan in Appendix 2.

3.3 Laboratory Testing

The soil samples and the bedrock core were recovered from the subject site and visually examined in Paterson's laboratory to review the field logs.

All samples will be stored in the laboratory for a period of one month after issuance of this report. The samples will then be discarded unless otherwise directed.

3.4 Analytical Testing

One soil sample was submitted for analytical testing to assess the corrosion potential for exposed ferrous metals and the potential of sulphate attacks against subsurface concrete structures. The sample was submitted to determine the concentration of sulphate and chloride, the resistivity and the pH of the sample. The results are presented in Appendix 1 and are discussed further in Subsection 6.7.

4.0 Observations

4.1 Surface Conditions

The subject property is presently occupied by two slab on grade commercial buildings. A former mechanical shop located on southeastern portion of the site was recently demolished. The area was backfilled with granular material. A parking lot and pavement structure covers the majority of the site. Some landscaped areas were noted along Ridgewood Avenue.

The ground surface across the subject site is relatively flat and slightly below grade from Ridgewood Avenue and the property to the west. The site is bordered to the west by a residential high rise structure, to the north and east by a residential and institutional development, and Ridgewood Avenue to the south.

4.2 Subsurface Profile

Overburden

Generally, the subsurface profile encountered at the boreholes consist of asphaltic concrete overlying a fill layer consisting of crushed stone and silty sand. The fill layer is underlain by a stiff to hard layer of brown silty clay with sand seams. Glacial till was encountered below the above noted layers consisting of a compact to a very dense silty sand with clay, gravel, cobbles, and boulders. Seams of coarse sand where encountered in the glacial till layer at some test hole locations. Reference should be made to the Soil Profile and Test Data sheets in Appendix 1 for the details of the soil profile encountered at each test hole location.

Bedrock

Bedrock was cored at one borehole location to confirm refusal. Limestone bedrock was encountered at a depth of 9.7 m below the existing ground surface at BH6. Refusal was encountered in the other boreholes between a depth of 4.8 to 8.7 m. It should be noted that boulders are to be expected.

Upon review of the core hole sample, the upper first meter of the bedrock was found to be of good quality.

Based on available geological mapping, the subject site is located in an area where the bedrock consists of limestone of the Bobcaygeon Formation. The overburden drift thickness is anticipated to be between 5 to 15 m in depth.

4.3 Groundwater

Flexible piezometers were installed as part of our geotechnical investigation. Groundwater level measurements were recorded at the borehole locations and our findings are presented in Table 1. It should also be noted that the groundwater level is subject to seasonal fluctuations. Therefore, groundwater could vary at the time of construction. It should be further noted that groundwater measurements at monitoring well locations can be influenced by surface water entering the backfilled borehole, which can lead to higher than normal groundwater level readings. Long-term groundwater levels can also be determined based on observations of the recovered soil samples, such as moisture levels, colouring and consistency. Based on these observations, the long-term groundwater level is expected at a 5 to 6 m depth.

Table 1 - Groundwater Measurements at Monitoring Well Locations											
Test Hole Location	Ground Surface Elevation (m)	GW Level Reading (m)	GW Level Elevation (m)	Date							
BH 1	82.55	2.75	79.80	July 7, 2020							
BH 2	81.92	3.54	78.38	July 7, 2020							
BH 3	82.05	4.72	77.33	July 7, 2020							
BH 4	81.35	4.01	77.34	July 7, 2020							
BH 5	81.71	3.28	78.43	July 7, 2020							
BH 6	82.02	1.88	80.14	July 7, 2020							
BH 7	81.61	3.15	78.46	July 7, 2020							

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Geotechnical Investigation Proposed Multi Storey Building 729 Ridgewood Avenue - Ottawa

5.0 Discussion

5.1 Geotechnical Assessment

From a geotechnical perspective, the subject site is considered satisfactory for the proposed development. The proposed mid-rise residential building is anticipated to be founded on spread footings placed directly or indirectly by the use of a lean concrete in-filled trench on a clean, surface sounded bedrock bearing surface or compact glacial till bearing surface.

Bedrock removal may be required to complete the underground level. Hoe ramming is an option where only small quantities of bedrock need to be removed. Line drilling and controlled blasting where large quantities of bedrock need to be removed is recommended. The blasting operations should be planned and completed under the guidance of a professional engineer with experience in blasting operations.

The above and other considerations are further discussed in the following sections.

5.2 Site Grading and Preparation

Stripping Depth

Since the building will occupy the entire boundaries of the subject site, it is expected that most of the overburben will be removed to bedrock. Topsoil and deleterious fill, such as those containing organic materials, should be stripped from under any buildings, paved areas, pipe bedding and other settlement sensitive structures

Bedrock Removal

Bedrock removal can be accomplished by hoe ramming where only small quantity of the bedrock needs to be removed. Sound bedrock may be removed by line drilling and controlled blasting and/or hoe ramming.

Prior to considering blasting operations, the blasting effects on the existing services, buildings and other structures should be addressed. A pre-blast or pre-construction survey of the existing structures located in proximity of the blasting operations should be completed prior to commencing site activities. The extent of the survey should be determined by the blasting consultant and should be sufficient to respond to any inquiries/claims related to the blasting operations.

As a general guideline, peak particle velocities (measured at the structures) should not exceed 25 mm/s during the blasting program to reduce the risks of damage to the existing structures. The blasting operations should be planned and conducted under the supervision of a licensed professional engineer who is also an experienced blasting consultant.

Excavation side slopes in sound bedrock can be carried out using almost vertical side walls. A minimum 1 m horizontal ledge, should be left between the bottom of the overburden excavation and the top of the bedrock surface to provide an area to allow for potential sloughing or to provide a stable base for the overburden shoring system.

Lean Concrete In-Filled Trenches

Where bedrock is encountered below the design underside of footing elevation, consideration should be given to excavating vertical trenches to expose the underlying bedrock surface and backfilling with lean concrete (15 MPa 28-day compressive strength). Typically, the excavation sidewalls will be used as the form to support the concrete. The additional width of the concrete poured against an undisturbed trench sidewall will suffice in providing a direct transfer of the footing load to the underlying bedrock.

The effectiveness of this operation will depend on the ability of maintaining vertical trenches until the lean concrete can be poured. It is suggested that once the bottom of the excavation is exposed, an assessment should be completed to determine the water infiltration and stability of the excavation sidewalls extending to the bedrock surface.

The trench excavation should be at least 150 mm wider than all sides of the footing at the base of the excavation. The excavation bottom should be relatively clean using the hydraulic shovel only (workers will not be permitted in the excavation below a 1.5 m depth). Once approved by the geotechnical engineer, lean concrete can be poured up to the proposed founding elevation.

Footings placed on lean concrete filled trenches extending to the bedrock surface can be designed using a factored bearing resistance value at ultimate limit states (ULS) of 1,500 kPa.

Vibration Considerations

Construction operations are the cause of vibrations, and possibly, sources of nuisance to the community. Therefore, means to reduce the vibration levels as much as possible should be incorporated in the construction operations to maintain, as much as possible, a cooperative environment with the residents.

The following construction equipments could be the source of vibrations: hoe ram, compactor, dozer, crane, truck traffic, etc. Vibrations, whether caused by blasting operations or by construction operations, could be the source of detrimental vibrations on the nearby buildings and structures. Therefore, all vibrations are recommended to be limited.

Two parameters are used to determine the permissible vibrations, namely, the maximum peak particle velocity and the frequency. For low frequency vibrations, the maximum allowable peak particle velocity is less than that for high frequency vibrations. As a guideline, the peak particle velocity should be less than 15 mm/s between frequencies of 4 to 12 Hz, and 50 mm/s above a frequency of 40 Hz (interpolate between 12 and 40 Hz). The guidelines are for current construction standards. Considering that these guidelines are above perceptible human level and, in some cases, could be very disturbing to some people, a pre-construction survey is recommended be completed to minimize the risks of claims during or following the construction of the proposed building.

Fill Placement

Fill used for grading purposes beneath the proposed buildings should consist of clean imported granular fill, such as Ontario Provincial Standard Specifications (OPSS) Granular A or Granular B Type II. The fill should be tested and approved prior to delivery to the site. It should be placed in lifts no greater than 300 mm in thickness and compacted using suitable compaction equipment for the specified lift thickness. Fill placed beneath the building areas should be compacted to at least 98% of its standard Proctor maximum dry density (SPMDD).

Non-specified existing fill along with site-excavated soil can be used as general landscaping fill where settlement of the ground surface is of minor concern. These materials should be spread in thin lifts and be compacted at minimum by the tracks of the spreading equipment to minimize voids. If these materials are to be used to build up the subgrade level for areas to be paved, they should be compacted in thin lifts to a minimum density of 95% of their respective SPMDD. Non-specified existing fill and site-excavated soils are not suitable for use as backfill against foundation walls unless used in conjunction with a composite drainage membrane.

5.3 Foundation Design

Bearing Resistance Values

Footings placed on an undisturbed, **dense glacial till bearing surface** can be designed using a bearing resistance value at serviceability limit states (SLS) of **250 kPa** and a factored bearing resistance value at ultimate limit states (ULS) of **500 kPa**. A geotechnical resistance factor of 0.5 was applied to the above noted bearing resistance value at ULS. Footings designed using the above-noted bearing resistance value at SLS will be subjected to potential post-construction total and differential settlements of 25 and 20 mm, respectively.

Footings placed on the upper levels of the **fractured limestone** bedrock bearing surface can be designed using a factored bearing resistance value at ultimate limit states (ULS) of **1,500 kPa**, incorporating a geotechnical resistance factor of 0.5. Where the design underside of footing is slightly above the bedrock surface, footings can be placed over concrete in-filled (17 MPa). zero entry, near vertical trenches extended to a surface sounded bedrock bearing surface using the same bearing resistance values. The concrete in-filled trenches should extend a minimum 300 mm beyond the footing faces in all directions.

A factored bearing resistance value at ULS of **4,000 kPa**, incorporating a geotechnical resistance factor of 0.5 if founded on **clean**, **surface sounded limestone bedrock** and the bedrock is free of seams, fractures and voids within 1.5 m below the founding level. This could be verified by completing and probing 50 mm diameter drill holes to a depth of 1.5 m below the founding level within the footing footprint(s). One drill hole should be completed per footing. The drill hole inspection should be completed by the geotechnical consultant.

A clean, surface-sounded bedrock bearing surface should be free of loose materials, and have no near surface seams, voids, fissures or open joints which can be detected from surface sounding with a rock hammer.

Footings bearing on an acceptable bedrock bearing surface and designed using the bearing resistance values provided herein will be subjected to negligible potential post-construction total and differential settlements.

Soil/Bedrock Transition

It is expected that not all footings will be founded on bedrock. Where the building is founded on the glacial till deposit, it is recommended to decrease the soil bearing capacity by 25% for the footing placed on soil bearing media to reduce the potential long term total and differential settlements. Also, at the soil/bedrock and bedrock/soil transitions, it is recommended that a 2 m transition zone composed of 0.5 m layer of nominally compacted OPSS Granular A or Granular B type II be placed directly on sound bedrock. Steel reinforcement, extending at least 3 m on both sides of the 2 m long transition should be placed in the top part of the footing and foundation walls.

Raft Foundation

Alternatively, consideration can be given to a raft foundation if the building loads exceed the bearing resistance values provided for a conventional spread footing foundation. The following parameters may be used for raft design.

The amount of settlement of the raft slab will be dependent on the sustained raft contact pressure. The bearing resistance value at SLS (contact pressure) of **250 kPa** can be used for design purposes. The loading conditions for the contact pressure are based on sustained loads, that are generally taken to be 100% Dead Load and 50% Live Load. The contact pressure provided considers the stress relief associated with the soil removal associated with one underground parking level. The factored bearing resistance (contact pressure) at ULS can be taken as **400 kPa**. A geotechnical resistance factor of 0.5 was applied to the bearing resistance value at ULS.

Based on the following assumptions for the raft foundation, the proposed building can be designed using the above parameters with a total and differential settlement of 25 and 15 mm, respectively.

Base on a single underground parking level or more it is expected that the raft foundation will be installed on the glacial till deposit. The modulus of subgrade reaction was calculated to be **30 MPa/m** for a contact pressure of 250 kPa. The design of the raft foundation is required to consider the relative stiffness of the reinforced concrete slab and the supporting bearing medium.

Lateral Support

The bearing medium under footing-supported structures is required to be provided with adequate lateral support with respect to excavations and different foundation levels. Adequate lateral support is provided to a sound bedrock bearing medium when a plane extending down and out from the bottom edge of the footing at a minimum of 1H:6V (or flatter) passes only through sound bedrock or a material of the same or higher capacity as the bedrock, such as concrete. A weathered bedrock bearing medium will require a lateral support zone of 1H:1V (or flatter).

5.4 Design for Earthquakes

The site class for seismic site response can be taken as **Class C** for the foundations considered at this site. However, a higher site class (**Class A or B**) can be achieved. The higher site class will require a site specific shear wave velocity test to be completed in confirmation of the seismic site classification. The soils underlying the subject site are not susceptible to liquefaction. Refer to the latest revision of the Ontario Building Code for a full discussion of the earthquake design requirements.

5.5 Basement Slab

With the removal of all topsoil and deleterious fill, containing organic matter, within the footprints of the proposed building, the native soil surface, bedrock or approved engineered fill pad will be considered an acceptable subgrade on which to commence backfilling for floor slab construction.

Any soft areas should be removed and backfilled with appropriate backfill material. A clear crushed stone fill is recommended for backfilling below the floor slab for limited span slab-on-grade areas, such as front porch or garage footprints. It is recommended that the upper 200 mm of sub-slab fill consist of 19 mm clear crushed stone below basement floor slabs.

It is expected that the basement area will be mostly parking and a rigid pavement structure designed by a structural engineer will be applicable. However, if storage or other uses of the lower level where a concrete floor slab will be used it is recommended that the upper 200 mm of sub-slab fill consists of 19 mm clear crushed stone. All backfill material within the footprint of the proposed building should be placed in maximum 300 mm thick loose layers and compacted to at least 98% of its SPMDD.

5.6 Basement Wall

It is understood that the basement walls are to be poured against a dampproofing system, which will be placed against the exposed bedrock face. Below the bedrock surface, a nominal coefficient for at-rest earth pressure of 0.01 is recommended in conjunction with a bulk unit weight of 24.5 kN/m³ (effective 15.5 kN/m³). A seismic earth pressure component will not be applicable for the foundation wall, which is to be poured against the bedrock face. It is expected that the seismic earth pressure will be transferred to the underground floor slabs, which should be designed to accommodate these pressures. A hydrostatic groundwater pressure should be added for the portion below the groundwater level.

Where soil is to be retained, the conditions can be well-represented by assuming the retained soil consists of a material with an angle of internal friction of 30 degrees and a bulk (drained) unit weight of 20 kN/m³. Undrained conditions are anticipated (i.e. below the groundwater level). Therefore, the applicable effective (undrained) unit weight of the retained soil can be taken as 13 kN/m^3 , where applicable. A hydrostatic pressure should be added to the total static earth pressure when using the effective unit weight.

Two distinct conditions, static and seismic, must be reviewed for design calculations. The parameters for design calculations for the two conditions are presented below.

Static Conditions

The static horizontal earth pressure (p_o) can be calculated using a triangular earth pressure distribution equal to $K_o \cdot \gamma \cdot H$ where:

- K_{o} = at-rest earth pressure coefficient of the applicable retained soil, 0.5
- γ = unit weight of fill of the applicable retained soil (kN/m³)
- H = height of the wall (m)

An additional pressure having a magnitude equal to $K_{o} \cdot q$ and acting on the entire height of the wall should be added to the above diagram for any surcharge loading, q (kPa), that may be placed at ground surface adjacent to the wall. The surcharge pressure will only be applicable for static analyses and should not be used in conjunction with the seismic loading case.

Actual earth pressures could be higher than the "at-rest" case if care is not exercised during the compaction of the backfill materials to maintain a minimum separation of 0.3 m from the walls with the compaction equipment.

Seismic Conditions

The total seismic force (P_{AE}) includes both the earth force component (P_o) and the seismic component (ΔP_{AE}).

The seismic earth force (ΔP_{AE}) can be calculated using $0.375 \cdot a_c \cdot \gamma \cdot H^2/g$ where:

 $a_c = (1.45 - a_{max}/g)a_{max}$ $\gamma = unit weight of fill of the applicable retained soil (kN/m³)$ H = height of the wall (m)g = gravity, 9.81 m/s²

The peak ground acceleration, (a_{max}) , for the Ottawa area is 0.32g according to OBC 2012. Note that the vertical seismic coefficient is assumed to be zero.

The earth force component (P_o) under seismic conditions can be calculated using P_o = 0.5 K_o γ H², where K_o = 0.5 for the soil conditions noted above.

The total earth force (P_{AE}) is considered to act at a height, h (m), from the base of the wall, where:

 $h = \{P_{o} \cdot (H/3) + \Delta P_{AE} \cdot (0.6 \cdot H)\} / P_{AE}$

The earth forces calculated are unfactored. For the ULS case, the earth loads should be factored as live loads, as per OBC 2012.

5.7 Pavement Structure

For design purposes, the flexible pavement structure presented in the following table could be used for the design of car only parking areas in the lower level of the parking garage.

Table 4 - Recommended Pavement Structure - Parking Areas										
Thickness (mm)	Material Description									
50	Wear Course - HL 3 or Superpave 12.5 Asphaltic Concrete									
150	BASE - OPSS Granular A Crushed Stone									
300	SUBBASE - OPSS Granular B Type II									
SUBGRADE - situ soil.	Either fill, in situ silty clay or sand or crushed stone material placed over in									

 Table 5 - Recommended Pavement Structure - Local Roadways, Access Lanes and

 Heavy Vehicle Parking

Thickness (mm)	Material Description
40	Wear Course - Superpave 12.5 Asphaltic Concrete
50	Binder Course - Superpave 19.0 Asphaltic Concrete
150	BASE - OPSS Granular A Crushed Stone
400	SUBBASE - OPSS Granular B Type II
SUBGRADE - situ soil.	Either fill, in situ silty clay or sand or crushed stone material placed over in

Minimum Performance Graded (PG) 58-34 asphalt cement should be used for parking areas and local roadways and PG 64-34 asphalt cement should be used for roadways with bus traffic. The pavement granular base and subbase should be placed in maximum 300 mm thick lifts and compacted to a minimum of 100% of the material's SPMDD using suitable vibratory equipment.

The proposed pavement structure, where it abuts the existing pavement, should match the existing pavement layers. It is recommended that a 300 mm wide and 50 mm deep stepped joint be provided where the new asphalt layer joins with the existing asphalt layer to provide more resistance to cracking at the joint.



6.0 Design and Construction Precautions

6.1 Foundation Drainage and Backfill

Foundation Drainage and Waterproofing

It is expected that the building foundation walls will be placed in close proximity to all the boundaries. It is expected that the foundation wall will be blind poured against a drainage system and waterproofing system fastened against the shoring system.

A waterproofing membrane will be required to lessen the effect of water infiltration for the lower P-2 basement level. The waterproofing membrane can be placed and fastened to the shoring system (soldier pile and timber lagging) and should extend to the bottom of the excavation at the founding level of the raft foundation.

It is recommended that the composite drainage system, such as Delta Drain 6000 or equivalent, extend from the exterior finished grade to the founding elevation (underside of raft slab). The purpose of the composite drainage system is to direct any water infiltration resulting from a breach of the waterproofing membrane to the building sump pit. It is recommended that 150 mm diameter sleeves at 3 m centres be cast in the foundation wall at the raft slab interface to allow the infiltration of water to flow to an interior perimeter underfloor drainage pipe. The perimeter drainage pipe should direct water to sump pit(s) within the lower basement area.

Foundation Raft Slab Construction Joints

If applicable, it is expected that the raft slab will be poured in sections. For the construction joint at each pour should incorporate a rubber water stop along with a chemical grout (Xypex or equivalent) applied to the entire vertical joint of the raft slab. Furthermore, a rubber water stop should be incorporated in the horizontal interface between the foundation wall and the raft slab.

Underfloor Drainage

Underfloor drainage will be required to control water infiltration due to groundwater infiltration at the proposed founding elevation. For design purposes, we recommend that 150 mm in diameter perforated pipes be placed along the interior perimeter of the foundation wall and one drainage line within each bay. The spacing of the underfloor drainage system should be confirmed at the time of backfilling the floor completing the excavation when water infiltration can be better assessed.

Adverse Effects of Dewatering on Adjacent Properties

It is understood that up to 2 underground parking levels are planned for the proposed development, with the lower portion of the foundation having a groundwater infiltration control system in place. The existing buildings along the west portion are expected to be founded over bedrock or within the glacial till above the bedrock surface.

Based on field observations and assessment, the groundwater level is anticipated at a 5 to 6 m depth below existing grade. A local groundwater lowering is expected under short-term conditions due to construction of the proposed building. It should be noted that the extent of any significant groundwater lowering will take place within a limited range of the subject site due to the minimal groundwater lowering. It should also be noted that the lower portion of the foundation walls will be waterproofed which will limit groundwater lowering within the subject site and surroundings.

Since the neighbouring structures are founded within native glacial till or directly over a bedrock bearing surface based on available soils information. No issues are expected with respect to groundwater lowering that would cause long term damage to adjacent structures surrounding the proposed building.

Foundation Backfill

Above the bedrock surface, backfill against the exterior sides of the foundation walls should consist of free-draining non frost susceptible granular materials. The greater part of the site excavated materials will be frost susceptible and, as such, are not recommended for re-use as backfill against the foundation walls, unless used in conjunction with a drainage geocomposite, such as Miradrain G100N or Delta Drain 6000, connected to the perimeter foundation drainage system. Imported granular materials, such as clean sand or OPSS Granular B Type I granular material, should otherwise be used for this purpose.

6.2 **Protection of Footings Against Frost Action**

The parking garage may require protection against frost action depending on the founding depth. Unheated structures, such as the access ramp wall footings, may be required to be insulated against the deleterious effect of frost action. A minimum of 2.1 m of soil cover alone, or a minimum of 0.6 m of soil cover, in conjunction with foundation insulation, should be provided.

Perimeter footings of heated structures are required to be insulated against the deleterious effects of frost action. A minimum of 1.5 m of soil cover alone, or a minimum of 0.6 m of soil cover, in conjunction with adequate foundation insulation, should be provided. More details regarding foundation insulation can be provided, if requested.

The effectiveness of this operation will depend on the ability of maintaining vertical trenches until the lean concrete can be poured. It is suggested that once the bottom of the excavation is exposed, an assessment should be completed to determine the water infiltration and stability of the excavation sidewalls extending to the bedrock surface.

Exterior unheated footings, such as those for isolated exterior piers, are more prone to deleterious movement associated with frost action than the exterior walls of the heated structure and require additional protection, such as soil cover of 2.1 m or an equivalent combination of soil cover and foundation insulation.

6.3 Excavation Side Slopes

Unsupported Side Slopes

The side slopes of excavations in the soil and fill overburden materials should either be excavated at acceptable slopes or should be retained by shoring systems from the beginning of the excavation until the structure is backfilled. Insufficient room is expected for majority of the excavation to be constructed by open-cut methods (i.e. unsupported excavations).

The excavation side slopes above the groundwater level extending to a maximum depth of 3 m should be excavated at 1H:1V or shallower. The shallower slope is required for excavation below groundwater level. The subsurface soils are considered to be a Type 2 and 3 soil according to the Occupational Health and Safety Act and Regulations for Construction Projects.

Excavated soil should not be stockpiled directly at the top of excavations and heavy equipment should be kept away from the excavation sides.

Slopes in excess of 3 m in height should be periodically inspected by the geotechnical consultant in order to detect if the slopes are exhibiting signs of distress. A trench box is recommended to protect personnel working in trenches with steep or vertical sides. Services are expected to be installed by "cut and cover" methods and excavations should not remain open for extended periods of time.

Temporary Shoring

Temporary shoring will be required to support the overburden soils. The design and implementation of these temporary systems will be the responsibility of the excavation contractor or the shoring contractor and their design team. Inspections and approval of the temporary system will also be the responsibility of the designer. Geotechnical information provided below is to assist the designer in completing a suitable and safe shoring system. The designer should take into account the impact of a significant precipitation event and designate design measures to ensure that a precipitation will not negatively impact the shoring system or soils supported by the system. Any changes to the approved shoring design system should be reported immediately to the owner's representative prior to implementation.

Temporary shoring may be required to complete the required excavations where insufficient room is available for open cut methods. The shoring requirements will depend on the depth of the excavation, the proximity of the adjacent buildings and underground structures and the elevation of the adjacent building foundations and underground services. Additional information can be provided when the above details are known.

For design purposes, the temporary system may consist of soldier pile and lagging system or interlocking steel sheet piling. Any additional loading due to street traffic, construction equipment, adjacent structures and facilities, etc., should be added to the earth pressures described below. These systems can be cantilevered, anchored or braced. The earth pressures acting on the shoring system may be calculated using the following parameters.

Table 6 - Soil Parameters for Shoring System Design									
Parameters	Values								
Active Earth Pressure Coefficient (K _a)	0.33								
Passive Earth Pressure Coefficient (K _p)	3								
At-Rest Earth Pressure Coefficient (K _o)	0.5								
Unit Weight (γ), kN/m³	20								
Submerged Unit Weight (γ), kN/m ³	13								

Generally, it is expected that the shoring systems will be provided with tie-back rock anchors to ensure their stability. It is further recommended that the toe of the shoring be adequately supported to resist toe failure.

The geotechnical design of grouted rock anchors in sedimentary bedrock is based upon two possible failure modes. The anchor can fail either by shear failure along the grout/rock interface or by pullout of a 60 to 90 degree cone of rock with the apex of the cone near the middle of the bonded length of the anchor.

The anchor derives its capacity from the bonded portion, or fixed anchor length, at the base of the anchor. An unbonded portion, or free anchor length, is also usually provided between the rock surface and the start of the bonded length. A factored tensile grout to rock bond resistance value at ULS of **1.0 MPa**, incorporating a resistance factor of 0.3, can be used. A minimum grout strength of 40 MPa is recommended.

It is recommended that the anchor drill hole diameter be within 1.5 to 2 times the rock anchor tendon diameter and the anchor drill holes be inspected by geotechnical personnel and should be flushed clean prior to grouting. The use of a grout tube to place grout from the bottom up in the anchor holes is further recommended.

The geotechnical capacity of each rock anchor should be proof tested at the time of construction. More information on testing can be provided upon request. Compressive strength testing is recommended to be completed for the rock anchor grout. A set of grout cubes should be tested for each day grout is prepared.

Soldier Pile and Lagging System

The active earth pressure acting on a soldier pile and lagging shoring system can be calculated using a rectangular earth pressure distribution with a maximum pressure of 0.65 K γ H for strutted or anchored shoring or a triangular earth pressure distribution with a maximum value of K γ H for a cantilever shoring system. H is the height of the excavation.

The active earth pressure should be used where wall movements are permissible while the at-rest pressure should be used if no movement is permissible.

The total unit weight should be used above the groundwater level while the submerged unit weight should be used below the groundwater level.

The hydrostatic groundwater pressure should be added to the earth pressure distribution wherever the submerged unit weights are used for earth pressure calculations should the level on the groundwater not be lowered below the bottom of the excavation. If the groundwater level is lowered, the total unit weight for the soil should be used full weight, with no hydrostatic groundwater pressure component.

Concrete Underpinning

Based on proximity of existing adjacent buildings support in the form of concrete underpinning maybe required during excavation for the proposed building. It is expected that the founding elevations of the existing foundations will be in close proximity to the bedrock surface (less than 1.5 m) and conventional concrete underpinning may be used to support the full width and length of the foundation.

It is expected that the structural engineer along with the geotechnical engineer will review the site conditions at the time of construction and finalize the underpinning program based on their observations at that time.

6.4 Pipe Bedding and Backfill

Bedding and backfill materials should be in accordance with the most recent Material Specifications & Standard Detail Drawings from the Department of Public Works and Services, Infrastructure Services Branch of the City of Ottawa.

A minimum of 150 mm of OPSS Granular A should be placed for bedding for sewer or water pipes when placed on soil subgrade. If the bedding is placed on bedrock, the thickness of the bedding should be increased to 300 mm for sewer pipes. The bedding should extend to the spring line of the pipe. Cover material, from the spring line to a minimum of 300 mm above the obvert of the pipe should consist of OPSS Granular A (concrete or PSM PVC pipes) or sand (concrete pipe). The bedding and cover materials should be placed in maximum 225 mm thick lifts and compacted to 95% of the SPMDD.

Where hard surface areas are considered above the trench backfill, the trench backfill material within the frost zone (about 1.8 m below finished grade) should match the soils exposed at the trench walls to reduce the potential differential frost heaving. The trench backfill should be placed in maximum 300 mm thick loose lifts and compacted to a minimum of 95% of the SPMDD.

6.5 Groundwater Control

It is anticipated that groundwater infiltration into the excavations should be controllable using open sumps. The contractor should be prepared to direct water away from all bearing surfaces and subgrades, regardless of the source, to prevent disturbance to the founding medium. A temporary Ministry of Environment, Conservation and Parks (MECP) Category 3 Permit to Take Water (PTTW) may be required if more than 400,000 L/day are to be pumped during the construction phase. At least 4 to 5 months should be allowed for completion of the application and issuance of the permit by the MECP.

For typical ground or surface water volumes being pumped during the construction phase, typically between 50,000 to 400,000 L/day, it is required to register on the Environmental Activity and Sector Registry (EASR). A minimum of two to four weeks should be allotted for completion of the EASR registration and the Water Taking and Discharge Plan to be prepared by a Qualified Person as stipulated under O.Reg. 63/16. If a project qualifies for a PTTW based upon anticipated conditions, an EASR will not be allowed as a temporary dewatering measure while awaiting the MECP review of the PTTW application.

6.6 Winter Construction

Precautions must be taken if winter construction is considered for this project. The subsoil conditions at this site mostly consist of frost susceptible materials. In presence of water and freezing conditions ice could form within the soil mass. Heaving and settlement upon thawing could occur.

In the event of construction during below zero temperatures, the founding stratum should be protected from freezing temperatures by the use of straw, propane heaters and tarpaulins or other suitable means. In this regard, the base of the excavations should be insulated from sub-zero temperatures immediately upon exposure and until such time as heat is adequately supplied to the building and the footings are protected with sufficient soil cover to prevent freezing at founding level.

The trench excavations should be carried out in a manner to avoid the introduction of frozen materials, snow or ice into the trenches. Precaution must be taken where excavations are carried in proximity of existing structures which may be adversely affected due to the freezing conditions. In particular, it should be recognized that where a shoring system is used, the soil behind the shoring system will be subjected to freezing conditions and could result in heaving of the structure(s) placed within or above frozen soil. Provisions should be made in the contract document to protect the walls of the excavations from freezing, if applicable.

6.7 Corrosion Potential and Sulphate

The analytical testing results indicate that the sulphate content is less tan 0.1%. This results indicates that Type 10 Portland Cement (i.e. normal cement) would be appropriate for this site. The chloride content and pH of the samples indicate that they are not significant factors in creating a corrosive environment, whereas the resistivity is indicative of an aggressive corrosive environment.

7.0 Recommendations

A materials testing and observation services program is a requirement for the provided foundation design data to be applicable. The following aspects of the program should be performed by the geotechnical consultant:

- Review of the geotechnical aspects of the excavating contractor's shoring design, prior to construction.
- **Q** Review the bedrock stabilization and excavation requirements.
- **Q** Review proposed foundation drainage design and requirements.
- Observation of all bearing surfaces prior to the placement of concrete.
- □ Sampling and testing of the concrete and fill materials used.
- Observation of all subgrades prior to backfilling.
- **□** Field density tests to determine the level of compaction achieved.

A report confirming that these works have been conducted in general accordance with our recommendations could be issued, upon request, following the completion of a satisfactory materials testing and observation program by the geotechnical consultant.



8.0 Statement of Limitations

The recommendations provided in this report are in accordance with our present understanding of the project. We request permission to review our recommendations when the drawings and specifications are completed.

A soils investigation is a limited sampling of a site. Should any conditions at the site be encountered which differ from those at the test locations, we request immediate notification to permit reassessment of our recommendations.

The recommendations provided herein should only be used by the design professionals associated with this project. They are not intended for contractors bidding on or undertaking the work. The latter should evaluate the factual information provided in this report and determine its suitability and completeness for their intended construction schedule and methods. Additional testing may be required for their purposes.

The present report applies only to the project described in this document. Use of this report for purposes other than those described herein or by person(s) other than Brigil or their agents is not authorized without review by Paterson for the applicability of our recommendations to the alternative use of the report.

Paterson Group Inc.

Joey R. Villeneuve, M.A.Sc., P.Eng.



David J. Gilbert, P.Eng.

Report Distribution

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- Paterson Group

APPENDIX 1

SOIL PROFILE AND TEST DATA SHEETS

SYMBOLS AND TERMS

ANALYTICAL TESTING RESULTS

SOIL PROFILE AND TEST DATA

154 Colonnade Road South, Ottawa, Ontario K2E 7J5

DATUM Geodetic									FILE NO	PG5172		
REMARKS									HOLE N	^{0.} BH 1		
BORINGS BY CME-55 Low Clearance I	Drill			D	DATE .	June 25,	2020					
SOIL DESCRIPTION	PLOT		DEP			DEPTH (m)	DEPTH ELEV.	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone				
	TRATA	ТҮРЕ	UMBER	COVER	VALUE r RQD			• v	Vater Co	ntent %	ezomet	
GROUND SURFACE	ν ν		Z	RE	z ^o	0-	-82 55	20	40	60 80	ΞÖ	
Asphaltic concrete 0.08 FILL: Brown silty sand with crushed 0.51 stone		AU	1				02.00					
		ss	2	67	10	1-	-81.55					
Hard to very stiff, brown SILTY CLAY		ss	3	67	14	2-	-80.55					
2.05		ss	4	100	10							
<u>3.05</u>		ss	5	100	28	3-	- 79.55					
GLACIAL TILL: Compact to dense, brown silty sand and gravel		ss	6	33	8	4-	-78.55					
		ss	7	33	4	5-	-77.55					
<u>5.84</u>		ss	8	43	50+							
End of Borehole												
Practical refusal to augering at 5.84m depth.												
(GWL @ 2.75m - July 7, 2020)												
								20 Shea ▲ Undist	40 ar Streng urbed 2	60 80 1 jth (kPa) ∆ Remoulded	1 00	

SOIL PROFILE AND TEST DATA

154 Colonnade Road South, Ottawa, Ontario K2E 7J5

DATUM Geodetic								FILE NO). PG5172	
REMARKS	الانعا			-		luna 05	2020	HOLE N	^{ю.} BH 2	
BORINGS BY CIVIE-55 LOW Clearance			SVI		Don Dociot B	Don Booist Blows/0.2m				
SOIL DESCRIPTION	PLOT		SAN	MPLE X	61	DEPTH (m)	ELEV. (m)	• 50 mm Di	ia. Cone	ter
		ТҮРЕ	UMBER	COVER	VALUE E ROD			O Water Co	ntent %	ezome
GROUND SURFACE	N N		Z	RE	z °	0-	-81 92	20 40	60 80	ie (
Asphaltic concrete 0.10 FILL: Brown silty sand with crushed stone 0.60		AU	1				01.02			
Very stiff, brown SILTY CLAY, trace sand <u>1.3</u> 7	7	ss	2	29	11	1-	-80.92			
GLACIAL TILL: Compact to dense, brown silty sand with clay and gravel, some cobbles		ss	3	67	18	2-	-79.92		· · · · · · · · · · · · · · · · · · ·	
		ss	4	33	19					
		ss	5	33	25	3-	-78.92			
		ss	6	25	16	4-	-77.92			
		ss	7	33	19	5-	-76.92			
		ss	8	83	4					
6.35	5	⊻ ∑ss	9	50	50+	6-	-75.92			
Practical refusal to augering at 6.35m										
(GWL @ 3.54m - July 7, 2020)										
								20 40 Shear Streng ▲ Undisturbed	oo 80 10 3th (kPa) △ Remoulded	UU

SOIL PROFILE AND TEST DATA

154 Colonnade Road South, Ottawa, Ontario K2E 7J5

DATUM Geodetic									FILE NO.	PG5172	
REMARKS				_		L			HOLE NO	BH 3	
BORINGS BY CME-55 LOW Clearance					ATE .	June 25,	2020			200	
SOIL DESCRIPTION	PLOT		SAN			DEPTH (m)	ELEV. (m)	Pen. Re ● 50	esist. Bio) mm Dia	ws/0.3m . Cone	er ion
	RATA	ЗЧХ	MBER	%	VALUE ROD			• w	ater Con	tent %	zomet istruct
GROUND SURFACE	LS I	F	NN	REC	N O			20	40 6	0 80	Pie: Cor
Asphaltic concrete 0.10 FILL: Brown silty sand with crushed 0.60 stone 0.60		AU	1			- 0-	-82.05				
		ss	2	75	12	1-	-81.05				
Very stiff, brown SILTY CLAY, trace sand		ss	3	100	12	2-	-80.05				
<u>2.90</u>		ss	4	100	11						
		ss	5	50	21	3-	- 79.05				
GLACIAL TILL: Comapct to dense.		ss	6	58	21	4-	-78.05			· · · · · · · · · · · · · · · · · · ·	
brown silty sand, some clay and gravel, trace cobbles		ss	7	58	8	5-	-77.05				
		ss	8	17	44	6-	-76.05				
		ss	9	8	8						
7.26		ss	10	25	50+	7-	-75.05				
Practical refusal to augering at 7.26m depth.											
(GWL @ 4.72m - July 7, 2020)											
								20 Shea ▲ Undistu	40 60 r Strengt urbed △	0 80 1 h (kPa) Remoulded	00

SOIL PROFILE AND TEST DATA

154 Colonnade Road South, Ottawa, Ontario K2E 7J5

DATUM Geodetic									FILE NO	PG5172	
REMARKS									HOLE N	0.	
BORINGS BY CME-55 Low Clearance	Drill			D	ATE .	June 25,	2020			BH 4	
SOIL DESCRIPTION	LOT		SAN	IPLE	1	DEPTH	ELEV.	Pen. Re	esist.B	lows/0.3m a. Cone	, _
		ы	ER	ERY	50 E	(m)	(m)		neter		
	STRA	đХТ	NUMB		L VAJ			• N	ater Co	ntent %	ezor onsti
GROUND SURFACE		x	-	R	ZŬ	0-	-81.35	20	40	60 80	С <u>Б</u>
		AU	1								
FILL: Brown silty sand with crushed		∛ss	2	67	47	1-	-80.35				
		\square									
2.29		ss	3	8	8	2-	-79.35				
Very stiff brown SILTY CLAY , trace		ss	4	100	11					· · · · · · · · · · · · · · · · · · ·	
sand			5		44	3-	-78.35				
- sand seam at 3.4m depth <u>3.81</u>		A 33	5								
GLACIAL TILL: Dense, brown silty sand, some gravel, trace clay, cobbles		ss	6		41	4-	-77.35				
and boulders 4.80		<u>∑</u> ss	7		50+						
End of Borehole											
Practical refusal to augering at 4.80m depth.											
(GWL @ 4.01m - July 7, 2020)											
								20	40	60 80 1	⊣ 00
								Shea ▲ Undist	r Streng urbed 2	gth (kPa) ∆ Remoulded	

SOIL PROFILE AND TEST DATA

154 Colonnade Road South, Ottawa, Ontario K2E 7J5

DATUM Geodetic								FIL	.E NO. PG5172			
REMARKS BORINGS BY CME-55 Low Clearance	Drill			п		lune 25	2020	нс	BH 5			
	E E	SAMPLE						Pen. Resis	Resist. Blows/0.3m			
SOIL DESCRIPTION	A PLO		<i>«</i>	ЗХ	Що	(m)	ELEV. (m)	• 50 m	m Dia. Cone	ster		
	TRAT	ТҮРЕ	UMBEI	COVEI	VALU r RQI			○ Wate	r Content %	some		
GROUND SURFACE	N N		Z	REC	z ^ö	0-	-81 71	20 40	40 60 80			
Asphaltic concrete 0.10 FILL: Brown slty sand with crushed 0.60 stone 0.60		AU	1									
		ss	2	100	8	1-	80.71					
Hard to very stiff, brown SILTY CLAY		ss	3	100	13	2.	-70 71					
		ss	4	100	9		79.71					
2.90		∬ ss	5	67	8	3-	-78.71					
GLACIAL TILL: Loose to compact.		∬ ss	6	42	15	4-	-77.71					
brown silty sand, some gravel, trace clay and cobbles		∦ ₩ <<	7	67	12							
- grey by 4.9m depth			,	50	12	5-	-76.71					
5.8	9 <u>\^^^^</u>	∦ ₽-	0	50				·····	<u></u>			
Practical refusal to augering at 5.89m depth.												
(GWL @ 3.28m - July 7, 2020)												
								20 40 Shear Si ▲ Undisturbe	60 80 1 irength (kPa) d △ Remoulded	00		

SOIL PROFILE AND TEST DATA

154 Colonnade Road South, Ottawa, Ontario K2E 7J5

DATUM Geodetic									FILE NO.	PG5172		
REMARKS								-	HOLE NO).		
BORINGS BY CME-55 Low Clearance	Drill	1		D	ATE .	June 26, 1	2020			BH 6		
SOIL DESCRIPTION	PLOT		SAN			DEPTH (m)	ELEV.	Pen. Res • 50	sist. Bl mm Dia	ows/0.3m a. Cone	er ion	
	STRATA	ТҮРЕ	NUMBER	SCOVER'	VALUE Dr RQD	(,	(,	⊖ Wa	• Water Content %			
GROUND SURFACE		×	~	R	ZŬ	0-	-82.02	20	40 6	60 80	С <u>Б</u>	
Asphaltic concrete 0.08 FILL: Brown silty sand with crushed0.51 stone		AU	1									
FILL: Brown silty sand, some gravel,		ss	2	25	54	1-	-81.02					
trace coddles		ss	3	17	33	2-	-80.02					
		ss	4	92	10	_						
GLACIAL TILL: Brow silty clay with		ss	5	92	11	3-	-79.02		· · · · · · · · · · · · · · · · · · ·			
sand and gravel		ss	6	75	18	4-	-78.02					
		ss	7	75	23	5-	-77.02					
		ss	8	42	13							
<u>6.10</u>		ss	9	67	3	6-	-76.02					
GLACIAL TILL: Grey silty sand, some gravel, trace clay		ss	10	17	1	7-	-75.02					
		n N ss	11		25	0	74.00					
Dynamic Cone Penetration Test commenced at 8.23m depth.		<u></u>				0-	-74.02					
Inferred GLACIAL TILL						9-	-73.02				-	
End of Borehole		+									•	
Practical DCPT refusal at 9.47m depth												
(GWL @ 1.88m - July 7, 2020)												
								20 Shear ▲ Undistur	40 € Streng	50 80 1 th (kPa) Remoulded	⊣ 00	

SOIL PROFILE AND TEST DATA

154 Colonnade Road South, Ottawa, Ontario K2E 7J5

DATUM Geodetic								FILE NO. PG5172
REMARKS								
BORINGS BY CME-55 Low Clearance	Drill			D	ATE .	June 25, 2	020	
SOIL DESCRIPTION	PLOT		SAN	IPLE		DEPTH (m)	ELEV. (m)	Pen. Resist. Blows/0.3m ● 50 mm Dia. Cone
	STRATA	ТҮРЕ	IUMBER	COVER'	VALUE Pr RQD		. ,	• Water Content %
GROUND SURFACE		~	4	RE	z º	0+	81 61	20 40 60 80 🗖
Asphaltic concrete0.10 FILL: Brown silty sand with crushed stone0.76		AU	1				01.01	
FILL: Brown silty clay, trace sand and gravel1.37	7	ss	2	83	10	1-	80.61	
FILL: Brown silty clay, trace sand		ss	3	100	14	2-	79.61	
		ss	4	83	22		-	
GLACIAL TILL: Brown silty clay with sand and gravel, trace cobbles		ss	5	92	39	3+	78.61	
4.57	7	ss	6	4	29	4-	77.61	
GLACIAL TILL: Grey silty sand, trace		ss	7	33	15	5-	76.61	
5.87	7 ^ ^ ^ ^ / ^ / / ^ / / ^ / / / / / / /	ss	8	38	19	6+	75.61	
		RC	1	92	65	7-	74.61	
BEDROCK: Fair to poor quality, grey limestone		- RC	2	100	48	8-	73.61	
9.04 End of Borehole						9-	72.61	
(GWL @ 3.15m - July 7, 2020)								
								20 40 60 80 100 Shear Strength (kPa) ▲ Undisturbed △ Remoulded

SYMBOLS AND TERMS

SOIL DESCRIPTION

Behavioural properties, such as structure and strength, take precedence over particle gradation in describing soils. Terminology describing soil structure are as follows:

Desiccated	-	having visible signs of weathering by oxidation of clay minerals, shrinkage cracks, etc.
Fissured	-	having cracks, and hence a blocky structure.
Varved	-	composed of regular alternating layers of silt and clay.
Stratified	-	composed of alternating layers of different soil types, e.g. silt and sand or silt and clay.
Well-Graded	-	Having wide range in grain sizes and substantial amounts of all intermediate particle sizes (see Grain Size Distribution).
Uniformly-Graded	-	Predominantly of one grain size (see Grain Size Distribution).

The standard terminology to describe the strength of cohesionless soils is the relative density, usually inferred from the results of the Standard Penetration Test (SPT) 'N' value. The SPT N value is the number of blows of a 63.5 kg hammer, falling 760 mm, required to drive a 51 mm O.D. split spoon sampler 300 mm into the soil after an initial penetration of 150 mm.

Relative Density	'N' Value	Relative Density %
Very Loose	<4	<15
Loose	4-10	15-35
Compact	10-30	35-65
Dense	30-50	65-85
Very Dense	>50	>85

The standard terminology to describe the strength of cohesive soils is the consistency, which is based on the undisturbed undrained shear strength as measured by the in situ or laboratory vane tests, penetrometer tests, unconfined compression tests, or occasionally by Standard Penetration Tests.

Consistency	Undrained Shear Strength (kPa)	'N' Value
Very Soft	<12	<2
Soft	12-25	2-4
Firm	25-50	4-8
Stiff	50-100	8-15
Very Stiff	100-200	15-30
Hard	>200	>30

SYMBOLS AND TERMS (continued)

SOIL DESCRIPTION (continued)

Cohesive soils can also be classified according to their "sensitivity". The sensitivity is the ratio between the undisturbed undrained shear strength and the remoulded undrained shear strength of the soil.

Terminology used for describing soil strata based upon texture, or the proportion of individual particle sizes present is provided on the Textural Soil Classification Chart at the end of this information package.

ROCK DESCRIPTION

The structural description of the bedrock mass is based on the Rock Quality Designation (RQD).

The RQD classification is based on a modified core recovery percentage in which all pieces of sound core over 100 mm long are counted as recovery. The smaller pieces are considered to be a result of closely-spaced discontinuities (resulting from shearing, jointing, faulting, or weathering) in the rock mass and are not counted. RQD is ideally determined from NXL size core. However, it can be used on smaller core sizes, such as BX, if the bulk of the fractures caused by drilling stresses (called "mechanical breaks") are easily distinguishable from the normal in situ fractures.

RQD % ROCK QUALITY

90-100	Excellent, intact, very sound
75-90	Good, massive, moderately jointed or sound
50-75	Fair, blocky and seamy, fractured
25-50	Poor, shattered and very seamy or blocky, severely fractured
0-25	Very poor, crushed, very severely fractured

SAMPLE TYPES

SS	-	Split spoon sample (obtained in conjunction with the performing of the Standard
		Penetration Test (SPT))

- TW Thin wall tube or Shelby tube
- PS Piston sample
- AU Auger sample or bulk sample
- WS Wash sample
- RC Rock core sample (Core bit size AXT, BXL, etc.). Rock core samples are obtained with the use of standard diamond drilling bits.
SYMBOLS AND TERMS (continued)

GRAIN SIZE DISTRIBUTION

MC%	-	Natural moisture content or water content of sample, %				
LL	-	Liquid Limit, % (water content above which soil behaves as a liquid)				
PL	-	Plastic limit, % (water content above which soil behaves plastically)				
PI	-	Plasticity index, % (difference between LL and PL)				
Dxx	-	Grain size which xx% of the soil, by weight, is of finer grain sizes These grain size descriptions are not used below 0.075 mm grain size				
D10	-	Grain size at which 10% of the soil is finer (effective grain size)				
D60	-	Grain size at which 60% of the soil is finer				
Сс	-	Concavity coefficient = $(D30)^2 / (D10 \times D60)$				
Cu	-	Uniformity coefficient = D60 / D10				
Cc and Cu are used to assess the grading of sands and gravels:						

Well-graded gravels have: 1 < Cc < 3 and Cu > 4Well-graded sands have: 1 < Cc < 3 and Cu > 4Well-graded sands have: 1 < Cc < 3 and Cu > 6Sands and gravels not meeting the above requirements are poorly-graded or uniformly-graded. Cc and Cu are not applicable for the description of soils with more than 10% silt and clay (more than 10% finer than 0.075 mm or the #200 sieve)

CONSOLIDATION TEST

p'o	-	Present effective overburden pressure at sample depth
p'c	-	Preconsolidation pressure of (maximum past pressure on) sample
Ccr	-	Recompression index (in effect at pressures below p'c)
Сс	-	Compression index (in effect at pressures above p'c)
OC Ratio		Overconsolidaton ratio = p'c / p'o
Void Ratio	D	Initial sample void ratio = volume of voids / volume of solids
Wo	-	Initial water content (at start of consolidation test)

PERMEABILITY TEST

k - Coefficient of permeability or hydraulic conductivity is a measure of the ability of water to flow through the sample. The value of k is measured at a specified unit weight for (remoulded) cohesionless soil samples, because its value will vary with the unit weight or density of the sample during the test.

SYMBOLS AND TERMS (continued) STRATA PLOT Topsoil Asphalt Peat Sand Silty Sand Fill Δ Sandy Silt Clay Silty Clay Clayey Silty Sand Glacial Till Shale Bedrock

MONITORING WELL AND PIEZOMETER CONSTRUCTION









Certificate of Analysis Client: Paterson Group Consulting Engineers Client PO: 24713

Report Date: 03-Jul-2020

Order Date: 26-Jun-2020

Project Description: PG5172

	Client ID:	BH3-SS3	-	-	-
	Sample Date:	25-Jun-20 13:00	-	-	-
	Sample ID:	2026529-01	-	-	-
	MDL/Units	Soil	-	-	-
Physical Characteristics					
% Solids	0.1 % by Wt.	74.6	-	-	-
General Inorganics					
pН	0.05 pH Units	7.58	-	-	-
Resistivity	0.10 Ohm.m	13.3	-	-	-
Anions					
Chloride	5 ug/g dry	179	-	-	-
Sulphate	5 ug/g dry	389	-	-	-

APPENDIX 2

FIGURE 1 - KEY PLAN

DRAWING PG5172-1 - TEST HOLE LOCATION PLAN



RIDEAUVIEW



LEGEND:

BOREHOLE LOCATION

82.05 GROUND SURFACE ELEVATION (m)

(74.79) REFUSAL TO AUGERING ELEVATION (m)

[72.29] BEDROCK SURFACE ELEVATION (m)

ALL GROUND SURFACE ELEVATIONS REFERENCE A GEODETIC DATUM (NAD83)

SCALE: 1:750

	0	10	20 30	50m
	Scale:		Date:	
		1:750		07/2020
	Drawn by:		Report No.:	
		NFRV		PG5172-1
ONTARIO	Checked by:		Dwg. No.:	
		JV	PG!	5172-1
	Approved by:			
		DJG	Revision No.:	

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Phase One Environmental Site Assessment

729 Ridgewood Avenue Ottawa, Ontario

Prepared for: 11684663 Canada Inc.



July 27, 2020

Table of Contents

1.		Ex	ecutive Summary	1
2.		Int	troduction	4
3.		Sc	ope of Investigation	5
4.		Re	ecords Review	6
ä	a)		General	6
		i.	Phase One Study Area	6
		ii.	First Developed Use Determination	6
		iii.	Fire Insurance Plans	6
		iv.	Chain of Title	6
		v.	Environmental Reports	8
I	c)		Environmental Source Information	10
(c)		Physical Setting Sources	16
		i.	Aerial Photographs	16
		ii.	Topography, Hydrology, Geology	18
		iii.	Fill Materials	19
		iv.	Water Bodies and Areas of Natural Significance & Ground Water Information	19
		v.	Well Records	19
(d)		Site Operating Records	20
5.		Int	terviews	20
6.		Sit	te Reconnaissance	21
i	a)		General Requirements	21
l	o)		Specific Observations at Phase One Property	22
		i.	Enhanced Investigation Property	24
(c)		Land Use Observations of the Phase One Study Area	24
7.		Re	eview and Evaluation of Information	25
ä	a)		Current and Past Land Use	25
I	c)		Potentially Contaminating Activity	26

C	:)	Areas of Potential Environmental Concern	. 27
C	1)	Phase One Conceptual Site Model	. 28
8.	C	onclusions	. 31
	i. C	Whether Phase Two Environmental Site Assessment Required Before Record of Site ondition Submitted	. 31
	ii.	Record of Site Condition Based on Phase One Environmental Site Assessment Alone	. 31
	iii	. Signatures	. 32
	iv	z. Limitations	. 32
9.	R	eferences	. 33
10.	A	ppendices	. 35

List of Figures

Figure 1:	Key Plan
Figure 2:	Site Plan
Figure 3:	Surrounding Land Use

List of Tables

Table 1: Potentially Contaminating Activities and Areas of Potential Environmental Concern	3
Table 2: Chain of Title Ownership Summary	7
Table 3: Building Occupants	
Table 4: Current and Past Land Use	25
Table 5: Potentially Contaminating Activities at the Phase One Property	
Table 6: Potentially Contaminating Activities in the Phase One Study Area	
Table 7: Areas of Potential Environmental Concern	

List of Appendices

- Appendix A Legal Survey Plan
- Appendix B Current Proposed Design Concept Plan

Appendix C - Chain of Title

- Appendix D Environmental Risk Information Systems (ERIS) database Search
- Appendix E Ministry of Environment, Conservation and Parks Freedom of Information (FOI) Request
- Appendix F Technical Standards and Safety Association Correspondence
- Appendix G City of Ottawa Historic Land Use Inventory (HLUI)
- Appendix H Aerial Photographs
- Appendix I Topographic Map
- Appendix J Photographic Log
- Appendix K Qualifications of Assessors

1. Executive Summary

Lopers & Associates (Lopers) was retained by 11684663 Canada Inc. (Brigil) to complete a Phase One Environmental Site Assessment (Phase One ESA) of the commercial property with Civic address No. 729 Ridgewood Avenue, Ottawa, Ontario ("Phase One Property", "Property" or "Site").

This Phase One ESA is being completed as part of due diligence requirements associated with the submission of a Development Application to the City of Ottawa Municipal Planning Department. This Phase One ESA can also be used to support the filing of a record of site condition for the Property.

The Phase One Property was undeveloped prior to the 1950's when initial development was interpreted to have been for residential purposes. A commercial lease was registered at the Phase One Property in 1965 and it is inferred that commercial redevelopment of the Property occurred at this time. Demolition of the former residential building was completed prior to 1991. A retail fuel outlet and automotive service garage were present as part of the original commercial development at the Phase One Property and operated on the southeast portion of the Property until 2002 and 2018, respectively. The automotive garage moved to the south unit of the south commercial building in 2018 and has operated at that location on the Phase One Property since that time.

The Property is currently used for commercial purposes and is zoned for mixed use. Brigil purchased the Phase One Property in November of 2019, and it is understood that the intended future use is for mixed use, with commercial ground floors and residential apartments on the subsequent levels. The Phase One Property is immediately surrounded by a municipal Right-of-Way to the south followed by a mixed institutional/commercial property and by residential properties to the north, east and west.

The presence of a former retail fuel outlet and automotive service garage on the southeast portion of the Phase One Property are a significant potentially contaminating activities (PCAs) which represent areas of potential environmental concern (APECs) for the Property. Given that previous reports were provided which document remnant petroleum hydrocarbon (PHC) and benzene, toluene, ethylbenzene and xylenes (BTEXs) soil contamination and that groundwater quality was not confirmed following the completion of a remediation program, further investigation is warranted. The contaminants of potential concern associated with retail fuelling are generally PHCs and BTEXs, and metals as this was an older facility and lead was historically present in gasoline. Based on historical soil analysis in this area of the Property, polycyclic aromatic hydrocarbons (PAH) and volatile organic compounds (VOCs) are also considered contaminants of potential concern associated with the former automotive garage operations. The practice of backfilling following demolition activities at the Phase One Property is also a significant PCA which represents an APEC for the Property. Given that no reports were provided with analytical data to support the environmental quality of the backfill used to fill the former residential building footprint on the central-south portion of the Property, this area warrants further investigation. The contaminants of potential concern commonly found in poor environmental quality backfill are PHCs/BTEXs, PAHs and metals.

The presence of an active automotive service garage was observed during the Site walk over on the central portion of the Phase One Property at the time of the Site Investigation. Although this garage has only been operating for a short time period (2018 to present), these operations are a PCA which represents an APEC for the Property. Based on the observations at this automotive garage, that contaminants of potential concern are considered to be PHCs and BTEXs.

Three active and/or historical fuel storage tank locations at neighbouring properties in the Phase One Study Area constitute PCAs. The PCAs at neighbouring properties in the Phase One Study Area are located significant distances and at down- or cross-gradient orientations with respect to the Phase One Property and are not considered to represent APECs for the Phase One Property.

The PCAs identified at the Phase One Property and neighbouring properties in the Phase One Study Area and APECs at the Phase One Property are included in Table 1 below.

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PCA Report Reference No.	Potentially Contaminating Activity	Location	APEC Report Reference No.
1	Former retail fuel outlet (O.Reg. 153/04 PCA Item 28: Gasoline and Associated Products Storage in Fixed Tanks)	Southeast portion of the Phase One Property	APEC #1 / 2
2	Former automotive service garage (O.Reg. 153/04 PCA Item 52: Storage, Maintenance, Fuelling and Repair of Equipment, Vehicles, and Material used to Maintain Transportation Systems)	Southeast portion of the Phase One Property	APEC #1 / 2
3	Fill placement following demolition activities (O.Reg. 153/04 PCA Item 30: Importation of Fill Material of Unknown Quality)		APEC #3
4	Active automotive service garage (O.Reg. 153/04 PCA Item 52: Storage, Maintenance, Fuelling and Repair of Equipment, Vehicles, and Material used to Maintain Transportation Systems)	Central portion of the Phase One Property (south unit in south commercial plaza building)	APEC #4
5	Suspected fuel (heating oil) storage tank, reported heating oil spill (O.Reg. 153/04 PCA Item 28: Gasoline and Associated Products Storage in Fixed Tanks)	2707 Springland Drive (Residential Dwelling), located approximately 160 m southeast	Not Applicable
6	Suspected fuel storage tank(s), waste generator (O.Reg. 153/04 PCA Item 28: Gasoline and Associated Products Storage in Fixed Tanks)	2960 Riverside Drive (Parkland), located approximately 200 m northwest	Not Applicable
7	Suspected former fuel storage tank(s), waste generator (O.Reg. 153/04 PCA Item 28: Gasoline and Associated Products Storage in Fixed Tanks)	2865 Riverside Drive (Residential property), located approximately 80 m north	Not Applicable

Based on the identification of PCAs and APECs at the Phase One Property, it is recommended that a Phase Two Environmental Site Assessment be completed to assess the soil and groundwater quality in the vicinity of the four APECs.

2. Introduction

Lopers & Associates (Lopers) was retained by 11684663 Canada Inc. (Brigil) to complete a Phase One Environmental Site Assessment (Phase One ESA) of the commercial property with Civic address No. 729 Ridgewood Avenue, Ottawa, Ontario ("Site" or "Phase One Property").

The Phase One Property is legally described as Part of Block C, Registered Plan 749, Part of Block C, Registered Plan 775 and Part of Lot 23 Junction Gore, Township of Gloucester, now in the City of Ottawa and has a property identifier number of 04071-0125, as obtained from a Legal Survey completed by Fairhall, Moffatt & Woodland Limited, on January 8, 2018, provided by Brigil; a copy of the Legal Survey is presented in Appendix A.

Based on approximate dimensions obtained from the City of Ottawa's GIS mapping software, the Phase One Property has an approximate area of 13,200 m² (1.32 Hectares) and a zoning designation of GM1 F(1.0), which signifies a general mixed use zone with a gross floor area restriction of 1 m² per m² of Property. The approximate elevation of the Phase One Property as indicated on the Legal Survey and confirmed through City of Ottawa mapping and Google Earth is between approximately 81 and 84 m above mean sea level (m AMSL). The approximate centre of the Phase One Property has Latitude and Longitude coordinates of 45° 22′ 06″ N and 75° 41′ 16″ W and Universal Transverse Mercator (UTM) coordinates of 446131 m E and 5024099 m N.

The Phase One Property is currently owned by 11684663 Canada Inc., a subsidiary company of Brigil Construction ("Brigil"). It is Lopers' understanding that Brigil intends to redevelop the Phase One Property for mixed use (commercial and residential purposes), including the current concept for construction of one building with five adjoining segments ranging from seven to twenty storeys in height, with subgrade parking, commercial ground floors and residential units above. A copy of an artist's rendering of the current Site development design concept plan, as provided by Brigil, is presented in Appendix B.

This Phase One ESA was commissioned by Mr. Jean-Luc Rivard, Director of Land Development and Infrastructure for Brigil Construction (Brigil), operating as 11684663 Canada Inc. Brigil has a business address of 98 Rue Lois, Gatineau, Quebec, J8Y 3R7 and a business telephone number of 819-243-7392.

3. Scope of Investigation

This Phase One ESA has been completed as per the details of scope presented in Lopers' Letter entitled "Proposal for Phase One Environmental Site Assessment and Phase Two Environmental Site Assessment, Proposed Residential Re-development, 729 Ridgewood Avenue, Ottawa, ON", dated May 8, 2020, reference No. PRO-002-20-Brigil.

The Phase One ESA has been prepared in accordance with the technical requirements and formatting guidance as presented by the Ministry of Environment, Conservation and Parks (MECP) in Ontario Regulation (O.Reg.)153/04, as amended July 1, 2020. This format is based on the provincial regulation for brownfields redevelopment and has been adopted as a standard by the City of Ottawa for development applications.

The scope of work for the Phase One ESA involved the following components:

- Historical Research (Review of available historical reports, public environmental databases, Fire Insurance Plans (FIPs), City Directories, Aerial Photographs, geological mapping and any other relevant environmental records which were readily accessible at the time of the Phase One ESA);
- Requests for Information from the MECP Freedom of Information (FOI), Technical Standards and Safety Authority (TSSA), and City of Ottawa Historical Land Use Inventory (HLUI);
- Subcontracted research of environmental databases through Environmental Risk Information Services (ERIS);
- Property Title Search (subcontracted through READ Abstracts Limited and reviewed herein)
- Physical Site inspection
- Interviews with persons knowledgeable about the Property and past uses
- Interpretation of findings
- Preparation of a Phase One ESA report

The specific objectives of the Phase One ESA are to:

- Provide an overview of the Phase One Environmental Site Assessment conducted with respect to the Phase One Property.
- Provide an environmental record of the Phase One Property, in a manner that can be assessed, tested and reconstructed, to document and demonstrate:
 - How the objectives of the Phase One ESA were achieved and how the requirements for the objectives were met;
 - Whether further investigation is required to submit a Record of Site Condition (RSC) for filing;
 - Whether there exists an adequate basis for further investigation; and,
 - The basis for required certifications.

4. Records Review

- a) General
- i. Phase One Study Area

The Phase One Study Area includes the Phase One Property and properties with the boundaries within 250 m of the Phase One Property limits. Based on a review of the Phase One Property and properties in the Phase One Study Area, their associated historical and/or current uses and operations and physical characteristics of the Phase One Study Area, it was determined that an assessment of properties within 250 m of the Phase One property was sufficient to meet the objectives of the scope of this investigation for a Phase One ESA.

ii. First Developed Use Determination

A land title search was completed by READ Abstracts Limited for the Phase One Property. The title search indicates that the Phase One Property was owned by individuals since at least 1904 until 1959 when ownership of the portions of the Property began to be transferred to Campeau Construction Company Limited. A commercial lease was registered at the Phase One Property in 1965.

Aerial photographs reviewed from 1933 and 1956 show that the Phase One Property use was agricultural use. The 1965 aerial photograph shows the presence of one small building on the central-south portion of the Phase One Property, which was interpreted to be used for residential purposes, and the rest of the Property is undeveloped. The 1976 aerial photograph shows full commercial development at the Phase One Property.

Based on the information reviewed as part of this Phase One ESA, specifically the title search and aerial photographs, the first developed use of the Phase One Property is considered to be 1959.

iii. Fire Insurance Plans

Fire insurance plans (FIPs), were reviewed where available, for the City of Ottawa as part of this Phase One ESA.

There was no coverage in the FIPs for the Phase One Property or for properties located in the Phase One Study Area as part of available FIPs.

iv. Chain of Title

A chronological chain of title was prepared by READ Abstracts Limited for the Phase One Property. The chain of title provides the names of historical owners, lessees and dates of ownership for the Phase One Property dating back to 1904. The legal description as obtained from the Chain of Title was Part of Block C, Registered Plan 749, Part of Block C, Registered Plan

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775 and Part of Lot 23 Concession JG, Gloucester, with a property identifier number of 04071-0125.

Based on additional historical research completed as part of this Phase One ESA and a review of the chain of title, the Phase One Property was agricultural with no developed use prior to 1959. The Phase One Property ownership summary included the evolution of legal descriptions with additional references to Parts of Block C on Plans 749 and 775, respectively with time. A chain of title ownership summary was prepared dating back to 1904 and is presented in Table 2 below. A copy of the Chain of Title for the Phase One Property, as prepared by READ Abstracts Limited for the Phase One Property is provided in Appendix C.

Year(s)	Phase One Property Ownership				
Part of Lot 23, Concession JG					
Prior to September 7, 1904	Ed P. Gleason				
1904 to 1925	Ellen Foran				
1925 to 1938	Robert M. Graham and John H. Graham				
1938 to 1944	John H. Graham				
1944 to 1959	Mary E. Coombs				
1959 to 1959	Norman H. Moody				
1959 to 1963	Campeau Construction Company Limited				
	Part of Block C, Plan 749				
1904 to 1917	Ellen Foran				
1917 to 1920	Thomas C. Bate, Robert S. Low, and Edward McMahon carrying on business as Bate, McMahon and Co.				
1921 to 1926	Thomas C. Bate and Edward McMahon				
1926 to 1943	Edward McMahon				
1943 to 1945	Sidney Munro				
1945 to 1949	Sidney Munro, John W. Lucas and Jessie J. Lucas				
1949 to 1958	Sidney Munro, William Lucas				
1958 to 1959	William Lucas and J. M. Patrick Kelly				
1959 to 1961	William Lucas and Campeau Construction Company Limited				
1961 to 1963	Campeau Construction Company Limited				
	Part of Block C, Plan 775				
1904 to 1926	Ellen Foran				
1926 to 1930 Edward Rutledge					

Table	2:	Chain	of	Title	Ownership	Summary
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Year(s)	Phase One Property Ownership		
1930 to 1947	John W. Dale		
1947 to 1949	Yvonne Griffin		
1949 to 1952	R. Walter Hamilton		
1952 to 1955	James and Sarah Western		
1955 to 1959	Frederick R. Francis		
1959 to 1963	Joseph B. Kearney and Terrace Investments Limited		
Entire Phase One Property			
1963 to 1983	Campeau Construction Company Limited		
1983 to 2019	561266 Ontario Inc.		
November 6, 2019 to Present	11684663 Canada Inc.		

Five leases were registered at the Phase One Property including:

- OT67899 Oct 20, 1965 Supertest Petroleum Corporation Limited
- CT188136 Feb 21, 1974 B. P. Oil Products Limited
- CT209810 May 15, 1975 The Bank of Nova Scotia
- N297929 Jul 31, 1985 Petro-Canada Products Inc.
- N319366 0 Dec 20, 1985 The Bank of Nova Scotia

Based on the chain of title ownership summary there was one Potentially Contaminating Activity (PCA) known to be associated with the ownership of the Phase One Property; the presence of a retail fuel outlet (operated by a sequence of companies), with associated Gasoline and Associated Products Storage in Fixed Tanks. This PCA represents Area of Potential Environmental Concern #1 (APEC #1) on the southeast portion of the Phase One Property. Additional research in subsequent sections of this assessment further reviews and identifies the location of APEC #1 on the Phase One Property.

v. Environmental Reports

Brigil provided the following two reports for review as part of this Phase One ESA:

- "Phase II Environmental Site Assessment, 729 Ridgewood Avenue, Ottawa, Ontario", dated January 12, 2018, completed by Pinchin Ltd. for Canadian Rental Development Services Inc.
- 2. "Verification Soil Sampling Program, 729 Ridgewood Avenue, Ottawa, Ontario", dated October 19, 2018, completed by Pinchin Ltd. for 561226 Ontario Inc.

2018 Phase II Environmental Site Assessment by Pinchin (2018 Pinchin Phase II ESA)

The 2018 Pinchin Phase II Environmental Site Assessment (2018 Pinchin Phase II ESA) was completed to assess the APECs identified through a review of the 2005 Intera Phase II ESA, namely, the potential for residual soil and groundwater contamination in the vicinity of the former retail fuel outlet on the southeast portion of the Phase One Property following its decommissioning and underground storage tank (UST) and pump island removal.

Five boreholes, each instrumented with a groundwater monitoring well, were drilled on the former retail fuel outlet and automotive service garage portion of the Property as part of the 2018 Phase II ESA. Soil conditions were generally found to consist of a layer of sand and gravel fill, underlain by clayey silt, followed by silty sand till. Refusal on interpreted bedrock was encountered at depths ranging from 7.2 to 7.6 m below ground surface (m BGS). Groundwater was encountered at depths ranging from approximately 3.8 to 4.8 m BGS.

Soil and groundwater samples were submitted from each of the borehole/monitoring well locations, which were analyzed for petroleum hydrocarbons (PHCs), volatile organic compounds (VOCs) and polycyclic aromatic hydrocarbons (PAHs). All of the soil and groundwater samples had exceedances of the site condition standards, primarily with respect to PHCs and benzene, toluene, ethylbenzene and xylenes (BTEXs), which were associated with the storage and dispensing of fuel at the Phase One Property. Pinchin recommended a delineative investigation to determine the extents of the soil and groundwater contamination at the Property.

The 2018 Pinchin Phase II ESA report was completed as environmental due diligence of the Phase One Property prior to potential purchase by the Canadian Rental Development Services Inc. As part of the due diligence, Pinchin reviewed one previous environmental report for the Phase One Property:

3. "Phase II Environmental Site Assessment and UST Removal, 753 Ridgewood Avenue, Ottawa, Ontario" completed by INTERA Engineering Ltd. (Intera) for 561266 Ontario Inc. dated September 2005.

The aforementioned report was summarized by Pinchin, however it was not provided by Brigil as part of this Phase One ESA, so a summary of this report is based on information provided in the 2018 Pinchin Phase II ESA.

2005 Phase II Environmental Site Assessment by Intera (2005 Intera Phase II ESA)

This Phase II ESA was completed following the removal of five underground storage tanks (USTs) and associated pump islands for the former retail fuel outlet at the Phase One Property in approximately 2005. Based on the soil and groundwater analytical results from the 2005 Intera Phase II ESA and the introduction of new soil and groundwater standards by the MECP in 2011, Pinchin suspected that there was the potential for the soil and groundwater quality at the Phase One Property to exceed the current site condition standards. A Phase II ESA was subsequently

recommended to assess the soil and groundwater quality in the vicinity of the former retail fuel outlet.

2018 Versification Soil Sampling Program by Pinchin (2018 Pinchin VSSP)

The 2018 Pinchin Versification Soil Sampling Program (2018 Pinchin VSSP) was completed to address the residual soil and groundwater contamination in the vicinity of the former retail fuel outlet on the southeast portion of the Phase One Property following its decommissioning and UST/pump island removal.

An environmental remediation program, consisting of excavation and off-Site disposal of contaminated soil was completed in two separate excavation footprints on the southeast portion of the Property in 2018. A total of 4,078 tonnes of soil was excavated and removed from the Property as part of remediation work in 2018, while approximately 13,019 L of water were removed and transported off-Site from the excavation. Approximately 3,485 tonnes of clean granular fill was brought to the Property to backfill the excavation; five samples were analyzed from the backfill material and it was found to be in compliance with the site condition standards.

Following completion of the excavation work, Pinchin collected a total of 14 worst case soil samples from the south excavation (Excavation #1) and 19 worst case soil samples from the north excavation (Excavation #2). All of the confirmatory samples were in compliance with the site condition standards, with the exception of 5 samples collected from the south and east portions of Excavation #1. It was noted that these exceedances were present in locations which were not practical for remediation without specialized excavation procedures (i.e. shoring). Pinchin estimated that only a small volume of soil, estimated 5-10 m³ of contaminated soil remained in these areas. No further remediation work was recommended at that time and it was stated that residual contamination could be removed at the time of Site redevelopment. Pinchin recommended that passive remediation measures could be applied to address residual groundwater impacts at the Property.

The 2018 Pinchin VSSP reviewed and referenced a delineation study completed in 2018. No summary was provided for this study, however, the borehole and monitoring well locations completed as part of this study were inferred to have been presented on Figure 3 in the 2018 Pinchin VSSP. The delineation study, which was not provided by Brigil as part of this Phase One ESA was entitled:

4. "Supplemental Phase II Environmental Site Assessment, 753 Ridgewood Avenue, Ottawa, Ontario" dated March 2018, completed by Pinchin Ltd. for 561226 Ontario Inc.

The presence of a former retail fuel outlet at the Phase One Property was previously identified as APEC #1. The presence of residual soil contamination and unknown groundwater quality in 2018 in the vicinity of APEC #1 further reinforces that this APEC requires further investigation.

b) Environmental Source Information

A review of the readily available environmental source information records was completed as part of this Phase One ESA.

As part of environmental source information review, Environmental Risk Information Systems (ERIS) was also contracted to complete a search of their records of environmental data bases within 250 m of the Site. The pertinent search results to this Phase One ESA are presented in the following subsections. A copy of the ERIS database search is included as Appendix D.

National Pollutant Release Inventory

The National Pollutant Release Inventory (NPRI) is a database maintained by Environment and Climate Change Canada (ECCC). Reporting of releases of pollutants into the natural environment are reported annually by corporations and/or their representatives and posted for public record by ECCC. Presently, data is available and posted for the years 1994 through 2017. No records were identified within 250 m of the Phase One Property during a review of the posted NPRI data on the ECCC electronic website on June 18, 2020 and the results were confirmed through the subcontracted ERIS search, dated June 12, 2020.

Polychlorinated Biphenyl (PCB) Inventories

The MECP, formerly known as the Ministry of Environment and Energy, published the "Ontario Inventory of PCB Storage Sites". The inventory documented the company information, physical address, number of tonnes of liquid PCBs by region. No records were identified within 250 m of the Phase One Property during a review this document and the results were confirmed through the subcontracted ERIS search, dated June 12, 2020.

The ERIS search also reviewed the National PCB Inventory, which details in use PCB containing equipment in federal, provincial and private facilities; this database was last updated in 2008. No records were identified within 250 m of the Phase One Property during a review this database.

Environmental Instruments

Environmental Instruments, such as Environmental Compliance Approvals (ECAs), Certificates of Approval (CAs), Permits to Take Water (PTTWs), Risk Management Plans (RMPs), and Certificates of Property Use (CPUs) are maintained by the MECP on a property specific basis and can generally be obtained by submitting a Freedom of Information (FOI) request. If records exist, they can generally be obtained through the MECP through additional communications. The subcontracted ERIS search also confirms the filing of any such records associated with properties.

An FOI request was submitted to the MECP as part of this Phase One ESA; however, a response was not received in the timeframe permitted as part of this mandate; a copy of the FOI request is included as Appendix E. The ERIS search did not identify any records of environmental

instruments at the Phase One Property, however, three records of CAs were identified within 250 m of the Phase One Property. Two CAs were issued to Tamarack Developments Corp. at 2991 Riverside Drive, located approximately 160 m southwest of the Phase One Property, in October of 1990 for municipal water and municipal sewage. One CA was issued to The City of Ottawa at the intersection of Springland Drive and Hobson Road, located approximately 150 m northeast of the Phase One Property, in July of 1994 for municipal sewage. These activities associated with the aforementioned CAs are not PCAs and do not represent APECs for the Phase One Property.

Inventory of Coal Gasification Plants

The document "Inventory of Industrial Sites Producing or Using Coal Tar and Related Tars in Ontario, Volume II", produced by Intera Technologies Ltd. for the Ontario Ministry of the Environment, dated July 1988 was reviewed as part of this Phase One ESA. No records were identified within 250 m of the Phase One Property during a review of this document and the results were confirmed through the subcontracted ERIS search, dated June 12, 2020.

Environmental Records of Incidents, Orders, Offences, Spills, Discharges of Contaminants or Inspections maintained by the Ministry

Environmental records of incidents, orders, offences, spills, discharges of contaminants or inspections are maintained by the MECP on a property specific basis and can generally be obtained by submitting a Freedom of Information (FOI) request. If records exist, they can generally be obtained through the MECP through additional communications. The subcontracted ERIS search also confirms the filing of such records associated with properties.

An FOI request was submitted to the MECP as part of this Phase One ESA; however, a response was not received in the timeframe permitted as part of this mandate; a copy of the FOI request is included as Appendix E. The ERIS search did not identify any records of environmental records at the Phase One Property; however, two records of spills were identified within 250 m of the Phase One Property. A spill of hydraulic oil from an equipment hose break was recorded at 750 Ridgewood Avenue, approximately 20 m south of the Phase One Property, in June of 2018; the spill was reportedly contained and an environmental remediation contractor was engaged for the clean up; this spill is not a PCA as it was cleaned up immediately following its occurrence and therefore does not represent an APEC for the Phase One Property. A spill of heating oil was recorded at 2707 Springland Drive, approximately 160 m southeast of the Phase One Property, in January of 1992. The heating oil spill is associated with the PCA of "Gasoline and Associated Products Storage in Fixed Tanks" (PCA #5). Given the separation distance of this property with respect to the Phase One Property, this PCA #5 is not considered to represent an APEC for the Phase One Property and PEC for the Phase One Property.

Waste Management Records

Waste management records, including current and historical waste storage locations and waste generator and waste receiver information maintained pursuant to Regulation 347 of the Revised

Regulations of Ontario, 1990 (General — Waste Management) made under the Act, or its predecessors are maintained by the MECP on a property specific basis and can generally be obtained by submitting a Freedom of Information (FOI) request. If records exist, they can generally be obtained through the MECP through additional communications. The subcontracted ERIS search also confirms the filing of such records associated with properties.

An FOI request was submitted to the MECP as part of this Phase One ESA, however, a response was not received in the timeframe permitted as part of this mandate; a copy of the FOI request is included as Appendix E. The ERIS search identified five records of environmental waste generators at the Phase One Property; and two additional properties with records of waste generators were identified within 250 m of the Phase One Property.

561226 Ontario Inc., identified at the Phase One Property, was listed as a generator of Oil Skimmings and Sludges in 2005, 2016 and 2017. It is suspected that these waste registrations were associated with the former retail automotive service garage present on the southeast portion of the Phase One Property. The presence of an automotive service garage is associated with the PCA of "Storage, Maintenance, Fuelling and Repair of Equipment, Vehicles, and Material used to Maintain Transportation Systems" (PCA #2). This PCA #2, in conjunction with PCA #1 is considered to represent APEC #1/2 for the Phase One Property.

Rick McCloskey's Service, identified on the Phase One Property, was listed as a generator of light fuels in 2018. It is suspected that this waste registration is associated with waste generated at the active automotive service garage on the central portion of the Phase One Property. The presence of an operating automotive service garage is associated with the PCA of "Storage, Maintenance, Fuelling and Repair of Equipment, Vehicles, and Material used to Maintain Transportation Systems" (PCA #4). This PCA #4 is considered to represent APEC #4 for the Phase One Property.

Riverside Pharmacy, identified at the Phase One Property, was listed as a "Professional Organizations" in 2004, with no waste generator details provided. It is suspected that based on the nature of this business, waste generated may have included pharmaceutical waste, which is not a PCA and does not represent an APEC for the Phase One Property.

The City of Ottawa with a property at 2960 Riverside Drive, with property limits located approximately 110 m east of the Phase One Property, was listed as a generator of Oil Skimmings and Sludges and Light Fuels related to bulk liquids trucking in 2013, 2014, 2015, 2016, 2018 and 2019. It is suspected that the waste generated is related to the PCA of "Gasoline and Associated Products Storage in Fixed Tanks" PCA #6. It is suspected based on features at this property that PCA #6 is located approximately 200 m northwest of the Phase One Property. Given the separation distance of this property with respect to the Phase One Property and the inferred down- or cross-gradient orientation, this PCA #6 is not considered to represent an APEC for the Phase One Property.

St. Patrick's Home of Ottawa and Medical Arts Dispensary of Ottawa with a property at 2865 Riverside Drive, with property limits located approximately 10 m north of the Phase One Property was listed as a generator of pathological wastes and pharmaceuticals related to nursing/health care from 1992 through 2001 and 2015, 2016, 2018 and 2019. It is suspected that based on the nature of this facility, waste generated is not a PCA and does not represent an APEC for the Phase One Property. PCB waste was also registered at this facility in 2014, however, given the nature of the facility, it is suspected that this waste class is associated with decommissioning and replacement of older equipment, is not a PCA and does not represent an APEC for the Phase One Property. St. Patrick's Home of Ottawa Inc. was listed as a generator of Oil Skimmings and Sludges in 2009. It is suspected that the waste generated is related to the PCA of "Gasoline and Associated Products Storage in Fixed Tanks". It is suspected based on features at this property that PCA #7 was located approximately 80 m north of the Phase One Property. Given the inferred down- or cross-gradient orientation, this PCA #7 is not considered to represent an APEC for the Phase One Property.

The locations of these PCAs are depicted on Figure 3: Surrounding Land Use and are summarized in Table 6 in Section 7. (b).

MECP Property Specific Reports

Reports submitted to the Ministry related to environmental conditions are maintained by the MECP on a property specific basis and can generally be obtained by submitting a Freedom of Information (FOI) request. If records exist, they can generally be obtained through the MECP through additional communications. The subcontracted ERIS search also confirms the filing of such records associated with properties.

An FOI request was submitted to the MECP as part of this Phase One ESA; however, a response was not received in the timeframe permitted as part of this mandate; a copy of the FOI request is included as Appendix E. The ERIS search did not identify any records of environmental reports at the Phase One Property, or properties within 250 m of the Phase One Property.

Technical Standards and Safety Authority

Records of retail fuel storage tanks, retail fuel outlets, spills, releases, and other associated information is maintained by the Technical Standards and Safety Authority (TSSA). These records can be obtained through electronic communications with the TSSA. The subcontracted ERIS search also confirms the filing of such records associated with properties.

The TSSA was contacted by email to complete a search of available records associated with the current property address, the known former property address of the former retail fuel outlet and addresses of surrounding properties with historical environmental listings (based on other historical research). The TSSA response, received on June 12, 2020, identified the presence of an expired (decommissioned) gasoline service station and three expired underground fuel storage

tanks at 753 Ridgewood Avenue (a former address associated with the southeast portion of the Phase One Property). A copy of the TSSA response is included as Appendix F.

The subcontracted ERIS search identified records of an expired private and retail fuel outlet, three underground fuel storage tanks and fuel distribution piping at 753 Ridgewood Avenue. The ERIS search results indicate that the fuel dispensing facility and associated features expired in 2002.

As previously noted, the former presence of a retail fuel outlet at the Phase One Property is a PCA of "Gasoline and Associated Products Storage in Fixed Tanks" (PCA #1) and represents APEC #1 for the southeast portion of the Phase One Property.

Registry Filings

Records of notices and instruments, including records of site condition (RSC), which have been posted in the environmental registry, are maintained by the MECP. These records can be reviewed electronically on the MECP Environmental Site Registry (ESR) website. The subcontracted ERIS search also confirms the filing of such records associated with properties. The website was reviewed for RSCs filed at the Phase One Property and in the Phase One Study Area; no RSCs have been filed for the Phase One Property or for any properties in the Phase One Study Area.

Areas of Natural and Scientific Interest

Records of areas of natural and scientific interest (ANSIs) formerly referred to as areas of natural significance, are maintained by the Ministry of Natural Resources and Forestry (MNRF), and are available for review on the Ontario GeoHub website. The website was reviewed on June 18, 2020 for records of ANSIs in the Phase One Study Area. There were no ANSIs identified within 250 m of the Phase One Property.

Current and Historical Landfills

Records of historical and operating landfills is maintained by the MECP. The document "Waste Disposal Site Inventory", produced by the Ontario Ministry of the Environment, dated June 1991 was reviewed as part of this Phase One ESA. No records were identified within 250 m of the Phase One Property during a review of this document.

The City of Ottawa contracted Golder Associates Ltd. to conduct an inventory and assessment of former waste disposal sites in within the City of Ottawa. The document "Old Landfill Management Strategy, Phase 1 – Identification of Sites, City of Ottawa, Ontario", produced by Golder Associates Ltd., finalized October 2004, was reviewed as part of this Phase One ESA. No records of active or former landfills were identified within 250 m of the Phase One Property during a review of this document.

City of Ottawa Historical Land Use Inventory

The City of Ottawa's Planning, Infrastructure and Economic Development department was contacted to complete a search of the Historical Land Use Inventory (HLUI) maintained by the City. The response, received on July 28, 2020, indicated that the HLUI search identified five activities (of environmental significance) associated with the Phase One Property, including:

- Riverside Supertest Gas Station was present in 1960, 1970 and 1980 city directory listings. This listing was previously identified as PCA/APEC # 1.
- Rick McCloskey's Service Limited, identified in 2001 and 2005. This listing was previously identified as PCA/APEC # 2.
- Edwards Upholstery, identified in 2001 this activity is not associated with a PCA.
- Fotomat was present in 1960, 1970 and 1980 city directory listings this activity is not associated with a PCA.
- Mooney's Bay Electronics Inc., identified in 2001 this activity is not associated with a PCA.

Additional activities were identified at properties in the HLUI study area; however, these activities were either located outside of the Phase One Study Area and/or are not associated with PCAs. None of the identified listed 'activities' at neighbouring properties are PCAs and no APECs were identified for the Phase One Property as part neighbouring land use during a review of the HLUI. A copy of the HLUI response letter is included in Appendix G.

- c) Physical Setting Sources
- i. Aerial Photographs

Aerial Photographs were reviewed for the Phase One Property and Phase One Study Area from available sources as part of the historical review. Aerial photographs were reviewed from historical research previously completed in the Phase One Study Area, Google Earth Aerial Imagery and from the City of Ottawa's geoOttawa GIS tool. Supplemental aerial photographs were ordered through (ERIS) and were reviewed. Aerial Photographs were reviewed over the period of 1933 through 2017, which depict development at the Phase One Property. A summary of the information gleaned from the aerial photographs is provided below. Copies of the aerial photographs reviewed are provided in Appendix H.

1933 Aerial Photograph

The Phase One Property appears to be undeveloped or used for agricultural purposes in the 1933 Aerial Photograph. What appears to be a gravel road (present day Ridgewood Avenue Right-of-Way) runs along the south limit of the Phase One Property. The Phase One Study Area appears to be used primarily for agricultural purposes, with some rural residential buildings present to the south and northwest of the Phase One Property. Riverside Drive has been constructed further west of the Phase One Property.

1956 Aerial Photograph

No significant changes appear to have been made to the Phase One Property. Some of the rural and/or agricultural buildings in the Phase One Study Area have been demolished and increased residential development is apparent along Riverside Drive, further southwest of the Phase One Property.

1965 Aerial Photograph

What appears to be a small (interpreted residential) building has been constructed on the central-south portion of the Phase One Property. No other significant changes appear to have been made at the Phase One Property. Ridgewood Avenue has been constructed to the south of the Phase One Property. Residential development, including single family residential dwellings and residential apartment buildings have been constructed to the northeast, southeast and east of the Phase One Property in the Phase One Study Area. Construction of Mooney's Bay beach and park appear to be on-going further west of the Phase One Property.

1976 Aerial Photograph

The Phase One Property has been developed with what appears to be the present-day commercial plaza buildings on the north portion of the Property. What appears to be a retail fuel outlet and automotive service garage has been constructed on the southeast portion of the Phase One Property. The majority of the undeveloped ground surface at the Property appears to have been surfaced with asphalt. The adjacent properties to the east and west of the Phase One Property have been developed with residential townhouses and a residential apartment building, respectively. A tennis court is present to the northeast of the Phase One Property. Increased residential development is apparent in the north portion of the Phase One Study Area.

1991 Aerial Photograph

The small, former (interpreted residential) building on the central-south portion of the Phase One Property has been demolished. The property to the south of Ridgewood Avenue, further south of the Phase One Property, has been developed with the present day institutional/commercial buildings. No other significant changes appear to have been made to the Phase One Property or neighbouring properties in the Phase One Study Area.

1999 Aerial Photograph

No significant changes appear to have been made to the Phase One Property or neighbouring properties in the Phase One Study Area.

2008 Aerial Photograph

No significant changes appear to have been made to the Phase One Property or neighbouring properties in the Phase One Study Area.

2017 Aerial Photograph

No significant changes appear to have been made to the Phase One Property or neighbouring properties in the Phase One Study Area.

As previously noted, the presence of a retail fuel outlet and automotive service garage at the Phase One Property are PCAs and represent APEC #1 / 2 for the southeast portion of the Phase One Property. The placement of fill material of unknown quality, associated with backfilling of the former inferred residential building's foundation at the Phase One Property also represents a PCA and APEC #2 for the central-south portion of the Phase One Property.

ii. Topography, Hydrology, Geology

The Ontario Ministry of Natural Resources and Forestry's (MNRF's) Topographic Map GIS website was used to produce a topographic map showing the location of the Phase One Property, nearby water bodies and the regional topography of the Phase One Study Area. A copy of the Topographic Map is provided in Appendix I. The regional topography in the Phase One Study Area is undulating but generally slopes downward to the west-northwest, toward the Rideau River. The topography on the south portion of the Phase One Property slopes downward from west to east, with the neighbouring property to the east at an elevation approximately 1.5 m lower than the southeast Property limits. A local topographical high is present approximately 200 m west of the Phase One Property, which may be associated with local bedrock undulation. The Rideau River is located approximately 550 m west of the Phase One Property.

Information on the regional surficial soil was obtained from the Geological Survey of Canada map 1425A titled Surficial Materials and Terrain features Ottawa Hull. Based on a review of the map, the natural soil conditions in the Phase One Study Area consist of "Abandoned River Channel Deposits of silt and silty clay; commonly including lenses of sand and generally underlain at variable depth by unit 3. 7. Stratified, buff, medium grained sand; unfossiliferous; locally reworked into low dunes".

Information on the regional bedrock was obtained from the Ontario Geological Survey Map P2716 titled 'Paleozoic Geology Ottawa Area'. Based on a review of the map, the Phase One Study Area is underlain by bedrock of the Bobcaygeon Formation, described as a "two member formation that can be distinguished with the lower member consisting of sublithographic to fine crystalline limestone, and the upper member of calcarenite with interbeds of sublithographic to fine crystalline limestone and shale".

Well records and borehole logs, obtained from the MECP Water Well Records database, the subcontracted ERIS search and from historical investigations at the Phase One Property were reviewed. Based on these records, the general stratigraphy of the Phase One Property and Phase One Study Area consists of sand and gravel fill, underlain by silty clay, followed by silty sand and gravel (till). The overburden soil is underlain by interbedded shale and limestone bedrock.

iii. Fill Materials

The Phase One Property was historically developed with what was interpreted to be a small residential building on the central-south portion of the Property; this building was subsequently demolished and was suspected to have had its foundation backfilled; this activity is a PCA associated with "Importation of Fill Material of Unknown Quality" PCA #3. The importation of fill to the Phase One Property represents APEC #3 on the central-south portion of the Phase One Property.

The Property was developed with the present-day commercial plaza buildings and paved asphalt parking areas. Granular base fill material is expected to have been used as part of construction of the aforementioned features; this fill type is not considered to represent a PCA, as gravel does not meet the definition of soil.

Various remediation work has been completed for the former retail fuel outlet and automotive service garage, which included excavation and off-site disposal of soil, which were subsequently backfilled with imported granular material; this granular fill was analyzed as part of the backfilling process and was determined to be in compliance with the site condition standards.

iv. Water Bodies and Areas of Natural Significance & Ground Water Information

The closest significant water body to the Phase One Property is the Rideau River, located approximately 550 m to the west. There were no areas of natural and scientific interest (ANSIs or areas of natural significance) identified in the Phase One Study Area.

The Phase One Property and Study Area are not located in the vicinity of any well-head protection areas or other designation identified by the City of Ottawa in its official plan for the protection of ground water. The Phase One Study Area is serviced by municipally treated drinking water. No private or agricultural water supply wells are located within the Phase One Study Area.

v. Well Records

Well records and borehole logs, obtained from the MECP Water Well Records database, the subcontracted ERIS search and from historical investigations at the Phase One Property were reviewed. Monitoring well clusters (a total of 12 groundwater monitoring wells) are located on the southeast portion of the Phase One Property, in the vicinity of the former retail fuel outlet. Based on these records, the general stratigraphy of the Phase One Property and Phase One Study Area consists of sand and gravel fill, underlain by silty clay, underlain by silty sand. The approximate depth to bedrock is expected to range from 6 to 8 m below ground surface (m BGS) in the area of the existing monitoring wells at the Phase One Property, with a groundwater table at approximately 4 to 5 m BGS.

Three historic potable water supply wells were identified in the Phase One Study Area during a review of the MECP Water Well Records database, however, these wells were drilled in the early

LOPERS & ASSOCIATES

1950s and were located at properties that have since been redeveloped. Additionally, the Phase One Study Area is provided with municipally treated non-potable water and as such it is not suspected that these wells remain in use.

d) Site Operating Records

Waste management, material orders and material safety data sheet records were available for Rick McCloskey's Automotive Service, which operates in the south unit of the east commercial plaza building. Since relocating to this commercial unit from the former automotive garage in 2018, this facility performs general repairs, oil changes, tire rotation/balancing. The presence of an operating automotive service garage, in a different location than the former automotive garage, is associated with the PCA of "Storage, Maintenance, Fuelling and Repair of Equipment, Vehicles, and Material used to Maintain Transportation Systems" (PCA #4). This PCA #4 is considered to represent APEC #4 for the Phase One Property.

Since it is known that there was a former retail fuel outlet and automotive service garage on the southeast portion of the Phase One Property Phase One Property and these are PCAs and are considered to represent APEC #1/2, the absence any such historical records from the former automotive service garage on the southeast portion of the Property is not expected to change the findings or the conclusions of this assessment.

5. Interviews

An in-person interview was completed on the day of the Site Investigation (June 19, 2020) with Mr. Philip Thibert, Project Manager – Land Development and Infrastructure for Brigil Construction. Mr. Thibert has been familiar with the Phase One Property since 2019 when Brigil purchased the Property. Mr. Thibert stated that Brigil purchased the property following the decommissioning of the former retail fuel outlet and automotive service garage on the southeast portion of the Property and the subsequent completion of an environmental remediation program by others in 2018. Mr. Thibert stated that the former tenants of the automotive service garage on the southeast portion of the southeast portion of the Property relocated to the south unit of the south commercial building following decommissioning and demolition of the former garage building circa 2018. Mr. Thibert was not aware of any spills or poor environmental management practices associated with the current operation of the automotive service garage or any of the other tenants at the Phase One Property. Mr. Thibert stated that to the best of his knowledge, no dry-cleaning equipment has ever been operated at the Phase One Property. Mr. Thibert was not aware of any former building present on the central-south portion of the Phase One Property and was not aware of the year of demolition or backfilling procedures.

Mr. Rick McCloskey, manager of the active automotive service garage, was interviewed in person by Mr. Philip Thibert of Brigil on July 17, 2020. Mr. McCloskey has been familiar with the Phase

One Property for at least 40 years. Mr. McCloskey stated that the garage relocated to the south commercial unit following demolition of the former service garage building on the southeast portion of the Property circa 2018. Mr. McCloskey stated that the garage completes regular maintenance, repairs and oil changes. Mr. McCloskey stated that waste oil is stored in an aboveground storage tank inside the garage and that GFL Environmental Inc. currently handles removal of waste oil on an as needed basis. He stated that there are no underground tanks at this facility. Mr. McCloskey stated that there have not been any spills at the Property to his knowledge and the garage has good housekeeping practices including regular cleaning of the floors. Mr. McCloskey stated that no chlorinated solvents are used as part of operations. Mr. McCloskey stated that no dry cleaners have operated at the Phase One Property to his knowledge, however, he stated that a dry-cleaning drop off depot was located within the grocery store for a few years in the 1980's.

The interviews identified the presence of the former retail fuel outlet and automotive service garage in the southeast portion of the Phase One Property and the current operation of an automotive service garage on the central portion (south unit of south commercial building) of the Phase One Property. All of the aforementioned activities represent PCAs which are interpreted as APECs for the Phase One Property. The information gleaned through interviews is consistent other information sources reviewed as part of this Phase One ESA and information gleaned from the interviews is considered to be valid.

6. Site Reconnaissance

a) General Requirements

The Phase One Site Investigation was completed on June 19, 2020 between the hours of 10:00 AM and 1:00 PM. Weather conditions were sunny with an ambient air temperature of approximately 29 degrees Celsius. The Phase One Property was occupied by two commercial plaza style buildings at the time of the Site Investigation. The commercial plaza style buildings were partially occupied at the time of the Site Investigation. The Site Investigation was completed by Mr. Luke Lopers, who is a registered Professional Engineer (Environmental) in the province of Ontario and a Qualified Person (QP) for Environmental Site Assessments, and has been conducting Phase I/One Environmental Site Assessments and environmental reconnaissance since 2006. Mr. Lopers was accompanied by Mr. Philip Thibert, Project Manager – Land Development and Infrastructure for Brigil Construction.

Photographs were taken of the exterior of the Phase One Property, documenting any areas of potential environmental concern and areas of disturbed soils, including fill areas, and existing groundwater monitoring wells. A copy of the Photographic Log and written descriptions of the photos are provided in Appendix J.

b) Specific Observations at Phase One Property

The Phase One Property was developed with two multi-unit commercial buildings at the time of the Site Investigation.

The north commercial building is generally a single storey slab-on-grade building with a partial basement below the north portion of the building, and a partial second storey on the east portion of the building. The west commercial unit in the north commercial building, formerly a grocery store, has a partial mezzanine level. The exterior of this building is finished with brick, concrete block, or precast concrete panels, has a flat tar and gravel roof and steel or glass doors. One sump was identified in the northwest portion of this building. The sump extended approximately 1.2 m below the floor slab and was dry at the time of the Site Investigation; when active, this sump discharges to the municipal sanitary sewer system.

The south commercial building is a single storey slab-on-grade building. The exterior of this building is finished with brick, concrete block, or precast concrete panels, has a flat tar and gravel roof and steel or glass doors.

The north commercial building consists of six commercial units, while the south commercial building consists of four commercial units. The occupants of the buildings were observed as presented in Table 3 below. The occupant identifiers (A through J) in Table 3 below are depicted on Figure 2: Site Plan.

North Commercial Building		South Commercial Building	
А	Vacant (former grocery store)	G	Pharmasave
В	iCook Persian Cuisine	Н	Picco Accounting Limited
С	Vacant (former Yoga studio)	Ι	Feras Barber Shop
D	Riverside Mall Hair Design	J	Rick McCloskey Auto Service (Automotive
Е	Caiger Watson Insurance		service garage)
F	Vacant (former Doctor's office)		

Table 3: Building Occupants

One double-walled steel aboveground storage tank (AST) was present on the interior of the automotive service garage in the south commercial building. The AST was approximately 1,100 L in size, was used to store waste oil, was in good condition was not fixed in place, and was located above concrete floors which were free of visible cracks and staining. The exact age and dimensions of the AST could not be determined as there was mechanical equipment obscuring the specification tag (if present). No visual indications of the presence of underground storage tanks (USTs), such as vent and fill pipes or access hatches, were observed as part of the Site Investigation.

No potable water wells were observed at the Phase One Property during the Site Investigation. Two groundwater monitoring wells, associated with previous investigations by others at the Property were present on the southeast portion of the Phase One Property, in the vicinity of the former retail fuel outlet. The Phase One Property is provided with potable water by the City of Ottawa through two underground connections from Ridgewood Avenue to the south which connect to each of the commercial plaza buildings.

Underground utility corridors for sanitary and storm sewers, potable water, private electricity and natural gas lines lead to the commercial plaza buildings, generally from Ridgewood Avenue to the south. Underground electrical services are supplied to the commercial buildings through connections on the northwest portion of the Property.

The commercial plaza buildings are heated with natural gas fired furnaces and or heating, ventilating and air conditioning units. There were no details regarding former heating and cooling systems, including historical fuel sources for buildings at the Phase One Property, however, given the date of development of the Property, it is suspected that the current buildings have always been heated and cooled using natural gas or electricity. It is suspected that the former residential building, which was present on the central-south portion of the Property may have been historically heated using furnace oil.

There were no significant cracks on stains on the concrete or finished floors of the commercial plaza buildings. Some minor staining, associated with routine automotive service, was observed on the floor of the automotive service garage.

The commercial buildings are connected to the City of Ottawa municipal sanitary sewer system. There were no septic tanks or leaching beds observed at the Phase One Property as part of the Site Investigation. Given the approximate years (circa 1960s) of commercial development at the Phase One Property, and that paving of the entire undeveloped portion of the Property was completed, it is expected that any former sewage tank/bed (if present) associated with the inferred former residential building on the central-south portion of the property would have been removed during demolition/development.

Approximately 25% of the Phase One Property is developed with two commercial buildings, which the majority of the remainder of the Property is surfaced with asphalt. A small portion of the Property in the southeast corner (former retail fuel outlet remediation) is surfaced with granular fill.

There were no current or former railway lines, tracks or spurs identified at the Phase One Property or in the Phase One Study Area as part of the Site Investigation.

No surficial staining was observed on the asphalt or gravel covered surfaces of the Phase One Property during the Site Investigation. No stressed vegetation was observed.

The presence of an operating automotive service garage, in a different location as the former automotive garage, is associated with the PCA of "Storage, Maintenance, Fuelling and Repair of

Equipment, Vehicles, and Material used to Maintain Transportation Systems" (PCA #4). This PCA #4 is considered to represent APEC #4 for the Phase One Property.

i. Enhanced Investigation Property

The Phase One Property is currently operating as a garage and formerly had operated as a garage and as a bulk liquid dispensing facility in a different location. The Phase One Property is hence an enhanced investigation property.

Motor oil is used for automotive oil changes, and waste motor oil is generated in the process. No bulk storage of new motor oil or any other fluids were observed in the garage at the time of the Site Investigation; new motor oil is stored in 1L plastic containers provided by the manufacturer and is stored on shelves in the storage area of the automotive service garage. As noted above, waste oil is stored in an AST on the interior of the automotive garage unit.

The automotive garage unit was originally constructed as a retail commercial unit and was previously used as a bank, as such, there are no strip drains, sumps or oil/water separators present in this building. No spills have been reported at the automotive garage unit and none are known to have occurred through interviews with the Property owner and manager of the automotive service garage.

Three aboveground lifts are present in the automotive service garage, two of the lifts are hydraulic, while the third is electrically powered.

c) Land Use Observations of the Phase One Study Area

Properties in the Phase One Study Area were reviewed from publicly accessible Rights-of-Way as part of the Site Investigation on June 19, 2020. Uses of these lands were noted and any potential presence of PCAs was also assessed. Neighbouring land uses were recorded as follows:

North: Residential properties and a retirement residence.

East: Residential townhouses, followed by Springland Drive, followed by residential apartment buildings.

South: Ridgewood Avenue, followed by an institutional/commercial property (St. Elias Church and conference centre), followed by residential dwellings.

West: Residential apartment building, followed by Riverside Drive, followed by Parkland (Mooney's Bay Park).

Neighbouring land uses are shown on Figure 3: Surrounding Land Use. No indications of PCAs were observed during the review of land use in the Phase One Study Area. The current uses of the neighbouring properties are not considered to represent any APECs for the Phase One Property.

7. Review and Evaluation of Information

a) Current and Past Land Use

The current and past land use of the Phase One Property, dating back to the first developed use, is provided in Table 4 below.

Year	Name of Owner	Description of Property Use	Property Use	Other observations from historical sources
1904 - 1959	Individuals	Unknown	Agricultural or other use	Property owned by individuals. 1933 and 1956 aerial photographs show Property in undeveloped condition.
1959 - 1963	Individuals & Campeau Construction Company Limited	Interpreted to have been partially used for residential purposes	Residential Use	Title search indicates a land developer started purchasing portions of the Property in 1959. Aerial photograph from 1965 shows the likely presence of a residential building, based on shape and location, on the central-south portion of the Property.
1963 - 1983	Campeau Construction Company Limited	Interpreted to have been used for residential purposes with commercial development commencing in the 1960s	Commercial Use and Residential Use	Commercial lease registered at the Property in 1965. Aerial photograph from 1976 shows the presence of commercial plaza buildings on north portion of Property and continued presence of inferred residential building on the central-south portion of the Property.
1983 - 2019	561266 Ontario Inc.	Commercial plazas and automotive service garage present at the Property and retail fuel outlet present until 2002.	Commercial Use	Aerial photographs from 1991 through 2017 show the presence of commercial plazas and automotive service garage. TSSA records indicated retail fuel outlet was decommissioned in 2002.
November 2019- Present	11684663 Canada Inc.	Commercial plazas and automotive service garage (within plaza building) present at the Property	Commercial Use	Site Investigation confirmed continued commercial use of the Property with automotive service on-going.

Table 4: Current and Past Land Use

b) Potentially Contaminating Activity

Four Potentially Contaminating Activities were identified at the Phase One Property and are summarized in Table 5 below.

Table 5: Potentially Contaminating	Activities at the	Phase One	Property
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PCA Report Reference No.	Potentially Contaminating Activity	Location
1	Former retail fuel outlet (O.Reg. 153/04 PCA Item 28: Gasoline and Associated Products Storage in Fixed Tanks)	Southeast portion of the Phase One Property
2	Former automotive service garage (O.Reg. 153/04 PCA Item 52: Storage, Maintenance, Fuelling and Repair of Equipment, Vehicles, and Material used to Maintain Transportation Systems)	Southeast portion of the Phase One Property
3	Fill placement following demolition activities (O.Reg. 153/04 PCA Item 30: Importation of Fill Material of Unknown Quality)	Central-south portion of the Phase One Property
4	Active automotive service garage (O.Reg. 153/04 PCA Item 52: Storage, Maintenance, Fuelling and Repair of Equipment, Vehicles, and Material used to Maintain Transportation Systems)	Central portion of the Phase One Property (south unit in south commercial plaza building)

Additionally, three PCAs were identified at neighbouring properties in the Phase One Study Area and are summarized in Table 6 below.

Table 6: Potentially	Contaminating	Activities in	the Phase	One Study	Area
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PCA Report Reference No.	Potentially Contaminating Activity	Location
5	Suspected fuel (heating oil) storage tank, reported heating oil spill (O.Reg. 153/04 PCA Item 28: Gasoline and Associated Products Storage in Fixed Tanks)	2707 Springland Drive (Residential Dwelling), located approximately 160 m southeast
6	Suspected fuel storage tank(s), waste generator (O.Reg. 153/04 PCA Item 28: Gasoline and Associated Products Storage in Fixed Tanks)	2960 Riverside Drive (Parkland), located approximately 200 m northwest
7	Suspected former fuel storage tank(s), waste generator (O.Reg. 153/04 PCA Item 28: Gasoline and Associated Products Storage in Fixed Tanks)	2865 Riverside Drive (Residential property), located approximately 80 m north

The PCAs at neighbouring properties in the Phase One Study Area are located significant distances and/or at down- or cross-gradient orientations with respect to the Phase One Property and are not considered to represent APECs for the Phase One Property.

c) Areas of Potential Environmental Concern

Four PCAs identified are considered to represent APECs for the Phase One Property and are summarized in Table 7 below.

APEC Report Reference No.	Location of APEC on Phase One Property	PCA	Location of PCA (on-site or off-site)	Contaminants of Potential Concern	Media Potentially Impacted (Groundwater, soil and/or Sediment)
1	Southeast portion of the Phase One Property	PCA Item 28: Gasoline and Associated Products Storage in Fixed Tanks	On-site: associated with former retail fuel outlet	PHCs/BTEXs Metals	Soil Groundwater
2	Southeast portion of the Phase One Property	PCA Item 52: Storage, Maintenance, Fuelling and Repair of Equipment, Vehicles, and Material used to Maintain Transportation Systems	On-site: associated with fill placement following decommissioning, demolition and remediation activities	PHCs/VOCs PAHs Metals	Soil Groundwater
3	Central- south portion of Phase One Property	PCA Item 30: Importation of Fill Material of Unknown Quality	On-site: associated with fill placement following demolition activities	PHCs/BTEXs PAHs Metals	Soil Groundwater
4	Central portion of the Phase One Property (south unit of south commercial plaza building)	PCA Item 52: Storage, Maintenance, Fuelling and Repair of Equipment, Vehicles, and Material used to Maintain Transportation Systems	On-site: associated with a current active automotive garage	PHCs/BTEXs	Soil Groundwater

Table 7: Areas of Potential Environmental Concern

The presence of a former retail fuel outlet and automotive service garage on the southeast portion of the Phase One Property are a significant PCAs which represents APECs for the Property. Given that reports were provided which document remnant PHC/BTEXs soil contamination and that groundwater quality was not confirmed following the completion a remediation program, further investigation is warranted. The contaminants of potential concern
associated with retail fuelling are generally PHCs and BTEXs, with older facilities also having concerns associated with metals, as lead was historically present in gasoline. Based on historical soil analysis in this area of the Property, PAH and VOCs are also considered contaminants of potential concern associated with the former automotive garage operations.

The practice of backfilling following demolition activities at the Phase One Property is a significant PCA which represents an APEC for the Property. Given that no reports were provided with analytical data to support the environmental quality of the backfill used to fill the interpreted former residential building footprint on the central-south portion of the Property, this area warrants further investigation. The contaminants of potential concern commonly found in poor environmental quality backfill are PHCs/BTEXs, PAHs and metals.

The presence of an active automotive service garage was observed on the central portion of the Phase One Property at the time of the Site Investigation. Although this garage has only been operating for a short time period (2018 to present), these operations are a PCA which represents an APEC for the Property. Based on the observations at this automotive garage, the contaminants of potential concern are considered to be PHCs and BTEXs.

Given that PCAs, interpreted as APECs were identified at the Phase One Property, the uncertainty or absence of information obtained in each of the components of the Phase One ESA is not considered to affect the conclusions.

d) Phase One Conceptual Site Model

Three Figures are provided to visually depict the Conceptual Site Model. Figure 1: Key Plan shows the location of the Phase One Property within the City of Ottawa. Figure 2: Site Plan depicts the current and former structures and environmentally significant features at the Phase One Property; this figure is provided with an overlay of the 2017 aerial imagery, which depicts the current general conditions of the Phase One Property. Figure 3: Surrounding Land Use shows the current uses of properties in the Phase One Study Area, location of PCAs and the location of APECs.

The Phase One Property is located at Civic No. 729 Ridgewood Avenue, Ottawa, Ontario and has an approximate area of 1.32 Hectares.

The Phase One Property was undeveloped prior to the late 1950's when a single residential building appears to have been was constructed on the central-south portion of the Phase One Property. Initial commercial development began circa 1965. The central and north portions of the Phase One Property have been occupied by two commercial plaza style buildings from circa 1965 to present. The southeast portion of the Phase One Property was formerly occupied by a retail fuel outlet and automotive service garage from 1965 to 2002 (retail fuel outlet) and the central portion of the Phase One Property has had an automotive service garage since 2018. Demolition of the former inferred residential building occurred between 1965 and 1991. The

LOPERS & ASSOCIATES

remaining undeveloped areas of the Phase One Property are paved with asphalt and used for access or parking.

The Property is currently used for commercial purposes and is zoned for mixed use. 11684663 Canada Inc. (Brigil) purchased the Phase One Property in November of 2019, and it is understood that the intended future use is for residential purposes, with commercial use on the ground floor. The Phase One Property is immediately surrounded by a municipal Right-of-Way to the south followed by a mixed institutional/commercial property and by residential properties to the north, east and west.

The Phase One Study Area includes the Phase One Property and properties with the boundaries within 250 m of the Phase One Property limits. Based on a review of the Phase One Property and properties in the Phase One Study Area, their associated historical and/or current uses and operations and physical characteristics of the Phase One Study Area, it was determined that an assessment of properties within 250 m of the Phase One property was sufficient to meet the objectives of the scope of this investigation for a Phase One ESA.

No water bodies or areas of natural significance are located at the Phase One Property or in the Phase One Study Area. No drinking water wells are located at the Phase One Property and the Phase One Study Area is serviced by municipally treated non-potable water. Two existing groundwater monitoring wells were present at the Phase One Property; the locations of these wells are presented on Figure 2.

The regional topography in the Phase One Study Area is undulating but generally slopes downward to the west-northwest, toward the Rideau River. The topography on the south portion of the Phase One Property slopes downward from west to east, with the neighbouring property to the east at an elevation approximately 1.2 m lower than the southeast Property limits. A local topographical high is present approximately 200 m west of the Phase One Property, which may be associated with local bedrock undulation. The Rideau River is located approximately 550 m west of the Phase One Property.

Based on the historical research, the general stratigraphy of the Phase One Property and Phase One Study Area consists of sand and gravel fill, underlain by silty clay, followed by silty sand and gravel (till). Overburden soils are expected to be up to 8 m thick and underlain by interbedded shale and limestone bedrock. Groundwater is expected at a depth of approximately 4 to 5 m BGS and flow in a predominantly northwest direction.

The presence of a former retail fuel outlet and automotive service garage on the southeast portion of the Phase One Property are a significant PCAs which represent APECs for the Property. Given that reports were provided which document remnant PHC/BTEXs soil contamination and that groundwater quality was not confirmed following the completion a remediation program, further investigation is warranted. The contaminants of potential concern associated with retail fuelling are generally PHCs and BTEXs and metals as this was an older facilities and lead was historically present in gasoline. Based on historical soil analysis in this area of the Property, PAH and VOCs are also considered contaminants of potential concern associated with the former automotive garage operations.

The practice of backfilling following demolition activities at the Phase One Property is a significant PCA which represents an APEC for the Property. Given that no reports were provided with analytical data to support the environmental quality of the backfill used to fill the former inferred residential building footprint on the central-south portion of the Property, this area warrants further investigation. The contaminants of potential concern commonly found in poor environmental quality backfill are PHCs/BTEXs, PAHs and metals.

The presence of an active automotive service garage was observed on the central portion of the Phase One Property at the time of the Site Investigation. Although this garage has only been operating for a short time period (2018 to present), these operations are a PCA which represents an APEC for the Property. Based on the observations at this automotive garage, that contaminants of potential concern are considered to be PHCs and BTEXs.

Three active and/or historical fuel storage tank locations at neighbouring properties in the Phase One Study Area constitute PCAs. The PCAs at neighbouring properties in the Phase One Study Area are located significant distances and/or at down- or cross-gradient orientations with respect to the Phase One Property and are not considered to represent APECs for the Phase One Property.

Underground utility service trenches are present at the Phase One Property. The underground utility corridors have the potential to affect contaminant distribution and transport, as they would create preferential pathways for lateral migration. It should be noted that the groundwater table is expected to be present approximately 4 to 5 m BGS, while the underground utilities are expected to be present at depths of 2 to 3 m BGS, therefore it is not suspected that significant migration of contaminants has occurred through underground utility corridors.

Given that APECs have been identified from several sources of information for the Phase One Property, any uncertainty or absence of information obtained in the components of this Phase One ESA are not expected to affect the validity of the conclusions or conceptual site model.

8. Conclusions

i. Whether Phase Two Environmental Site Assessment Required Before Record of Site Condition Submitted

The presence of a former retail fuel outlet and automotive service garage on the southeast portion of the Phase One Property are a significant PCAs which represent APECs for the Property. Given that reports were provided which document remnant PHC/BTEXs soil contamination and that groundwater quality was not confirmed following the completion a remediation program, further investigation is warranted. The contaminants of potential concern associated with retail fuelling are generally PHCs and BTEXs and metals as this was an older facility and lead was historically present in gasoline. Based on historical soil analysis in this area of the Property, PAH and VOCs are also considered contaminants of potential concern associated with the former automotive garage operations.

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The presence of an active automotive service garage was observed on the central portion of the Phase One Property at the time of the Site Investigation. Although this garage has only been operating for a short time period (2018 to present), these operations are a PCA which represents an APEC for the Property. Based on the observations at this automotive garage, that contaminants of potential concern are considered to be PHCs and BTEXs.

Based on the identification of APECs at the Phase One Property, it is recommended that a Phase Two Environmental Site Assessment be completed to assess the soil and/or groundwater quality in the vicinity of the APECs.

ii. Record of Site Condition Based on Phase One Environmental Site Assessment Alone

Given that there were APECs identified at the Phase One Property, a Phase Two Environmental Site Assessment is required before a record of site condition (RSC) may be submitted with respect to all or part of the Phase One Property.

iii. Signatures

The Qualified Person for this study is Mr. Luke Lopers, P. Eng. Mr. Lopers is a Professional Engineer registered in Ontario since 2012 and has been working on environmental site assessments since 2006. Mr. Lopers has been an author, project manager and/or peer reviewer for hundreds of Phase One ESAs and Phase Two ESAs as well as previously filed RSCs

The reviewer for this study is Mr. Don Plenderleith, P.Eng. Mr. Plenderleith is a Professional Engineer registered in Ontario since 1994 and has authored and/or reviewed hundreds of Phase One and Two ESAs in Ontario and the rest of Canada. The qualifications of the assessor/Qualified Person and reviewer are included in Appendix K.

Sincerely,

Luke Lopers, P.Eng., QP_{ESA}



Don Plenderletto

Don Plenderleith, P.Eng., QP_{ESA}

iv. Limitations

The findings and conclusions of this Phase One ESA are based on the information provided and/or reviewed as part of this study.

This Phase One ESA has been completed with the standard of care generally expected in the industry for a study of this nature.

This Phase One ESA has been prepared for the sole use of 11684663 Canada Inc. for the purposes of a due diligence assessment of the potential liabilities which may exist at the Phase One Property. No other party is permitted to rely on the conclusions or findings of this report without the written consent of Lopers & Associates and 11684663 Canada Inc..

There were no portions of the Phase One Property which were inaccessible, or components of this ESA where insufficient information was available to complete the interpretation.

Changes to the physical setting of the Phase One Property, Phase One Study Area and applicable regulations governing Phase One Environmental Site Assessments have the potential to influence the validity of the conclusions and opinions presented in this Phase One ESA.

9. References

Legal Survey Plan, Fairhall, Moffatt & Woodland Limited, dated January 8, 2018.

City of Ottawa, geoOttawa GIS mapping tool, Visited June through July, 2020. <u>http://maps.ottawa.ca/geoottawa/</u>

City of Ottawa, Development Applications website, Visited July 22, 2020. <u>http://ottwatch.ca/devapps?since=999</u>

Google Earth, Visited June through July, 2020.

Current Site Development Design Concept Plan, Brigil, 2020.

"Phase II Environmental Site Assessment, 729 Ridgewood Avenue, Ottawa, Ontario", dated January 12, 2018, completed by Pinchin Ltd. for Canadian Rental Development Services Inc.

"Verification Soil Sampling Program, 729 Ridgewood Avenue, Ottawa, Ontario", dated October 19, 2018, completed by Pinchin Ltd. for 561226 Ontario Inc.

National Pollutant Release Inventory – Environmental Climate Change Canada online website, visited June 18, 2020. <u>https://www.canada.ca/en/services/environment/pollution-waste-management/national-pollutant-release-inventory.html</u>

"Ontario Inventory of PCB Storage Sites", Ministry of Environment and Energy, dated January 1993.

"Inventory of Industrial Sites Producing or Using Coal Tar and Related Tars in Ontario, Volume II", produced by Intera Technologies Ltd. For the Ontario Ministry of the Environment, dated July 1988.

"Waste Disposal Site Inventory", produced by the Ontario Ministry of the Environment, dated June 1991.

"Inventory of Industrial Sites Producing or Using Coal Tar and Related Tars in Ontario, Volume II", produced by Intera Technologies Ltd. For the Ontario Ministry of the Environment, dated July 1988.

"Old Landfill Management Strategy, Phase 1 – Identification of Sites, City of Ottawa, Ontario", produced by Golder Associates Ltd., Dated October 2004.

Ministry of Environment, Conservation and Parks, Environmental Site Registry website, Visited June 18, 2020.

https://www.lrcsde.lrc.gov.on.ca/BFISWebPublic/pub/viewDetail?submissionId=226318

Ministry of Natural Resources and Forestry, Ontario GeoHub website, Visited June 18, 2020. <u>https://geohub.lio.gov.on.ca/datasets/b88037cdb71e4daf9445afa6fb999194_3?geometry=-</u> <u>75.706%2C45.443%2C-75.543%2C45.464</u>

Ministry of Natural Resources and Forestry, Make a Topographic Map website, Visited July 22, 2020.

https://www.gisapplication.lrc.gov.on.ca/matm/Index.html?site=Make A Topographic Map&vie wer=MATM&locale=en-US

Ministry of Environment, Conservation and Parks, Water Well Records database website, Visited July 2, 2020. <u>https://www.ontario.ca/environment-and-energy/map-well-records</u>

10. Appendices

- Appendix A Legal Survey Plan
- Appendix B Site Development Design Concept Plan
- Appendix C Chain of Title
- Appendix D Environmental Risk Information Systems (ERIS) database Search

Appendix E – Ministry of Environment, Conservation and Parks Freedom of Information (FOI) Request

- Appendix F Technical Standards and Safety Association Correspondence
- Appendix G City of Ottawa Historic Land Use Inventory (HLUI)
- Appendix H Aerial Photographs
- Appendix I Topographic Map
- Appendix J Photographic Log
- Appendix K Qualifications of Assessors

Figures







Appendix A

Legal Survey Plan



(SU) Vorth East x1 ^{8,17} Tod of Low Concrete East Edge of Wall 0.04 East	TOPOGRAPHIC SURVEY OF PART OF BLOCK 'C' REGISTERED PLAN 749, PART OF BLOCK 'C' REGISTERED PLAN 775 AND PART OF LOT 23, JUCTION GORE TOWNSHIP OF GLOUCESTER Now CITY OF OTTAWA SCALE 1: 250 0 5 10 20 25 metres FAIRHALL, MOFFATT & WOODLAND LIMITED
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	ONTARIO LAND SURVEYORS ELEVATION NOTES 1. ELEVATIONS SHOWN HEREON ARE REFERRED TO GEODETIC DATUM (GGVD28).
	2. ELEVATIONS FOR MANHOLE COVERS AND CATCH BASINS HAVE TO BE INDEPENDENTLY CONFIRMED BEFORE THEY CAN BE ACCEPTED FOR FINAL DESIGN OR CONSTRUCTION PURPOSES.
) • > -) •	3. IT IS THE RESPONSIBILITY OF THE USER OF THIS INFORMATION TO VERIFY THAT THE JOB BENCHMARKS HAVE NOT BEEN ALTERED OR DISTUREDE AND THAT THEIR RELATIVE LELVATION AND DESCRIPTION AGREE WITH THE INFORMATION SHOWN ON THIS DRAWING.
	<u>UTILITY NOTES</u> 1. THIS DRAWING CANNOT BE ACCEPTED AS ACKNOWLEDGING ALL UNDEROROUND UTILITES AND IT WILL BE THE RESPONSIBILITY OF THE USER TO CONTACT THE RESPECTIVE UTILITY AUTHORITIES FOR CONFIRMATION OR LOCATION.
) × 79,71 ;> ∩ 70 ° 70 ° 70 ° 70 ° 70	2. UNDERGROUND UTILITIES, AS REPORTED ON THIS DRAWING, ARE NOT BASED ON AN ACTUAL 'FIELD LOCATE' BY THE RESPECTIVE UTILITY AGENCIES BUT HAVE BEEN COMPILED FROM DATA OBTAINED FROM THE FOLLOWING SOURCE: 0) CITY OF OTTAWA PUBLIC UTILITIES REGISTRY
	 b) USL-1 UNDERGROUND SERVICE LOCATORS INC. BEFORE ANY WORK INVOLVING PROBING, EXCAVATING, ETC., A FIELD LOCATION OF UNDERGROUND PLANT BY THE PERTINENT UTILITY AUTHORITY IS MANDATORY.
	NOTES 1. BEARINGS ARE GRID, DERIVED FROM OBSERVED REFERENCE POINTS () AND() BY REAL TIME NETWORK OBSERVATIONS AND ARE REFERRED TO THE CENTRAL MERIDIAN, 76'30'W LONGITUDE MTM ZONE 9, (NADB3 ORIGINAL).
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	LECEND \square SURVEY MONUMENT SET \blacksquare SURVEY MONUMENT FOUND \blacksquare IRON BAR SIB STANDARD IRON BAR (P) OTTAWA-CARLETON CONDOMINUM PLAN N' 32 (P2) OTTAWA-CARLETON CONDOMINUM PLAN N' 32 (P3) REGISTERED PLAN 749 (S) SET (M) MEASURED (1982) R. A. DENIS, OLLS. (HJM) H. J. MARTIN, OLLS. (SU) SOURCE UNKNOWN PIN PROPERTY IDENTIFIER NUMBER SAN. SANTERY DIA. DIAMETER IIIOB CATCHBASIN © UP UTILITY POLE CMH MANHOI F.
C) F × 79.75 S S S S S S S S S S S S S	Image: Sign OFH FIRE HYDRANT QLS - LAMP STANDARD MWV - WATER VALVE Image: Sign - BOLLARDS Image: Sign - BOLLARDS Image: Sign - Declopoous TREE Image: Sign - OVERHEAD UTILITY WIRE
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× 30 m 80.59	
- Z80.60 C.B.	

LOPERS & ASSOCIATES

Appendix B

Current Proposed Design Concept Plan













Appendix C

Chain of Title



READ Abstracts Limited

331 Cooper Street, Suite 300, Ottawa, Ontario K2P 0A4 Email: search@readsearch.com Tel.: 613-236-0664 Fax: 613-236-3677

ENVIRONMENTAL SEARCH

Lopers & Associates Attn: Luke Lopers

BRIEF DESCRIPTION OF LAND:

729 Ridgewood Ave., Ottawa Part Block C, Plan 775; Part Block C, Plan 749; Part Lot 23, Concession JG Gloucester

PIN: 04071-0125

LAST REGISTERED OWNER: 11684663 Canada Inc.

CHAIN OF TITLE:

Part Lot 23, Con JG.

Deed GL18011 registered Sep 7, 1904 From Ed P. Gleason to Ellen Foran

Vesting Order GL23147 registered Dec 18, 1917 To Thomas C. Bate, Robert S. Low, and Edward McMahon carrying on business as Bate, McMahon and Co.

Deed GL30343 registered April 13, 1920 From Thomas C. Bate and Edward McMahon surviving partners of Bate, McMahon and Co. to Thomas C. Bate and Edward McMahon

Deed GL31223 registered Oct 4, 1921 From Ellen Foran to Thomas C. Bate and Edward McMahon

Deed GL33374 registered Sep 5, 1925 From Ellan Foran to Robert M. Graham and John H. Graham

Deed GL33595 registered Feb 2, 1926 From Ellen Foran to Edward Rutledge Deed GL33631 registered Mar 1, 1926 From Thomas C. Bate to Edward McMahon

Deed GL35202 registered Jan 3, 1930 From Edward Rutledge to John W. Dale

Deed GL37870 registered Jul 7, 1938 From estate of Robert M. Graham to John H. Graham

Deed GL40184 registered Sep 1943 From estate of Edward McMahon to Sidney Munro

Deed GL40430 registered Mar 9, 1944 From John H. Graham to Mary E. Coombs

Deed GL41671 registered Nov 21, 1945 From Sidney J. Munro to John W. Lucas and Jessie J. Lucas

Deed GL43757 registered Aug 16, 1947 From estate of John W. Dale to Robert and Yvonne Griffin

Deed GL46025 registered Apr 6, 1949 From John W. Lucas and Jessie J. Lucas to William Lucas

Foreclosure GL46822 registered Sep 7, 1949 From Robert and Yvonne Griffin to R. Walter Hamilton

Deed OT7066 registered Jul 3, 1952 From R. Walter Hamilton to James and Sarah Western

Deed OT9038 registered Jun 15, 1955 From James and Sarah Western to Frederick R. Francis

Deed OT31099 registered Jun 27, 1958 From estate of Sidney Munro to J. M. Patrick Kelly

Deed OT63114 registered Jun 4, 1959 From Mary E. Coombs to Norman H. Moody

Deed OT36513 registered Jun 30, 1959 From Frederick R. Francis to Joseph B. Kearney and Terrace Investments Limited

Deed OT37999 registered Oct 19, 1959 From Norman H. Moody to Campeau Construction Company Limited Deed OT38000 registered Oct 19, 1959 From J. M. Patrick Kelly to Campeau Construction Company Limited

Deed OT40311 registered Jun 22, 1960 From estate of Ellen Foran and Frederick R. Francis to Joseph B. Kearney and Terrace Investments Limited

Deed OT42828 registered Jan 16, 1961 From William Lucas to Campeau Construction Company Limited

Deed OR42834 registered Jan 17, 1961 From Jessie J. Lucas and estate of John W. Lucas to Campeau Construction Company Limited

Plan 749 registered Jan 18, 1961 By Campeau Construction Company Limited

Plan 775 registered Dec 19, 1962 By Joseph B. Kearney and Terrace Investments Limited

Part Block C, Plan 775

Deed OT54279 registered Jan 9, 1963 From Joseph B. Kearney and Terrace Investments Limited to Campeau Construction Company Limited

Part Block C, Plan 775 and Part Block C, Plan 749 and Part Lot 23, Con. JG.

Deed NS223177 registered Dec 16, 1983 From Campeau Corporation to 561266 Ontario Inc.

Deed OC2162469 registered Nov 6, 2019 From 561266 Ontario Inc. to 11684663 Canada Inc.

Leases

OT67899 – Oct 20, 1965 – Supertest Petroleum Corporation Limited CT188136 – Feb 21, 1974 – B. P. Oil Products Limited CT209810 – May 15, 1975 - The Bank of Nova Scotia N297929 – Jul 31, 1985 – Petro-Canada Products Inc. N319366 0 Dec 20, 1985 – The Bank of Nova Scotia LOPERS & ASSOCIATES

Appendix D

Environmental Risk Information Systems (ERIS) database Search



Project Property:

Project No: Report Type: Order No: Requested by: Date Completed: Phase One Environmental Site Assessment 729 Ridgewood Avenue Ottawa ON K1V 6M8 LOP20-002 Standard Report 20200610241 Lopers & Associates June 12, 2020

Environmental Risk Information Services A division of Glacier Media Inc. 1.866.517.5204 | info@erisinfo.com | erisinfo.com

Table of Contents

Table of Contents	2
Executive Summary	3
Executive Summary: Report Summary	4
Executive Summary: Site Report Summary - Project Property	6
Executive Summary: Site Report Summary - Surrounding Properties	10
Executive Summary: Summary By Data Source	13
Мар	21
Aerial	22
Topographic Map	23
Detail Report	24
Unplottable Summary	67
Unplottable Report	69
Appendix: Database Descriptions	76
Definitions	85

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Reliance on information in Report: This report DOES NOT replace a full Phase I Environmental Site Assessment but is solely intended to be used as a database review of environmental records.

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Executive Summary

Property Information:

Project Property:		Phase One Environmental Site Assessment 729 Ridgewood Avenue Ottawa ON K1V 6M8			
Project No:		LOP20-002			
Coordinates:					
	Latitude:	45.3681874			
	Longitude:	-75.6879375			
	UTM Northing:	5,024,082.83			
	UTM Easting:	446,128.39			
	UTM Zone:	18T			
Elevation:		268 FT			
		81.82 M			
Order Information	<u>.</u>				

Order No:
Date Requested:
Requested by:
Report Type:

20200610241 June 10, 2020 Lopers & Associates Standard Report

Historical/Products:

Aerial Photographs	Aerials - National Collection
City Directory Search	CD - Subject Site plus 250m Radius

Executive Summary: Report Summary

Database	Name	Searched	Project Property	Within 0.25 km	Total
AAGR	Abandoned Aggregate Inventory	Y	0	0	0
AGR	Aggregate Inventory	Y	0	0	0
AMIS	Abandoned Mine Information System	Y	0	0	0
ANDR	Anderson's Waste Disposal Sites	Y	0	0	0
AST	Aboveground Storage Tanks	Y	0	0	0
AUWR	Automobile Wrecking & Supplies	Y	0	0	0
BORE	Borehole	Y	1	0	1
СА	Certificates of Approval	Y	0	3	3
CDRY	Dry Cleaning Facilities	Y	0	0	0
CFOT	Commercial Fuel Oil Tanks	Y	0	0	0
CHEM	Chemical Register	Y	0	0	0
CNG	Compressed Natural Gas Stations	Y	0	0	0
COAL	Inventory of Coal Gasification Plants and Coal Tar Sites	Y	0	0	0
CONV	Compliance and Convictions	Y	0	0	0
CPU	Certificates of Property Use	Y	0	0	0
DRL	Drill Hole Database	Y	0	0	0
EASR	Environmental Activity and Sector Registry	Y	0	0	0
EBR	Environmental Registry	Y	0	0	0
ECA	Environmental Compliance Approval	Y	0	0	0
EEM	Environmental Effects Monitoring	Y	0	0	0
EHS	ERIS Historical Searches	Y	0	6	6
EIIS	Environmental Issues Inventory System	Y	0	0	0
EMHE	Emergency Management Historical Event	Y	0	0	0
EPAR	Environmental Penalty Annual Report	Y	0	0	0
EXP	List of Expired Fuels Safety Facilities	Y	11	0	11
FCON	Federal Convictions	Y	0	0	0
FCS	Contaminated Sites on Federal Land	Y	0	0	0
FOFT	Fisheries & Oceans Fuel Tanks	Y	0	0	0
FRST	Federal Identification Registry for Storage Tank Systems	Y	0	0	0
FST	Fuel Storage Tank	Y	0	0	0
FSTH	Fuel Storage Tank - Historic	Y	0	0	0
GEN	Ontario Regulation 347 Waste Generators Summary	Y	5	14	19
GHG	Greenhouse Gas Emissions from Large Facilities	Y	0	0	0
HINC	TSSA Historic Incidents	Y	0	0	0
IAFT	Indian & Northern Affairs Fuel Tanks	Y	0	0	0
INC	Fuel Oil Spills and Leaks	Y	0	0	0

Database	Name	Searched	Project Property	Within 0.25 km	Total
LIMO	Landfill Inventory Management Ontario	Y	0	0	0
MINE	Canadian Mine Locations	Y	0	0	0
MNR	Mineral Occurrences	Y	0	0	0
NATE	National Analysis of Trends in Emergencies System	Y	0	0	0
NCPL	(NATES) Non-Compliance Reports	Y	0	0	0
NDFT	National Defense & Canadian Forces Fuel Tanks	Y	0	0	0
NDSP	National Defense & Canadian Forces Spills	Y	0	0	0
NDWD	National Defence & Canadian Forces Waste Disposal	Y	0	0	0
NEBI	Sites National Energy Board Pipeline Incidents	Y	0	0	0
NEBP	National Energy Board Wells	Y	0	0	0
NEES	National Environmental Emergencies System (NEES)	Y	0	0	0
NPCB	National PCB Inventory	Y	0	0	0
NPRI	National Pollutant Release Inventory	Y	0	0	0
OGWE	Oil and Gas Wells	Y	0	0	0
OOGW	Ontario Oil and Gas Wells	Y	0	0	0
OPCB	Inventory of PCB Storage Sites	Y	0	0	0
ORD	Orders	Y	0	0	0
PAP	Canadian Pulp and Paper	Y	0	0	0
PCFT	Parks Canada Fuel Storage Tanks	Y	0	0	0
PES	Pesticide Register	Y	4	0	4
PINC	Pipeline Incidents	Y	0	0	0
PRT	Private and Retail Fuel Storage Tanks	Y	1	0	1
PTTW	Permit to Take Water	Y	0	0	0
REC	Ontario Regulation 347 Waste Receivers Summary	Y	0	0	0
RSC	Record of Site Condition	Y	0	0	0
RST	Retail Fuel Storage Tanks	Y	3	0	3
SCT	Scott's Manufacturing Directory	Y	0	1	1
SPL	Ontario Spills	Y	0	2	2
SRDS	Wastewater Discharger Registration Database	Y	0	0	0
TANK	Anderson's Storage Tanks	Y	0	0	0
TCFT	Transport Canada Fuel Storage Tanks	Y	0	0	0
VAR	Variances for Abandonment of Underground Storage Tanks	Y	0	0	0
WDS	Waste Disposal Sites - MOE CA Inventory	Y	0	0	0
WDSH	Waste Disposal Sites - MOE 1991 Historical Approval Inventory	Y	0	0	0
WWIS	Water Well Information System	Y	10	3	13
		Total:	35	29	64

Executive Summary: Site Report Summary - Project Property

1 PES THE COUNTRY GROCER INC. DY ANNO NATIVE MARE -0.0 -0.25 1 PES THE COUNTRY GROCER INC. DY ANNO NATIVE MARE -0.0 -0.25 1 PES THE COUNTRY GROCER INC. DY ANNO NATIVE MARE -0.0 -0.25 1 PES THE COUNTRY GROCER INC. DY ANNO NATIVE MARE -0.0 -0.25 1 PES THE COUNTRY GROCER INC. DY ANNO NATIVE MARE -0.0 -0.25 1 PES THE COUNTRY GROCER INC. DY ANNO NATIVE MARE -0.0 -0.25 1 GEN 561266 Ontario INC. 729 Ridgewood Ottawa ON K1VE MARE -0.0 -0.25 1 GEN 561266 Ontario INC. 729 Ridgewood Ottawa ON K1VE MARE -0.0 -0.25 1 GEN 561266 Ontario INC. 729 Ridgewood Ottawa ON K1VE MARE -0.0 -0.25 1 PES THE COUNTRY GROCER INC. OTTAWA ON K1VE MARE -0.0 -0.25 -0.0 2 BORE THE COUNTRY GROCER INC. OTTAWA ON K1VE MARE -0.0 -0.25 -0.0 2 WWIS THE COUNTRY GROCER INC. ON TAWA ON K1VE MARE -0.0 -0.25 -0.0 2	Мар Кеу	DB	Company/Site Name	Address	Dir/Dist (m)	Elev diff (m)	Page Number
1 PES THE COUNTRY GROCER INC. OA COUNTRY GROCER 729 RIDGEWOOD AVE OTTAWA ON KIV 6MB -0.0 -0.25 1 PES THE COUNTRY GROCER INC. AC COUNTRY GROCER 729 RIDGEWOOD AVE OTTAWA ON KIV 6MB -0.0 -0.25 1 GEN 561266 Ontario INC. 729 RIDGEWOOD AVE Ottawa ON KIV 6MB -0.0 -0.25 1 GEN 561266 Ontario INC. 729 RIDGEWOOD AVE Ottawa ON KIV 6MB -0.0 -0.25 1 GEN 561266 Ontario INC. 729 RIDGEWOOD AVE Ottawa ON KIV 6MB -0.0 -0.25 1 GEN 561266 Ontario INC. 729 RIDGEWOOD AVE Ottawa ON KIV 6MB -0.0 -0.25 1 GEN 561266 Ontario INC. 729 RIDGEWOOD AVE OTTAWA ON KIV 6MB -0.0 -0.25 2 BORE THE COUNTRY GROCER INC. OA ON NNW20.8 -0.71 3 BORE ON ON NNW20.8 -0.71 4 WWIS THE COUNTRY GROCER INC. ON ON ESE/08.7 -0.92 2 BORE OTTAWA ON OTTAWA ON ESE/08.7 -0.92 3 WWIS ON ON ON <t< td=""><td><u>1</u></td><td>PES</td><td>THE COUNTRY GROCER INC. O/A COUNTRY GROCER FRESHMART</td><td>729 RIDGEWOOD AVENUE OTTAWA ON K1V 6M8</td><td>-/0.0</td><td>-0.25</td><td><u>24</u></td></t<>	<u>1</u>	PES	THE COUNTRY GROCER INC. O/A COUNTRY GROCER FRESHMART	729 RIDGEWOOD AVENUE OTTAWA ON K1V 6M8	-/0.0	-0.25	<u>24</u>
1PESTHE COUNTRY GROCER INC. PRESHMART729 RIDGEWOOD AVE OTTAWA ON KTV 6MB-0.0-0.251GEN561266 Ontario INc.729 Ridgewood Ottawa ON KTV 6MB-0.0-0.251GEN661266 Ontario INc.729 Ridgewood Ottawa ON KTV 6MB-0.0-0.251GEN161266 Ontario INc.729 Ridgewood Ottawa ON KTV 6MB-0.0-0.251PESTHE COUNTRY GROCER INC. OVA COUNTRY GROCER729 RIDGEWOOD AVE OTTAWA ON KTV 6MB-0.0-0.252BOREONNIW/20.8-0.71-0.253WWISLECOUNTRY GROCER PRESHMARTONNIW/20.8-0.714WWISONOTTAWA ON WEI ID: 1555713ESE/38.7-0.92	<u>1</u>	PES	THE COUNTRY GROCER INC. O/A COUNTRY GROCER FRESHMART	729 RIDGEWOOD AVE OTTAWA ON K1V 6M8	-/0.0	-0.25	<u>24</u>
1 GEN 561266 Ontario INc. 729 Ridgewood Ottawa ON K1V 6M8 -0.0 -0.25 1 GEN 561266 Ontario INc. 729 Ridgewood Ottawa ON K1V 6M8 -/0.0 -0.25 1 PES THE COUNTRY GROCER INC. O/A COUNTRY GROCER INC. O/A COUNTRY GROCER INC. PRESHMART 700 -0.25 2 BORE ON NNW/20.8 -0.71 3 WWIS OTTAWA ON K1V 6M9 NNW/20.8 -0.72 4 WWIS ON ESE/38.7 -0.92	<u>1</u>	PES	THE COUNTRY GROCER INC. O/A COUNTRY GROCER FRESHMART	729 RIDGEWOOD AVE OTTAWA ON K1V 6M8	-/0.0	-0.25	<u>24</u>
1 GEN 561266 Ontario INc. 729 Ridgewood Ottawa ON K1V 6M8 -0.0 -0.25 1 PES THE COUNTRY GROCER INC. O/A COUNTRY GROCER INC. O/A COUNTRY GROCER PRESHMART 729 RIDGEWOOD AVE OTTAWA ON K1V6M8 -0.0 -0.25 2 BORE ON NNW/20.8 -0.71 3 WWIS OTTAWA ON K1V6M8 ESE/38.7 -0.92 4 WWIS ON ESE/33.73 -0.92	<u>1</u>	GEN	561266 Ontario INc.	729 Ridgewood Ottawa ON K1V 6M8	-/0.0	-0.25	<u>25</u>
1 PES THE COUNTRY GROCER INC. PRESHMART 729 RIDGEWOOD AVE OTTAWA ON K1V6M8 -0.0 -0.25 2 BORE ON NNW/20.8 -0.71 3 WWIS OTTAWA ON MUI D: 1535713 ESE/38.7 -0.92 4 WWIS ON ESE/45.3 -0.95	<u>1</u>	GEN	561266 Ontario INc.	729 Ridgewood Ottawa ON K1V 6M8	-/0.0	-0.25	<u>25</u>
2 BORE ON NNW/20.8 -0.71 3 WWIS OTTAWA ON Well ID: 1535713 ESE/38.7 -0.92 4 WWIS ON ESE/45.3 -0.95	<u>1</u>	PES	THE COUNTRY GROCER INC. O/A COUNTRY GROCER FRESHMART	729 RIDGEWOOD AVE OTTAWA ON K1V6M8	-/0.0	-0.25	<u>25</u>
3 WWIS OTTAWA ON ESE/38.7 -0.92 4 WWIS WWIS ESE/45.3 -0.95	2	BORE		ON	NNW/20.8	-0.71	<u>26</u>
4 WWIS ON ESE/45.3 -0.95	<u>3</u>	WWIS		OTTAWA ON Well ID: 1535713	ESE/38.7	-0.92	<u>27</u>
	<u>4</u>	WWIS		ON	ESE/45.3	-0.95	<u>29</u>

Map Key	DB	Company/Site Name	Address	Dir/Dist (m)	Elev diff (m)	Page Number
			Well ID: 7313098			
<u>5</u>	GEN	Riverside Pharmacy	737 Ridgewood Ave. Ottawa ON K1V 6M8	SSE/45.4	-0.64	<u>30</u>
<u>6</u>	WWIS		ON	ESE/47.3	-0.95	<u>30</u>
			Well ID: 7303717			
<u>Z</u>	WWIS		ON	SE/53.4	-0.95	<u>34</u>
			Well ID: 7313097			
<u>8</u>	WWIS		ON	ESE/54.2	-0.95	<u>34</u>
			Well ID: 7303718			
<u>9</u>	wwis		ON	E/54.7	-0.92	<u>38</u>
			Well ID: 7313096			
<u>9</u>	WWIS		ON	E/54.7	-0.92	<u>38</u>
			Well ID: 7313099			
<u>10</u>	PRT	RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON K1V 6M8	ESE/55.4	-0.95	<u>39</u>
10	PST	MCCLOSKEY'S RICK	753 RIDGEWOOD AVE	ESE/55 4	-0.95	
<u>10</u>	101	SERVICE LTD	OTTAWA ON K1V6M8	202,00.4	0.00	<u>39</u>
<u>10</u>	RST	MCCLOSKEY'S RICK SERVICE LTD	753 RIDGEWOOD AVE OTTAWA ON K1V 6M8	ESE/55.4	-0.95	<u>39</u>

Map Key	DB	Company/Site Name	Address	Dir/Dist (m)	Elev diff (m)	Page Number
<u>10</u>	RST	MCCLOSKEY'S RICK SERVICE LTD	OTTAWA ON K1V 6M8	ESE/55.4	-0.95	<u>39</u>
<u>10</u>	GEN	561266 ONT. INC.	753 RIDGEWOOD AVE OTTAWA ON K1V 6M8	ESE/55.4	-0.95	<u>40</u>
<u>10</u>	EXP	RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON K1V 6M8	ESE/55.4	-0.95	<u>40</u>
<u>10</u>	EXP	RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON	ESE/55.4	-0.95	<u>40</u>
<u>10</u>	EXP	RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON K1V 6M8	ESE/55.4	-0.95	<u>40</u>
<u>10</u>	EXP	RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON K1V 6M8	ESE/55.4	-0.95	<u>41</u>
<u>10</u>	EXP	RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON K1V 6M8	ESE/55.4	-0.95	<u>41</u>
<u>10</u>	ЕХР	RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON	ESE/55.4	-0.95	<u>41</u>
<u>10</u>	EXP	RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON	ESE/55.4	-0.95	<u>41</u>
<u>10</u>	EXP	RICK MCCLOSKEYS SERVICE	753 RIDGEWOOD AV OTTAWA ON	ESE/55.4	-0.95	<u>42</u>
				.		

Map Key	DB	Company/Site Name	Address	Dir/Dist (m)	Elev diff (m)	Page Number
<u>10</u>	EXP	RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON K1V 6M8	ESE/55.4	-0.95	42
<u>10</u>	EXP	RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON K1V 6M8	ESE/55.4	-0.95	<u>42</u>
<u>10</u>	EXP	RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON K1V 6M8	ESE/55.4	-0.95	<u>42</u>
<u>10</u>	GEN	McCloskey's Rick Service	753 Ridgewood Ave Ottawa ON K1V 6M8	ESE/55.4	-0.95	<u>43</u>
<u>11</u>	WWIS		ON Well ID: 7303720	E/57.6	-2.10	<u>43</u>
<u>12</u>	WWIS		ON <i>Well ID:</i> 7303719	ESE/59.3	-0.95	<u>46</u>
<u>13</u>	WWIS		ON <i>Well ID:</i> 7303721	ESE/64.0	-1.25	<u>48</u>

Executive Summary: Site Report Summary - Surrounding Properties

Map Key	DB	Company/Site Name	Address	Dir/Dist (m)	Elev Diff (m)	Page Number
<u>14</u>	SPL	Clean Water Works Inc.	Ottawa ON	ESE/87.2	-1.15	<u>51</u>
<u>15</u>	WWIS		ON <i>Well ID:</i> 1508797	W/100.6	2.60	<u>52</u>
<u>16</u>	EHS		753 Springland Drive Ottawa ON K1V 6L9	ENE/116.2	-3.95	<u>54</u>
<u>16</u>	EHS		753 Springland Drive Ottawa ON K1V 6L9	ENE/116.2	-3.95	<u>54</u>
<u>16</u>	EHS		753 A Springland Dr Ottawa ON K1V6L9	ENE/116.2	-3.95	<u>54</u>
<u>17</u>	SCT	ITALIAN TELEPHONE DIRECTORY	770 RIDGEWOOD AVE OTTAWA ON K1V 6M9	ESE/137.1	-1.95	<u>54</u>
<u>18</u>	WWIS		ON <i>Well ID:</i> 1507898	W/188.7	5.76	<u>55</u>
<u>19</u>	EHS		2887 Riverside Dr Ottawa ON K1V8N4	NW/192.9	1.00	<u>57</u>
<u>20</u>	WWIS		Ottawa ON <i>Well ID:</i> 7285490	WNW/197.7	2.36	<u>57</u>
<u>21</u>	CA	TAMARACK DEVELOPMENTS CORP RIVERSIDE	2991 RIVERSIDE DR/BAYPORT PRIV OTTAWA CITY ON K1V 8N6	SSW/200.2	1.25	<u>59</u>
<u>21</u>	CA	TAMARACK DEVELOPMENTS CORP RIVERSIDE	2991 RIVERSIDE DR/BAYPORT PRIV OTTAWA CITY ON K1V 8N6	SSW/200.2	1.25	<u>60</u>
<u>22</u>	CA	OTTAWA CITY	SPRINGLAND DR./HOBSON RD. OTTAWA CITY ON	ENE/234.0	-5.92	<u>60</u>
Мар Кеу	DB	Company/Site Name	Address	Dir/Dist (m)	Elev Diff (m)	Page Number
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<u>23</u>	SPL	PRIVATE RESIDENCE	2707 SPRINGLAND DRIVE FURNACE OIL TANK OTTAWA CITY ON K1V 6M2	ESE/246.4	-3.89	<u>60</u>
<u>24</u>	GEN	City of Ottawa	2960 Riverside Dr. ottawa ON	WNW/249.5	5.11	<u>61</u>
<u>24</u>	GEN	City of Ottawa	2960 Riverside Dr. ottawa ON K2G 6J8	WNW/249.5	5.11	<u>61</u>
<u>24</u>	GEN	City of Ottawa	2960 Riverside Dr. ottawa ON K2G 6J8	WNW/249.5	5.11	<u>62</u>
<u>24</u>	GEN	City of Ottawa	2960 Riverside Dr. ottawa ON K2G 6J8	WNW/249.5	5.11	<u>62</u>
<u>24</u>	GEN	City of Ottawa RPAM	2960 Riverside Dr. ottawa ON K2G 6J8	WNW/249.5	5.11	<u>62</u>
<u>24</u>	GEN	City of Ottawa RPAM	2960 Riverside Dr. ottawa ON K2G 6J8	WNW/249.5	5.11	<u>63</u>
<u>25</u>	GEN	ST. PATRICK'S HOME OF OTTAWA 34-692	2865 RIVERSIDE DRIVE OTTAWA ON K1V 8N5	NNW/249.6	0.09	<u>63</u>
<u>25</u>	GEN	ST. PATRICK'S HOME OF OTTAWA	2865 RIVERSIDE DRIVE OTTAWA ON K1V 8N5	NNW/249.6	0.09	<u>63</u>
<u>25</u>	EHS		2865 Riverside Drive Ottawa ON K1V 8N5	NNW/249.6	0.09	<u>63</u>
<u>25</u>	EHS		2865 Riverside Drive Ottawa ON K1V 8N5	NNW/249.6	0.09	<u>64</u>
<u>25</u>	GEN	St. Patrick's Home of Ottawa Inc.	2865 Riverside Dr. Ottawa ON K1V 8N5	NNW/249.6	0.09	<u>64</u>
<u>25</u>	GEN	Medical Arts Dispensary of Ottawa	2865 Riverside Drive Ottawa ON K1V 8N5	NNW/249.6	0.09	<u>64</u>

Мар Кеу	DB	Company/Site Name	Address	Dir/Dist (m)	Elev Diff (m)	Page Number
<u>25</u>	GEN	Medical Arts Dispensary of Ottawa	2865 Riverside Drive Ottawa ON K1V 8N5	NNW/249.6	0.09	<u>64</u>
<u>25</u>	GEN	St. Patrick's Home of Ottawa Inc.	2865 Riverside Dr. Ottawa ON K1V 8N5	NNW/249.6	0.09	<u>65</u>
<u>25</u>	GEN	Medical Arts Dispensary of Ottawa	2865 Riverside Drive Ottawa ON K1V 8N5	NNW/249.6	0.09	<u>65</u>
<u>25</u>	GEN	Medical Arts Dispensary of Ottawa	2865 Riverside Drive Ottawa ON K1V 8N5	NNW/249.6	0.09	<u>65</u>

Executive Summary: Summary By Data Source

BORE - Borehole

A search of the BORE database, dated 1875-Jul 2018 has found that there are 1 BORE site(s) within approximately 0.25 kilometers of the project property.

Lower Elevation	<u>Address</u>	Direction	<u>Distance (m)</u>	<u>Map Key</u>
	ON	NNW	20.83	<u>2</u>

CA - Certificates of Approval

A search of the CA database, dated 1985-Oct 30, 2011* has found that there are 3 CA site(s) within approximately 0.25 kilometers of the project property.

Equal/Higher Elevation	Address	Direction	<u>Distance (m)</u>	<u>Map Key</u>
TAMARACK DEVELOPMENTS CORP RIVERSIDE	2991 RIVERSIDE DR/BAYPORT PRIV OTTAWA CITY ON K1V 8N6	SSW	200.25	<u>21</u>
TAMARACK DEVELOPMENTS CORP RIVERSIDE	2991 RIVERSIDE DR/BAYPORT PRIV OTTAWA CITY ON K1V 8N6	SSW	200.25	<u>21</u>

Lower Elevation	Address	Direction	<u>Distance (m)</u>	<u>Map Key</u>
OTTAWA CITY	SPRINGLAND DR./HOBSON RD. OTTAWA CITY ON	ENE	233.98	<u>22</u>

EHS - ERIS Historical Searches

A search of the EHS database, dated 1999-Jan 31, 2020 has found that there are 6 EHS site(s) within approximately 0.25 kilometers of the project property.

Equal/Higher Elevation	Address	Direction	<u>Distance (m)</u>	<u>Map Key</u>
	2887 Riverside Dr Ottawa ON K1V8N4	NW	192.93	<u>19</u>
	2865 Riverside Drive Ottawa ON K1V 8N5	NNW	249.56	<u>25</u>

Equal/Higher Elevation	<u>Address</u>	Direction	<u>Distance (m)</u>	<u>Map Key</u>
	2865 Riverside Drive Ottawa ON K1V 8N5	NNW	249.56	<u>25</u>
Lower Elevation	Address 753 Springland Drive Ottawa ON K1V 6L9	Direction ENE	<u>Distance (m)</u> 116.17	<u>Map Key</u> <u>16</u>
	753 A Springland Dr Ottawa ON K1V6L9	ENE	116.17	<u>16</u>
	753 Springland Drive Ottawa ON K1V 6L9	ENE	116.17	<u>16</u>

EXP - List of Expired Fuels Safety Facilities

A search of the EXP database, dated Feb 28, 2017 has found that there are 11 EXP site(s) within approximately 0.25 kilometers of the project property.

Lower Elevation	Address	Direction	<u>Distance (m)</u>	<u>Map Key</u>
RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON K1V 6M8	ESE	55.43	<u>10</u>
RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON	ESE	55.43	<u>10</u>
RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON K1V 6M8	ESE	55.43	<u>10</u>
RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON	ESE	55.43	<u>10</u>
RICK MCCLOSKEYS SERVICE	753 RIDGEWOOD AV OTTAWA ON	ESE	55.43	<u>10</u>

RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON	ESE	55.43	<u>10</u>
RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON K1V 6M8	ESE	55.43	<u>10</u>
RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON K1V 6M8	ESE	55.43	<u>10</u>
RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON K1V 6M8	ESE	55.43	<u>10</u>
RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON K1V 6M8	ESE	55.43	<u>10</u>
RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON K1V 6M8	ESE	55.43	<u>10</u>

GEN - Ontario Regulation 347 Waste Generators Summary

A search of the GEN database, dated 1986-Jan 31, 2020 has found that there are 19 GEN site(s) within approximately 0.25 kilometers of the project property.

Equal/Higher Elevation	Address	Direction	<u>Distance (m)</u>	<u>Map Key</u>
City of Ottawa	2960 Riverside Dr. ottawa ON	WNW	249.51	<u>24</u>
City of Ottawa	2960 Riverside Dr. ottawa ON K2G 6J8	WNW	249.51	<u>24</u>
City of Ottawa	2960 Riverside Dr. ottawa ON K2G 6J8	WNW	249.51	<u>24</u>
City of Ottawa	2960 Riverside Dr. ottawa ON K2G 6J8	WNW	249.51	<u>24</u>
City of Ottawa RPAM	2960 Riverside Dr. ottawa ON K2G 6J8	WNW	249.51	<u>24</u>

Equal/Higher Elevation	Address	Direction	<u>Distance (m)</u>	<u>Map Key</u>
City of Ottawa RPAM	2960 Riverside Dr. ottawa ON K2G 6J8	WNW	249.51	<u>24</u>
ST. PATRICK'S HOME OF OTTAWA 34-692	2865 RIVERSIDE DRIVE OTTAWA ON K1V 8N5	NNW	249.56	<u>25</u>
ST. PATRICK'S HOME OF OTTAWA	2865 RIVERSIDE DRIVE OTTAWA ON K1V 8N5	NNW	249.56	<u>25</u>
St. Patrick's Home of Ottawa Inc.	2865 Riverside Dr. Ottawa ON K1V 8N5	NNW	249.56	<u>25</u>
Medical Arts Dispensary of Ottawa	2865 Riverside Drive Ottawa ON K1V 8N5	NNW	249.56	<u>25</u>
Medical Arts Dispensary of Ottawa	2865 Riverside Drive Ottawa ON K1V 8N5	NNW	249.56	<u>25</u>
St. Patrick's Home of Ottawa Inc.	2865 Riverside Dr. Ottawa ON K1V 8N5	NNW	249.56	<u>25</u>
Medical Arts Dispensary of Ottawa	2865 Riverside Drive Ottawa ON K1V 8N5	NNW	249.56	<u>25</u>
Medical Arts Dispensary of Ottawa	2865 Riverside Drive Ottawa ON K1V 8N5	NNW	249.56	<u>25</u>
Lower Elevation 561266 Ontario INc.	Address 729 Ridgewood Ottawa ON K1V 6M8	<u>Direction</u> -	Distance (m) 0.00	<u>Map Key</u> <u>1</u>
561266 Ontario INc.	729 Ridgewood Ottawa ON K1V 6M8	-	0.00	1

Order No: 20200610241

Riverside Pharmacy	737 Ridgewood Ave. Ottawa ON K1V 6M8	SSE	45.36	<u>5</u>
McCloskey's Rick Service	753 Ridgewood Ave Ottawa ON K1V 6M8	ESE	55.43	<u>10</u>
561266 ONT. INC.	753 RIDGEWOOD AVE OTTAWA ON K1V 6M8	ESE	55.43	<u>10</u>

PES - Pesticide Register

A search of the PES database, dated 1988 - Apr 2020 has found that there are 4 PES site(s) within approximately 0.25 kilometers of the project property.

Lower Elevation	Address	Direction	<u>Distance (m)</u>	<u>Map Key</u>
THE COUNTRY GROCER INC. O/A COUNTRY GROCER FRESHMART	729 RIDGEWOOD AVE OTTAWA ON K1V 6M8	-	0.00	<u>1</u>
THE COUNTRY GROCER INC. O/A COUNTRY GROCER FRESHMART	729 RIDGEWOOD AVENUE OTTAWA ON K1V 6M8	-	0.00	<u>1</u>
THE COUNTRY GROCER INC. O/A COUNTRY GROCER FRESHMART	729 RIDGEWOOD AVE OTTAWA ON K1V6M8	-	0.00	<u>1</u>
THE COUNTRY GROCER INC. O/A COUNTRY GROCER FRESHMART	729 RIDGEWOOD AVE OTTAWA ON K1V 6M8	-	0.00	<u>1</u>

PRT - Private and Retail Fuel Storage Tanks

A search of the PRT database, dated 1989-1996* has found that there are 1 PRT site(s) within approximately 0.25 kilometers of the project property.

Lower Elevation	Address	Direction	<u>Distance (m)</u>	<u>Map Key</u>
RICK MCCLOSKEYS SERVICE LTD	753 RIDGEWOOD AV OTTAWA ON K1V 6M8	ESE	55.43	<u>10</u>

<u>RST</u> - Retail Fuel Storage Tanks

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A search of the RST database, dated 1999-Jan 31, 2020 has found that there are 3 RST site(s) within approximately 0.25 kilometers of the project property.

Lower Elevation	Address	Direction	<u>Distance (m)</u>	<u>Map Key</u>
MCCLOSKEY'S RICK SERVICE LTD	753 RIDGEWOOD AVE OTTAWA ON K1V6M8	ESE	55.43	<u>10</u>
MCCLOSKEY'S RICK SERVICE LTD	753 RIDGEWOOD AVE OTTAWA ON K1V 6M8	ESE	55.43	<u>10</u>
MCCLOSKEY'S RICK SERVICE	OTTAWA ON K1V 6M8	ESE	55.43	<u>10</u>

<u>SCT</u> - Scott's Manufacturing Directory

A search of the SCT database, dated 1992-Mar 2011* has found that there are 1 SCT site(s) within approximately 0.25 kilometers of the project property.

Lower Elevation	Address	Direction	<u>Distance (m)</u>	<u>Map Key</u>	
ITALIAN TELEPHONE DIRECTORY	770 RIDGEWOOD AVE OTTAWA ON K1V 6M9	ESE	137.10	<u>17</u>	

SPL - Ontario Spills

A search of the SPL database, dated 1988-Nov 2019 has found that there are 2 SPL site(s) within approximately 0.25 kilometers of the project property.

Lower Elevation	<u>Address</u>	Direction	<u>Distance (m)</u>	<u>Map Key</u>
Clean Water Works Inc.	Ottawa ON	ESE	87.21	<u>14</u>
PRIVATE RESIDENCE	2707 SPRINGLAND DRIVE FURNACE OIL TANK OTTAWA CITY ON K1V 6M2	ESE	246.45	<u>23</u>

WWIS - Water Well Information System

A search of the WWIS database, dated Feb 28, 2019 has found that there are 13 WWIS site(s) within approximately 0.25 kilometers of the project property.

Equal/Higher Elevation	Address	Direction	<u>Distance (m)</u>	<u>Map Key</u>
	ON	W	100.64	<u>15</u>

Equal/Higher Elevation	Address Well ID: 1508797	<u>Direction</u>	<u>Distance (m)</u>	<u>Map Key</u>
	ON	W	188.67	<u>18</u>
	Well ID: 1507898			
	Ottawa ON	WNW	197.74	<u>20</u>
	Well ID: 7285490			

Lower Elevation	<u>Address</u>	Direction ESE	<u>Distance (m)</u> 38.67	<u>Map Key</u> 3
	OTTAWA ON			-
	Well ID: 1535713			
	ON	ESE	45.27	<u>4</u>
	Well ID: 7313098			
	ON	ESE	47.33	<u>6</u>
	Well ID: 7303717			
	ON	SE	53.37	<u>7</u>
	Well ID: 7313097			
	ON	ESE	54.23	<u>8</u>
	Well ID: 7303718			
	ON	E	54.66	<u>9</u>
	Well ID: 7313099			
	ON	E	54.66	<u>9</u>
	Well ID: 7313096			
	ON	E	57.63	<u>11</u>
	Well ID: 7303720			
	ON	ESE	59.26	<u>12</u>

Well ID: 7303719

	ESE	63.98	13
ON			

Well ID: 7303721



Source: © 2015 DMTI Spatial Inc.

© ERIS Information Limited Partnership



Aerial Year: 2019

0

Address: 729 Ridgewood Avenue, Ottawa, ON

m

250

Source: ESRI World Imagery

125

250

Order Number: 20200610241

1:10000 ES/Airbus DS, 45°22'30"N



Maxar, GeoEye, Earthstar Geographics, CNES// , AeroGRID, IGN, and the GIS User Community

© ERIS Information Limited Partnership



Topographic Map

Address: 729 Ridgewood Avenue, ON

Source: ESRI World Topographic Map



© ERIS Information Limited Partnership

Detail Report

Map Key	Numbe Record	r of 's	Direction/ Distance (m)	Elev/Diff (m)	Site	DB
<u>1</u>	1 of 6		-/0.0	81.6 / -0.25	THE COUNTRY GROCER INC. O/A COUNTRY GROCER FRESHMART 729 RIDGEWOOD AVENUE OTTAWA ON K1V 6M8	PES
Detail Licence Licence No: Status: Approval Dat Report Source Licence Type Licence Clas Licence Con Latitude: Longitude: Longitude: Lot: Concession: Region: District: County: Trade Name: PDF Link:	te: te: ce: e: e Code: s: trol:	23-01-1185 11853 Limited Ver 23 01 0	i3-0 ndor		Operator Box: Operator Class: Operator No: Operator Type: Oper Area Code: Oper Area Code: Oper Phone No: Operator Ext: Operator Lot: Oper Concession: Operator District: 2 Operator District: 2 Operator County: 15 Op Municipality: Post Office Box: MOE District: SWP Area Name:	
1	2 of 6		-/0.0	81.6 / -0.25	THE COUNTRY GROCER INC. O/A COUNTRY GROCER FRESHMART 729 RIDGEWOOD AVE OTTAWA ON K1V 6M8	PES
Detail Licence Licence No: Status: Approval Dat Report Sourd Licence Type Licence Cas Licence Cos Licence Cos Licence Cos Licence Cos Longitude: Longitude: Lot: Concession: Region: District: County: Trade Name: PDF Link:	te: te: ce: e: e Code: s: trol:	Limited Ver 23	ndor		Operator Box: Operator Class: Operator No: Operator Type: Oper Area Code: Oper Phone No: Operator Ext: Operator Lot: Operator Contession: Operator District: Operator District: Operator County: Op Municipality: Post Office Box: MOE District: SWP Area Name:	
<u>1</u>	3 of 6		-/0.0	81.6 / -0.25	THE COUNTRY GROCER INC. O/A COUNTRY GROCER FRESHMART 729 RIDGEWOOD AVE OTTAWA ON K1V 6M8	PES

Мар Кеу	Number Records	of	Direction/ Distance (m)	Elev/Diff (m)	Site		DB
Detail Licence Licence No: Status: Approval Date Report Source Licence Type Licence Type Licence Class Licence Conte Latitude: Longitude: Lot: Concession: Region: District: County: Trade Name: PDF Link:	e No: e: code: s: rol:	Vendor			Operator Box: Operator Class: Operator No: Operator Type: Oper Area Code: Oper Phone No: Operator Ext: Operator Lot: Operator Lot: Operator Region: Operator Region: Operator District: Operator County: Op Municipality: Post Office Box: MOE District: SWP Area Name:		
<u>1</u>	4 of 6		-/0.0	81.6 / -0.25	561266 Ontario INc. 729 Ridgewood Ottawa ON K1V 6M8		GEN
Generator No Status: Approval Yea Contam. Facili MHSW Facilit SIC Code: SIC Descriptio	rs: lity: iy: on:	ON3535704 2016 No 531310 R	4 REAL ESTATE PRO	DPERTY MANAGE	PO Box No: Country: Choice of Contact: Co Admin: Phone No Admin:	Canada CO_OFFICIAL Brenda M Kennedy 6132743742 Ext.223	
<u>Detail(s)</u>							
Waste Class: Waste Class D	Desc:	2 C	51 DIL SKIMMINGS &	SLUDGES			
<u>1</u>	5 of 6		-/0.0	81.6 / -0.25	561266 Ontario INc. 729 Ridgewood Ottawa ON K1V 6M8		GEN
Generator No. Status: Approval Yea. Contam. Facilit MHSW Facilit SIC Code: SIC Descriptio	e: lity: iy: on:	ON3535704 Registered As of Dec 2	4 2017		PO Box No: Country: Choice of Contact: Co Admin: Phone No Admin:	Canada	
<u>Detail(s)</u>							
Waste Class: Waste Class D	Desc:	2 V	51 L Vaste oils/sludges (petroleum based)			
<u>1</u>	6 of 6		-/0.0	81.6 / -0.25	THE COUNTRY GROC GROCER FRESHMAR 729 RIDGEWOOD AVE OTTAWA ON K1V6M8	ER INC. O/A COUNTRY T 3	PES
Detail Licence Licence No:	e No:	23-01-1185 11853	3-0		Operator Box: Operator Class:		
25	erisinfo.co	m Enviror	nmental Risk Info	rmation Services	3	Order No	: 20200610241

Map Key	Numbe Record	er of Is	Direction/ Distance (m)	Elev/Diff (m)	Site		DB
Status: Approval Da Report Sour Licence Typ Licence Clas Licence Con Latitude: Longitude: Lot: Concession. Region: District: County: Trade Name: PDF Link:	nte: ce: e Code: ss: htrol:	Legacy Lic Limited Ve 23 01 0	enses (Excluding ndor	TS)	Operator No: Operator Type: Oper Area Code: Oper Phone No: Operator Ext: Operator Lot: Operator Concession: Operator Region: Operator District: Operator County: Op Municipality: Post Office Box: MOE District: SWP Area Name:	613 7316883 4 2 15	
2	1 of 1		NNW/20.8	81.1 / -0.71	ON		BORE
Borehole ID:		612687			Inclin FLG:	No	

Bulenule ID.	012087	monn FLG.	INU
OGF ID:	215513993	SP Status:	Initial Entry
Status:		Surv Elev:	No
Туре:	Borehole	Piezometer:	No
Use:		Primary Name:	
Completion Date:		Municipality:	
Static Water Level:		Lot:	
Primary Water Use:		Township:	
Sec. Water Use:		Latitude DD:	45.368361
Total Depth m:	-999	Longitude DD:	-75.688038
Depth Ref:	Ground Surface	UTM Zone:	18
Depth Elev:		Easting:	446121
Drill Method:		Northing:	5024102
Orig Ground Elev m:	74.7	Location Accuracy:	
Elev Reliabil Note:		Accuracy:	Not Applicable
DEM Ground Elev m:	82.4		
Concession:			
Location D:			
Survey D:			
Comments:			

Borehole Geology Stratum

Geology Stratum ID: Top Depth: Bottom Depth: Material Color: Material 1: Material 2: Material 3: Material 4: Gsc Material Description	218392090 .6 .9 Gravel	Mat Consistency: Material Moisture: Material Texture: Non Geo Mat Type: Geologic Formation: Geologic Group: Geologic Period: Depositional Gen:
Stratum Description:	GRAVEL.	
Geology Stratum ID: Top Depth: Bottom Depth: Material Color: Material 1: Material 2: Material 3: Material 4: Gsc Material Description	218392091 .9 Bedrock	Mat Consistency: Material Moisture: Material Texture: Non Geo Mat Type: Geologic Formation: Geologic Group: Geologic Period: Depositional Gen:

Map Key Nu Re	umber o ecords	of	Direction/ Distance (m	Elev/Diff) (m)	Site		DB
Stratum Descripti	ion:		BEDROCK. 0350 records provided	093FEET.SAND. BI	EDROCK. ROCK. BEDROC have a truncated [Stratum De	:K. 00000 040 00000009 000 **Note: Man escription] field.	у
Geology Stratum Top Depth: Bottom Depth: Material Color: Material 1: Material 2: Material 3: Material 4: Gsc Material Desc Stratum Descripti	DID: cription: ion:	21839208 0 .6 Silt	39 SILT.		Mat Consistency: Material Moisture: Material Texture: Non Geo Mat Type: Geologic Formation: Geologic Group: Geologic Period: Depositional Gen:		
Source							
Source Type: Source Orig: Source Date: Confidence: Observatio: Source Name: Source Details: Confiden 1:		Data Surv Geologica 1956-197 M	vey al Survey of Cana 2 Urban Geology A File: OTTAWA2.t Logs are approxi	da utomated Informatic xt RecordID: 05195(mately correct. Lack	Source Appl: Source Iden: Scale or Res: Horizontal: Verticalda: In System (UGAIS) NTS_Sheet: 31G05B of information. Doubtful terr	Spatial/Tabular 1 Varies NAD27 Mean Average Sea Level minology.	
Source List Source Identifier Source Type: Source Date: Scale or Resoluti Source Name: Source Originatol	: ion: rs:	1 Data Surv 1956-197 Varies	vey 2 Urban Geology A Geological Surve	utomated Informatic y of Canada	Horizontal Datum: Vertical Datum: Projection Name: n System (UGAIS)	NAD27 Mean Average Sea Level Universal Transverse Mercator	
<u>3</u> 10	of 1		ESE/38.7	80.9 / -0.92	OTTAWA ON		wwis
Well ID: Construction Date Primary Water Us Sec. Water Use: Final Well Status Water Type: Casing Material: Audit No: Tag: Construction Method: Elevation (m): Elevation Reliabi Depth to Bedrocol Well Depth: Overburden/Bedi Pump Rate: Static Water Leve Flowing (Y/N): Flow Rate: Clear/Cloudy:	te: se: : : : k: rock: el:	1535713 Observati Z29930 A027929	on Wells		Data Entry Status: Data Src: Date Received: Selected Flag: Abandonment Rec: Contractor: Form Version: Owner: Street Name: County: Municipality: Site Info: Lot: Concession: Concession Name: Easting NAD83: Northing NAD83: Zone: UTM Reliability:	8/16/2005 Yes 7241 3 753 RIDGEWOOD DR OTTAWA-CARLETON OTTAWA CITY	
Bore Hole Informa Bore Hole ID:	<u>ation</u>	11316252	2		Elevation:	82.05619	

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Мар Кеу	Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site		DB
DP2BR: Spatial Status Code OB: Code OB Dese Open Hole: Cluster Kind: Date Complet Remarks: Elevrc Desc: Location Sour Improvement I Source Revisio Supplier Com	c: 0 c: Overburd ed: 7/15/2005 ce Date: Location Source: Location Method: on Comment: ment:	en 5		Elevrc: Zone: East83: North83: Org CS: UTMRC: UTMRC Desc: Location Method:	18 446164.1 5024068 G83a 4 margin of error : 30 m - 100 m wwr	
<u>Overburden ar</u> Materials Inter	nd Bedrock Ival					
Formation ID: Layer: Color: General Color: Mat1: Most Common Mat2: Other Material Mat3: Other Material Formation Top Formation Enc Formation Enc Formation ID: Layer: Color: General Color: Mat1:	: Material: s: Depth: Depth: Depth UOM: <u>nd Bedrock</u> <u>val</u>	932996993 3 2 GREY 05 CLAY 28 SAND 3 5 m 932996991 1 6 BROWN 28				
Most Common Mat2: Other Material Mat3: Other Material Formation Top Formation Enc Formation Enc Overburden au	n Material: s: o Depth: d Depth: d Depth UOM: nd Bedrock	SAND 11 GRAVEL 0 1.2 m				
Aterials Inter Materials Inter Color: General Color: Mat1: Most Common Mat2: Other Material Mat3: Other Material Formation Top	<u>val</u> Material: s: Depth:	932996992 2 GREY 05 CLAY 28 SAND 11 GRAVEL 1.2				

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Map Key	Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site		DB
Formation E Formation E	nd Depth: nd Depth UOM:	3 m				
<u>Method of Co Use</u>	onstruction & Well					
Method Cons Method Cons Method Cons Other Metho	struction ID: struction Code: struction: d Construction:	B Other Method				
Pipe Informa	<u>tion</u>					
Pipe ID: Casing No: Comment: Alt Name:		11331107 1				
Constructior	Record - Casing					
Casing ID: Layer: Material: Open Hole o Depth From: Depth To: Casing Diam Casing Diam Casing Dept	r Material: eter: eter UOM: h UOM:	930855621 1 5 PLASTIC 0 1.83 1.25 cm m				
Constructior	<u> Record - Screen</u>					
Screen ID: Layer: Slot: Screen Top I Screen End I Screen Mate Screen Diam Screen Diam	Depth: Depth: rial: h UOM: eter UOM: eter:	933414209 1 10 1.83 5 5 m cm 3.17				
Hole Diamete	<u>ər</u>					
Hole ID: Diameter: Depth From: Depth To: Hole Depth L Hole Diamete	IOM: er UOM:	11533805 8.89 0 3.96 m cm				
<u>4</u>	1 of 1	ESE/45.3	80.9 / -0.95	ON		WWIS
Well ID: Constructio Primary Wat Sec. Water (Final Well S Water Type: Casing Mate	731309 n Date: ter Use: Jse: tatus: tatus:	8		Data Entry Status: Data Src: Date Received: Selected Flag: Abandonment Rec: Contractor: Form Version:	Yes 6/19/2018 Yes 7241 7	
20	erisinfo.com Env	vironmental Risk Inf	ormation Service	es		Order No: 20200610241

Map Key	Number Records	of S	Direction/ Distance (m)	Elev/Diff (m)	Site		DB
Audit No: Tag: Construction		Z212322 A189924			Owner: Street Name: County:	OTTAWA-CARLETON	
Elevation (m) Elevation Rel Depth to Bed Well Depth: Overburden/E Pump Rate: Static Water I Flowing (Y/N) Flow Rate: Clear/Cloudy.	: iability: rock: Bedrock: Level:): :				Municipality: Site Info: Lot: Concession: Concession Name: Easting NAD83: Northing NAD83: Zone: UTM Reliability:	GLOUCESTER TOWNSHIP	
Bore Hole Info	ormation						
Bore Hole ID: DP2BR:		1007115287	7		Elevation: Elevrc:		
Spatial Status Code OB: Code OB Des Open Hole: Cluster Kind: Date Complet Remarks: Elevrc Desc:	s: sc: ted:	2/22/2018			Zone: East83: North83: Org CS: UTMRC: UTMRC Desc: Location Method:	18 446170 5024065 UTM83 4 margin of error : 30 m - 100 m wwr	
Location Sour	rce Date:						
Improvement Improvement Source Revisi Supplier Com	Location S Location N ion Comme ment:	ource: lethod: ent:					
<u>5</u>	1 of 1		SSE/45.4	81.2 / -0.64	Riverside Pharmacy 737 Ridgewood Ave. Ottawa ON K1V 6M8		GEN
Generator No):	ON7903814	1		PO Box No:		
Approval Yea Contam. Faci MHSW Facilit	nrs: lity: ty:	04			Choice of Contact: Co Admin: Phone No Admin:		
SIC Code: SIC Descriptio	on:	813920 Pi	rofessional Organi	zations			
<u>6</u>	1 of 1		ESE/47.3	80.9/-0.95	ON		wwis
Well ID: Construction Primary Wate Sec. Water Us Final Well Sta Water Type: Casing Mater Audit No: Tag: Construction Method: Elevation (m) Elevation Rel Depth to Bed Well Depth:	Date: rr Use: se: atus: ial: ial: iability: rock:	7303717 Test Hole Monitoring Observation Z212364 A182473	n Wells		Data Entry Status: Data Src: Date Received: Selected Flag: Abandonment Rec: Contractor: Form Version: Owner: Street Name: County: Municipality: Site Info: Lot: Concession:	1/19/2018 Yes 7241 7 749 RIDGEWOOD AVENUE OTTAWA-CARLETON OTTAWA CITY	

erisinfo.com | Environmental Risk Information Services

Order No: 20200610241

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Мар Кеу	Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site		DB
Overburden/I Pump Rate: Static Water I Flowing (Y/N, Flow Rate: Clear/Cloudy	Bedrock: Level:): :			Concession Name: Easting NAD83: Northing NAD83: Zone: UTM Reliability:		
Bore Hole Info	ormation					
Bore Hole ID: DP2BR: Spatial Status Code OB: Code OB Des Open Hole: Cluster Kind:	10069723 s: sc:	12		Elevation: Elevrc: Zone: East83: North83: Org CS: UTMBC:	18 446173 5024067 UTM83 4	
Cluster Kind: Date Comple Remarks: Elevrc Desc: Location Soul Improvement Improvement Source Revisi Supplier Com	ted: 11/27/201 rce Date: Location Source: Location Method: ion Comment: ment:	7		UTMRC: UTMRC Desc: Location Method:	4 margin of error : 30 m - 100 m wwr	
<u>Overburden a</u> <u>Materials Inte</u>	<u>nd Bedrock</u> rval					
Formation ID: Layer: Color: General Color Mat1: Most Common Mat2:	: n Material:	1007122157 1 6 BROWN 28 SAND 11 00 00051				
Other Materia Mat3: Other Materia Formation To Formation En Formation En	ls: p Depth: d Depth: d Depth UOM:	GRAVEL 0 5 m				
<u>Overburden a</u> Materials Inte	<u>nd Bedrock</u> rval					
Formation ID: Layer: Color: General Color Mat1: Most Commol Mat2: Other Materia Mat3: Other Materia Formation To	: n Material: ls: ls: p Depth:	1007122160 4 2 GREY 05 CLAY 66 DENSE				
Formation En Formation En <u>Overburden a</u> <u>Materials Inte</u>	d Depth: d Depth UOM: <u>nd Bedrock</u> rval	20 m				
31	erisinfo.com Enviro	nmental Risk Info	rmation Servic	es	Order No: 20200	0610241

M	lap Key	Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site	DB
Fo La Co Ge Ma Ma Ot Ma	ormation ID: over: olor: eneral Color at1: ost Common at2: ther Material at3: ther Material	: n Material: ls:	1007122161 5 2 GREY 05 CLAY 66 DENSE			
Fo Fo Fo	ormation Top ormation En ormation En	o Depth: d Depth: d Depth UOM:	20 25 m			
<u>Ov</u> Ma	verburden a aterials Inter	<u>nd Bedrock</u> rval				
Fo La Co Ge Ma Ot Fo Fo Fo	ormation ID: oper: olor: at1: ost Common at2: ther Material of Material ormation Top ormation Encormation Encormat	: n Material: ls: s: Depth: d Depth: d Depth:	1007122159 3 2 GREY 06 SILT 11 GRAVEL 10 15 m			
<u>Ov</u> <u>Ma</u>	verburden a aterials Inter	nd Bedrock rval				
Fo La Ge Ma Ma Ot Fo Fo Fo	ormation ID: oper: oper: at1: opst Common at2: ther Material at3: ther Material ormation Top ormation Endormation Endormation Endormation	: n Material: ls: ls: o Depth: d Depth: d Depth UOM:	1007122158 2 GREY 28 SAND 11 GRAVEL 5 10 m			
<u>Ar</u> Se	nular Space aling Recor	e/Abandonment ːd				
Pli La Pli Pli Pli	ug ID: nyer: ug From: ug To: ug Depth U(DM:	1007122171 3 14 25 m			
<u>Ar</u> Se	nular Space aling Recor	e/Abandonment ːd				
Pli La	ug ID: iyer:		1007122169 1			

	Мар Кеу	Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site	Ľ)B
•	Plua From:		0				
	Plug To:		1				
	Plug Depth U	ОМ:	m				
	<u>Annular Spac</u> <u>Sealing Recor</u>	<u>e/Abandonment</u> r <u>d</u>					
			1007122170				
	Flug ID. Laver:		2				
	Plua From:		1				
	Plug To:		14				
	Plug Depth U	ОМ:	m				
	<u>Method of Co</u> <u>Use</u>	nstruction & Well					
	Method Const	truction ID:					
	Method Const	truction Code:	D				
	Method Const	truction:	Direct Push				
	Other Method	Construction:					
		1					
	<u>Pipe informati</u>	<u>1011</u>					
	Pipe ID:		1007122156				
	Casing No:		0				
	Comment:						
	Alt Name:						
	Construction	<u>Record - Casing</u>					
	Casing ID [.]		1007122164				
	Laver:		1				
	Material:		5				
	Open Hole or	Material:	PLASTIC				
	Depth From:		0				
	Depth To:		15				
	Casing Diame	ter:	1.5				
	Casing Diame	eter UOM:	cm				
	Casing Depth		111				
	Onmatrustian	Deserved Company					
	CONSTRUCTION	Record - Screen					
	Screen ID:		1007122165				
	Layer:		1				
	Slot:		10				
	Screen Top D	epth:	15				
	Screen End D	epth:	25				
	Screen Materi	al:	5				
	Screen Depth		(II) CM				
	Screen Diame		19				
	Gueen Dialite		1.0				
	<u>Hole Diameter</u>	r					
			1007122462				
	noie ID: Diameter:		3 25				
	Denth From		0				
	Depth To:		25				
	Hole Depth U	OM:	m				
	Hole Diameter	r UOM:	cm				

Map Key	Numbe Record	r of s	Direction/ Distance (m)	Elev/Diff (m)	Site		DB
<u>7</u>	1 of 1		SE/53.4	80.9/ -0.95	ΟΝ		wwis
Well ID: Construction Primary Wate Sec. Water U Final Well St Water Type: Casing Mate Audit No: Tag: Construction Method: Elevation (m Elevation Re Depth to Beo Well Depth: Overburden/ Pump Rate: Static Water Flowing (Y/N Flow Rate: Clear/Cloudy	n Date: er Use: Ise: atus: rial: n liability: drock: /Bedrock: Level: I): /:	7313097 Z212323 A182692			Data Entry Status: Data Src: Date Received: Selected Flag: Abandonment Rec: Contractor: Form Version: Owner: Street Name: County: Municipality: Site Info: Lot: Concession: Concession: Concession Name: Easting NAD83: Northing NAD83: Zone: UTM Reliability:	Yes 6/19/2018 Yes 7241 7 OTTAWA-CARLETON GLOUCESTER TOWNSHIP	
Bore Hole Inf Bore Hole ID DP2BR: Spatial Statu Code OB: Code OB De Open Hole: Cluster Kind Date Comple Remarks: Elevrc Desc: Location Sou Improvement Source Revis Supplier Con	iormation : : : : : : : : : : : : :	100711528 2/22/2018 Source: Method: ient:	1		Elevation: Elevrc: Zone: East83: North83: Org CS: UTMRC: UTMRC Desc: Location Method:	18 446165 5024044 UTM83 4 margin of error : 30 m - 100 m wwr	
8	1 of 1		ESE/54.2	80.9 / -0.95	ON		wwis
Well ID: Construction Primary Wate Sec. Water U Final Well St Water Type: Casing Mate Audit No: Tag: Construction Tag: Construction Method: Elevation (m Elevation Re Depth to Beo Well Depth: Overburden/ Pump Rate:	n Date: er Use: Ise: atus: rial: n liability: drock: Bedrock:	7303718 Test Hole Monitoring Observation Z212360 A182474	n Wells		Data Entry Status: Data Src: Date Received: Selected Flag: Abandonment Rec: Contractor: Form Version: Owner: Street Name: County: Municipality: Site Info: Lot: Concession: Concession Name: Easting NAD83:	1/19/2018 Yes 7241 7 749 RIDGEWOOD AVENUE OTTAWA-CARLETON OTTAWA CITY	

Map Key	Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site		DE
Static Water	Level:			Northing NAD83:		
Flowing (Y/N)):			Zone:		
Flow Rate:				UTM Reliability:		
Clear/Cloudy						
Bore Hole Info	ormation					
Bore Hole ID:	: 100697	2315		Elevation:		
DFZDR. Snatial Status	e.			Zone:	18	
Code OB:	5.			East83:	446173	
Code OB Des	SC:			North83:	5024052	
Open Hole:				Org CS:	UTM83	
Cluster Kind:				UTMRC:	4	
Date Comple	ted: 11/27/2	017		UTMRC Desc:	margin of error : 30 m - 100 m	
Remarks:				Location Method:	wwr	
Location Sou	rce Date:					
Improvement	Location Source:					
Improvement	Location Method:					
Source Revisi	ion Comment:					
Supplier Com	iment:					
<u>Overburden a</u> Materials Inter	nd Bedrock rval					
Formation ID:		1007122198				
Layer:		1				
Color:		6				
General Color	r:	BROWN				
Mat1: Maat Commo	n Matarial					
Most Common Mat2.	n waterial:	SAND 85				
Other Materia	ls:	SOFT				
Mat3:						
Other Materia	ls:					
Formation To	p Depth:	0				
Formation En	d Depth:	5				
Formation En	a Depth UOW:	m				
<u>Overburden a</u> Materials Inte	nd Bedrock rval					
Formation ID:		1007122200				
Layer:		3				
Color:		2 CREV				
General Color Mat1.	r:	28				
Most Commo	n Material:	SAND				
Mat2:		11				
Other Materia	ls:	GRAVEL				
Mat3:						
Other Materia	ls: n Donth	10				
Formation 10	p Depth: d Denth:	10				
Formation En	d Depth UOM:	m				
. simulon Ell						
<u>Overburden a</u> <u>Materials Inte</u>	nd Bedrock rval					
Formation ID:		1007122202				
Layer:		5				
35	erisinfo.com Env	rironmental Risk Info	rmation Servic	es	Order No: 20200	0610241

Мар Кеу	Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site	DB
Color: General Color Mat1: Most Commo Mat2: Other Materia Mat3: Other Materia Formation To Formation En	r: n Material: ls: ls: p Depth: d Depth: d Depth UOM:	2 GREY 05 CLAY 66 DENSE 20 24 m			
<u>Overburden a</u> <u>Materials Inte</u>	<u>nd Bedrock</u> rval				
Formation ID: Layer: Color: General Color Mat1: Most Commo Mat2: Other Materia Mat3: Other Materia Formation To Formation En Formation En	r: n Material: ls: ls: p Depth: d Depth: d Depth: d Depth UOM:	1007122201 4 2 GREY 06 SILT 28 SAND 15 20 m			
<u>Overburden a</u> <u>Materials Inte</u>	<u>nd Bedrock</u> rval				
Formation ID: Layer: Color: General Color Mat1: Most Commo Mat2: Other Materia Mat3: Other Materia Formation To Formation En	r: n Material: ls: ls: p Depth: d Depth: d Depth UOM:	1007122199 2 GREY 28 SAND 11 GRAVEL 5 10 m			
<u>Annular Spac</u> <u>Sealing Reco</u>	e/Abandonment rd				
Plug ID: Layer: Plug From: Plug To: Plug Depth U	ОМ:	1007122211 2 1 14 m			
<u>Annular Spac</u> Sealing Reco	e/Abandonment_ rd				
Plug ID: Layer: Plug From: Plug To:		1007122212 3 14 24			

Order No: 20200610241

erisinfo.com | Environmental Risk Information Services

Map Key Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site	DB
Plug Depth UOM:	m			
<u>Annular Space/Abandonment</u> <u>Sealing Record</u>				
Plug ID: Layer: Plug From: Plug To: Plug Depth UOM:	1007122210 1 0 1 m			
<u>Method of Construction & Well</u> <u>Use</u>				
Method Construction ID: Method Construction Code: Method Construction: Other Method Construction:	D Direct Push			
Pipe Information				
Pipe ID: Casing No: Comment: Alt Name:	1007122197 0			
Construction Record - Casing				
Casing ID: Layer: Material: Open Hole or Material: Depth From: Depth To: Casing Diameter: Casing Diameter UOM: Casing Depth UOM:	1007122205 1 5 PLASTIC 0 14 1.5 cm m			
Construction Record - Screen				
Screen ID: Layer: Slot: Screen Top Depth: Screen End Depth: Screen Material: Screen Depth UOM: Screen Diameter UOM: Screen Diameter:	1007122206 1 10 14 24 5 m cm 1.9			
Hole Diameter				
Hole ID: Diameter: Depth From: Depth To: Hole Depth UOM: Hole Diameter UOM:	1007122203 3.25 0 24 m cm			

Мар Кеу	Number Record	r of s	Direction/ Distance (m)	Elev/Diff (m)	Site		DB
9	1 of 2		E/54.7	80.9 / -0.92	ON		wwis
Well ID: Construction Primary Wate Sec. Water U Final Well St Water Type: Casing Mate Audit No: Tag:	n Date: er Use: Ise: atus: rial:	7313096 Z212321			Data Entry Status: Data Src: Date Received: Selected Flag: Abandonment Rec: Contractor: Form Version: Owner: Street Name:	Yes 6/19/2018 Yes 7241 7	
Tag: Construction Method: Elevation (m Elevation Re Depth to Bec Well Depth: Overburden/ Pump Rate: Static Water Flowing (Y/N Flow Rate: Clear/Cloudy): liability: drock: Bedrock: Level: l): /:	A199391			Street Name: County: Municipality: Site Info: Lot: Concession: Concession Name: Easting NAD83: Northing NAD83: Zone: UTM Reliability:	OTTAWA-CARLETON GLOUCESTER TOWNSHIP	
Bore Hole Inf DP2BR: Spatial Statu Code OB: Code OB De: Open Hole: Cluster Kind Date Comple Remarks: Elevrc Desc: Location Sou Improvement Source Revis Supplier Com	iormation : : : : : : : : : : : : : : : : : : :	100711523 2/22/2018 Source: Method: ent:	3		Elevation: Elevrc: Zone: East83: North83: Org CS: UTMRC: UTMRC Desc: Location Method:	18 446181 5024068 UTM83 4 margin of error : 30 m - 100 m wwr	
<u>9</u>	2 of 2		E/54.7	80.9 / -0.92	ON		wwis
Well ID: Construction Primary Wate Sec. Water U Final Well St Water Type: Casing Mate Audit No: Tag: Construction Tag: Construction Method: Elevation (m Elevation Re Depth to Beo Well Depth: Overburden/ Pump Rate: Static Water	n Date: er Use: Ise: atus: rial: n liability: frock: Bedrock: Level:	7313099 Z212320 A189923			Data Entry Status: Data Src: Date Received: Selected Flag: Abandonment Rec: Contractor: Form Version: Owner: Street Name: County: Municipality: Site Info: Lot: Concession: Concession: Easting NAD83: Northing NAD83:	Yes 6/19/2018 Yes 7241 7 OTTAWA-CARLETON GLOUCESTER TOWNSHIP	

Map Key	Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site		DB
Flowing (Y/N) Flow Rate: Clear/Cloudy:	:			Zone: UTM Reliability:		
Bore Hole Info	ormation					
Bore Hole ID: DP2BR:	100	7115293		Elevation: Elevrc:		
Spatial Status Code OB: Code OB Des Open Hole: Cluster Kind: Date Complet Remarks: Elevrc Desc:	s: c: ted: 2/22	2/2018		Zone: East83: North83: Org CS: UTMRC: UTMRC Desc: Location Method:	18 446181 5024068 UTM83 4 margin of error : 30 m - 100 m wwr	
Improvement Improvement Source Revisi Supplier Com	Location Source Location Metho on Comment: ment:	re: od:				
<u>10</u>	1 of 17	ESE/55.4	80.9 / -0.95	RICK MCCLOSKEYS 753 RIDGEWOOD AV OTTAWA ON K1V 61	SERVICE LTD / M8	PRT
Location ID: Type: Expiry Date: Capacity (L): Licence #:		11073 retail 1995-06-30 55000 0048045001				
<u>10</u>	2 of 17	ESE/55.4	80.9/ -0.95	MCCLOSKEY'S RICK 753 RIDGEWOOD AV OTTAWA ON K1V6M	(SERVICE LTD /E 18	RST
Headcode: Headcode Des Phone: List Name: Description:	5C:	1186800 Service Stations-Ga 6137330322	usoline, Oil & Natu	ral Gas		
<u>10</u>	3 of 17	ESE/55.4	80.9 / -0.95	MCCLOSKEY'S RICK 753 RIDGEWOOD AV OTTAWA ON K1V 61	(SERVICE LTD /E M8	RST
Headcode: Headcode Des Phone: List Name: Description:	SC:	1186800 Service Stations-Ga 6137331749	usoline, Oil & Natu	ral Gas		
<u>10</u>	4 of 17	ESE/55.4	80.9 / -0.95	MCCLOSKEY'S RICK	SERVICE LTD	RST
Headcode: Headcode Des	sc:	1186800 Service Stations-Ga	soline, Oil & Natu	OTTAWA ON K1V 61 ral Gas	М8	
			<i></i>		0 1 00	

Мар Кеу	Number Records	of ;	Direction/ Distance (m)	Elev/Diff (m)	Site	DB
Phone: List Name: Description:			6137331749			
<u>10</u>	5 of 17		ESE/55.4	80.9 / -0.95	561266 ONT. INC. 753 RIDGEWOOD AVE OTTAWA ON K1V 6M8	GEN
Generator No.	:	ON8776	822		PO Box No: Country:	
Approval Yea Contam. Facili MHSW Facilit	rs: lity: y:	05			Choice of Contact: Co Admin: Phone No Admin:	
SIC Code: SIC Descriptio	n:	447 190	Other Gasoline Sta	tions		
<u>Detail(s)</u>						
Waste Class: Waste Class D	esc:		251 OIL SKIMMINGS &	SLUDGES		
<u>10</u>	6 of 17		ESE/55.4	80.9 / -0.95	RICK MCCLOSKEYS SERVICE LTD 753 RIDGEWOOD AV OTTAWA ON K1V 6M8	EXP
Instance No:			9673997			
Instance ID: Instance Type:	:		FS Facility			
Description: Status: TSSA Program Area: Maximum Hazard Rank:			EXPIRED			
Expired Date:			4/13/2002			
<u>10</u>	7 of 17		ESE/55.4	80.9 / -0.95	RICK MCCLOSKEYS SERVICE LTD 753 RIDGEWOOD AV OTTAWA ON	EXP
Instance No: Instance ID: Instance Type Description: Status: TSSA Progran Maximum Haz Facility Type: Expired Date:	: 1 Area: ard Rank:		10155239 12837 FS Facility FS Propane Cylr Ha EXPIRED	andling Facility		
<u>10</u>	8 of 17		ESE/55.4	80.9 / -0.95	RICK MCCLOSKEYS SERVICE LTD 753 RIDGEWOOD AV OTTAWA ON K1V 6M8	EXP
Instance No:			10906415			
Instance ID: Instance Type	:		FS Liquid Fuel Tan	k		
Description: Status:			EXPIRED			

Мар Кеу	Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site	DB
TSSA Program Maximum Haz Facility Type: Expired Date:	n Area: ard Rank:	4/13/2002			
<u>10</u>	9 of 17	ESE/55.4	80.9/ -0.95	RICK MCCLOSKEYS SERVICE LTD 753 RIDGEWOOD AV OTTAWA ON K1V 6M8	EXP
Instance No:		10906439			
Instance Type):	FS Liquid Fuel Tank	¢		
Status: TSSA Program Maximum Haz	n Area: ard Rank:	EXPIRED			
Facility Type: Expired Date:		4/13/2002			
<u>10</u>	10 of 17	ESE/55.4	80.9/ -0.95	RICK MCCLOSKEYS SERVICE LTD 753 RIDGEWOOD AV OTTAWA ON K1V 6M8	EXP
Instance No:		10906455			
Instance Type	n Area: ard Rank:	FS Liquid Fuel Tank	(
Description: Status: TSSA Program Maximum Haza		EXPIRED			
Facility Type: Expired Date:		4/13/2002			
<u>10</u>	11 of 17	ESE/55.4	80.9 / -0.95	RICK MCCLOSKEYS SERVICE LTD 753 RIDGEWOOD AV OTTAWA ON	EXP
Instance No: Instance ID: Instance Type Description: Status: TSSA Progran Maximum Haz Facility Type: Expired Date:	e: n Area: rard Rank:	10906431 51384 FS Piping FS Piping EXPIRED			
<u>10</u>	12 of 17	ESE/55.4	80.9 / -0.95	RICK MCCLOSKEYS SERVICE LTD 753 RIDGEWOOD AV OTTAWA ON	EXP
Instance No: Instance ID: Instance Type Description: Status: TSSA Progran Maximum Haz	e: n Area: rard Rank:	10906446 51538 FS Piping FS Piping EXPIRED			

Мар Кеу	Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site	DB
Facility Type: Expired Date:					
<u>10</u>	13 of 17	ESE/55.4	80.9/-0.95	RICK MCCLOSKEYS SERVICE LTD 753 RIDGEWOOD AV OTTAWA ON	EXP
Instance No: Instance ID: Instance Type Description: Status: TSSA Progran Maximum Haz Facility Type: Expired Date:	: n Area: ard Rank:	10906461 51967 FS Piping FS Piping EXPIRED			
<u>10</u>	14 of 17	ESE/55.4	80.9 / -0.95	RICK MCCLOSKEYS SERVICE LTD 753 RIDGEWOOD AV OTTAWA ON K1V 6M8	EXP
Instance No:		10906415			
Instance ID:		ES Liquid Eucl Took			
Description:	•	FS Gasoline Station	- Full Serve		
Status:	_	EXPIRED			
TSSA Progran	n Area: ard Rank:				
Facility Type:		FS Liquid Fuel Tank			
Expired Date:		4/13/2002			
<u>10</u>	15 of 17	ESE/55.4	80.9 / -0.95	RICK MCCLOSKEYS SERVICE LTD 753 RIDGEWOOD AV OTTAWA ON K1V 6M8	EXP
Instance No:		10906439			
Instance ID:		ES Liquid Euel Tank			
Description:		FS Gasoline Station	- Full Serve		
Status:		EXPIRED			
A A A A A A A A A A A A A A A A A A A	n Area: ard Rank:				
Facility Type:		FS Liquid Fuel Tank			
Expired Date:		4/13/2002			
<u>10</u>	16 of 17	ESE/55.4	80.9/-0.95	RICK MCCLOSKEYS SERVICE LTD 753 RIDGEWOOD AV OTTAWA ON K1V 6M8	EXP
Instance No:		10906455			
Instance ID:		FS Liquid Fuel Teek			
Description:	•	FS Gasoline Station	- Full Serve		
Status:		EXPIRED			
ISSA Progran Maximum Haz	n Area: ard Rank:				
Facility Type:		FS Liquid Fuel Tank			
Expired Date:		4/13/2002			

Мар Кеу	Number Records	r of s	Direction/ Distance (m)	Elev/Diff (m)	Site		DB
<u>10</u>	17 of 17		ESE/55.4	80.9 / -0.95	McCloskey's Rick Ser 753 Ridgewood Ave Ottawa ON K1V 6M8	vice	GEN
Generator No Status: Approval Yea Contam. Fac MHSW Facili SIC Code: SIC Descripti	o: ars: illity: ity: ion:	ON9824428 Registered As of Dec 2	3 2018		PO Box No: Country: Choice of Contact: Co Admin: Phone No Admin:	Canada	
<u>Detail(s)</u> Waste Class: Waste Class	Desc:	2 L	21 L ight fuels				
<u>11</u>	1 of 1		E/57.6	79.7/-2.10	ON		wwis
Well ID: Construction Primary Wate Sec. Water U Final Well St Water Type: Casing Mate Audit No: Tag: Construction Method: Elevation (m Elevation Re Depth to Beo Well Depth: Overburden/ Pump Rate: Static Water Flowing (Y/N Flow Rate: Clear/Cloudy	n Date: er Use: Jse: iatus: rial: n): liability: drock: /Bedrock: /Bedrock: Level: I): /:	7303720 Test Hole Monitoring Observation Z212363 A182475	n Wells		Data Entry Status: Data Src: Date Received: Selected Flag: Abandonment Rec: Contractor: Form Version: Owner: Street Name: County: Municipality: Site Info: Lot: Concession: Concession: Concession Name: Easting NAD83: Northing NAD83: Zone: UTM Reliability:	1/19/2018 Yes 7241 7 749 RIDGEWOOD ROAD OTTAWA-CARLETON OTTAWA CITY	
Bore Hole Inf	ormation						
Bore Hole ID DP2BR: Spatial Statu Code OB: Code OB De Open Hole: Cluster Kind Date Comple Remarks: Elevrc Desc: Location Sou Improvement): IS: SC: I: eted: Irce Date: t Location I t Location I	100697232 11/27/2017 Source: Method:	1		Elevation: Elevrc: Zone: East83: North83: Org CS: UTMRC: UTMRC Desc: Location Method:	18 446185 5024072 UTM83 4 margin of error : 30 m - 100 m wwr	
Open Hole: Cluster Kind Date Comple Remarks: Elevrc Desc: Location Sou Improvement Source Revis Supplier Con	l: ated: t Location S t Location I tion Comm nment:	11/27/2017 Source: Method: ent:			Org CS: UTMRC: UTMRC Desc: Location Method:	UTM83 4 margin of error : 30 m - 100 m wwr	

Мар Кеу	Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site	DE
Overburden Materials Inte	and Bedrock erval				
Formation ID Layer:	:	1007122245 2			
Color:		2			
General Cold	or:	GREY 05			
Most Commo	on Material:	CLAY			
Mat2:		06			
Other Materia	als:	SILT			
Mats: Other Materia	als				
Formation To	op Depth:	2.43			
Formation E	nd Depth:	4.57			
Formation E	nd Depth UOM:	m			
<u>Overburden a</u> <u>Materials Inte</u>	and Bedrock erval				
Formation ID	2	1007122246			
Layer:		3			
General Colo	r.	2 GREY			
Mat1:		06			
Most Commo	on Material:	SILT			
Mat2: Othor Matori	aler	11 GRAVEI			
Mat3:	ais.	GRAVEL			
Other Materia	als:				
Formation To	op Depth:	4.57			
Formation El Formation El	nd Depth: nd Depth UOM:	7.31 m			
<u>Overburden a</u> <u>Materials Inte</u>	<u>and Bedrock</u> erval				
Formation ID	:	1007122244			
Layer:		1			
General Colo	r.	6 BROWN			
Mat1:		28			
Most Commo	on Material:	SAND			
Mat2: Other Materi	ale	11 GRAVEI			
Mat3:	ais.	ONAVEL			
Other Materia	als:				
Formation To	op Depth:	0			
Formation El	nd Depth: nd Depth UOM:	2.43 M			
<u>Annular Spaces Sealing Reco</u>	<u>ce/Abandonment</u> ord				
Plug ID:		1007122254			
Layer:		1			
Plug From:		0			
Flug 10: Plug Denth I	IOM:	0.31 M			
2					

Annular Space/Abandonment Sealing Record

Map Key	Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site	DB
Plug ID: Layer: Plug From: Plug To: Plug Depth U	IOM:	1007122256 3 3.96 7.31 m			
<u>Annular Spa</u> Sealing Reco	<u>ce/Abandonment</u> ord				
Plug ID: Layer: Plug From: Plug To: Plug Depth U	IOM:	1007122255 2 0.31 3.96 m			
<u>Method of Co Use</u>	onstruction & Well				
Method Cons Method Cons Method Cons Other Metho	struction ID: struction Code: struction: d Construction:	D Direct Push			
<u>Pipe Informa</u>	<u>tion</u>				
Pipe ID: Casing No: Comment: Alt Name:		1007122243 0			
<u>Construction</u>	Record - Casing				
Casing ID: Layer: Material: Open Hole o Depth From: Depth To: Casing Diam Casing Diam Casing Dept	r Material: eter: eter UOM: h UOM:	1007122249 1 5 PLASTIC 0 4.26 4.03 cm m			
<u>Constructior</u>	<u> Record - Screen</u>				
Screen ID: Layer: Slot: Screen Top I Screen End I Screen Mate Screen Dept Screen Diam	Depth: Depth: rial: h UOM: eter UOM: eter:	1007122250 1 10 4.26 7.31 5 m cm 4.82			
Hole Diamete	er				
Hole ID: Diameter: Depth From: Depth To:		1007122247 8.5 0 7.31			

Мар Кеу	Number Records	of S	Direction/ Distance (m)	Elev/Diff (m)	Site		DB
Hole Depth U Hole Diameter	OM: r UOM:	m C	า m				
<u>12</u>	1 of 1		ESE/59.3	80.9/ -0.95	ON		WWIS
Well ID: Construction Primary Wate Sec. Water U. Final Well Sta Water Type: Casing Mater Audit No: Tag: Construction Method: Elevation (m) Elevation Rel Depth to Bed Well Depth: Overburden/I Pump Rate: Static Water I Flowing (Y/N), Flow Rate: Clear/Cloudy	Date: er Use: se: atus: ial: iability: liability: lrock: Bedrock: Level:): :	7303719 Test Hole Monitoring Monitoring Z212362 A182477	and Test Hole		Data Entry Status: Data Src: Date Received: Selected Flag: Abandonment Rec: Contractor: Form Version: Owner: Street Name: County: Municipality: Site Info: Lot: Concession? Concession Name: Easting NAD83: Northing NAD83: Zone: UTM Reliability:	1/19/2018 Yes 7241 7 749 RIDGEWOOD ROAD OTTAWA-CARLETON OTTAWA CITY	
Bore Hole Info	ormation						
Bore Hole ID: DP2BR: Spatial Status Code OB: Code OB Des Open Hole: Cluster Kind: Date Comple Remarks: Elevrc Desc:	s: sc: ted:	1006972318 11/27/2017			Elevation: Elevrc: Zone: East83: North83: Org CS: UTMRC: UTMRC Desc: Location Method:	18 446174 5024045 UTM83 4 margin of error : 30 m - 100 m wwr	
Location Soul Improvement Improvement Source Revisi Supplier Com	rce Date: Location S Location N ion Commo ment:	Source: Method: ent:					
<u>Overburden a</u> Matorials Into	nd Bedroc	<u>k</u>					
Formation ID: Layer: Color: General Color Mat1: Most Commol Mat2: Other Materia Mat3: Other Materia Formation For Formation En	r: n Material: ls: ls: p Depth: d Depth:	1 1 6 8 2 5 1 6 9 0 2 0 0 2 0 0 0 2 0 0 0 2 0 0 0 2 0 0 0 2 0 0 0 2 0 0 0 2 0	007122227 SROWN 8 SAND 1 SRAVEL				
rormation En	a vepth U	יא <i>וו:</i> מאונ: מ	1				
Мар	o Key	Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site	DB	
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<u>Over</u> Mate	burden a rials Inte	and Bedrock erval					
Form	nation ID r·	:	1007122229 3				
Color	r:		2				
Gene	eral Colo	r:	GREY				
Mat1:	:	•• • • •	06				
Most	Commo	on Material:					
Othe	: r Materia	als	GRAVEI				
Mat3	: : r Materi:	als:	0.0.0.22				
Form	ation To	op Depth:	4.57				
Form	nation Er	nd Depth:	7.31				
Form	ation Er	nd Depth UOM:	m				
<u>Over</u> Mate	<u>burden a</u> rials Inte	and Bedrock erval					
Form	ation ID	:	1007122228				
Laye	r:		2				
Color	r:		2				
Gene Mat1	eral Colo	r:	GREY 05				
Most	Commo	on Material:	CLAY				
Mat2	:		06				
Othe	r Materia	als:	SILT				
Mat3	: r Matari:						
Form	ation To	ns. op Depth:	2.43				
Form	ation Er	nd Depth:	4.57				
Form	nation Er	nd Depth UOM:	m				
<u>Annu</u> Seali	ilar Spac ing Reco	ce/Abandonment_ rd					
Plua	ID:		1007122237				
Laye	r:		1				
Plug	From:		0				
Plug	To: Donth I		0.31				
Plug	Depth U	ОМ:	111				
<u>Annu</u> Seali	<u>ılar Spac</u> İng Recc	ce/Abandonment ord					
Plug	ID:		1007122238				
Laye	r:		2				
Plug	From:		0.31				
Plug	10: Denth II	IOM·	3.90 m				
riug	Depai 0	U 111.					
<u>Annu</u> <u>Seali</u>	ilar Spacing Reco	ce/Abandonment ard					
Plua	ID:		1007122239				
Laye	r:		3				
Plug	From:		3.96				
Plug	To:		7.31				
riug	ט eptn		m				

Мар Кеу	Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site	DB
Method of Co	onstruction & Well				
Method Cons Method Cons Method Cons Other Method	truction ID: truction Code: truction: Construction:	D Direct Push			
<u>Pipe Informat</u>	tion				
Pipe ID: Casing No: Comment: Alt Name:		1007122226 0			
Construction	Record - Casing				
Casing ID: Layer: Material: Open Hole or Depth From: Depth To: Casing Diame Casing Diame Casing Depth	Material: eter: eter UOM: o UOM:	1007122232 1 5 PLASTIC 0 4.26 4.03 cm m			
<u>Construction</u>	Record - Screen				
Screen ID: Layer: Slot: Screen Top D Screen End D Screen Mater Screen Diame Screen Diame	Depth: Depth: ial: i UOM: eter UOM: eter:	1007122233 1 10 4.26 7.31 5 m cm 4.52			
<u>Hole Diamete</u>	<u>er</u>				
Hole ID: Diameter: Depth From: Depth To: Hole Depth U Hole Diamete	OM: r UOM:	1007122230 8.5 0 7.31 m cm			
<u>13</u>	1 of 1	ESE/64.0	80.6 / -1.25	ON	wwis

OOD ROAD
RLETON
/

Order No: 20200610241

Map Key	Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site		DB
Method: Elevation (m): Elevation Relia Depth to Bedro Well Depth: Overburden/Bo Pump Rate: Static Water Lo Flowing (Y/N): Flow Rate: Clear/Cloudy:	ability: ock: edrock: evel:			Municipality: Site Info: Lot: Concession: Concession Name: Easting NAD83: Northing NAD83: Zone: UTM Reliability:	OTTAWA CITY	
Bore Hole Infor	rmation					
Bore Hole ID: DP2BR: Spatial Status: Code OB: Code OB Desc Open Hole: Cluster Kind: Date Complete Remarks: Elevrc Desc: Location Source Improvement L Improvement L Source Revisio Supplier Comm	100697232 : ed: 11/27/2017 ce Date: .ocation Source: .ocation Method: on Comment: nent:			Elevation: Elevrc: Zone: East83: North83: Org CS: UTMRC: UTMRC Desc: Location Method:	18 446186 5024055 UTM83 4 margin of error : 30 m - 100 m wwr	
<u>Overburden an</u> <u>Materials Interv</u>	nd Bedrock val					
Formation ID: Layer: Color: General Color: Mat1: Most Common Mat2: Other Materials Mat3: Other Materials Formation Top Formation End Formation End	1 32 6 0 0 0 0 1 5: 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	007122269 3 2 3 3 3 3 8 3 8 5 7 5 7 5 7 5 7 5 7 1 9 8 8 7 7 9 8 9 9 9 9 9 9 9 9 9 9 9 9 9				
<u>Overburden an</u> <u>Materials Interv</u>	nd Bedrock val					
Formation ID: Layer: Color: General Color: Mat1: Most Common Mat2: Other Materials Mat3: Other Materials Formation Top Formation End Formation End	1 1 6 2 2 Material: 5: 5: 5: 5 5 5 5 5 7 5 7 5 7 5 7 5 7 5	007122267 BROWN 28 SAND 1 GRAVEL 2.43 n				

Overburden and Bedrock Materials Interval

Formation ID:	1007122268
Layer:	2
Color:	2
General Color:	GREY
Mat1:	05
Most Common Material:	CLAY
Mat2:	06
Other Materials:	SILT
Mat3:	
Other Materials:	
Formation Top Depth:	2.43
Formation End Depth:	4.57
Formation End Depth UOM:	m

<u>Annular Space/Abandonment</u> <u>Sealing Record</u>

7122279

<u>Annular Space/Abandonment</u> <u>Sealing Record</u>

Plug ID:	1007122277
Layer:	1
Plug From:	0
Plug To:	0.31
Plug Depth UOM:	m

<u>Annular Space/Abandonment</u> <u>Sealing Record</u>

Plug ID:	1007122278
Layer:	2
Plug From:	0.31
Plug To:	3.96
Plug Depth UOM:	m

Method of Construction & Well Use

Method Construction ID:	
Method Construction Code:	D
Method Construction:	Direct Push
Other Method Construction:	

Pipe Information

Pipe ID:	1007122266
Casing No:	0
Comment:	
Alt Name:	

Мар Кеу	Number Records	of Direction/ Distance (m)	Elev/Diff (m)	Site		DB
Construction	Record - C	Casing				
Casing ID: Layer: Material: Open Hole of Depth From: Depth To: Casing Diam Casing Diam Casing Depth	r Material: eter: eter UOM: n UOM:	1007122272 1 5 PLASTIC 0 4.28 4.03 cm m				
Construction	Record - S	creen				
Screen ID: Layer: Slot: Screen Top L Screen End L Screen Mater Screen Diam Screen Diam	Depth: Depth: rial: n UOM: eter UOM: eter:	1007122273 1 10 4.26 7.31 5 m cm 4.82				
Hole Diamete	<u>er</u>					
Hole ID: Diameter: Depth From: Depth To: Hole Depth U Hole Diamete	IOM: er UOM:	1007122270 8.5 0 7.31 m cm				
14	1 of 1	ESE/87.2	80.7/-1.15	Clean Water Works In	IC.	SPL
				Ottawa ON		
Ref No: Site No: Incident Dt: Year: Incident Cau Incident Even Contaminant Contaminant Contaminant Contaminant Contaminant Environment Nature of Imp Receiving Me Receiving Me Receiving En MOE Respon Dt MOE ArvI MOE Responte Dt Document Incident Reas Site Name: Site County/I Site Geo Ref Incident Sum	se: Code: Name: Limit 1: t Freq 1: UN No 1: Impact: bact: se: on Scn: ed Dt: closed: son: closed: son: District: Meth: mary:	3508-B23PFT NA 2018/06/25 Leak/Break 15 HYDRAULIC OIL n/a Land No 2018/06/25 2018/07/13 Equipment Failure 750 Ridgewood Dr<	UNOFFICIAL>	Discharger Report: Material Group: Health/Env Conseq: Client Type: Sector Type: Agency Involved: Nearest Watercourse: Site Address: Site District Office: Site Postal Code: Site Postal Code: Site Region: Site Region: Site Kegion: Site Conc: Northing: Easting: Site Geo Ref Accu: Site Geo Ref Accu: Site Map Datum: SAC Action Class: Source Type:	2 - Minor Environment Corporation Miscellaneous Communal Ottawa Eastern Ottawa 5024026.15 446194.67 Primary Assessment of Spills Motor Vehicle	
Contaminant	Qty:	50 gal-US		•		

Records	Distance (m)	(m)	Sne		DB
<u>15</u> 1 of 1	W/100.6	84.4 / 2.60	ON		wwis
Well ID: Construction Date: Primary Water Use: Sec. Water Use: Final Well Status: Water Type: Casing Material: Audit No: Tag: Construction Method: Elevation Reliability: Depth to Bedrock: Well Depth: Overburden/Bedrock: Pump Rate: Static Water Level: Flowing (Y/N): Flow Rate: Clear/Cloudy:	1508797 Domestic 0 Water Supply		Data Entry Status: Data Src: Date Received: Selected Flag: Abandonment Rec: Contractor: Form Version: Owner: Street Name: County: Municipality: Site Info: Lot: Concession: Concession Name: Easting NAD83: Northing NAD83: Zone: UTM Reliability:	1 4/1/1952 Yes 3725 1 OTTAWA-CARLETON OTTAWA CITY	
Bore Hole Information Bore Hole ID: DP2BR: Spatial Status: Code OB: Code OB Desc: Open Hole: Cluster Kind: Date Completed: Remarks: Elevrc Desc: Location Source Date: Improvement Location S Improvement Location M Source Revision Comment Supplier Comment:	10030831 23 r Bedrock 10/25/1951 Source: Method: ent:		Elevation: Elevrc: Zone: East83: North83: Org CS: UTMRC: UTMRC Desc: Location Method:	84.175674 18 446030.7 5024107 9 unknown UTM p9	
Overburden and Bedrock Materials Interval Formation ID: Layer: Color: General Color: Mat1: Most Common Material: Mat2: Other Materials: Mat3: Other Materials: Formation Top Depth: Formation End Depth: Formation End Depth UC	k 931010616 1 05 CLAY 13 BOULDERS 0 23 tt k				

Map Key	Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site	DB
Formation ID	:	931010617			
Layer:		2			
Color:					
General Colo	r:	45			
Mat1: Most Commo	n Motorial:				
Most Commo Mat2:	n Malendi.	LINESTONE			
Other Materia	als.				
Mat3:					
Other Materia	nls:				
Formation To	p Depth:	23			
Formation Er	nd Depth:	90			
Formation Er	d Depth UOM:	ft			
<u>Method of Co</u> <u>Use</u>	onstruction & Well				
Method Cons	truction ID:				
Method Cons	truction Code:	1			
Method Cons	truction:	Cable Tool			
Other Method	Construction:				
<u>Pipe Informat</u>	tion				
Pipe ID:		10579401			
Casing No:		1			
Comment:					
Alt Name:					
<u>Construction</u>	Record - Casing				
Casing ID:		930054296			
Laver:		2			
Material:		4			
Open Hole or	Material:	OPEN HOLE			
Depth From:					
Depth To:	- 4 - <i>m</i>	90			
Casing Diamo	eter:	4 inch			
Casing Depth	UOM:	ft			
5 1					
Construction	<u>Record - Casing</u>				
Casing ID:		930054295			
Layer:		1			
Material:	Matarial				
Denth From:	wateriai:	SIEEL			
Depth To:		31			
Casing Diam	eter:	4			
Casing Diam	eter UOM:	inch			
Casing Depth	UOM:	ft			
<u>Results of We</u>	ell Yield Testing				
Pump Test ID):	991508797			
Pump Set At:					
Static Level:		12			

Map Key	Number Records	of Direction/ Distance (m)	Elev/Diff (m)	Site		DB
Flowing Rate Recommend Levels UOM: Rate UOM: Water State J Water State J Pumping Du Pumping Du	e: led Pump Ra : After Test Co After Test: st Method: iration HR: ration MN·	te: ft GPM ode: 1 CLEAR 1				
Flowing:		Ν				
Water Detail	<u>s</u>					
Water ID: Layer: Kind Code: Kind: Water Found Water Found	l Depth: l Depth UOM	933463473 1 FRESH 60 I: ft				
<u>16</u>	1 of 3	ENE/116.2	77.9 / -3.95	753 Springland Drive Ottawa ON K1V 6L9		EHS
Order No: Status: Report Type Report Date: Date Receive Previous Sitt Lot/Building Additional In	: ed: e Name: Size: nfo Ordered:	20040311001 C Complete Report Upgrade 3/16/04 3/11/04		Nearest Intersection: Municipality: Client Prov/State: Search Radius (km): X: Y:	ON 0.35 -75.686188 45.369211	
<u>16</u>	2 of 3	ENE/116.2	77.9 / -3.95	753 Springland Drive Ottawa ON K1V 6L9		EHS
Order No: Status: Report Type Report Date: Date Receive Previous Site Lot/Building Additional In	: ed: e Name: Size: nfo Ordered:	20040310012w C Online Mapless 3/10/04 3/10/04		Nearest Intersection: Municipality: Client Prov/State: Search Radius (km): X: Y:	ON 0.25 0 0	
<u>16</u>	3 of 3	ENE/116.2	77.9 / -3.95	753 A Springland Dr Ottawa ON K1V6L9		EHS
Order No: Status: Report Type Report Date: Date Receive Previous Site	: ed: e Name: Size:	20170206030 C RSC Report (Urban) 10-FEB-17 06-FEB-17		Nearest Intersection: Municipality: Client Prov/State: Search Radius (km): X: Y:	ON .3 -75.686603 45.368644	
Additional In	of Ordered:	Fire Insur. Maps a	nd/or Site Plans; A	Aerial Photos		
<u>17</u>	1 of 1	ESE/137.1	79.9/-1.95	ITALIAN TELEPHONE 770 RIDGEWOOD AVE OTTAWA ON K1V 6MS	DIRECTORY	SCT
54	erisinfo.co	m Environmental Risk Inf	ormation Servic	es		Order No: 20200610241

Map Key	Number Records	r of s	Direction/ Distance (m)	Elev/Diff (m)	Site		DB
Established: Plant Size (ft [:] Employment:	²): :	(1970 0 2				
<u>Details</u> Description: SIC/NAICS C	ode:	 !	Database and Direct 511140	ctory Publishers			
<u>18</u>	1 of 1		W/188.7	87.6 / 5.76	ON		wwis
Well ID: Construction Primary Wate Sec. Water U Final Well Sta Water Type: Casing Mater Audit No: Tag: Construction Elevation (m) Elevation Rei Depth to Bed Well Depth: Overburden/I Pump Rate: Static Water Flowing (Y/N) Flow Rate: Clear/Cloudy	n Date: er Use: lse: atus: rial: iability: liability: frock: Bedrock: Level:):	1507898 Domestic 0 Water Sup	ply		Data Entry Status: Data Src: Data Src: Selected Flag: Abandonment Rec: Contractor: Form Version: Owner: Street Name: County: Municipality: Site Info: Lot: Concession Name: Easting NAD83: Northing NAD83: Zone: UTM Reliability:	1 11/16/1951 Yes 3601 1 OTTAWA-CARLETON OTTAWA CITY	
Bore Hole Int	formation						
Bore Hole ID: DP2BR: Spatial Statu: Code OB: Code OB Des Open Hole: Cluster Kind: Date Comple Remarks: Elevrc Desc: Location Sou Improvement Source Revis Supplier Con	: sc: sc: teted: t Location S t Location I sion Comm nment:	10029933 14 r Bedrock 8/18/1951 Source: Method: ent:			Elevation: Elevrc: Zone: East83: North83: Org CS: UTMRC: UTMRC Desc: Location Method:	87.029212 18 445940.7 5024102 9 unknown UTM p9	
<u>Overburden a</u> <u>Materials Inte</u>	and Bedroo erval	: <u>k</u>					
Formation ID Layer: Color: General Colo Mat1: Most Commo Mat2: Other Materia): or: on Material: als:		931008314 3 15 LIMESTONE				

	Мар Кеу	Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site	DB
-	Mat3: Other Materia Formation To, Formation En Formation En	ls: p Depth: d Depth: d Depth UOM:	23 80 ft			
	<u>Overburden a</u> <u>Materials Inte</u>	<u>nd Bedrock</u> rval				
	Formation ID: Layer: Color: General Color Mat1: Most Common Mat2: Other Materia Mat3:	:: n Material: Is:	931008312 1 05 CLAY			
	Other Materia Formation To Formation En Formation En	ls: p Depth: d Depth: d Depth UOM:	0 14 ft			
	<u>Overburden a</u> <u>Materials Inte</u>	<u>nd Bedrock</u> rval				
	Formation ID: Layer: Color: General Color Mat1: Most Commo Mat2: Other Materia Mat3: Other Materia	: n Material: ls: ls:	931008313 2 17 SHALE			
	Formation To Formation En Formation En	p Depth: d Depth: d Depth UOM:	14 23 ft			
	<u>Method of Co</u> <u>Use</u>	nstruction & Well				
	Method Const Method Const Method Const Other Method	truction ID: truction Code: truction: Construction:	1 Cable Tool			
	<u>Pipe Informat</u>	ion				
	<i>Pipe ID: Casing No: Comment: Alt Name:</i>		10578503 1			
	<u>Construction</u>	<u>Record - Casing</u>				
	Casing ID: Layer: Material: Open Hole or Depth From:	Material:	930052520 2 4 OPEN HOLE			

Records	Distance (m)	(m)			
Depth To:	80				
Casing Diameter:	4				
Casing Diameter UOM:	inch				
Casing Depth UOM:	п				
Construction Record - Casi	na				
<u>oonoauouon noooru</u> ouon					
Casing ID:	930052519				
Layer:	1				
Material: Open Hole or Material:	STEEL				
Depth From:	0.222				
Depth To:	16				
Casing Diameter:	4				
Casing Diameter UOM:	inch				
Casing Depth COM.	n				
Results of Well Yield Testin	g				
Pump Test ID:	991507898				
Pump Set At:					
Static Level:	8				
Final Level After Pumping:					
Pumping Rate:	l.				
Flowing Rate:					
Recommended Pump Rate:					
Levels UOM:	ft				
Water State After Test Code	GPM • 1				
Water State After Test:	CLEAR				
Pumping Test Method:	1				
Pumping Duration HR:	1				
Pumping Duration Min: Flowing:	U N				
riowing.					
<u>Water Details</u>					
Water ID:	933462188				
Layer:	1				
Kind Code:	1				
Kind: Water Found Depth:	FRESH 50				
Water Found Depth UOM:	ft				
·					
<u>19</u> 1 of 1	NW/192.9	82.8 / 1.00	2887 Riverside Dr Ottawa ON K1V8N4		EHS
Order No: 20	170313066		Nearest Intersection		
Status: C	110010000		Municipality:	Ottawa	
Report Type: Sta	andard Express Report		Client Prov/State:	ON	
Report Date: 14	-MAR-17		Search Radius (km):	.25	
Previous Site Name	-WAR-17		χ. γ.	-75.00957 45.369488	
Lot/Building Size: 0.4	15 Acres				
Additional Info Ordered:	Fire Insur. Maps ar	nd/or Site Plans; (City Directory		
20 1 of 1	WNW/197.7	84.2 / 2.36	Ottowo ON		WWIS
Well ID: 72	85490		Data Entry Status:		
	Environmental Dials lat	armation Carda			Order Net 20200040044
57 erisinto.com		ormation Servic	60		Order 1NO: 20200610241

Number of

Мар Кеу

Direction/

Elev/Diff

Site

DB

Map Key Number o Records	f Direction/ Distance (m)	Elev/Diff (m)	Site	DB
Construction Date:Primary Water Use:TSec. Water Use:MFinal Well Status:CWater Type:Casing Material:Audit No:ZTag:AConstruction Method:Elevation (m):Elevation Reliability:Depth to Bedrock:Well Depth:Overburden/Bedrock:Pump Rate:Static Water Level:Flowing (Y/N):Flow Rate:Clear/Cloudy:C	Test Hole Monitoring Dbservation Wells 2245905 A216094		Data Src: Date Received: Selected Flag: Abandonment Rec: Contractor: Form Version: Owner: Street Name: County: Municipality: Site Info: Lot: Concession: Concession Name: Easting NAD83: Northing NAD83: Zone: UTM Reliability:	4/21/2017 Yes 7529 7 2887 RIVERSIDE DR OTTAWA-CARLETON GLOUCESTER TOWNSHIP
Bore Hole Information				
Bore Hole ID:1DP2BR:Spatial Status:Code OB:Code OB Desc:Open Hole:Cluster Kind:Date Completed:4Remarks:Elevrc Desc:Location Source Date:Improvement Location SolImprovement Location MeSource Revision CommenSupplier Comment:	006383549 \/3/2017 urce: thod: t:		Elevation: Elevrc: Zone: East83: North83: Org CS: UTMRC: UTMRC Desc: Location Method:	82.631134 18 445985 5024219 UTM83 4 margin of error : 30 m - 100 m wwr
<u>Overburden and Bedrock</u> <u>Materials Interval</u>				
Formation ID: Layer: Color: General Color: Mat1: Most Common Material: Mat2: Other Materials: Mat3: Other Materials: Formation Top Depth: Formation End Depth: Formation End Depth UON <u>Annular Space/Abandonm</u> <u>Sealing Record</u> Plug ID: Layer: Plug From: Plug To:	1006689984 1 6 BROWN 28 SAND 05 CLAY 73 HARD 0 15 M: ft 1006689993 2 5 15			
Plug Depth UOM:	ft	rmation Service	20	Order No: 20200610241

Map Key Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site
<u>Annular Space/Abandonme</u> <u>Sealing Record</u>	<u>nt</u>		
Plug ID: Layer: Plug From: Plug To: Plug Depth UOM:	10066899992 1 0 5 ft		
<u>Method of Construction & V</u> <u>Use</u>	<u>Vell</u>		
Method Construction ID: Method Construction Code: Method Construction: Other Method Construction	6 Boring :		
Pipe Information			
Pipe ID: Casing No: Comment: Alt Name:	1006689983 0		
Construction Record - Casi	ng		
Casing ID: Layer: Material: Open Hole or Material: Depth From: Depth To: Casing Diameter: Casing Diameter UOM: Casing Depth UOM:	1006689987 1 5 PLASTIC 0 5 2 inch ft		
Construction Record - Scre	en		
Screen ID: Layer: Slot: Screen Top Depth: Screen End Depth: Screen Material: Screen Depth UOM: Screen Diameter UOM: Screen Diameter:	1006689988 1 5 15 5 ft inch 2		
Hole Diameter			
Hole ID: Diameter: Depth From: Depth To: Hole Depth UOM: Hole Diameter UOM:	1006689985 9 0 15 ft inch		

<u>21</u>

1 of 2

SSW/200.2 83.1 / 1.25

TAMARACK DEVELOPMENTS CORP. -RIVERSIDE

CA

Мар Кеу	Number Records	of Direction/ Distance (m)	Elev/Diff (m)	Site	DB
				2991 RIVERSIDE DR/BAYPORT PRIV OTTAWA CITY ON K1V 8N6	
Certificate #: Application Y Issue Date: Approval Tyj Status: Application T Client Name: Client Name: Client Addre Client City: Client Postal Project Desc Contaminant Emission Co	: Year: pe: Type: : sss: I Code: cription: ts: ontrol:	3-1919-90- 90 10/19/1990 Municipal sewage Approved			
<u>21</u>	2 of 2	SSW/200.2	83.1 / 1.25	TAMARACK DEVELOPMENTS CORP RIVERSIDE 2991 RIVERSIDE DR/BAYPORT PRIV OTTAWA CITY ON K1V 8N6	СА
Certificate #: Application V Issue Date: Approval Tyj Status: Application T Client Name: Client Name: Client Addre Client City: Client Postal Project Desc Contaminant Emission Co	: Year: pe: Type: : sss: l Code: cription: ts: ontrol:	7-1571-90- 90 10/19/1990 Municipal water Approved			
<u>22</u>	1 of 1	ENE/234.0	75.9 / -5.92	OTTAWA CITY SPRINGLAND DR./HOBSON RD. OTTAWA CITY ON	CA
Certificate #: Application M Issue Date: Approval Tyj Status: Application M Client Name: Client Name: Client Addre Client City: Client Postal Project Desc Contaminant Emission Co	Year: pe: Type: : sss: Code: cription: ts: ontrol:	3-0909-94- 94 7/22/1994 Municipal sewage Approved			
<u>23</u>	1 of 1	ESE/246.4	77.9 / -3.89	PRIVATE RESIDENCE 2707 SPRINGLAND DRIVE FURNACE OIL TANK OTTAWA CITY ON K1V 6M2	SPL
Ref No: Site No: Incident Dt:		65843 1/3/1992		Discharger Report: Material Group: Health/Env Conseq:	
60	erisinfo.cor	n Environmental Risk Info	ormation Service	os Order No: 2	0200610241

Map Key	Number Records	r of s	Direction/ Distance (m)	Elev/Diff (m)	Site		DB
Year: Incident Caus Incident Ever Contaminant Contaminant Contaminant Contam Limit Contaminant	se: nt: Code: Name: Limit 1: t Freq 1: UN No 1:	PIPE/HC	DSE LEAK		Client Type: Sector Type: Agency Involved: Nearest Watercourse: Site Address: Site District Office: Site Postal Code: Site Region:		
Environment Nature of Imp Receiving Me Receiving En	Impact: bact: edium: iv:	CONFIR Multi-me LAND	MED dia Pollution		Site Municipality: Site Lot: Site Conc: Northing:	20101	
MOE Respon Dt MOE Arvi MOE Reporte Dt Document Incident Reas Site Name: Site County/I	se: on Scn: ed Dt: Closed: son: District:	1/3/1992 MATERI	AL FAILURE		Easting: Site Geo Ref Accu: Site Map Datum: SAC Action Class: Source Type:	MCCR	
Site Geo Ref Incident Sum Contaminant	Meth: mary: Qty:		RESIDENCE: OIL	LEAKING FROM F	URNACE TANK OIL LINE I	JNDER BASEMENT SLAB	
<u>24</u>	1 of 6		WNW/249.5	86.9 / 5.11	City of Ottawa 2960 Riverside Dr. ottawa ON		GEN
Generator No Status: Approval Yea Contam. Faci MHSW Facilit SIC Code: SIC Descripti	o: ars: ility: ty: ion:	ON59393 2013 484221	702 BULK LIQUIDS TF	RUCKING, LOCAL	PO Box No: Country: Choice of Contact: Co Admin: Phone No Admin:		
<u>Detail(s)</u> Wasta Classi			254				
Waste Class	Desc:		OIL SKIMMINGS &	& SLUDGES			
<u>24</u>	2 of 6		WNW/249.5	86.9 / 5.11	City of Ottawa 2960 Riverside Dr. ottawa ON K2G 6J8		GEN
Generator No):	ON59397	702		PO Box No:	Conada	
Status: Approval Yea Contam. Faci MHSW Facilit SIC Code: SIC Descripti	ars: ility: ty: ion:	2016 No No 484221	BULK LIQUIDS TF	RUCKING, LOCAL	<i>Country: Choice of Contact: Co Admin: Phone No Admin:</i>	Canada CO_OFFICIAL Mark Hennigar 613-580-2424 Ext.33331	
<u>Detail(s)</u>							
Waste Class: Waste Class	Desc:		221 LIGHT FUELS				
Waste Class: Waste Class	Desc:		251 OIL SKIMMINGS 8	& SLUDGES			

Map Key Number Records	r of S	Direction/ Distance (m)	Elev/Diff (m)	Site		DB
24 3 of 6		WNW/249.5	86.9 / 5.11	City of Ottawa 2960 Riverside Dr. ottawa ON K2G 6J8		GEN
Generator No:	ON5939	702		PO Box No:		
Status: Approval Years:	2015			Country: Choice of Contact:	Canada CO_OFFICIAL	
Contam. Facility: MHSW Facility:	No No			Co Admin: Phone No Admin:	Mark Hennigar 613-580-2424 Ext.33331	
SIC Code: SIC Description:	484221	BULK LIQUIDS TR	UCKING, LOCAL			
<u>Detail(s)</u>						
Waste Class: Waste Class Desc:		251 OIL SKIMMINGS &	SLUDGES			
Waste Class: Waste Class Desc:		221 LIGHT FUELS				
<u>24</u> 4 of 6		WNW/249.5	86.9/5.11	City of Ottawa 2960 Riverside Dr. ottawa ON K2G 6J8		GEN
Generator No:	ON5939	702		PO Box No:		
Status: Approval Years:	2014			Country: Choice of Contact:	Canada CO_OFFICIAL	
Contam. Facility: MHSW Facility:	No No			Co Admin: Phone No Admin:	Mark Hennigar 613-580-2424 Ext.33331	
SIC Code: SIC Description:	484221	BULK LIQUIDS TR	UCKING, LOCAL			
<u>Detail(s)</u>						
Waste Class: Waste Class Desc:		251 OIL SKIMMINGS &	SLUDGES			
Waste Class: Waste Class Desc:		221 LIGHT FUELS				
24 5 of 6		WNW/249.5	86.9/5.11	City of Ottawa RPAM 2960 Riverside Dr. ottawa ON K2G 6J8		GEN
Generator No:	ON5939	702		PO Box No:	O	
Status: Approval Years:	Register As of De	ed c 2018		Country: Choice of Contact:	Canada	
Contam. Facility: MHSW Facility:				Co Admin: Phone No Admin:		
SIC Code: SIC Description:						
<u>Detail(s)</u>						
Waste Class: Waste Class Desc:		221 L Light fuels				
Waste Class: Waste Class Desc:		251 L Waste oils/sludges	(petroleum based)			

Мар Кеу	Number Records	of S	Direction/ Distance (m)	Elev/Diff (m)	Site		DB
<u>24</u>	6 of 6		WNW/249.5	86.9/5.11	City of Ottawa RPAM 2960 Riverside Dr. ottawa ON K2G 6J8		GEN
Generator No Status: Approval Yea Contam. Faci MHSW Facilit SIC Code: SIC Descripti	o: ars: ility: ty: ion:	ON59397 Registere As of Oct	02 d 2019		PO Box No: Country: Choice of Contact: Co Admin: Phone No Admin:	Canada	
<u>Detail(s)</u>							
Waste Class: Waste Class	Desc:		251 L Waste oils/sludges	(petroleum based)			
Waste Class: Waste Class	Desc:		221 L Light fuels				
<u>25</u>	1 of 10		NNW/249.6	81.9 / 0.09	ST. PATRICK'S HOME 2865 RIVERSIDE DRIV OTTAWA ON K1V 8N5	OF OTTAWA 34-692 /E	GEN
Generator No Status:): 	ON16685	00		PO Box No: Country:		
Approval Yea Contam. Fac	ars: ility:	92,93,94,	95,96,97,98		Choice of Contact: Co Admin:		
MHSW Facilia SIC Code:	ty:	8621			Phone No Admin:		
SIC Descripti	ion:		PERS./NURS. CAF	RE H.			
<u>Detail(s)</u>							
Waste Class: Waste Class	Desc:		312 PATHOLOGICAL V	WASTES			
<u>25</u>	2 of 10		NNW/249.6	81.9 / 0.09	ST. PATRICK'S HOME 2865 RIVERSIDE DRIV OTTAWA ON K1V 8N5	OF OTTAWA /E	GEN
Generator No	o:	ON16685	00		PO Box No: Country:		
Approval Yea Contam. Faci	ars: ility:	99,00,01			Choice of Contact: Co Admin:		
MHSW Facili SIC Code: SIC Descripti	ty: ion:	8621	PERS./NURS. CAF	RE H.	Phone No Admin:		
<u>Detail(s)</u>							
Waste Class: Waste Class	Desc:		312 PATHOLOGICAL V	WASTES			
<u>25</u>	3 of 10		NNW/249.6	81.9/0.09	2865 Riverside Drive Ottawa ON K1V 8N5		EHS
Order No: Status: Report Type: Report Date:		20081014 C Standard 10/23/200	1031 Report)8		Nearest Intersection: Municipality: Client Prov/State: Search Radius (km):	Riverside Drive and Brookfield Road Ottawa ON 0.25	

erisinfo.com | Environmental Risk Information Services

Order No: 20200610241

Map Key Number Records	of Direction/ Distance (m)	Elev/Diff (m)	Site		DB
Date Received: Previous Site Name: Lot/Building Size: Additional Info Ordered:	10/14/2008 approx. 5.9 acres City Directory		Х: Ү:	-75.690139 45.370423	
25 4 of 10	NNW/249.6	81.9 / 0.09	2865 Riverside Drive Ottawa ON K1V 8N5		EHS
Order No: Status: Report Type: Report Date: Date Received: Previous Site Name: Lot/Building Size: Additional Info Ordered:	20110412033 C Standard Select Report 4/21/2011 4/12/2011 1:06:29 PM		Nearest Intersection: Municipality: Client Prov/State: Search Radius (km): X: Y:	ON 0.25 -75.689562 45.370416	
25 5 of 10	NNW/249.6	81.9 / 0.09	<i>St. Patrick's Home of 2865 Riverside Dr. Ottawa ON K1V 8N5</i>	Ottawa Inc.	GEN
Generator No: Status: Approval Years: Contam. Facility: MHSW Facility: SIC Code: SIC Description:	ON4394512 2009 623310 Community Care Fa	acilities for the Ele	PO Box No: Country: Choice of Contact: Co Admin: Phone No Admin: derly		
<u>Detail(s)</u> Waste Class: Waste Class Desc:	251 OIL SKIMMINGS &	SLUDGES			
25 6 of 10	NNW/249.6	81.9/0.09	Medical Arts Dispens 2865 Riverside Drive Ottawa ON K1V 8N5	ary of Ottawa	GEN
Generator No: Status: Approval Years: Contam. Facility: MHSW Facility: SIC Code: SIC Description:	ON4099831 2016 No 621110 OFFICES OF PHYS	SICIANS	PO Box No: Country: Choice of Contact: Co Admin: Phone No Admin:	Canada CO_OFFICIAL	
<u>Detail(s)</u>					
Waste Class: Waste Class Desc:	312 PATHOLOGICAL V	VASTES			
Waste Class: Waste Class Desc:	261 PHARMACEUTICA	LS			
25 7 of 10	NNW/249.6	81.9 / 0.09	Medical Arts Dispensa 2865 Riverside Drive Ottawa ON K1V 8N5	ary of Ottawa	GEN
Generator No:	ON4099831		PO Box No:		

Map Key Nu Rec	mber of cords	Direction/ Distance (m)	Elev/Diff (m)	Site		DB
Status: Approval Years: Contam. Facility: MHSW Facility: SIC Code: SIC Description:	2015 No No 621110	OFFICES OF PHYS	SICIANS	Country: Choice of Contact: Co Admin: Phone No Admin:	Canada CO_OFFICIAL	
<u>Detail(s)</u>						
Waste Class: Waste Class Desc:		312 PATHOLOGICAL V	VASTES			
Waste Class: Waste Class Desc:		261 PHARMACEUTICA	LS			
25 8 of	10	NNW/249.6	81.9 / 0.09	St. Patrick's Home of 2865 Riverside Dr. Ottawa ON K1V 8N5	Ottawa Inc.	GEN
Generator No: Status: Approval Years: Contam. Facility: MHSW Facility: SIC Code: SIC Description:	ON7471 2014 No 623310	928 623310		PO Box No: Country: Choice of Contact: Co Admin: Phone No Admin:	Canada CO_OFFICIAL	
<u>Detail(s)</u>						
Waste Class: Waste Class Desc:		243 PCBS				
<u>25</u> 9 of ¹	10	NNW/249.6	81.9 / 0.09	Medical Arts Dispens 2865 Riverside Drive Ottawa ON K1V 8N5	ary of Ottawa	GEN
Generator No: Status: Approval Years: Contam. Facility: MHSW Facility: SIC Code: SIC Description:	ON4099 Register As of De	831 red ec 2018		PO Box No: Country: Choice of Contact: Co Admin: Phone No Admin:	Canada	
<u>Detail(s)</u>						
Waste Class: Waste Class Desc:		261 A Pharmaceuticals				
Waste Class: Waste Class Desc:		261 I Pharmaceuticals				
Waste Class: Waste Class Desc:		312 P Pathological wastes	8			
25 10 of	10	NNW/249.6	81.9/0.09	<i>Medical Arts Dispens 2865 Riverside Drive Ottawa ON K1V 8N5</i>	ary of Ottawa	GEN
Generator No: Status:	ON4099 Register	9831 red		PO Box No: Country:	Canada	

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Order No: 20200610241

Мар Кеу	Number of Records	Direction/ Distance (m)	Elev/Diff (m)	Site	DB
Approval Yea Contam. Fac MHSW Facili SIC Code: SIC Descript	ars: As of ility: ty: ion:	Oct 2019		Choice of Contact: Co Admin: Phone No Admin:	
<u>Detail(s)</u>					
Waste Class. Waste Class	: Desc:	261 I Pharmaceuticals			
Waste Class. Waste Class	: Desc:	261 A Pharmaceuticals			
Waste Class. Waste Class	: Desc:	312 P Pathological wastes	;		

Unplottable Summary

Total: 24 Unplottable sites

DB	Company Name/Site Name	Address	City	Postal
СА	1213225 Ontario Ltd. and Tamarack Developments Corporation	Block 165 of Registered Plan 4M-1224	Ottawa ON	
CA	1213225 Ontario Ltd. and Tamarack Developments Corporation	Block 167 - Reg. Plan 4M-1224	Ottawa ON	
СА	Clean Water Works Inc.		Ottawa ON	
СА	Clean Water Works Inc.		Ottawa ON	
CA	CLARIDGE HOMES (RIVERSIDE) INC.	ST.#1/RIVERSIDE DR/CUL-DE-SAC	OTTAWA CITY ON	
CA	CAMPEAU CORP.	RIVERSIDE DR.	OTTAWA ON	
СА	CAMPEAU CORP.	RIVERSIDE DR.	OTTAWA ON	
CA	CLARIDGE HOMES (RIVERSIDE) INC.	ST.#1/RIVERSIDE DR/CUL-DE-SAC	OTTAWA CITY ON	
CA	WEDGEWOOD BUILDING CORPORATION	RIDGEWOOD PRIVATE	OTTAWA CITY ON	
CA	PEREZ CORPORATION	STREET NO. 1 RIVERSIDE DR.	OTTAWA CITY ON	
CA	Clean Water Works Inc.		Ottawa ON	
CA	Clean Water Works Inc.		Ottawa ON	
СА	Clean Water Works Inc.	Mobile Unit	Ottawa ON	
CA	R.M. OF OTTAWA-CARL.S.E. TRANSITWAY ST. 1	E. SIDE OF RIVERSIDE DR.	OTTAWA CITY ON	
CA	WEDGEWOOD BUILDING CORPORATION	RIDGEWOOD PRIVATE	OTTAWA CITY ON	
CA	J. PEREZ CORPORATION STM MGN. 3-0842-87	STREET #1 RIVERSIDE DR.	OTTAWA CITY ON	
ECA	City of Ottawa	Cummings Ave., Norberry Cres., Rainbow Cres.	Ottawa ON	K1S 5K2

ECA	1213225 Ontario Ltd. and Tamarack Developments Corporation	Block 165 of Registered Plan 4M-1224	Ottawa ON	K2P 0G5
ECA	1213225 Ontario Ltd. and Tamarack Developments Corporation		Ottawa ON	K2P 0G5
ECA	Clean Water Works Inc.	Mobile Unit	Ottawa ON	K1B 5L6
ECA	City of Ottawa	Riverside Drive	Ottawa ON	K1S 5K2
RSC		Part Lot 23	Ottawa ON	
SPL	ULTRAMAR	RIVERSIDE DRIVE AT TRANSIT WAY (NEAR POST OFFICE) TANK TRUCK (CARGO)	OTTAWA CITY ON	
SPL	Clean Water Works Inc.		Ottawa ON	

Unplottable Report

Site: 1213225 Ontario Ltd. and Tamarack Developments Corporation Block 165 of Registered Plan 4M-1224 Ottawa ON

Certificate #: Application Year: Issue Date: Approval Type: Status: Application Type: Client Name: Client Address: Client City: Client Postal Code: **Project Description:** Contaminants: **Emission Control:**

2967-6DMNBF 2005 6/24/2005 Municipal and Private Sewage Works Approved

1213225 Ontario Ltd. and Tamarack Developments Corporation Site: Block 167 - Reg. Plan 4M-1224 Ottawa ON

2005

2786-6DNMQD

Certificate #: Application Year: Issue Date: Approval Type: Status: Application Type: Client Name: Client Address: Client City: **Client Postal Code: Project Description:** Contaminants: **Emission Control:**

6/28/2005 Municipal and Private Sewage Works Approved

Site: Clean Water Works Inc. Ottawa ON

Certificate #: Application Year: Issue Date: Approval Type: Status: Application Type: Client Name: Client Address: Client City: Client Postal Code: **Project Description:** Contaminants: **Emission Control:**

3664-6GGPRM 2005 10/3/2005 Waste Management Systems Revoked and/or Replaced

CA

<u>Site:</u>	Clean Water Works Inc. Ottawa ON		Database: CA
Certific Applica	ate #: ation Year:	3664-6GGPRM 2006	
60	erisinfo.com Envi	ronmental Risk Information Services	Order No: 20200610241

Database:

CA

Database: CA

Database:

Issue Date: Approval Type: Status: Application Type: Client Name: Client Address: Client City: Client Postal Code: Project Description: Contaminants: Emission Control: 1/20/2006 Waste Management Systems Approved

<u>Site:</u> CLARIDGE HOMES (RIVERSIDE) INC. ST.#1/RIVERSIDE DR/CUL-DE-SAC OTTAWA CITY ON

Certificate #: Application Year: Issue Date: Approval Type: Status: Application Type: Client Name: Client Address: Client City: Client Postal Code: Project Description: Contaminants: Emission Control: 3-1703-95-006 95 12/11/95 Municipal sewage Approved

<u>Site:</u> CAMPEAU CORP. RIVERSIDE DR. OTTAWA ON

Certificate #: Application Year: Issue Date: Approval Type: Status: Application Type: Client Name: Client Address: Client Address: Client City: Client Postal Code: Project Description: Contaminants: Emission Control:

Site:

7-0165-85-006 85 3/29/85 Municipal water Approved

RIVERSIDE DR. OTTAWA ON

Certificate #: Application Year: Issue Date: Approval Type: Status: Application Type: Client Name: Client Address: Client City: Client Postal Code: Project Description: Contaminants: Emission Control:

CAMPEAU CORP.

3-0118-85-006 85 3/1/85 Municipal sewage Approved Database:

Database:

CA

Database: CA

<u>Site:</u> CLARIDGE HOMES (RIVERSIDE) INC. ST.#1/RIVERSIDE DR/CUL-DE-SAC OTTAWA CITY ON

Certificate #: Application Year: Issue Date: Approval Type: Status: Application Type: Client Name: Client Name: Client Address: Client City: Client Postal Code: Project Description: Contaminants: Emission Control: 7-1203-95-006 95 12/11/95 Municipal water Approved

<u>Site:</u> WEDGEWOOD BUILDING CORPORATION RIDGEWOOD PRIVATE OTTAWA CITY ON

Certificate #: Application Year: Issue Date: Approval Type: Status: Application Type: Client Name: Client Address: Client City: Client Postal Code: Project Description: Contaminants: Emission Control: 7-1096-87-87 8/4/1987 Municipal water Approved

<u>Site:</u> PEREZ CORPORATION STREET NO. 1 RIVERSIDE DR. OTTAWA CITY ON

Certificate #: Application Year: Issue Date: Approval Type: Status: Application Type: Client Name: Client Name: Client Address: Client City: Client Postal Code: Project Description: Contaminants: Emission Control: 7-0478-87-87 5/5/1987 Municipal water Approved

<u>Site:</u> Clean Water Works Inc. Ottawa ON

Certificate #: Application Year: Issue Date: Approval Type: Status: Application Type: Client Name: Client Name: Client Address: Client City: Client Postal Code: Project Description: 6489-6GTPNX 2005 10/5/2005 Waste Management Systems Revoked and/or Replaced

71



Database: CA

Database: CA

Database: CA

<u>Site:</u> Clean Water Works Inc. Ottawa ON

Certificate #: Application Year: Issue Date: Approval Type: Status: Application Type: Client Name: Client Address: Client City: Client Postal Code: Project Description: Contaminants: Emission Control: 6489-6GTPNX 2006 3/3/2006 Waste Management Systems Approved

<u>Site:</u> Clean Water Works Inc. Mobile Unit Ottawa ON

Certificate #: Application Year: Issue Date: Approval Type: Status: Application Type: Client Name: Client Address: Client City: Client Postal Code: Project Description: Contaminants: Emission Control: 9392-8HTPQD 2011 10/25/2011 Industrial Sewage Works Approved

<u>Site:</u> R.M. OF OTTAWA-CARL.S.E.TRANSITWAY ST. 1 E. SIDE OF RIVERSIDE DR. OTTAWA CITY ON

Certificate #: Application Year: Issue Date: Approval Type: Status: Application Type: Client Name: Client Name: Client Address: Client City: Client Postal Code: Project Description: Contaminants: Emission Control: 7-0818-89-89 5/29/1989 Municipal water Approved

<u>Site:</u> WEDGEWOOD BUILDING CORPORATION RIDGEWOOD PRIVATE OTTAWA CITY ON

Certificate #: Application Year: Issue Date: Approval Type: Status: Application Type: 3-1320-87-87 8/4/1987 Municipal sewage Approved

72



Database: CA

Database:

Database: CA



J. PEREZ CORPORATION STM MGN. 3-0842-87 Site: STREET #1 RIVERSIDE DR. OTTAWA CITY ON

ECA

IDS

Certificate #: Application Year: Issue Date: Approval Type: Status: Application Type: Client Name: Client Address: Client City: Client Postal Code: Project Description: Contaminants: **Emission Control:**

3-0563-87-87 5/5/1987 Municipal sewage Approved

Site:	City	of Ot	tawa
	-	-	-

Cummings Ave., Norberry Cres., Rainbow Cres. Ottawa ON K1S 5K2

Approval No: Approval Date: Status: Record Type: Link Source: SWP Area Name: Approval Type: Project Type: Address: Full Address: Full PDF Link:

1904-5QCPU7 2003-08-26 Approved ECA-Municipal Drinking Water Systems Municipal Drinking Water Systems Cummings Ave., Norberry Cres., Rainbow Cres.

MOE District:

Longitude:

Geometry X:

Geometry Y:

Latitude:

City:

Site: 1213225 Ontario Ltd. and Tamarack Developments Corporation Block 165 of Registered Plan 4M-1224 Ottawa ON K2P 0G5

Approval No:	2967-6DMNBF	MOE District:
Approval Date:	2005-06-24	City:
Status:	Approved	Longitude:
Record Type:	ECA	Latitude:
Link Source:	IDS	Geometry X:
SWP Area Name:		Geometry Y:
Approval Type:	ECA-MUNICIPAL AN	ID PRIVATE SEWAGE WORKS
Project Type:	MUNICIPAL AND PF	IVATE SEWAGE WORKS
Address:	Block 165 of Registe	red Plan 4M-1224
Full Address:		
Full PDF Link:	https://www.accesse	nvironment.ene.gov.on.ca/instruments/8345-6DKP82-14.pdf

Site: 1213225 Ontario Ltd. and Tamarack Developments Corporation Ottawa ON K2P 0G5

Approval No:	2786-6DNMQD	MOE District:
Approval Date:	2005-06-28	City:
Status:	Approved	Longitude:
Record Type:	ECA	Latitude:
Link Source:	IDS	Geometry X:

73

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Database: CA

Database: **ECA**

Database: **ECA**

Database: ECA



SWP Area Name: Approval Type: Project Type: Address: Full Address: Full PDF Link:

https://www.accessenvironment.ene.gov.on.ca/instruments/6293-6DKPCR-14.pdf

<u>Site:</u> Clean Water W Mobile Unit	orks Inc. ttawa ON K1B 5L6		Database: ECA
Approval No: Approval Date: Status: Record Type: Link Source: SWP Area Name: Approval Type: Project Type: Address: Full Address: Full PDF Link:	9392-8HTPQD 2011-10-25 Approved ECA IDS ECA-INDUSTRIAL SEWAGE WORKS INDUSTRIAL SEWAGE WORKS Mobile Unit https://www.accessenvironment.ene.go	MOE District: City: Longitude: Latitude: Geometry X: Geometry Y:	
<u>Site:</u> City of Ottawa Riverside Drive	e Ottawa ON K1S 5K2		Database: ECA
Approval No: Approval Date: Status: Record Type: Link Source: SWP Area Name: Approval Type: Project Type: Address: Full Address: Full PDF Link:	6330-5XEKCD 2004-03-29 Approved ECA IDS ECA-Municipal Drinking Water System Municipal Drinking Water Systems Riverside Drive	MOE District: City: Longitude: Latitude: Geometry X: Geometry Y:	
<u>Site:</u> Part Lot 23 Ot	tawa ON		Database: RSC
RSC ID: RA No: RSC Type: Curr Property Use: Ministry District: Filing Date: Date Ack: Date Returned: Restoration Type: Soil Type: Criteria: CPU Issued Sect 1686: Asmt Roll No: Prop ID No (PIN): Property Municipal Add Mailing Address: Latitude & Latitude:	Ottawa 07/05/01 08/14/01 Generic Medium/Fine Res/parkland + Nonpotable	Cert Date: Cert Prop Use No: Intended Prop Use: Qual Person Name: Stratified (Y/N): N Audit (Y/N): Entire Leg Prop. (Y/N): Accuracy Estimate: Telephone: Fax: Email:	
Latitude & Latitude: UTM Coordinates: Consultant: Legal Desc: Measurement Method: Applicable Standards: RSC PDF:	DST Consulting Engineers Inc.		

<u>Site:</u> ULTRAMAR RIVERSIDE DRIVE AT TRANSIT WAY (NEAR POST OFFICE) TANK TRUCK (CARGO) OTTAWA CITY ON

Database: <mark>SPL</mark>

Ref No:	76621	Discharger Report:	
Site No:	- / /	Material Group:	
Incident Dt:	9/22/1992	Health/Env Conseq:	
Year:		Client Type:	
Incident Cause:	TRUCK/TRAILER OVERTURN	Sector Type:	
Incident Event:		Agency Involved:	
Contaminant Code:		Nearest Watercourse:	
Contaminant Name:		Site Address:	
Contaminant Limit 1:		Site District Office:	
Contam Limit Freq 1:		Site Postal Code:	
Contaminant UN No 1:		Site Region:	
Environment Impact:	NOT ANTICIPATED	Site Municipality:	20101
Nature of Impact:		Site Lot:	
Receiving Medium:	LAND	Site Conc:	
Receiving Env:		Northing:	
MOE Response:		Easting:	F.D., FRANCIS WASTE MGT.
Dt MOE Arvl on Scn:		Site Geo Ref Accu:	
MOE Reported Dt:	9/22/1992	Site Map Datum:	
Dt Document Closed:		SAC Action Class:	
Incident Reason:	UNKNOWN	Source Type:	
Site Name:			
Site County/District:			
Site Geo Ref Meth:			
Incident Summary:	ULTRAMAR GASOLINE TANKER - U	NKNOWN QUANTITY GAS	FROM MOTOR TO ROAD.
Contaminant Qty:			

<u>Site:</u> Clean Water Works Inc. Ottawa ON

Ref No:	6517-B3EKFG	Discharger Report:	
Site No:	NA	Material Group:	
Incident Dt:	2018/08/03	Health/Env Conseq:	2 - Minor Environment
Year:		Client Type:	Corporation
Incident Cause:		Sector Type:	Miscellaneous Industrial
Incident Event:	Leak/Break	Agency Involved:	
Contaminant Code:	15	Nearest Watercourse:	
Contaminant Name:	HYDRAULIC OIL	Site Address:	
Contaminant Limit 1:		Site District Office:	Ottawa
Contam Limit Freq 1:		Site Postal Code:	
Contaminant UN No 1:	n/a	Site Region:	Eastern
Environment Impact:		Site Municipality:	Ottawa
Nature of Impact:		Site Lot:	
Receiving Medium:		Site Conc:	
Receiving Env:	Land	Northing:	
MOE Response:	No	Easting:	
Dt MOE Arvl on Scn:		Site Geo Ref Accu:	
MOE Reported Dt:	2018/08/07	Site Map Datum:	
Dt Document Closed:	2018/09/04	SAC Action Class:	Land Spills
Incident Reason:	Equipment Failure	Source Type:	Motor Vehicle
Site Name:	20 Marie Curie Drive (University of C	Ottawa) <unofficial></unofficial>	
Site County/District:			
Site Geo Ref Meth:			
Incident Summary:	Ottawa 25L of hydraulic oil to grnd		
Contaminant Qty:	25 L		
•			

Appendix: Database Descriptions

Environmental Risk Information Services (ERIS) can search the following databases. The extent of historical information varies with each database and current information is determined by what is publicly available to ERIS at the time of update. Note: Databases denoted with " * " indicates that the database will no longer be updated. See the individual database description for more information.

Abandoned Aggregate Inventory: Provincial AAGR The MAAP Program maintains a database of abandoned pits and guarries. Please note that the database is only referenced by lot and concession and city/town location. The database provides information regarding the location, type, size, land use, status and general comments.* Government Publication Date: Sept 2002*

Provincial Aggregate Inventory: AGR The Ontario Ministry of Natural Resources maintains a database of all active pits and quarries. The database provides information regarding the registered owner/operator, location name, operation type, approval type, and maximum annual tonnage. Government Publication Date: Up to Sep 2019

Provincial Abandoned Mine Information System: AMIS The Abandoned Mines Information System contains data on known abandoned and inactive mines located on both Crown and privately held lands. The information was provided by the Ministry of Northern Development and Mines (MNDM), with the following disclaimer: "the database provided has been compiled from various sources, and the Ministry of Northern Development and Mines makes no representation and takes no responsibility that such information is accurate, current or complete". Reported information includes official mine name, status, background information, mine start/end date, primary commodity, mine features, hazards and remediation.

Anderson's Waste Disposal Sites:

Government Publication Date: 1860s-Present

Government Publication Date: 1999-Jan 31, 2020

Aboveground Storage Tanks:

Historical listing of aboveground storage tanks made available by the Department of Natural Resources and Forestry. Includes tanks used to hold water or petroleum. This dataset has been retired as of September 25, 2014 and will no longer be updated. Government Publication Date: May 31, 2014

Automobile Wrecking & Supplies:

Borehole: Provincial BORE A borehole is the generalized term for any narrow shaft drilled in the ground, either vertically or horizontally. The information here includes geotechnical investigations or environmental site assessments, mineral exploration, or as a pilot hole for installing piers or underground utilities. Information is from many sources such as the Ministry of Transportation (MTO) boreholes from engineering reports and projects from the 1950 to 1990's in Southern Ontario. Boreholes from the Ontario Geological Survey (OGS) including The Urban Geology Analysis Information System (UGAIS) and the York Peel Durham Toronto (YPDT) database of the Conservation Authority Moraine Coalition. This database will include fields such as location, stratigraphy,

depth, elevation, year drilled, etc. For all water well data or oil and gas well data for Ontario please refer to WWIS and OOGW. Government Publication Date: 1875-Jul 2018

Government Publication Date: 1800-Oct 2018

The information provided in this database was collected by examining various historical documents which aimed to characterize the likely position of former waste disposal sites from 1860 to present. The research initiative behind the creation of this database was to identify those sites that are missing from the Ontario MOE Waste Disposal Site Inventory, as well as to provide revisions and corrections to the positions and descriptions of sites currently listed in the MOE inventory. In addition to historic waste disposal facilities, the database also identifies certain auto wreckers and scrap yards that have been extrapolated from documentary sources. Please note that the data is not warranted to be complete, exhaustive or authoritative. The information was collected for research purposes only.

Provincial AST

This database provides an inventory of known locations that are involved in the scrap metal, automobile wrecking/recycling, and automobile parts & supplies industry. Information is provided on the company name, location and business type.

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ANDR

Private

Private

AUWR

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Certificates of Approval:

Chemical Register:

Dry Cleaning Facilities:

Government Publication Date: Feb 28, 2017

distribute chemicals. The production of these chemical substances may involve one or more chemical reactions and/or chemical separation processes (i.e. fractionation, solvent extraction, crystallization, etc.).

Compressed Natural Gas Stations: Canada has a network of public access compressed natural gas (CNG) refuelling stations. These stations dispense natural gas in compressed form at 3,000 pounds per square inch (psi), the pressure which is allowed within the current Canadian codes and standards. The majority of natural gas refuelling is located at existing retail gasoline that have a separate refuelling island for natural gas. This list of stations is made available by the

Government Publication Date: Dec 2012 - Feb 2020

Compliance and Convictions:

Certificates of Property Use:

Drill Hole Database:

77

Inventory of Coal Gasification Plants and Coal Tar Sites:

or use coal tar and other related tars. Detailed information is available and includes: facility type, size, land use, information on adjoining properties, soil condition, site operators/occupants, site description, potential environmental impacts and historic maps available. This was a one-time inventory.* Government Publication Date: Apr 1987 and Nov 1988*

have been found guilty of environmental offenses in Ontario courts of law. Government Publication Date: 1989-Dec 2019

This is a subset taken from Ontario's Environmental Registry (EBR) database. It will include all CPU's on the registry such as (EPA s. 168.6) -Certificate of Property Use.

The Ontario Drill Hole Database contains information on more than 113,000 percussion, overburden, sonic and diamond drill holes from assessment files on record with the department of Mines and Minerals. Please note that limited data is available for southern Ontario, as it was the last area to be completed. The database was created when surveys submitted to the Ministry were converted in the Assessment File Research Image Database (AFRI) project. However, the degree of accuracy (coordinates) as to the exact location of drill holes is dependent upon the source document submitted to the MNDM. Levels of accuracy used to locate holes are: centering on the mining claim; a sketch of the mining claim; a 1:50,000 map; a detailed company map; or from submitted a "Report of Work".

Government Publication Date: 1886 - Sep 2019

ground or surface water, provides potable water supplies, or stores, transports or disposes of waste, must have a Certificate of Approval before it can operate lawfully. Fields include approval number, business name, address, approval date, approval type and status. This database will no longer be updated, as CofA's have been replaced by either Environmental Activity and Sector Registry (EASR) or Environmental Compliance Approval (ECA). Please refer to those individual databases for any information after Oct.31, 2011. Government Publication Date: 1985-Oct 30, 2011*

List of dry cleaning facilities made available by Environment and Climate Change Canada. Environment and Climate Change Canada's Tetrachloroethylene (Use in Dry Cleaning and Reporting Requirements) Regulations (SOR/2003-79) are intended to reduce releases of tetrachloroethylene to the environment from dry cleaning facilities. Government Publication Date: Jan 2004-Dec 2017

This database contains the following types of approvals: Air & Noise, Industrial Sewage, Municipal & Private Sewage, Waste Management Systems and Renewable Energy Approvals. The MOE in Ontario states that any facility that releases emissions to the atmosphere, discharges contaminants to

Commercial Fuel Oil Tanks: CFOT Locations of commercial underground fuel oil tanks. This is not a comprehensive or complete inventory of commercial fuel tanks in the province; this listing is a copy of records of registered commercial underground fuel oil tanks obtained under Access to Public Information.

Note that the following types of tanks do not require registration: waste oil tanks in apartments, office buildings, residences, etc.; aboveground gas or diesel tanks. Records are not verified for accuracy or completeness.

This database includes information from both a one time study conducted in 1992 and private source and is a listing of facilities that manufacture or Government Publication Date: 1999-Jan 31, 2020

Canadian Natural Gas Vehicle Alliance.

This inventory includes both the "Inventory of Coal Gasification Plant Waste Sites in Ontario-April 1987" and the Inventory of Industrial Sites Producing or Using Coal Tar and Related Tars in Ontario-November 1988) collected by the MOE. It identifies industrial sites that produced and continue to produce

This database summarizes the fines and convictions handed down by the Ontario courts beginning in 1989. Companies and individuals named here

Government Publication Date: 1994-Apr 30, 2020

Provincial

Federal

CDRY

CA

Provincial

Private

CNG

COAL

CONV

CPU

CHEM

Provincial

Private

Provincial

Provincial

Provincial

DRI

Order No: 20200610241

Provincial

EASR

EBR

FCA

EHS

FIIS

EMHE

EPAR

Provincial

Provincial

Federal

Private

Federal

Provincial

Provincial

Environmental Activity and Sector Registry:

activities aren't subject to the EASR may apply for an ECA (Environmental Compliance Approval), Please see our ECA database. Government Publication Date: Oct 2011-Apr 30, 2020

The Environmental Registry lists proposals, decisions and exceptions regarding policies, Acts, instruments, or regulations that could significantly affect the environment. Through the Registry, thirteen provincial ministries notify the public of upcoming proposals and invite their comments. For example, if a local business is requesting a permit, license, or certificate of approval to release substances into the air or water; these are notified on the registry. Data includes: Approval for discharge into the natural environment other than water (i.e. Air) - EPA s. 9, Approval for sewage works - OWRA s. 53(1), and EPA s. 27 - Approval for a waste disposal site. For information regarding Permit to Take Water (PTTW), Certificate of Property Use (CPU) and (ORD) Orders please refer to those individual databases.

operation can be applied. The EASR is currently available for: heating systems, standby power systems and automotive refinishing. Businesses whose

Government Publication Date: 1994-Apr 30, 2020

Environmental Compliance Approval:

Environmental Registry:

On October 31, 2011, a smarter, faster environmental approvals system came into effect in Ontario. In the past, a business had to apply for multiple approvals (known as certificates of approval) for individual processes and pieces of equipment. Today, a business either registers itself, or applies for a single approval, depending on the types of activities it conducts. Businesses whose activities aren't subject to the EASR may apply for an ECA. A single ECA addresses all of a business's emissions, discharges and wastes. Separate approvals for air, noise and waste are no longer required. This database will also include Renewable Energy Approvals. For certificates of approval prior to Nov 1st, 2011, please refer to the CA database. For all Waste Disposal Sites please refer to the WDS database. Government Publication Date: Oct 2011-Apr 30, 2020

EEM The Environmental Effects Monitoring program assesses the effects of effluent from industrial or other sources on fish, fish habitat and human usage of fisheries resources. Since 1992, pulp and paper mills have been required to conduct EEM studies under the Pulp and Paper Effluent Regulations. This database provides information on the mill name, geographical location and sub-lethal toxicity data.

Government Publication Date: 1992-2007*

Environmental Effects Monitoring:

ERIS Historical Searches:

ERIS has compiled a database of all environmental risk reports completed since March 1999. Available fields for this database include: site location,

Profile" page.

Government Publication Date: 1999-Jan 31, 2020 Environmental Issues Inventory System:

The Environmental Issues Inventory System was developed through the implementation of the Environmental Issues and Remediation Plan. This plan was established to determine the location and severity of contaminated sites on inhabited First Nation reserves, and where necessary, to remediate those that posed a risk to health and safety; and to prevent future environmental problems. The EIIS provides information on the reserve under investigation, inventory number, name of site, environmental issue, site action (Remediation, Site Assessment), and date investigation completed. Government Publication Date: 1992-2001*

date of report, type of report, and search radius. As per all other databases, the ERIS database can be referenced on both the map and "Statistical

Emergency Management Historical Event:

List of locations of historical occurrences of emergency events, including those assigned to the Ministry of Natural Resources by Order-In-Council (OIC) under the Emergency Management and Civil Protection Act, as well as events where MNR provided requested emergency response assistance. Many of these events will have involved community evacuations, significant structural loss, and/or involvement of MNR emergency response staff. These events fall into one of ten (10) type categories: Dam Failure; Drought / Low Water; Erosion; Flood; Forest Fire; Soil and Bedrock Instability; Petroleum Resource Center Event, EMO Requested Assistance, Continuity of Operations Event, Other Requested Assistance. EMHE record details are reproduced by ERIS under License with the Ontario Ministry of Natural Resources © Queen's Printer for Ontario, 2017. Government Publication Date: Dec 31, 2016

Environmental Penalty Annual Report:

This database contains data from Ontario's annual environmental penalty report published by the Ministry of the Environment and Climate Change. These reports provide information on environmental penalties for land or water violations issued to companies in one of the nine industrial sectors covered by the Municipal Industrial Strategy for Abatement (MISA) regulations.

Government Publication Date: Jan 1. 2011 - Dec 31. 2019

List of Expired Fuels Safety Facilities:

in the province; this listing is a copy of previously registered tanks and facilities obtained under Access to Public Information. Includes private fuel outlets, bulk plants, fuel oil tanks, gasoline stations, marinas, propane filling stations, liquid fuel tanks, piping systems, etc; includes tanks which have been removed from the ground. Notes: registration was not required for private fuel underground/aboveground storage tanks prior to January 1990, nor for furnace oil tanks prior to May 1, 2002; registration is not required for waste oil tanks in apartments, office buildings, residences, etc., or aboveground gas or diesel tanks. Records are not verified for accuracy or completeness.

List of facilities and tanks for which there was once a fuel registration. This is not a comprehensive or complete inventory of expired tanks/tank facilities

Government Publication Date: Feb 28, 2017

Federal Convictions: Environment Canada maintains a database referred to as the "Environmental Registry" that details prosecutions under the Canadian Environmental Protection Act (CEPA) and the Fisheries Act (FA). Information is provided on the company name, location, charge date, offence and penalty.

Government Publication Date: 1988-Jun 2007

Contaminated Sites on Federal Land:

The Federal Contaminated Sites Inventory includes information on known federal contaminated sites under the custodianship of departments, agencies and consolidated Crown corporations as well as those that are being or have been investigated to determine whether they have contamination arising from past use that could pose a risk to human health or the environment. The inventory also includes non-federal contaminated sites for which the Government of Canada has accepted some or all financial responsibility. It does not include sites where contamination has been caused by, and which are under the control of, enterprise Crown corporations, private individuals, firms or other levels of government. Includes fire training sites and sites at which Per- and Polyfluoroalkyl Substances (PFAS) are a concern. Government Publication Date: Jun 2000-Apr 2020

Fisheries & Oceans Fuel Tanks:

Fisheries & Oceans Canada maintains an inventory of aboveground & underground fuel storage tanks located on Fisheries & Oceans property or controlled by DFO. Our inventory provides information on the site name, location, tank owner, tank operator, facility type, storage tank location, tank contents & capacity, and date of tank installation.

Government Publication Date: 1964-Sep 2019

Federal Identification Registry for Storage Tank Systems (FIRSTS):

A list of federally regulated Storage tanks from the Federal Identification Registry for Storage Tank Systems (FIRSTS). FIRSTS is Environment and Climate Change Canada's database of storage tank systems subject to the Storage Tank for Petroleum Products and Allied Petroleum Products Regulations. The main objective of the Regulations is to prevent soil and groundwater contamination from storage tank systems located on federal and aboriginal lands. Storage tank systems that do not have a valid identification number displayed in a readily visible location on or near the storage tank system may be refused product delivery.

Government Publication Date: May 31, 2018

Fuel Storage Tank:

List of registered private and retail fuel storage tanks. This is not a comprehensive or complete inventory of private and retail fuel storage tanks in the province; this listing is a copy of registered private and retail fuel storage tanks, obtained under Access to Public Information. Notes: registration was not required for private fuel underground/aboveground storage tanks prior to January 1990, nor for furnace oil tanks prior to May 1, 2002; registration is not required for waste oil tanks in apartments, office buildings, residences, etc., or aboveground gas or diesel tanks. Records are not verified for accuracy or completeness.

Government Publication Date: Feb 28, 2017

Fuel Storage Tank - Historic:

The Fuels Safety Branch of the Ontario Ministry of Consumer and Commercial Relations maintained a database of all registered private fuel storage tanks. Public records of private fuel storage tanks are only available since the registration became effective in September 1989. This information is now collected by the Technical Standards and Safety Authority.

Government Publication Date: Pre-Jan 2010*

Ontario Regulation 347 Waste Generators Summary:

Regulation 347 of the Ontario EPA defines a waste generation site as any site, equipment and/or operation involved in the production, collection, handling and/or storage of regulated wastes. A generator of regulated waste is required to register the waste generation site and each waste produced, collected, handled, or stored at the site. This database contains the registration number, company name and address of registered generators including the types of hazardous wastes generated. It includes data on waste generating facilities such as: drycleaners, waste treatment and disposal facilities, machine shops, electric power distribution etc. This information is a summary of all years from 1986 including the most currently available data. Some records may contain, within the company name, the phrase "See & Use..." followed by a series of letters and numbers. This occurs when one company is amalgamated with or taken over by another registered company. The number listed as "See & Use", refers to the new ownership and the other identification number refers to the original ownership. This phrase serves as a link between the 2 companies until operations have been fully transferred.

Government Publication Date: 1986-Jan 31, 2020

Provincial

EXP

FCON

FCS

FOFT

FRST

FST

FSTH

Federal

Federal

Federal

Federal

Provincial

Provincial

Provincial

GEN

Order No: 20200610241

List of greenhouse gas emissions from large facilities made available by Environment Canada. Greenhouse gas emissions in kilotonnes of carbon dioxide equivalents (kt CO2 eq). Government Publication Date: 2013-Dec 2017

TSSA Historic Incidents: HINC List of historic incidences of spills and leaks of diesel, fuel oil, gasoline, natural gas, propane, and hydrogen recorded by the TSSA in their previous incident tracking system. The TSSA's Fuels Safety Program administers the Technical Standards & Safety Act 2000, providing fuel-related safety services associated with the safe transportation, storage, handling and use of fuels such as gasoline, diesel, propane, natural gas and hydrogen. Under this Act, the TSSA regulates fuel suppliers, storage facilities, transport trucks, pipelines, contractors and equipment or appliances that use fuels. Records are not verified for accuracy or completeness. This is not a comprehensive or complete inventory of historical fuel spills and leaks in the province. This listing is a copy of the data captured at one moment in time and is hence limited by the record date provided here. Government Publication Date: 2006-June 2009*

Indian & Northern Affairs Fuel Tanks: IAFT The Department of Indian & Northern Affairs Canada (INAC) maintains an inventory of aboveground & underground fuel storage tanks located on both federal and crown land. Our inventory provides information on the reserve name, location, facility type, site/facility name, tank type, material & ID number, tank contents & capacity, and date of tank installation.

Government Publication Date: 1950-Aug 2003*

Fuel Oil Spills and Leaks:

Listing of spills and leaks of diesel, fuel oil, gasoline, natural gas, propane, and hydrogen reported to the Spills Action Centre (SAC). This is not a comprehensive or complete inventory of fuel-related leaks, spills, and incidents in the province; this listing in a copy of incidents reported to the SAC, obtained under Access to Public Information. Includes incidents from fuel-related hazards such as spills, fires, and explosions. Records are not verified for accuracy or completeness. Government Publication Date: Feb 28, 2017

Landfill Inventory Management Ontario: LIMO The Landfill Inventory Management Ontario (LIMO) database is updated every year, as the ministry compiles new and updated information. The inventory will include small and large landfills. Additionally, each year the ministry will request operators of the larger landfills complete a landfill data collection form that will be used to update LIMO and will include the following information from the previous operating year. This will include additional information such as estimated amount of total waste received, landfill capacity, estimated total remaining landfill capacity, fill rates, engineering designs, reporting and monitoring details, size of location, service area, approved waste types, leachate of site treatment, contaminant attenuation zone and more. The small landfills will include information such as site owner, site location and certificate of approval # and status.

Government Publication Date: Feb 28, 2019

Canadian Mine Locations:

Mineral Occurrences:

80

This information is collected from the Canadian & American Mines Handbook. The Mines database is a national database that provides over 290 listings on mines (listed as public companies) dealing primarily with precious metals and hard rocks. Listed are mines that are currently in operation, closed, suspended, or are still being developed (advanced projects). Their locations are provided as geographic coordinates (x, y and/or longitude, latitude). As of 2002, data pertaining to Canadian smelters and refineries has been appended to this database. Government Publication Date: 1998-2009*

In the early 70's, the Ministry of Northern Development and Mines created an inventory of approximately 19,000 mineral occurrences in Ontario, in regard to metallic and industrial minerals, as well as some information on building stones and aggregate deposits. Please note that the "Horizontal Positional Accuracy" is approximately +/- 200 m. Many reference elements for each record were derived from field sketches using pace or chain/tape measurements against claim posts or topographic features in the area. The primary limiting factor for the level of positional accuracy is the scale of the source material. The testing of horizontal accuracy of the source materials was accomplished by comparing the plan metric (X and Y) coordinates of that point with the coordinates of the same point as defined from a source of higher accuracy. Government Publication Date: 1846-Jan 2020

National Analysis of Trends in Emergencies System (NATES): In 1974 Environment Canada established the National Analysis of Trends in Emergencies System (NATES) database, for the voluntary reporting of significant spill incidents. The data was to be used to assist in directing the work of the emergencies program. NATES ran from 1974 to 1994. Extensive information is available within this database including company names, place where the spill occurred, date of spill, cause, reason and source of spill, damage incurred, and amount, concentration, and volume of materials released. Government Publication Date: 1974-1994*

Greenhouse Gas Emissions from Large Facilities:

Provincial

Provincial

Federal

NATE

GHG

Provincial

Federal

Federal

Provincial

INC

MINE

MNR

Private



National Defense & Canadian Forces Fuel Tanks:

DND lands. Our inventory provides information on the base name, location, tank type & capacity, tank contents, tank class, date of tank installation, date tank last used, and status of tank as of May 2001. This database will no longer be updated due to the new National Security protocols which have

prohibited any release of this database. Government Publication Date: Up to May 2001*

Sectoral Regulation or specific regulation/act. Government Publication Date: Dec 31, 2018

National Defense & Canadian Forces Spills:

The Department of National Defense and the Canadian Forces maintains an inventory of spills to land and water. All spill sites have been classified under the "Transportation of Dangerous Goods Act - 1992". Our inventory provides information on the facility name, location, spill ID #, spill date, type of spill, as well as the quantity of substance spilled & recovered. Government Publication Date: Mar 1999-Apr 2018

The Ministry of the Environment provides information about non-compliant discharges of contaminants to air and water that exceed legal allowable limits, from regulated industrial and municipal facilities. A reported non-compliance failure may be in regard to a Control Order, Certificate of Approval,

The Department of National Defence and the Canadian Forces maintains an inventory of waste disposal sites located on DND lands. Where available, our inventory provides information on the base name, location, type of waste received, area of site, depth of site, year site opened/closed and status. Government Publication Date: 2001-Apr 2007*

Federal National Energy Board Pipeline Incidents: **NEBI** Locations of pipeline incidents from 2008 to present, made available by the Canada Energy Regulator (CER) - previously the National Energy Board (NEB). Includes incidents reported under the Onshore Pipeline Regulations and the Processing Plant Regulations related to pipelines under federal jurisdiction, does not include incident data related to pipelines under provincial or territorial jurisdiction. Government Publication Date: 2008-Mar 31, 2020

the National Energy Board. Data is provided regarding the operator, well name, well ID No./UWI, status, classification, well depth, spud and release

National Defence & Canadian Forces Waste Disposal Sites:

National Energy Board Wells:

date.

Government Publication Date: 1920-Feb 2003*

National Environmental Emergencies System (NEES): In 2000, the Emergencies program implemented NEES, a reporting system for spills of hazardous substances. For the most part, this system only

captured data from the Atlantic Provinces, some from Quebec and Ontario and a portion from British Columbia. Data for Alberta, Saskatchewan, Manitoba and the Territories was not captured. However, NEES is also a repository for previous Environment Canada spill datasets. NEES is composed of the historic datasets ' or Trends ' which dates from approximately 1974 to present. NEES Trends is a compilation of historic databases, which were merged and includes data from NATES (National Analysis of Trends in Emergencies System), ARTS (Atlantic Regional Trends System), and NEES. In 2001, the Emergencies Program determined that variations in reporting regimes and requirements between federal and provincial agencies made national spill reporting and trend analysis difficult to achieve. As a consequence, the department has focused efforts on capturing data on spills of substances which fall under its legislative authority only (CEPA and FA). As such, the NEES database will be decommissioned in December 2004.

Government Publication Date: 1974-2003*

National PCB Inventory: NPCB Environment Canada's National PCB inventory includes information on in-use PCB containing equipment in Canada including federal, provincial and private facilities. Federal out-of-service PCB containing equipment and PCB waste owned by the federal government or by federally regulated industries such as airlines, railway companies, broadcasting companies, telephone and telecommunications companies, pipeline companies, etc. are also listed. Although it is not Environment Canada's mandate to collect data on non-federal PCB waste, the National PCB inventory includes some information on provincial and private PCB waste and storage sites. Some addresses provided may be Head Office addresses and are not necessarily the location of where the waste is being used or stored. Government Publication Date: 1988-2008*

National Pollutant Release Inventory: Federal Environment Canada has defined the National Pollutant Release Inventory ("NPRI") as a federal government initiative designed to collect comprehensive national data regarding releases to air, water, or land, and waste transfers for recycling for more than 300 listed substances. Government Publication Date: 1993-May 2017

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Provincial

NCPL

NDFT

NDSP

Federal The Department of National Defense and the Canadian Forces maintains an inventory of all aboveground & underground fuel storage tanks located on

Federal

Federal

Federal

NDWD

The NEBW database contains information on onshore & offshore oil and gas wells that are outside provincial jurisdiction(s) and are thereby regulated by

NEBP

NEES

Federal

Federal

NPRI

Order No: 20200610241

OGWE

OOGW

OPCB

Provincial

Provincial

Private

Private

Provincial

PAP

PES

PINC

PRT

PTTW

Provincial

Provincial

Provincial The Fuels Safety Branch of the Ontario Ministry of Consumer and Commercial Relations maintained a database of all registered private fuel storage

Provincial

Oil and Gas Wells: The Nickle's Energy Group (publisher of the Daily Oil Bulletin) collects information on drilling activity including operator and well statistics. The well

is updated on a monthly basis. More information is available at www.nickles.com. Government Publication Date: 1988-Feb 29, 2020

Ontario Oil and Gas Wells:

drilled in Ontario. The OGSR Library has over 20,000+ wells in their database. Information available for all wells in the ERIS database include well owner/operator, location, permit issue date, and well cap date, license No., status, depth and the primary target (rock unit) of the well being drilled. All

Inventory of PCB Storage Sites:

Canadian Pulp and Paper:

Pesticide Register:

Government Publication Date: 1800-Jun 2019

The Ontario Ministry of Environment, Waste Management Branch, maintains an inventory of PCB storage sites within the province. Ontario Regulation 11/82 (Waste Management - PCB) and Regulation 347 (Generator Waste Management) under the Ontario EPA requires the registration of inactive PCB storage equipment and/or disposal sites of PCB waste with the Ontario Ministry of Environment. This database contains information on: 1) waste quantities; 2) major and minor sites storing liquid or solid waste; and 3) a waste storage inventory. Government Publication Date: 1987-Oct 2004; 2012-Dec 2013

information database includes name, location, class, status and depth. The main Nickle's database is updated on a daily basis, however, this database

In 1998, the MNR handed over to the Ontario Oil, Gas and Salt Resources Corporation, the responsibility of maintaining a database of oil and gas wells

geology/stratigraphy table information, plus all water table information is also provide for each well record.

Orders: ORD This is a subset taken from Ontario's Environmental Registry (EBR) database. It will include all Orders on the registry such as (EPA s. 17) - Order for remedial work, (EPA s. 18) - Order for preventative measures, (EPA s. 43) - Order for removal of waste and restoration of site, (EPA s. 44) - Order for conformity with Act for waste disposal sites, (EPA s. 136) - Order for performance of environmental measures. Government Publication Date: 1994-Apr 30, 2020

This information is part of the Pulp and Paper Canada Directory. The Directory provides a comprehensive listing of the locations of pulp and paper mills and the products that they produce. Government Publication Date: 1999, 2002, 2004, 2005, 2009-2014

Parks Canada Fuel Storage Tanks: Federal PCFT Canadian Heritage maintains an inventory of known fuel storage tanks operated by Parks Canada, in both National Parks and at National Historic Sites. The database details information on site name, location, tank install/removal date, capacity, fuel type, facility type, tank design and owner/operator. Government Publication Date: 1920-Jan 2005*

The Ontario Ministry of the Environment and Climate Change maintains a database of licensed operators and vendors of registered pesticides. Government Publication Date: 1988 - Apr 2020

Pipeline Incidents: List of pipeline incidents (strikes, leaks, spills). This is not a comprehensive or complete inventory of pipeline incidents in the province; this listing in an historical copy of records previously obtained under Access to Public Information. Records are not verified for accuracy or completeness. Government Publication Date: Feb 28, 2017

Private and Retail Fuel Storage Tanks:

Authority (TSSA). Government Publication Date: 1989-1996*

Permit to Take Water:

This is a subset taken from Ontario's Environmental Registry (EBR) database. It will include all PTTW's on the registry such as OWRA s. 34 - Permit to take water.

tanks and licensed retail fuel outlets. This database includes an inventory of locations that have gasoline, oil, waste oil, natural gas and/or propane storage tanks on their property. The MCCR no longer collects this information. This information is now collected by the Technical Standards and Safety

Government Publication Date: 1994-Apr 30, 2020
Ontario Regulation 347 Waste Receivers Summary: Part V of the Ontario Environmental Protection Act ("EPA") regulates the disposal of regulated waste through an operating waste management system

or a waste disposal site operated or used pursuant to the terms and conditions of a Certificate of Approval or a Provisional Certificate of Approval. Regulation 347 of the Ontario EPA defines a waste receiving site as any site or facility to which waste is transferred by a waste carrier. A receiver of regulated waste is required to register the waste receiving facility. This database represents registered receivers of regulated wastes, identified by registration number, company name and address, and includes receivers of waste such as: landfills, incinerators, transfer stations, PCB storage sites, sludge farms and water pollution control plants. This information is a summary of all years from 1986 including the most currently available data. Government Publication Date: 1986-2016

The Record of Site Condition (RSC) is part of the Ministry of the Environment's Brownfields Environmental Site Registry. Protection from environmental cleanup orders for property owners is contingent upon documentation known as a record of site condition (RSC) being filed in the Environmental Site Registry. In order to file an RSC, the property must have been properly assessed and shown to meet the soil, sediment and groundwater standards appropriate for the use (such as residential) proposed to take place on the property. The Record of Site Condition Regulation (O. Reg. 153/04) details requirements related to site assessment and clean up. RSCs filed after July 1, 2011 will also be included as part of the new (O.Reg. 511/09).

Government Publication Date: 1997-Sept 2001, Oct 2004-Mar 2020

Retail Fuel Storage Tanks: This database includes an inventory of retail fuel outlet locations (including marinas) that have on their property gasoline, oil, waste oil, natural gas and /

Scott's Manufacturing Directory:

Government Publication Date: 1999-Jan 31, 2020

or propane storage tanks.

Ontario Spills:

Record of Site Condition:

the most comprehensive database of Canadian manufacturers available. Information concerning a company's address, plant size, and main products are included in this database. Government Publication Date: 1992-Mar 2011*

Scott's Directories is a data bank containing information on over 200,000 manufacturers across Canada. Even though Scott's listings are voluntary, it is

This database identifies information such as location (approximate), type and quantity of contaminant, date of spill, environmental impact, cause, nature of impact, etc. Information from 1988-2002 was part of the ORIS (Occurrence Reporting Information System). The SAC (Spills Action Centre) handles

Government Publication Date: 1988-Nov 2019

Wastewater Discharger Registration Database:

Government Publication Date: 1990-Dec 31, 2017

Information under this heading is combination of the following 2 programs. The Municipal/Industrial Strategy for Abatement (MISA) division of the Ontario Ministry of Environment maintained a database of all direct dischargers of toxic pollutants within nine sectors including: Electric Power Generation; Mining; Petroleum Refining; Organic Chemicals; Inorganic Chemicals; Pulp & Paper; Metal Casting; Iron & Steel; and Quarries. All

sampling information is now collected and stored within the Sample Result Data Store (SRDS).

all spills reported in Ontario. Regulations for spills in Ontario are part of the MOE's Environmental Protection Act, Part X.

The information provided in this database was collected by examining various historical documents, which identified the location of former storage tanks, containing substances such as fuel, water, gas, oil, and other various types of miscellaneous products. Information is available in regard to business operating at tank site, tank location, permit year, permit & installation type, no. of tanks installed & configuration and tank capacity. Data contained within this database pertains only to the city of Toronto and is not warranted to be complete, exhaustive or authoritative. The information was collected for research purposes only.

Government Publication Date: 1915-1953*

Anderson's Storage Tanks:

83

Transport Canada Fuel Storage Tanks:

List of fuel storage tanks currently or previously owned or operated by Transport Canada. This inventory also includes tanks on The Pickering Lands, which refers to 7,530 hectares (18,600 acres) of land in Pickering, Markham, and Uxbridge owned by the Government of Canada since 1972; properties on this land has been leased by the government since 1975, and falls under the Site Management Policy of Transport Canada, but is administered by Public Works and Government Services Canada. This inventory provides information on the site name, location, tank age, capacity and fuel type. Government Publication Date: 1970-Aug 2018

Provincial

RFC

RSC

RST

SCT

SPL

SRDS

TANK

TCFT

Private

Provincial

Private

Provincial

Provincial

Private

Federal

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Waste Disposal Sites - MOE CA Inventory:

The Ontario Ministry of Environment, Waste Management Branch, maintains an inventory of known open (active or inactive) and closed disposal sites in the Province of Ontario. Active sites maintain a Certificate of Approval, are approved to receive and are receiving waste. Inactive sites maintain Certificate(s) of Approval but are not receiving waste. Closed sites are not receiving waste. The data contained within this database was compiled from the MOE's Certificate of Approval database. Locations of these sites may be cross-referenced to the Anderson database described under ERIS's Private Source Database section, by the CA number. All new Environmental Compliance Approvals handed out after Oct 31, 2011 for Waste Disposal Sites will still be found in this database.

Government Publication Date: Oct 2011-Apr 30, 2020

Waste Disposal Sites - MOE 1991 Historical Approval Inventory:

In June 1991, the Ontario Ministry of Environment, Waste Management Branch, published the "June 1991 Waste Disposal Site Inventory", of all known active and closed waste disposal sites as of October 30st, 1990. For each "active" site as of October 31st 1990, information is provided on site location, site/CA number, waste type, site status and site classification. For each "closed" site as of October 31st 1990, information is provided on site location, site/CA number, closure date and site classification. Locations of these sites may be cross-referenced to the Anderson database described under ERIS's Private Source Database section, by the CA number.

Government Publication Date: Up to Oct 1990*

Water Well Information System:

This database describes locations and characteristics of water wells found within Ontario in accordance with Regulation 903. It includes such information as coordinates, construction date, well depth, primary and secondary use, pump rate, static water level, well status, etc. Also included are detailed stratigraphy information, approximate depth to bedrock and the approximate depth to the water table.

Government Publication Date: Feb 28, 2019

Variances for Abandonment of Underground Storage Tanks:

Listing of variances granted for storage tank abandonment. This is not a comprehensive or complete inventory of tank abandonment variances in the province; this listing is a copy of tank abandonment variance records previously obtained under Access to Public Information. In Ontario, registered underground storage tanks must be removed within two years of disuse; if removal of a tank is not feasible, an application may be sought for a variance from this code requirement.

Records are not verified for accuracy or completeness.

Government Publication Date: Feb 28, 2017

Provincial

Provincial

Provincial

VAR

WDS

WDSH

Provincial

WWIS

Definitions

Database Descriptions: This section provides a detailed explanation for each database including: source, information available, time coverage, and acronyms used. They are listed in alphabetic order.

Detail Report: This is the section of the report which provides the most detail for each individual record. Records are summarized by location, starting with the project property followed by records in closest proximity.

Distance: The distance value is the distance between plotted points, not necessarily the distance between the sites' boundaries. All values are an approximation.

Direction: The direction value is the compass direction of the site in respect to the project property and/or center point of the report.

Elevation: The elevation value is taken from the location at which the records for the site address have been plotted. All values are an approximation. Source: Google Elevation API.

Executive Summary: This portion of the report is divided into 3 sections:

'Report Summary'- Displays a chart indicating how many records fall on the project property and, within the report search radii.

'Site Report Summary'-Project Property'- This section lists all the records which fall on the project property. For more details, see the 'Detail Report' section.

'Site Report Summary-Surrounding Properties'- This section summarizes all records on adjacent properties, listing them in order of proximity from the project property. For more details, see the 'Detail Report' section.

<u>Map Key:</u> The map key number is assigned according to closest proximity from the project property. Map Key numbers always start at #1. The project property will always have a map key of '1' if records are available. If there is a number in brackets beside the main number, this will indicate the number of records on that specific property. If there is no number in brackets, there is only one record for that property.

The symbol and colour used indicates 'elevation': the red inverted triangle will dictate 'ERIS Sites with Lower Elevation', the yellow triangle will dictate 'ERIS Sites with Higher Elevation' and the orange square will dictate 'ERIS Sites with Same Elevation.'

<u>Unplottables:</u> These are records that could not be mapped due to various reasons, including limited geographic information. These records may or may not be in your study area, and are included as reference.

Appendix E

Ministry of Environment, Conservation and Parks – Freedom of Information (FOI) Request



Freedom of Information and Protection of Privacy Office 40 St. Clair Avenue West, 12th Floor Toronto ON M4V 1M2 Telephone 416 314-4075

Instructions

Use this form to request records that are in the Ministry's files on environmental concerns related to properties. Our fax number is 416 314-4285.

For Ministry Use C	Dnly									
FOI Request Number					Date Request Received (yyyy/mm/dd)					
Fee Paid				Cheque		A/MC		Cash/Mon	ey Order	
				WCR	IEB	EAA	EMR		СВ] SDW
1. Requester Data										
Last Name				First Name				Middle Init	ial	
Lopers					Luke				Α	
Title					Company Name					
Principal					Lopers & Associates					
Mailing Address									_	
Unit Number	Street Numb	er	Street Name	е					PO Box	
	30		Lansfield	Way						
City/Town					Province				Postal Co	de
Ottawa					Ontario				K2G 3V	8
Email Address					Telephone N	lumber			Fax Numb	er
Luke@Lopers.ca					613 327-90	073	ext.			
Project/Reference Nu	mber	Signatu	re of Reques	ter	1/ /	1				
LOP20-002				In	1 3					
2. Request Parame	eters									
Municipal Address (Municipal add	ress mai	ndatory for ci	ties, towns or i	regions)					
Unit Number	Street Numb	er	Street Name	е					PO Box	
	729		Ridgewoo	od Avenue						
Lot Number			Concession		Geographic -	Township				
City/Town/Village					Province				Postal Co	de
Ottawa					Ontario				K1V 6M	18
Present Property										
1. Owner							Date	of Owne	ership (yyyy	/mm/dd)
11684663 Canada Inc.							2019	9/11/06	-)	
Tenant (if applical	ble)									
Previous Property										
1. Owner							Date	of Owne	ership (yyyy	//mm/dd)
561266 Ontari	o Inc.						1983	3/12/16	<u>,</u>	
Tenant (if applical	ble)									

3. Search Parameters	
Search Parameters	Specify Year(s) Requested
Environmental concerns (General correspondence, occurrence reports, abatement)	All
Orders	All
Spills	All
Investigations/prosecutions Owner and tenant information must be provided	All
Waste Generator number/classes	All

Files older than 2 years may require \$60.00 retrieval cost. There is no guarantee that records responsive to your request will be located.

4. Environmental Compliance Approvals/Certificates of Approval

- -

Environmental Compliance Approvals/Certificates of Approval	SD	Specify Year(s) Requested
air - emissions	\checkmark	
renewable energy	\checkmark	
water - mains, treatment, ground level, standpipes & elevated storage, pumping stations (local & booster)	 Image: A start of the start of	
sewage - sanitary, storm, treatment, stormwater, leachate & leachate treatment & sewage pump stations	 Image: A start of the start of	
waste water - industrial discharge	\checkmark	
waste sites - disposal, landfill sites, transfer stations, processing sites, incinerator sites	<	
waste systems - haulers: sewage, non-hazardous & hazardous waste, mobile waste processing units, PCB destruction	 Image: A start of the start of	

Proponent information must be provided and Environmental Compliance Approval/Certificate of Approval number(s) (if known). 1985 and prior records are searched manually. Search fees in excess of \$300.00 may be incurred, depending on the types and years to be searched. Specify Approval number(s) (if known). If supporting documents are also required, mark SD box and specify type e.g. maps, plans, reports, etc.

LOPERS & ASSOCIATES

Appendix F

Technical Standards and Safety Authority Correspondence

From:	Public Information Services
To:	Luke Lopers
Subject:	RE: TSSA Records Search Request - Environmental Research
Date:	June 12, 2020 11:29:30 AM

Records Found Hello,

Thank you for your request for confirmation of public information.

Context	Address	City	Province	Postal	Status
		-		Code	
FS GASOLINE STATION - FULL	753 RIDGEWOOD AV	OTTAWA	ON	K1V 6M8	EXPIRED
SERVE					
FS PROPANE CYLR HANDLING	753 RIDGEWOOD AV	OTTAWA	ON	K1V 6M8	EXPIRED
FACILITY					
FS Liquid Fuel Tank	753 RIDGEWOOD AV	OTTAWA	ON	K1V 6M8	EXPIRED
FS Liquid Fuel Tank	753 RIDGEWOOD AV	OTTAWA	ON	K1V 6M8	EXPIRED
FS Liquid Fuel Tank	753 RIDGEWOOD AV	OTTAWA	ON	K1V 6M8	EXPIRED
	Context FS GASOLINE STATION - FULL SERVE FS PROPANE CYLR HANDLING FACILITY FS Liquid Fuel Tank FS Liquid Fuel Tank FS Liquid Fuel Tank	ContextAddressFS GASOLINE STATION - FULL SERVE753 RIDGEWOOD AVFS PROPANE CYLR HANDLING FACILITY753 RIDGEWOOD AVFS Liquid Fuel Tank753 RIDGEWOOD AV	ContextAddressCityFS GASOLINE STATION - FULL SERVE753 RIDGEWOOD AVOTTAWAFS PROPANE CYLR HANDLING FACILITY753 RIDGEWOOD AVOTTAWAFS Liquid Fuel Tank753 RIDGEWOOD AVOTTAWA	ContextAddressCityProvinceFS GASOLINE STATION - FULL SERVE753 RIDGEWOOD AV 753 RIDGEWOOD AVOTTAWAONFS PROPANE CYLR HANDLING FACILITY753 RIDGEWOOD AVOTTAWAONFS Liquid Fuel Tank753 RIDGEWOOD AVOTTAWAON	ContextAddressCityProvincePostal CodeFS GASOLINE STATION - FULL SERVE753 RIDGEWOOD AVOTTAWAONK1V 6M8FS PROPANE CYLR HANDLING FACILITY753 RIDGEWOOD AVOTTAWAONK1V 6M8FS Liquid Fuel Tank753 RIDGEWOOD AVOTTAWAONK1V 6M8

For a further search in our archives please complete our release of public information form found at https://www.tssa.org/en/about-tssa/release-of-publicinformation.aspx? mid =392 and email the completed form to publicinformationservices@tssa.org or through mail along with a fee of \$56.50 (including HST) per location. The fee is payable with credit card (Visa or MasterCard) or with a Cheque made payable to TSSA.

Although TSSA believes the information provided pursuant to your request is accurate, please note that TSSA does not warrant this information in any way whatsoever

Kind regards,

Gaya

From: Luke Lopers <Luke@lopers.ca>

Sent: June 12, 2020 11:02 AM

To: Public Information Services <publicinformationservices@tssa.org> Subject: TSSA Records Search Request - Environmental Research

[CAUTION]: This email originated outside the organisation.

Please do not click links or open attachments unless you recognise the source of this email and know the content is safe.

Good morning,

Could you please search the TSSA database for records of fuel storage tanks, spills, incidents or infractions for the following addresses located in the City of Ottawa, ON:

• 729, 753, 757, 758, 770 Ridgewood Avenue

• 2865, 2909, 2951, 2975 Riverside Drive

Thank you for your time,

Luke Lopers, P.Eng.

Principal LOPERS & ASSOCIATES Cell: 613-327-9073 Email: Luke@Lopers.ca 30 Lansfield Way, Ottawa, Ontario K2G 3V8

This electronic message and any attached documents are intended only for the named recipients. This communication from the Technical Standards and Safety Authority may contain information that is privileged, confidential or otherwise protected from disclosure and it must not be disclosed, copied, forwarded or distributed without authorization. If you have received this message in error, please notify the sender immediately and delete the original message.

Appendix G

City of Ottawa Historic Land Use Inventory (HLUI)



File Number: D06-03-20-0086

July 28, 2020

Luke Lopers Lopers & Associates 30 Lansfield Way, Ottawa

Sent via email [Luke@lopers.ca]

Dear Mr. Lopers,

Re: Information Request 729 Ridgewood Drive, Ottawa, Ontario ("Subject Property")

Internal Department Circulation

The Planning, Infrastructure and Economic Development Department has the following information in response to your request for information regarding the Subject Property:

• No information was returned on the Subject Property from Departmental circulation.

Search of Historical Land Use Inventory

This acknowledges receipt of the signed Disclaimer regarding your request for information from the City's Historical Land Use Inventory (HLUI 2005) database for the Subject Property.

A search of the HLUI database revealed the following information:

• There are five activities associated with the Subject Property; Activity Numbers 5134, 5948, 8806, 9038, and 12114.

The HLUI database was also searched for activity associated with properties located within 250m of the Subject Property. The search revealed the following:

• There are six activities associated with four properties located within 250m of the Subject Property; Activity Numbers 836, 6842, 10221, 12114, 13732, and 14215.

Shaping our future together Ensemble, formons notre avenir City of Ottawa Planning, Infrastructure and Economic Development Department

110 Laurier Avenue West, 4th Floor Ottawa, ON K1P 1J1 Tel: (613) 580-2424 ext. 21690 Fax: (613) 560-6006 www.ottawa.ca Ville d'Ottawa Services de la planification, de l'infrastructure et du développement économique

110, avenue Laurier Ouest, 4e étage Ottawa (Ontario) K1P 1J1 Tél.: (613) 580-2424 ext. 21690 Téléc: (613) 560-6006 www.ottawa.ca Please note that certain activities have been identified to have a PIN Certainty of "2". This identifier acknowledges that there is some uncertainty about the exact location of the land use activity and that the activity may or may not have been located on the property. All database entries with a PIN Certainty of "2" require independent verification as to their precise location.

A **site map** and **table** have been included to show the location of the Subject Property as well as the location of all the activities noted above, including the HLUI database's location of the Activity Numbers with a PIN Certainty of "2".

Additional information may be obtained by contacting:

Ontario's Environmental Registry

The Environmental Registry found at <u>http://www.ebr.gov.on.ca/ERS-WEB-External/</u> contains "public notices" about environmental matters being proposed by all government ministries covered by the Environmental Bill of Rights. The public notices may contain information about proposed new laws, regulations, policies and programs or about proposals to change or eliminate existing ones. By using keys words i.e. name of proponent/owner and the address one can ascertain if there is any information on the proponent and address under the following categories: Ministry, keywords, notice types, Notice Status, Acts, Instruments and published date (all years).

The Ontario Land Registry Office

Registration of real property is recorded in the Ontario Land Registry Office through the Land Titles Act or the Registry Act. Documents relating to title and other agreements that may affect your property are available to the public for a fee. It is recommended that a property search at the Land Registry Office be included in any investigation as to the historic use of your property. The City of Ottawa cannot comment on any documents to which it is not a party.

Court House 161 Elgin Street 4th Floor Ottawa ON K2P 2K1 Tel: (613) 239-1230 Fax: (613) 239-1422

Please note, as per the HLUI Disclaimer, that the information contained in the HLUI database has been compiled from publicly available records and other sources of information. The HLUI may contain erroneous information given that the records used as sources of information may be flawed. For instance, changes in municipal addresses over time may introduce error. Accordingly, all information from the HLUI database is provided on an "as is" basis with no representation or warranty by the City with respect to the information's accuracy or exhaustiveness in responding to the request.

Furthermore, the HLUI database and the results of this search in no way confirm the presence or absence of contamination or pollution of any kind. This information is provided on the assumption that it will not be relied upon by any person for any purpose whatsoever. The City of Ottawa denies all liability to any persons attempting to rely on any information provided from the HLUI database.

Please note that in responding to your request, the City of Ottawa does not guarantee or comment on the environmental condition of the Subject Property. You may wish to contact the Ontario Ministry of Environment and Climate Change for additional information.

If you have any further questions or comments, please contact Insert Your Name at 613-580-2424 ext. Insert Your Extension or HLUI@ottawa.ca

Sincerely,

GochAN

Sarah Ezzio

Per:

Michael Boughton, MCIP, RPP Senior Planner Development Review East Planning Services Planning, Infrastructure and Economic Development Department

cc: File no. D06-03-20-0086



Prepared By: Sarah Ezzio

Scale: 1

1 : 2176



Area	Associated HLUI Activities	Associated HLUI Activities with a PIN Certainty of "2" *
Subject Property	5134, 5948, 8806, 9038	12114
1	6842	14215
2	10221	
3		12114, 13732
4	836	

*This identifier acknowledges that there is some uncertainty about the exact location of the land use activity and that the activity may or may not have been located on the property. All database entries with a PIN Certainty of "2" require independent verification as to their precise location.



Historical Land Use Inventory Subject Property Activity Numbers

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Report:

Run On: 28 Jul 2020 at: 10:19:25

RPTC_OT_DEV0122

Study Year 1998	PIN 040710125		Multi-NAIC Y	Multiple Activities Y
Activity ID:	12114	Multiple PINS:	Y	
PIN Certainty:	2	Previous Activity ID(s) :	5563	
Related PINS:	150960000			
Name: Address:	RIVERSIDE SUPERTES 753 RIDGEWOOD AVEN	ST GAS STATION		
Facility Type: Comments 1: Comments 2:	Gasoline Service Station	S		
Generator Number: Storage Tanks:				
HL References 1: HL References 2:	M.1960, M.1970, M.1980; S	C98		
HL References 3:				
NAICS	SIC			
447190 6 811199 6 447110 6	333 333 333			

Company Name

McCloskey Supertest Gas Station

Riverside Supertest Gas Station

Year of Operation

c. 1980-1998

c. 1970



1998 040710125 Y Y	Study Year 1998	PIN 040710125	Multi-NAIC Y	Multiple Activities
--------------------	--------------------	-------------------------	-----------------	---------------------

Activity ID:	5134	Multiple PINS:	Ν
PIN Certainty:	1	Previous Activity ID(s) :	
Related PINS:	040710125		
Name: Address: Facility Type:	EDWARDS UPHOLSTE 747 RIDGEWOOD AVER	RY NUE, OTTAWA	
Comments 1: Comments 2:	Other Machinery, Equipr	nent and Supplies, wholesale	
Generator Number			
Storage Tanks:			
HL References 1:			
HL References 2:			
HL References 3:	2001 Employment Survey		
NAICS	SIC		
811420	0		
Company Name			Year of Operation

EDWARDS UPHOLSTERY

c. 2001

RPTC_OT_DEV0122

28 Jul 2020 at: 10:19:25

Report: Run On:



Study Year	PIN 040710125	Multi-NAIC	Multiple Activities
1000	040110120	ľ	ľ

Activity ID:	5948	Multiple PINS:	Ν
PIN Certainty:	1	Previous Activity ID(s) :	5657
Related PINS:	040710125		
Name: Address:	FOTOMAT 752 RIDGEWOOD AV	ENUE, OTTAWA	
Comments 1: Comments 2:	Platemaking, Typesett	ing and Bindery Industry	
Generator Number Storage Tanks:	:		
HL References 1: HL References 2:	M.1960, M.1970, M.1980		
HL References 3:			
NAICS	SIC		
323120 812921	282 282		
Company Name			Year of Operation

Fotomat

Year of Operation

Report: Run On:

c. 1980

RPTC_OT_DEV0122

28 Jul 2020 at: 10:19:25



Study Year	PIN	Multi-NAIC	Multiple Activities
1998	040710125	Y	Y

Activity ID:	8	806	Multiple PINS:	Ν
PIN Certainty:	1		Previous Activity ID(s) :	
Related PINS:		040710125		
Name:		MOONEY'S BAY ELECTR	RONICS INC.	
Address:		733 RIDGEWOOD AVEN	JE, OTTAWA	
Facility Type:		Electrical and Electronic N	Achinery, Equipment and Sup	plies, Wholesale
Comments 1:				
Comments 2:				
Generator Number	:			
Storage Tanks:				
HL References 1:				
HL References 2:				
HL References 3:		2001 Employment Survey		
NAICS	SIC			
443120	0			

Company Name

MOONEY'S BAY ELECTRONICS INC.

Year of Operation

Report: Run On:

c. 2001

RPTC_OT_DEV0122

28 Jul 2020 at: 10:19:25



Study Year	PIN 040710125	Multi-NAIC Y	Multiple Activities

Activity ID:	9038	Multiple PINS:	Ν
PIN Certainty:	1	Previous Activity ID(s)	:
Related PINS:	040710125		
Name: Address: Facility Type: Comments 1: Comments 2:	MCCLOSKEY 753 RIDGEW Motor Vehicles	'S RICK SERVICE LIMITED OOD AVENUE, s, Wholesale	
Generator Number	:		
Storage Tanks:			
HL References 1:			
HL References 3:	2005 Select Pho	one	
NAICS	SIC		
811111	0		
Company Name			Year of Operation
MCCLOSKEY'S RIC	K SERVICE LIMITED		c. 2001

MCCLOSKEY'S RICK SERVICE LIMITED

c. 2005

RPTC_OT_DEV0122

28 Jul 2020 at: 10:19:25

Report: Run On:



Historical Land Use Inventory Adjacent Properties within 250m Area & Activity Numbers

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Historical Land Use Inventory Area 1 Activity Numbers

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Report:

Run On:

RPTC_OT_DEV0122 28 Jul 2020 at: 10:20:56

Study Year 1998	PIN 040710122		Multi-NAIC Y	Multiple Activities Y
Activity ID:	14215	Multiple PINS:	N	
PIN Certainty	2	Previous Activity ID(s) :	5044	
Related PINS:	040710122	1011040710411 13 12(0)1		
Name:	TRANS TECH AUTO SE	RVICE		
Address:	2805 RIVERSIDE DRIVE	e, ottawa		
Facility Type:	Gasoline Service Station	s		
Comments 1:				
Comments 2:				
Generator Number:				
Storage Tanks:				
HL References 1:	M.1960, M.1970, M.1980; S	C98		
HL References 2:				
HL References 3:				
NAICS S	IC			
811119 6	35			
811112 6	35			
447190 6	33			
811199 6	33			
447110 6	33			

Company Name	Year of Operation
Bob Tait's Service Station	c. 1960
Trans Tech Auto Service	c. 1998
Gerald McConnell Service Station	c. 1970
Beaver Service Centre	c. 1980

811121

635



Study Year	PIN	Multi-NAIC	Multiple Activities
1998	040710122	Y	Y

Activity ID:	6842	Multiple PINS:	Ν
PIN Certainty:	1	Previous Activity ID(s) :	5043
Related PINS:	040710106		
Name:	PETRO-CANADA		
Address:	2801 RIVERSIDE D	ORIVE, OTTAWA	
Facility Type:	Gasoline Service St	ations	
Comments 1:			
Comments 2:			
Generator Number:	:		
Storage Tanks:			
HL References 1:	M.1960, M.1970, M.19	980; SC98	
HL References 2:			
HL References 3:	2005 Property Assess	ment	
NAICS	SIC		
811112	635		
447110	0		
811121	635		
447110	633		
447190	633		
811119	635		
811199	633		
447190	0		
811111	0		

Company Name	Year of Operation
PETRO-CANADA	c. 2005
Bud Tierney Service Station	c. 1970
HOGGS BACK SERVICE CENTRE	c. 2005
Bud Stephen's Garage	c. 1960
HOGGS BACK SERVICE CENTRE	c. 2001
Hog's Back Service Centre	c. 1998
H & D Lytle Gulf Station	c. 1980

RPTC_OT_DEV0122

28 Jul 2020 at: 10:20:56

Report: Run On:



Historical Land Use Inventory Area 2 Activity Numbers

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Report:

Run On:

28 Jul 2020 at: 10:23:14

RPTC_OT_DEV0122

Study Year 2005	P 04	IN 0710124	Multi-NAIC N	Multiple Activities N
Activity ID:	10221	Multiple PINS:	N	
PIN Certainty:	1	Previous Activity ID	(s) :	
Related PINS:	040710124			
Name: Address: Facility Type: Comments 1: Comments 2:	OTTAWA-CARL 824 BROOKFIE Elementary and BROOKFIELD F	ETON DISTRICT SCHOOL BO LD ROAD, OTTAWA Secondary Education HIGH SCHOOL	ARD - BROOKFIELD HIGH	SCHOOL
Generator Numbe Storage Tanks: HL References 1: HL References 2:	r: ON0375207			
HL References 3:	2000 PID			
NAICS 611110	SIC 0			
Company Name)		Year of Opera	ition
OTTAWA-CARLETC SCHOOL	IN DISTRICT SCHOOL B	OARD - BROOKFIELD HIGH	c. 2003	
OTTAWA-CARLETC SCHOOL	ON DISTRICT SCHOOL B	OARD - BROOKFIELD HIGH	c. 2000	

c. 2005

OTTAWA-CARLETON DISTRICT SCHOOL BOARD - BROOKFIELD HIGH SCHOOL



Historical Land Use Inventory Area 3 Activity Numbers

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Report:

Run On: 28 Jul 2020 at: 10:24:44

RPTC_OT_DEV0122

Study Year 1998		PIN 150960000	Multi-NAIC Y	Multiple Activities Y
Activity ID:	12114	Multiple PINS:	Y	
PIN Certainty:	2	Previous Activity ID(s) :	5563	
Related PINS:	150960000			
Name: Address:	RIVERSIDE S	SUPERTEST GAS STATION		
Facility Type: Comments 1:	Gasoline Serv	vice Stations		
Generator Numbe Storage Tanks:	r:			
HL References 1: HL References 2:	M.1960, M.1970	D, M.1980; SC98		
HL References 3:				
NAICS	SIC			
447190	633			
811199 447110	633 633			

Company Name

McCloskey Supertest Gas Station

Riverside Supertest Gas Station

Year of Operation

c. 1980-1998

c. 1970



Study Year	PIN	Multi-NAIC	Multiple Activities
1998	150960000	Y	

Activity ID:	13732	Multiple PINS:	Ν					
PIN Certainty:	2	Previous Activity ID(s) :	5562					
Related PINS:	150960000							
Name:	VAILS FABRIC CARE LIMITED							
Address:	749 RIDGEWOOD AVEN	749 RIDGEWOOD AVENUE, OTTAWA						
Facility Type:	lity Type: Laundries and Cleaners							
Comments 1:								
Comments 2:								
Generator Number:								
Storage Tanks:								
HL References 1: M.1960, M.1970, M.1980								
HL References 2:								
HL References 3:								
NAICS	SIC							
812320	972							
561740	972							
812310	972							

812310 812330

972

Company Name

Company Name	Year of Operation
Parker Clean	c. 1980
Vails Fabric Care Ltd.	c. 1970

RPTC_OT_DEV0122

28 Jul 2020 at: 10:24:44

Report: Run On:



Historical Land Use Inventory Area 4 Activity Numbers

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Report:

Run On:

RPTC_OT_DEV0122

28 Jul 2020 at: 10:25:34

Study Year 2005	PIN 040730008		Multi-NAIC N	Multiple Activities N
Activity ID:	836	Multiple PINS:	N	
PIN Certainty:	1	Previous Activity ID	(s) :	
Related PINS:	040730008			
Name: Address: Facility Type:	A-1 CARPET RI 2692 SPRINGL	EPAIRS & INSTALL AND DRIVE, Cleaners		
Comments 1: Comments 2:				
Generator Number:				
Storage Tanks:				
HL References 1:				
HL References 3:	2005 Select Phon	e		
NAICS	SIC			
561740	0			
Company Name			Year of Opera	tion
A-1 CARPET REPAIR	S & INSTALL		c. 2005	

Appendix H

Aerial Photographs



1933 Aerial Photograph

Aerial Photographs



1956 Aerial Photograph

Aerial Photographs



1965 Aerial Photograph

Aerial Photographs



1976 Aerial Photograph

Aerial Photographs


1991 Aerial Photograph

Aerial Photographs

729 Ridgewood Avenue, Ottawa



1999 Aerial Photograph

Aerial Photographs

729 Ridgewood Avenue, Ottawa



2008 Aerial Photograph

Aerial Photographs

729 Ridgewood Avenue, Ottawa



2017 Aerial Photograph

Aerial Photographs

Appendix I

Topographic Map



Topographic Map

Appendix J

Photographic Log



Photograph 1: View of Phase One Property looking north from Ridgewood Drive. View shows the north (right side) and south (left side) commercial buildings at the Property. The automotive service garage is visible in this view.



Photograph 2: View of Phase One Property looking northwest southeast corner of the Property. View shows former retail fuel outlet and automotive garage area in the southeast portion of the Site which has been backfilled with granular fill. The commercial buildings are also visible in this view.



Photograph 3: View of the north side of the Phase One Property looking southeast. View shows the north side of the north commercial building, including the loading dock for the former grocery store and the natural gas meters for the commercial units.



Photograph 4: View of the west side of the Phase One Property looking south. View shows the west side of the north commercial building.



Photograph 5: View of the east side of the Phase One Property looking south. View shows the east sides of the north and south commercial buildings.



Photograph 6: View of the interior of the automotive service garage in the south unit of the south commercial building. View shows a tire balancing machine and some equipment storage.



Photograph 7: View of the interior of the automotive service garage in the south unit of the south commercial building. View shows an aboveground hoist and some equipment storage.



Photograph 8: View of interior of the automotive service garage in the south unit of the south commercial building. View shows two of the aboveground hoists and general operations inside the garage.



Photograph 9: View of the basement electrical room in the north commercial plaza building.



Photograph 10: View of the refrigeration equipment in the penthouse level of the former grocery store in the northwest portion of the north commercial plaza building.



Photograph 11: View of interior of the former grocery store in the northwest portion of the north commercial plaza building.



Photograph 12: View of the interior (typical) of one of the commercial units in the north commercial plaza building.

Appendix K

Qualifications of Assessors



PROFILE

Mr. Lopers is an environmental engineer with over 12 years of experience in environmental engineering specializing in due diligence investigations. Mr. Lopers has extensive experience in Phase I and II Environmental Site Assessments; environmental remediation, and investigations; record of site condition submissions; asset inventory, designated substance surveys and abatement projects; environmental expertise on legal issues; and coordination of various monitoring programs (groundwater, surface water, air).

Mr. Lopers has participated in various Property Condition and Building Envelope mandates at various residential and commercial properties throughout Ontario.

Mr. Lopers has a strong commitment to health and safety, having experience leading a regional health and safety committee as a certified employee representative. Mr. Lopers has extensive training including OSHA 40-hour HAZWOPER, ASP Health and Safety on Construction Sites in Quebec, Ontario Working at Heights, Emergency First Aid/CPR and WHMIS.

CONTACT

EMAIL: Luke@Lopers.ca

LUKE LOPERS Principal LOPERS & ASSOCIATES

EDUCATION

University of Waterloo, B.A.Sc., Honours Environmental Engineering Management Science Option Designation - 2002 - 2008

PROFESSIONAL EXPERIENCE

Lopers & Associates, Principal, Project Manager, Senior Environmental Engineer

Ottawa, Ontario - 2020–Present

Responsible for the management, coordination, supervision, completion and delivery of Phase I/1 and II/2 Environmental Site Assessments, Environmental Remediation Programs, Environmental litigation support, Designated Substance Surveys, scope of work development, cost estimates and proposals

GHD Limited, Project Manager, Senior Environmental Engineer Ottawa, Ontario - 2013–2020

Responsible for the management, senior technical review, coordination, supervision, completion and delivery of Phase I/1 and II/2 Environmental Site Assessments, Environmental Remediation Programs, Environmental litigation support, Designated Substance Surveys, scope of work development, cost estimates and proposals Office Safety Captain and Joint Health and Safety Committee team leader

Paterson Group Inc., Project Manager, Environmental Engineer Ottawa, Ontario - 2009–2013

Responsible for supervision, completion and review for Phase I/1 and II/2 Environmental Site Assessments, Environmental Remediation Programs, Designated Substance Surveys

NEXT Environmental Inc., Site Investigation Staff

Burnaby, British Columbia - 2008–2009 Responsible for fieldwork and reporting for Stage/Phase I and II Environmental Site Assessments, Environmental Remediation Programs

PROFESSIONAL DESIGNATIONS

Licensed Professional Engineer (P.Eng.) with Professional Engineers Ontario (PEO) since 2012

Qualified Person (QP), Environmental Site Assessments with Ontario Ministry of the Environment, Conservation and Parks

PROJECT EXPERIENCE

Environmental Site Assessments

Project Engineer/Manager Phase 1 Environmental Site Assessment | Various Clients | Ontario, Quebec and British Columbia | 2006-2020

Project Engineer/Manager Phase Two Environmental Site Assessments | Various Clients | Various Locations | 2008-2020

Project Manager Phase One, Phase Two Environmental Site Assessments, Environmental Delineation Quality Assurance Program | Costco Wholesale | Ottawa, ON | 2014-2019

Environmental Remediation Programs

Project Engineer Underground Fuel Storage Tank Removals and Environmental Remediation Programs in Vicinity of Active Underground Services | Ottawa, ON | 2010, 2012 Project Engineer/Manager for Phase I Environmental Site Assessments in support of acquisition/divestiture/regulatory requirements for various properties in Ontario, Quebec and British Columbia, including the following:

- Canadian Tire Retail Store and Gas Bar, CTR 417 2560 Princess Street, Kingston, Ontario
- Former Automotive Dealership and Service Garage, North Vancouver, British Columbia
- Former Philips Cable Plant, Brockville, Ontario
- Former Cornwall Cotton Mill, Cornwall, Ontario
- Retail Fuel Outlet and Automotive Service Garage, Ottawa, Ontario
- Jack Garland Airport Land, North Bay, Ontario
- Various Commercial/Residential Properties, Ontario and British Columbia
- Various Residential Properties, Ontario, Quebec and British Columbia
- Rochester Heights (811, 818 Gladstone Avenue), Ottawa, Ontario

Project Engineer/Manager for the following field investigation and/or regulatory reporting requirements for Phase II ESAs and other Site Investigations:

- Proposed Canadian Tire Development, CTR 693P Terry Fox Drive at Eagleson Road, Stittsville, Ontario
- Former Retail/Private Fuel Outlets, Ottawa/North Bay/Vancouver, Canada
- Operational/Former Industrial Facilities, Ottawa/Cornwall/Sarnia/Brockville/Gananoque, Ontario
- Existing Dry Cleaning Facilities, Ottawa/Arnprior, Ontario
 - Automotive Service Garages, Ottawa/Vancouver, Canada
- Various Commercial/Residential Properties, Eastern Ontario
- Tetrachloroethylene Groundwater Plume, Commercial Property, Ottawa, Ontario
- Rochester Heights (811, 818 Gladstone Avenue), Ottawa, Ontario

Project Manager for the completion of a Phase One ESA for the potential acquisition of a commercial property. Upon discovery of APECs at the Site and significant data gaps in previous investigations, completed a Phase Two ESA to evaluate soil and groundwater quality at the Site. Further oversight of original owner's environmental consultants was completed to ensure adequate delineation and characterization of a dNAPL groundwater plume at the Site, present at significant depths in shale bedrock, which originated as a result of a former on-Site dry-cleaning operation.

Project Engineer for removal of underground heating oil storage tanks adjacent to residential buildings. Completed excavation supervision of contaminated soil around and below active underground services, including hydro, water and natural gas infrastructure at residential properties. Activities included oversight of removal of petroleum, impacted soil, and field screening and collection of confirmatory soil and groundwater samples for petroleum hydrocarbon analysis. Prepared Phase I, II and III Environmental Site Assessment reports. Project Engineer Retail Fuel Outlet Decommissioning and Remediation | Ottawa, ON | 2012

Project Engineer/Manager Former Fuel Outlet Investigation and Remediation | Merrickville, ON | 2016-2017

Record of Site Conditions

Project Manager/Engineer Residential Redevelopment | Environmental Remediation Program and Record of Site Condition Submission | Ottawa | 2015

Project Manager/Engineer Industrial Development | Environmental Assessment and Record of Site Condition Submission | Township of Edwardsburgh/Cardinal | 2015

Excess Soil Management

Project Engineer/Manager Management of Excess Soil | CTREL, Brigil, Ottawa Community Housing Corporation | Ottawa and Pembroke, Ontario | 2016, 2018

Designated Substance Surveys

Project Manager

Designated Substance Surveys and Hazardous Building Materials Assessment | Ottawa, Pembroke, Southeastern Ontario | 2010-2020

Environmental Litigation Support

Project Manager, Field Engineer, Expert Witness Ottawa, Ontario | 2014-2020 Project Engineer for UST removal and confirmatory soil sampling at former ESSO gas station in Ottawa, Ontario. Activities included oversight of removal of USTs and product lines, oversight of removal of petroleum-impacted soil and groundwater encountered and backfilling operations, and field screening and collection of confirmatory soil and groundwater samples for petroleum hydrocarbon analysis.

Project Engineer for confirmatory soil and groundwater sampling following UST removal at former Shell gas station. Activities included oversight of removal of petroleum-impacted soil, pumping of groundwater encountered and backfilling operations, and field screening and collection of confirmatory soil and groundwater samples for petroleum hydrocarbon analysis. Additional borehole/monitoring well drilling also completed.

Project Manager for delineation of soil contamination and groundwater sampling for a former automotive garage and gas station property in Ottawa, Ontario. Presented and implemented remedial action plan to remediate on-Site contamination. Directed staff in collection of post remediation confirmatory soil and groundwater samples for contaminants of concern. Prepared remediation closure report and record of site condition supporting documentation for submission to the Ministry of the Environment and Climate Change.

Project Manager for environmental assessments for a proposed industrial business park, in an existing industrial area within the Township of Edwardsburgh/Cardinal, Ontario. Prepared environmental assessment reports and record of site condition supporting documentation for submission to the Ministry of the Environment and Climate Change.

Project Engineer/Manager for sampling, analytical testing, development of soil management plans and monitoring during removal of excess soil generated as part of construction activities, including the following properties/facilities:

- Rochester Heights (811, 818 Gladstone Avenue), Ottawa, Ontario
- Residential redevelopment, 121 Parkdale Avenue, Ottawa, Ontario
- CTR 079, 1104 Pembroke Street East, Pembroke, Ontario
- CTR 297, 2010 Ogilvie Road, Ottawa, Ontario

Project Manager for asbestos containing material (ACM) surveys, designated substance surveys (DSSs), Hazardous Building Materials Assessments (HBMAs) or mould assessments at the following sites:

- DSSs at various municipal facilities for the City of Pembroke, Pembroke, Ontario. Preparation of Asbestos Management Plan.
- HBMAs at various institutional buildings for the Catholic District School Board of Eastern Ontario, Southeastern Ontario.
- DSSs and ACM surveys at various residential, buildings (dwellings and apartment buildings) for private residential clients, Ottawa, Ontario.
- DSS and abatement oversight during demolition, residential buildings (townhouses) for Ottawa Community Housing Corporation, 818 Gladstone Avenue, Ottawa, Ontario.

Project Manager, Field Engineer and Expert Witness for a fuel spill, remediation program, groundwater monitoring program and litigation review for redevelopment of a residential property adjacent to a central heating plant at an institutional facility.

Education

BEng Geological Engineering, École Polytechnique de Montreal, Montreal, Quebec, 1990

MSc Geophysics, University of British Columbia, Vancouver, British Columbia, 1983

BSc Geophysics, Honours, University of British Columbia, Vancouver, British Columbia, 1980

Certifications

Registered as PMP with Project Management Institute since 2012, requalified in 2018

Qualified Person (QP) for Environmental Site Assessments with Ontario Ministry of Environment and Conservation and Parks

Professional Affiliations

Licensed as P.Eng. with the Professional Engineers of Ontario (PEO) since 1994

Licensed as Ing. with l'Ordre des ingénieurs du Québec (OIQ), 1992

Licensed as P.Eng. with NAPEG (NWT and Nunavut), since 2009.

Licensed as P.Eng with Engineers Yukon since 2018

Federal Clearance Level

Secret ID # 95251065

DON PLENDERLEITH

Senior Environmental Engineer and Project Manager

PROFESSIONAL SUMMARY

Mr. Plenderleith has been an environmental engineer for 30 years. From 1990 to 2000 he worked at specialty firms in Montreal and Ottawa where he gained field and reporting experience in site assessment and remediation of retail fuel outlets and railway yards. In 1991 and 1992 he worked on a CIDA sponsored project to assess additional water resource potential in two provinces in Indonesia. He worked for Golder for 19 years on projects in Ottawa, the North and overseas.

His expertise covers all steps in contaminated site management: Phase I, II and III environmental site assessments (ESAs), risk assessments, remedial options evaluations, remedial action plans, tender plans and specifications, remediation project oversight, long-term monitoring and project closure. He has largely concentrated on federal sites since 2002 and was Golder's initial point of contact on the Environmental Standing Offer Agreement with PSPC in the National Capital over that time.

Don led Golder's national client service team for Federal government and was responsible to Golder's management for maintaining strong relations with the federal government. Locally, he provided project management and technical direction of a variety of environmental projects from the Ottawa office. Don mentored several junior professionals. His site portfolio included: military bases, Northern sites, navigational sites, correctional facilities, research labs, commercial buildings and Canadian embassies abroad. On several multi-year projects (Kingston Penitentiary and Connaught Ranges landfill) he directed all steps of site management from initial investigations, through to site closure.

Don is equally experienced at providing strategic and portfolio-level assistance to clients as well as site-specific level work. He has written contaminated sites management plans for several federal Departments. He helped to develop components of the FCSAP project manager's tool kit and has trained federal project managers in its use. He has provided program-level assistance to the FCSAP Secretariat for funding demand forecasting and long-term strategy and risk management. For nine years he led a multi-disciplinary team that performed contaminated site liability peer reviews for the Office of the Auditor General of Canada.

Don completed his engineering degree in French and is licensed to practice in Quebec. He frequently coordinates the French language component at bilingual meetings and workshops.

PROJECT EXPERIENCE – STANDING OFFER MANAGER

Public Services and Procurement Canada, National Capital Region, Environmental Engineering Standing Offer (2002-2019). Don managed Golder's Environmental Standing Offer Agreement (SOA) with PSPC in the National Capital Region from 2002 to 2019. He was the first point of contact with PSPC for new call-ups. He formed project teams from the approved resources and reviewed the work plans under each call-up. He was responsible and accountable for Golder's overall project performance to PSPC.

PROJECT EXPERIENCE – SENIOR PROJECT MANAGER

Environmental Site Assessment, Remediation Planning and Implementation for the Pittsburgh Institution and Kingston Penitentiary, Kingston, Ontario from 2007 Phase I, II, and III and to 2015 - Don was the Senior Project Manager and project reviewer for the **Remediation at Pittsburgh** Phase I, II and III of contaminated sites on two similar projects at these federal Institution and Kingston penitentiaries. Don performed project management and provided technical Penitentiary for PSPC/CSC direction during the full suite of services from site assessment through to near Kingston, Ontario remediation. Federal project management tools, and FCSAP technical tools (GOST) were used to assist with procedural compliance. Don assisted PSPC with the tender specification for both remediation projects and performed on-site supervision during the fast-track remediation work at Pittsburgh. Don also performed senior review of the draft and final reports.

Peer Review and Liability Review of US Steel Site in Hamilton Harbour for PSPC and Transport Canada (July-August 2016)

Contaminated Site Reporting and Review for Department of National Defence Ottawa, Ontario, Canada

Don was the Senior Project Manager for a Peer Review of reports pertaining to the US Steel site on Hamilton Harbour that the Hamilton Port Authority (HPA) was considering purchasing. TC requested the peer review and liability review in its oversight role over the HPA. Don brought a senior expert in at steel industry at Golder onto the project team. With his input some important gaps in the previous site assessments, management plans and liability estimates were identified to TC.

Don has managed several projects for DND's Director General Environment, related to the financial reporting of DND's contaminated sites. He managed the EcoNet validation project in 2006, in which the systems and procedures by which site cost and liability information are input to DND's Contaminated Site database, Econet. Several of DND's major projects being run out of headquarters were reviewed in that exercise. In 2008 he assisted DND by producing the 2008 update of their Contaminated Sites Management Plan (CSMP) for Treasury Board submission. Nine divisional CSMPs were reviewed, summarized and incorporated into the departmental CSMP.

PROGRAM LEVEL WORK – FEDERAL CONTAMINATED SITES

Project Management Tools for Contaminated Sites, Ottawa, Ontario, Canada Mr. Plenderleith developed two of the FCSAP Project Management Tools: Status Reporting and Project Risk Management. He has provided training in the tools to federal project managers country-wide. He has delivered training sessions at RPIC National Contaminated Sites workshops on several occasions on the PM Tools, the Sustainable Development Tool (SDAT), and Guidance Tool for Selection of Technologies Tools (GOST).

Assistance to FCSAP for program-level Risk Management, PWGSC/ECCC Ottawa, Ontario Don has led a team at Golder that provided assistance to the FCSAP Secretariat from 2013 to 2019 in the areas of cost projections for funding demand estimates. He devised a method of projecting the costs of unassessed sites based on closure costs of similar sites. This tool was used to estimate the funding demand for FCSAP Phase III and past Phase III. Don assisted the Secretariat with Long-Term Strategic planning for FSCAP post 2020 when the 15-year program is due to sunset.

Secondments to Federal Departments Mr. Plenderleith has been seconded from Golder to the Department of Foreign Affairs and International Trade (now Global Affairs Canada "GAC") on three occasions to develop their Contaminated Sites Management Plans and to fill in while GAC was staffing their full-time environmental engineer position. Through these secondments he has developed a greater understanding of the role of federal custodians in managing their programs.

PROJECT EXPERIENCE – NORTHERN SITES

Mr. Plenderleith was the project director of Golder's DEW Line Monitoring **DEW Line Site Monitoring,** contract with DND from four years 2015 to 2019. He was responsible for overall **Baffin Region, DND** program quality and liaison with the client and management of Inuit (2015-19)subcontractors. The project was multi-disciplinary, involving geotechnical and environmental components. Mr. Plenderleith has developed a very positive working relationship with the hamlet of Qikiqtarjuag and the Inuit staff from that community, many of whom have returned to work with Golder every year. All Inuit Participation Targets were exceeded. **Tundra Mine Remediation** Don was the Senior project director for Golder's Remediation Monitoring of Monitoring PSPC/INAC Tundra Mine (NWT) for PSPC and INAC. This project is multi-disciplinary (2016 - 2018)

Tundra Mine (NWT) for PSPC and INAC. This project is multi-disciplinary involving surface water and groundwater environmental monitoring and aquatic monitoring for the final stages of the remediation of Tundra Mine. Don has reviewed the monthly and annual monitoring reports produced for the Water Licence. His earlier experience with the RAP for Tundra has been valuable on this project.

Remedial Options Review and Remedial Action Planning Former Water Tanker Base, Inuvik Airport, NWT 2010-12 From 2010 to 2012, Mr. Plenderleith was the technical director for the Phase III ESA detailed site assessment and remediation planning of the former Water Tanker Base at the Inuvik Airport in NWT. The work included determining the contaminants of concern, delineation of contaminated soil and seasonal groundwater areas, and assessing remedial options. The remedial action plan reviewed chemical oxidation and removal & disposal options within the constraints of northern work season, and the distance to a disposal facility. Descriptions, costs, advantages and limitations were provided for several options. GNWT performed the remediation with own forces.

Phase Two Environmental Site Assessment

729 Ridgewood Avenue Ottawa, Ontario

Prepared for: 11684663 Canada Inc.



August 14, 2020

Table of Contents

1.	Exec	utive Summary	1	
2.	Intro	Introduction		
	i.	Site Description	4	
	ii.	Property Ownership	4	
	iii.	Current and Proposed Future Use	4	
	iv.	Applicable Site Condition Standard	5	
3.	Background Information			
	i.	Physical Setting	6	
	ii.	Past Investigations	6	
4.	Scop	pe of Investigation	7	
	i.	Overview of Site Investigation	7	
	ii.	Media Investigation	10	
	iii.	Phase One Conceptual Site Model	11	
	iv.	Deviations from Sampling and Analysis Plan	12	
	v.	Impediments	13	
5.	Inve	stigation Method	13	
	i.	General	13	
	ii.	Drilling	13	
	iii.	Soil Sampling	14	
	iv.	Field Screening Measurements	16	
	v.	Groundwater: Monitoring Well Installation	16	
	vi.	Groundwater: Field Measurement of Water Quality Parameters	17	
	vii.	Groundwater: Sampling	17	
	viii.	Sediment: Sampling	18	
	ix.	Analytical Testing	18	
	x.	Residue Management Procedures	18	
	xi.	Elevation Surveying	18	
	xii.	Quality Assurance and Quality Control Measures	19	
6.	Revi	ew and Evaluation	20	

LOPERS & ASSOCIATES

	i.	Geology	. 20		
	ii.	Groundwater and Elevations and Flow Direction	. 22		
	iii.	Groundwater: Hydraulic Gradients	.24		
	iv.	Fine-Medium Soil Texture	.24		
	v.	Soil Field Screening	.24		
	vi.	Soil Quality	.25		
	vii.	Groundwater Quality	.27		
	viii.	Sediment Quality	. 29		
	ix.	Quality Assurance and Quality Control Results	. 30		
	x.	Phase Two Conceptual Site Model	.31		
7.	Cond	clusions	. 35		
	i.	Signatures	. 37		
8.	Limi	Limitations			
9.	Refe	References			
10.	Appendices				

List of Figures

Figure 1:Key PlanFigure 2:Site Plan

List of Tables

Table 1: Monitoring Well Construction Details	
Table 2: Groundwater Table Elevations Measured on June 23, 2020	23
Table 3: Soil Samples Selected for Laboratory Analysis	25
Table 4: Groundwater Samples Selected for Laboratory Analysis	
Table 5: Soil Analytical Results	Following Text
Table 6: TCLP Analytical Results	Following Text
Table 7: Groundwater Analytical Results	Following Text

List of Appendices

- Appendix A Sampling and Analysis Plan
- Appendix B Underground Utility Locates
- Appendix C Borehole Logs
- Appendix D Certificates of Equipment Calibration
- Appendix E Laboratory Certificates of Analysis
- Appendix F Qualifications of Assessors

1. Executive Summary

Lopers & Associates (Lopers) was retained by 11684663 Canada Inc. (Brigil) to complete a Phase Two Environmental Site Assessment (Phase Two ESA) of the commercial property with Civic address No. 729 Ridgewood Avenue, Ottawa, Ontario ("Phase Two Property", "Property" or "Site").

This Phase Two ESA is being completed as part of due diligence requirements associated with the submission of a Development Application to the City of Ottawa Municipal Planning Department.

Lopers has previously completed a Phase One Environmental Site Assessment (Phase One ESA) (Reference No. LOP20-001A, dated July 27, 2020) for Brigil at the Property. The Phase One ESA identified the presence of four potentially contaminating activities (PCAs) at the Property which were interpreted to represent areas of potential environmental concern (APECs). The presence of a former retail fuel outlet and automotive service garage were identified on the southeast portion of the Phase One Property (APEC #1 / #2). The contaminants of potential concern associated with retail fuelling are petroleum hydrocarbons (PHCs) and benzene, toluene, ethylbenzene and xylenes (BTEX), and metals, since this was an older facility and lead was historically present in gasoline. Based on historical soil analysis in this area of the Property, polycyclic aromatic hydrocarbons (PAH) and volatile organic compounds (VOCs) are also considered contaminants of potential concern associated with the former automotive garage operations. The practice of backfilling following demolition activities was identified on the central-south portion of the Phase One Property (APEC #3). The contaminants of potential concern commonly found in poor environmental quality backfill are PHCs/BTEXs, polycyclic aromatic hydrocarbons (PAHs) and metals. Based on the operations observed at the active automotive service garage (APEC #4), the contaminants of concern associated with this activity are PHCs and BTEXs. A Phase Two ESA was recommended to assess the soil and groundwater quality in the vicinity of the identified APECs.

The scope of work for the Phase Two ESA included drilling seven boreholes at the Phase Two Property. Three of the boreholes were instrumented with groundwater monitoring wells with screens installed in the overburden.

Six soil samples, including one duplicate sample, were submitted for laboratory analysis for a combination of PHCs, BTEXs, volatile organic compounds (VOCs), PAHs, metals and inorganics. One sample was also submitted for toxicity leaching characteristic procedure (TCLP) for waste characterization purposes.

Groundwater sampling was completed of the newly installed monitoring wells and two existing groundwater monitoring wells at the Phase Two Property, which were installed as part of historical investigations. A total of seven groundwater samples, including a duplicate sample and a trip blank, were submitted for laboratory analysis for a combination of PHCs, BTEXs, VOCs, PAHs, metals and inorganics.

The applicable sites standard was determined to be the full depth generic site condition standard, in a non-potable groundwater condition, with course textured soil, for residential property use, as specified in Table 3 of the MECP Soil, Ground Water and Sediment Standards for Use Under Part XV.1 of the Environmental Protection Act, April 15, 2011.

At APEC #3 (placement of fill of unknown quality) the soil samples BH1-20-SS5 and BH1-20-SS5 (Duplicate of BH1-20-SS5), collected from a depth of approximately 3.1-3.7 m BGS, had reported concentrations of PHC F2 range (909 μ g/g and 306 μ g/g vs. 98 μ g/g), Methylnaphthalene (7.61 μ g/g and 2.26 μ g/g vs. 0.99 μ g/g) and reported concentrations of vanadium (101 μ g/g and 104 μ g/g vs. 86 μ g/g). These samples also had respective cobalt concentrations of 20.1 μ g/g and 22.5 μ g/g compared to the site condition standard of 22 μ g/g; since the average concentration of cobalt in these samples is less than the site condition standard, the marginal exceedance in the duplicate standard is not considered to exceed the site condition standard.

At APEC #1 (former retail fuel outlet) the soil sample BH3-20-SS6, collected from a depth of approximately 3.8-4.4 m BGS, had reported concentrations of PHC F1 range (117 μ g/g vs. 55 μ g/g), PHC F2 range (110 μ g/g vs. 98 μ g/g), benzene (3.02 μ g/g vs. 0.21 μ g/g), ethylbenzene (59 μ g/g vs. 2 μ g/g), toluene (73.5 μ g/g vs. 2.3 μ g/g) and xylenes (276 μ g/g vs. 3.1 μ g/g). Additionally, PAH exceedances from the same soil sample included Methylnaphthalene (1.95 μ g/g vs. 0.99 μ g/g) and Naphthalene (1.69 μ g/g vs. 0.6 μ g/g).

At APEC #1 (former retail fuel outlet), the groundwater samples BH3-20 and BH13-20 (Duplicate of BH3-20), collected from a screen depth of approximately 2.5-5.5 m BGS, had reported concentrations of PHC F1 range (3,600 μ g/g and 3,790 μ g/g vs. 750 μ g/g), PHC F2 range (52,400 μ g/g and 2,260 μ g/g vs. 150 μ g/g), PHC F3 range (3,940 μ g/g vs. 500 μ g/g), benzene (19,300 μ g/g and 19,700 μ g/g vs. 44 μ g/g), ethylbenzene (3,800 μ g/g and 3,700 μ g/g vs. μ g/g), toluene (65,200 μ g/g and 60,900 μ g/g vs. 18,000 μ g/g) and xylenes (27,600 μ g/g and 26,600 μ g/g vs. 4,200 μ g/g). Lead was also reported at concentrations of 51.6 μ g/g and 54.6 μ g/g vs. 25 μ g/g.

All of the other soil and groundwater results for the Phase Two Property are in compliance with the applicable site condition standards. The Phase Two Property is not in compliance with the Table 3 site condition standards as of the certification date of June 30, 2020.

An environmental remediation program, including the bulk removal and off-site disposal of soil and groundwater in excess of the site condition standards is recommended for the Phase Two Property. Given the scope and timeline for the proposed redevelopment and the requirements for specialized construction techniques to complete remediation of the Phase Two Property to

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meet the site condition standards, it is recommended that remediation be completed in conjunction with redevelopment of the Property. It should be noted that the proposed redevelopment includes excavation for at least two to three levels of underground parking, which is expected to be sufficient for remediation of the aforementioned environmental contamination at the Phase Two Property.

Further delineation and confirmation of remediation sampling will be required prior to the completion of an environmental remediation and program and confirmation of compliance with the site condition standards; however, these tasks can be completed at the time decommissioning and demolition of existing structures at the Phase Two Property. The submission of a record of site condition would be required in the event of a change of zoning of the Phase Two Property; however, these tasks can be completed at the time decommissioning and demolition of existing structures at the Phase Two Property; however, these tasks can be completed at the time decommissioning and demolition of existing structures at the Phase Two Property. The Phase Two ESA could be then updated at that time to show compliance with site condition standards.

Preparation of a soil management plan in accordance with O.Reg. 406/19 will be required as part of management of excess soil generated as part of construction activities. It is recommended that a remedial action plan be prepared to develop a strategy for remediation, including soil and groundwater management, during redevelopment.

2. Introduction

Lopers & Associates (Lopers) was retained by 11684663 Canada Inc. (Brigil) to complete a Phase Two Environmental Site Assessment (Phase Two ESA) of the commercial property with Civic address No. 729 Ridgewood Avenue, Ottawa, Ontario ("Phase Two Property", "Property" or "Site"). The location of the Phase Two Property within the City of Ottawa is presented on Figure 1: Key Plan.

i. Site Description

The Phase Two Property has a Civic address of 729 Ridgewood Avenue, Ottawa, Ontario. The Property is legally described as Part of Block C, Registered Plan 749, Part of Block C, Registered Plan 775 and Part of Lot 23 Junction Gore, Township of Gloucester, now in the City of Ottawa and has a property identifier number of 04071-0125. The boundaries of the Phase Two Property are presented on Figure 2: Site Plan.

Based on approximate dimensions obtained from the City of Ottawa's GIS mapping tool, the Phase Two Property has an approximate area of 13,200 m2 (1.32 Hectares). The Phase Two Property is immediately surrounded by a municipal Right-of-Way to the south followed by a mixed institutional/commercial property and by residential properties to the north, east and west.

ii. Property Ownership

The Phase Two Property is currently owned by 11684663 Canada Inc., a subsidiary company of Brigil Construction ("Brigil"). This Phase Two ESA was commissioned by Mr. Jean-Luc Rivard, Director of Land Development and Infrastructure for Brigil Construction (Brigil), operating as 11684663 Canada Inc. Brigil has a business address of 98 Rue Lois, Gatineau, Quebec, J8Y 3R7 and a business telephone number of 819-243-7392.

iii. Current and Proposed Future Use

It is Lopers' understanding that Brigil intends to redevelop the Phase Two Property for mixed use (commercial and residential purposes), including the current concept for construction of one building with five adjoining segments ranging from seven to twenty storeys in height, with subgrade parking, commercial ground floors and residential units above.

The redevelopment plan for the Phase Two Property includes mixed use (residential and commercial), which is the current zoning of the Phase Two Property. A certified Planner should assist 11684663 Canada Inc. in determining whether a record of site condition (RSC) will be required to be filed with the Ministry of Environment, Conservation and Parks (MECP) for the Phase Two Property. If so, and update to this Phase Two ESA (post-remediation) can be used as supporting documentation as part of filing of an RSC.

iv. Applicable Site Condition Standard

Through Ontario Regulation 153/04 (O.Reg. 153/04) the Ministry of Environment, Conservation and Parks (MECP) prescribes the conditions to determine the applicable site condition standard for a property.

The proposed future use of the Phase Two Property is for mixed ground floor commercial and residential use, however residential land use standards have been applied for the purposes of this report as they represent the more environmentally sensitive land use conditions.

The Phase Two Property and all other properties within 250 m of the property boundaries are supplied by the municipal drinking water system. The RSC does not specify agricultural use and there are no wells within 250 m of the property boundaries that are intended for use as a source of water for human consumption or agriculture. As such, the designation of non-potable groundwater setting is determined to be applicable [O.Reg. 153/04, section 35].

The soil and groundwater quality over the full depth of overburden was considered for this Phase Two ESA. The full depth generic site condition standards were selected for comparison for the Phase Two Property [O.Reg. 153/04, sections 36, 37, 38, 39 and 40].

The Phase Two Property is not situated within or adjacent to an area of natural significance and does not include any land within 30 m of an area of natural significance. The pH of the soil was analyzed as part of this Phase Two ESA and was found to range from 7.14 to 7.40. As such, the Phase Two Property is not considered to be an environmentally sensitive area [O.Reg. 153/04, section 41].

A substantial layer of native glacial till, consisting of silty sand and gravel, which would be classified as coarse grained soil, is present underlaying a silty clay unit to full depth to bedrock on the south and central portions of the Phase Two Property while sand and gravel fill is present near surface elsewhere at the Property. It is interpreted that greater than 1/3 of the Phase Two Property has coarse grained soil. For the purposes of this Phase Two ESA, the soil conditions are considered to be coarse grained, which provides a more conservative comparison to the MECP site condition standards than the fine-grained values [O.Reg. 153/04, section 42].

Review of the drilling program and borehole/monitoring well logs completed as part of this Phase Two ESA and previous investigations was completed. It was determined that greater than 2/3 of the Phase Two Property has greater than 2 m of overburden soil. The Phase Two Property is not considered a shallow soil property [O.Reg. 153/04, section 43.1].

The Phase Two Property does not include and does not have any land located within 30 m of a water body. The MECP site condition standards for use within 30 m of a water body do not apply [O.Reg. 153/04, section 43.1].

The full depth generic site condition standards, with non-potable groundwater, course textured soil, for residential/parkland/institutional property use, as specified in Table 3 of the MECP Soil,

Ground Water and Sediment Standards for Use Under Part XV.1 of the Environmental Protection Act, April 15, 2011 were determined to be the applicable site condition standards for the Phase Two Property as part of this Phase Two ESA.

3. Background Information

i. Physical Setting

No water bodies or areas of natural significance are located at the Phase Two Property or in the Phase One Study Area. There were no areas of natural and scientific interest (ANSIs) or areas of natural significance identified in the Phase One Study Area.

The regional topography in the Phase One Study Area is undulating but generally slopes downward to the west-northwest, toward the Rideau River. The topography on the south portion of the Phase Two Property slopes downward from west to east, with the neighbouring property to the east at an elevation approximately 1.5 m lower than the southeast Property limits. A local topographical high is present approximately 200 m west of the Phase Two Property, which may be associated with local bedrock undulation. The Rideau River is located approximately 550 m west of the Phase Two Property.

Surface water drainage at the Phase Two Property is by sheet drainage to catch basins located on the paved surfaces of the Property, which drain into the municipal stormwater sewer system.

No drinking water wells are located at the Phase Two Property and the Phase One Study Area are serviced by municipally treated drinking water. The Phase Two Property and Study Area are not located in the vicinity of any well-head protection areas or other designation identified by the City of Ottawa in its official plan for the protection of ground water. No private or agricultural water supply wells are located within the Phase One Study Area.

ii. Past Investigations

A Phase One ESA report was prepared concurrently with this Phase Two ESA: "Phase One Environmental Site Assessment, 729 Ridgewood Avenue, Ottawa, Ontario" dated July 27, 2020 prepared for 11684663 Canada Inc. by Lopers & Associates. The Phase One ESA identified four potentially contaminating activities (PCAs) at the Phase One Property, which include:

The presence of a former retail fuel outlet and automotive service garage was identified on the southeast portion of the Phase One Property (O.Reg. PCA item #28 and #52). This former retail fuel outlet and automotive service garage are significant potentially contaminating activities (PCAs) which represent areas of potential environmental concern # 1 and #2 (APEC #1 / #2) for the Property. The contaminants of potential concern associated with retail fuelling are generally PHCs and BTEXs, with older facilities also having concerns associated with metals, as lead was historically present in gasoline. Based on historical soil analysis in this area of the Property, PAH and VOCs are also considered contaminants of potential concern associated with the former automotive garage operations.

- The practice of backfilling following demolition activities was identified on the central-south portion of the Phase One Property (O.Reg. PCA item #30). Backfilling with fill of unknown environmental quality is a significant PCA which represents APEC #3 for the Property. The contaminants of potential concern commonly found in poor environmental quality backfill are PHCs/BTEXs, PAHs and metals.
- The presence of an active automotive service garage was observed on the central portion of the Phase One Property (O.Reg. PCA item #52). The active automotive service garage is a potentially contaminating activity (PCA) which represents area of potential environmental concern # 4 (APEC #4) for the Property. Based on the observations at this automotive garage, that contaminants of potential concern are considered to be PHCs and BTEXs.

Based on the identification of PCAs and APECs at the Phase One Property, a Phase Two Environmental Site Assessment was recommended to be completed to assess the soil groundwater quality in the vicinity of the APECs.

Additional reports and sources were reviewed and/or referenced as part of the aforementioned Phase One ESA, and included:

- "Phase II Environmental Site Assessment, 729 Ridgewood Avenue, Ottawa, Ontario", dated January 12, 2018, completed by Pinchin Ltd. for Canadian Rental Development Services Inc.
- 2. "Verification Soil Sampling Program, 729 Ridgewood Avenue, Ottawa, Ontario", dated October 19, 2018, completed by Pinchin Ltd. for 561226 Ontario Inc.

These reports confirm the findings of the Phase One ESA completed by Lopers & Associates in 2020 and provide some additional detail of historical investigation and remediation work at the Phase Two Property.

There were no discrepancies identified in review of documentation, information or data from previous investigations. As such, previous investigations are considered to be of adequate quality such that they can be relied upon for the purposes of this Phase Two ESA.

4. Scope of Investigation

i. Overview of Site Investigation

This Phase Two ESA was designed to meet the general requirements of O.Reg. 153/04 as amended, with details of scope presented in Lopers' Letter entitled "Proposal for Phase One Environmental Site Assessment and Phase Two Environmental Site Assessment, Proposed Residential Re-development, 729 Ridgewood Avenue, Ottawa, ON", dated May 8, 2020,

reference, No. LOP-002-20-BRIGIL. The scope of work for investigation was discussed with Brigil and sampling and analysis plan (SAP) was prepared to achieve the objectives of the Phase Two ESA; the SAP is provided in Appendix A. In the event that an RSC is required for the Phase Two Property, additional effort, including delineation, remediation and reporting would be required.

Underground utility locates were completed through Ontario 1-Call to identify any active public services on the Phase Two Property. Following the completion of the public locates, USL-1 Underground Service Locators completed scanning of the Phase Two Property proposed drilling locations to locate privately owned underground services prior to initiating the field program. Various underground utility services, including natural gas, electricity, water and sewers were identified at the Phase Two Property. The natural gas, water and sewer services are present in underground trenches which enter the Property from Ridgewood Avenue to the south and lead to the commercial buildings. Electricity enters the property through an underground service trench to the north of the north commercial building. Copies of the underground locates are provided in Appendix B.

On June 24, 2020, a total of seven boreholes (BH1-20 through BH7-20) were drilled at the Phase Two Property. The boreholes were drilled using a truck mounted CME 55 drill rig operated by George Downing Estate Drilling. Soil samples were collected using stainless steel split spoons. Soil samples recovered during the sampling program were screened in the field for volatile vapour concentrations, as well as visual and olfactory observations.

A total of three groundwater monitoring wells (BH1-20, BH3-20, BH5-20) were installed on the central-south, southeast and central portions of the Phase Two Property. The boreholes which were instrumented with groundwater monitoring wells were drilled to the localized depths ranging from 5.9 to 6.7 m below ground surface (m BGS) and were screened to straddle the shallow groundwater table. When possible, these groundwater monitoring wells were developed on day of drilling by removing at least three well volumes or by purging the wells dry three times.

A total of two existing groundwater monitoring wells were present on the southeast portion of the Phase Two Property prior to undertaking the field program for this Phase Two ESA. The existing monitoring wells were installed as part of past investigations by others. Based on the depths of these wells and the depth to bedrock in boreholes in the vicinity of these wells which were drilled as part of this Phase Two ESA, the existing monitoring wells are suspected to have their screens set within the overburden and may also straddle the shallow groundwater table. Both of the existing groundwater monitoring wells were developed on day of drilling by removing at least three well volumes.

The locations of the boreholes/monitoring wells drilled/installed as part of this Phase Two ESA as well as existing monitoring wells at the Phase Two Property are presented on Figure 2: Site Plan. The rationale for the placement of the boreholes/monitoring wells is provided below:

BH1-20 was drilled in the vicinity of the former suspected residential building at the Phase Two Property. This borehole was placed in a location to assess fill quality in the footprint of this former building (APEC #3). This borehole was instrumented with a groundwater monitoring well, with its screen installed within soil which was observed to be wet during the drilling/soil sample collection in an attempt to straddle the shallow groundwater table.

BH2-20 was drilled in the southeast portion of the Phase Two Property, near the east Property limit. This borehole was placed in a location to assess potential remnant soil contamination from the former retail fuel outlet and automotive service garage (APEC #1 / #2).

BH3-20 was drilled in the southeast portion of the Phase Two Property, near the south Property limit. This borehole was placed in a location to assess potential remnant soil and groundwater contamination from the former retail fuel outlet and automotive service garage (APEC #1 / #2). This borehole was instrumented with a groundwater monitoring well, with its screen installed within soil which was observed to be wet during the drilling/soil sample collection in an attempt to straddle the shallow groundwater table.

BH4-20 was drilled in the southeast portion of the Phase Two Property, near footprint of the former automotive service garage. This borehole was placed in a location to assess potential remnant soil contamination from the former retail fuel outlet and automotive service garage (APEC #1 / #2).

BH5-20 was drilled in the central portion of the Phase Two Property, near the south side of the active automotive service garage. This borehole was placed in a location to assess potential soil and groundwater contamination from the active automotive service garage (APEC #4). This borehole was instrumented with a groundwater monitoring well, with its screen installed within soil which was observed to be wet during the drilling/soil sample collection in an attempt to straddle the shallow groundwater table.

BH6-20 was drilled approximately 5 m to the west of the former suspected residential building at the Phase Two Property. This borehole was placed in a location to delineate potential soil impacts suspected in BH1-20.

BH7-20 was drilled approximately 8 m to the east of the former suspected residential building at the Phase Two Property. This borehole was also placed in a location to delineate potential soil impacts suspected in BH1-20.

Soil samples were selected for laboratory analysis of select contaminants of potential concern (CPCs) based on APECs and CPCs identified in the Phase One ESA, as described in Section 3.ii. above as well as field screening observations.

A waste characterization sample was selected for laboratory analysis of toxicity characteristic leaching procedure (TCLP) analysis for flashpoint, leachate metals & inorganics, leachate VOCs and leachate organics (PAHs and polychlorinated biphenyls (PCBs)). This sample was comprised

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of a composite of worst case samples, as determined by field screening parameters, from BH1-20 and BH3-20.

Groundwater monitoring and sampling of the monitoring wells installed as part of this Phase Two ESA (BH1-20, BH3-20, BH5-20) as well as both existing groundwater monitoring wells (MW-6 and MW-8, which were interpreted as being screened in overburden) was completed on June 30, 2020. Static groundwater levels were measured prior to disturbance of the water column. During purging, water quality parameters were measured at regular intervals to monitor groundwater quality stabilization; once groundwater quality parameters stabilized (were within approximately 10% on successive readings), groundwater samples were collected. Groundwater samples were selected for laboratory analysis of select CPCs based on APECs and CPCs identified in the Phase One ESA.

An elevation survey was completed of the boreholes/monitoring wells drilled as part of the Phase Two ESA as well as both existing monitoring wells at the Phase Two Property. The boreholes/monitoring wells were surveyed relative to a temporary benchmark of the top of spindle of the City of Ottawa fire hydrant located at the southwest corner of the Ridgewood Avenue and Dupont Street intersection; this benchmark was assigned a reference elevation of 100.000 m ("Site Datum") for the purposes of this Phase Two ESA.

ii. Media Investigation

Based on the finding of the Phase One ESA, the following APECs, and CPCs were identified for the following media:

The presence of a former retail fuel outlet and automotive service garage was identified on the southeast portion of the Phase One Property (O.Reg. PCA item #28 and #52). This former retail fuel outlet and automotive service garage are significant potentially contaminating activities (PCAs) which represent areas of potential environmental concern # 1 and #2 (APEC #1 / #2) for the Property. The contaminants of potential concern associated with retail fuelling are generally PHCs and BTEXs, with older facilities also having concerns associated with metals, as lead was historically present in gasoline. Based on historical soil analysis in this area of the Property, PAH and VOCs are also considered contaminants of potential concern associated with the former automotive garage operations.

The practice of backfilling following demolition activities was identified on the central-south portion of the Phase One Property (O.Reg. PCA item #30). Backfilling with fill of unknown environmental quality is a significant PCA which represents APEC #3 for the Property. The contaminants of potential concern commonly found in poor environmental quality backfill are PHCs/BTEXs, PAHs and metals.

The presence of an active automotive service garage was observed on the central portion of the Phase One Property (O.Reg. PCA item #52). The active automotive service garage is a potentially contaminating activity (PCA) which represents area of potential environmental

concern # 4 (APEC #4) for the Property. Based on the observations at this automotive garage, the contaminants of potential concern are considered to be PHCs and BTEXs.

Soil quality at the Phase Two Property was investigated through the collection of soil samples at varying depths facilitated by drilling using a truck mounted CME drill rig with stainless steel split spoon sampling.

Groundwater quality at the Phase Two Property was investigated through the installation and sampling of groundwater monitoring wells. The monitoring wells installed as part of the Phase Two ESA were drilled to the localized depths ranging from 5.9 to 6.7 m below ground surface (m BGS) and were screened to straddle the shallow groundwater table. The existing monitoring wells at the Phase Two Property were suspected to have monitoring well screens installed within the overburden. A bentonite seal was installed above the monitoring well screen's sand pack in each of the monitoring wells to prevent surface and precipitation water influence. Groundwater monitoring wells were sampled using a peristaltic pump.

There were no natural surface water bodies at the Phase Two Property, and as such no sediment sampling was completed as part of the Phase Two ESA.

iii. Phase One Conceptual Site Model

The Phase One Property, which has the same location orientation and property boundaries as the Phase Two Property, is located at Civic No. 729 Ridgewood Avenue, Ottawa, Ontario and has an approximate area of 1.32 Hectares.

The Phase One Property was undeveloped prior to the late 1950's when a suspected residential building was constructed on the central-south portion of the Phase One Property. Initial commercial development began circa 1965. The central and north portions of the Phase One Property have been occupied by two commercial plaza style buildings from circa 1965 to present. The southeast portion of the Phase One Property was formerly occupied by a retail fuel outlet and automotive service garage from 1965 to 2002 (retail fuel outlet) and 2017 (automotive service garage). Demolition of the former suspected residential building occurred prior to 1991. The remaining undeveloped areas of the Phase One Property are paved with asphalt and used for access or parking.

The Property is currently used for commercial purposes and is zoned for mixed use. 11684663 Canada Inc. (Brigil) purchased the Phase One Property in November of 2019, and it is understood that the intended future use is for mixed use, with commercial use on the ground floor and residential uses above the ground floor. The Phase One Property is immediately surrounded by a municipal Right-of-Way to the south followed by a mixed institutional/commercial property and by residential properties to the north, east and west.

The Phase One Study Area includes the Phase One Property and properties with the boundaries within 250 m of the Phase One Property limits. Based on a review of the Phase One Property and properties in the Phase One Study Area, their associated historical and/or current uses and

operations and physical characteristics of the Phase One Study Area, it was determined that an assessment of properties within 250 m of the Phase One property was sufficient to meet the objectives of the scope of this investigation for a Phase One ESA.

No water bodies or areas of natural significance are located at the Phase One Property or in the Phase One Study Area. No drinking water wells are located at the Phase One Property and the Phase One Study Area is serviced by municipally treated non-potable water. Two existing groundwater monitoring wells were present at the Phase One Property; the locations of these wells are presented on Figure 2.

The regional topography in the Phase One Study Area is undulating but generally slopes downward to the west-northwest, toward the Rideau River. The topography on the south portion of the Phase One Property slopes downward from west to east, with the neighbouring property to the east at an elevation approximately 1.2 m lower than the southeast Property limits. A local topographical high is present approximately 200 m west of the Phase One Property, which may be associated with local bedrock undulation. The Rideau River is located approximately 550 m west of the Phase One Property.

Based on the historical research, the general stratigraphy of the Phase One Property and Phase One Study Area consists of sand and gravel fill, underlain by silty clay, followed by silty sand and gravel (till). Overburden soils are expected to be up to 8 m thick and underlain by interbedded shale and limestone bedrock. Groundwater is expected at a depth of approximately 4 to 5 m BGS and flow in a predominantly northwest direction.

Three active and/or historical fuel storage tank locations at neighbouring properties in the Phase One Study Area were identified as PCAs. The PCAs at neighbouring properties in the Phase One Study Area are located significant distances and/or at down- or cross-gradient orientations with respect to the Phase One Property and are not considered to represent APECs for the Phase One Property.

Underground utility service trenches are present at the Phase One Property. The underground utility corridors do have the potential to affect contaminant distribution and transport, as they would create preferential pathways for lateral migration. It should be noted that the groundwater table is expected to be approximately 4 to 5 m BGS, while the underground utilities are expected to be present at depths of 2 to 3 m BGS, therefore it is not suspected that significant migration of contaminants has occurred through underground utility corridors.

iv. Deviations from Sampling and Analysis Plan

The Sampling and Analysis Plan (SAP) was designed to complete BH2-20 at the east Property limit, in the approximately location of remnant contaminated soil which was reported as part of historical sampling. The presence of fencing at the east Property limit prevented drilling with the CME 55 Tuck mounted drill within 3 m of the Property limit; BH2-20 was located as close as practical and safe drilling practices would permit.
v. Impediments

As noted above, the presence of fencing at the east Property limits, impeded the drilling investigation as planned in the SAP. It is suspected that remnant soil and/or groundwater contamination may be present in this area of the Phase Two Property based on historical sampling data, however, this could not be confirmed as part of this Phase Two ESA. Additional investigation and confirmation of soil and groundwater quality in this area of the Property is recommended at the time of excavation for site redevelopment.

5. Investigation Method

i. General

The investigation method for this Phase Two ESA involved an assessment of the soil and/or groundwater quality for the associated CPCs in the vicinity of the APECs identified during the Phase One ESA.

Investigation of soil was completed using a truck mounted CME drill rig, with stainless steel split spoons used to recover soil samples. Soil samples were screened in the field for volatile vapour concentrations, as well as visual and olfactory observations. Select soil samples were submitted based on all the indications mentioned above, as well as to capture representative soil and fill layers, for laboratory analysis for the CPCs.

Groundwater was assessed using groundwater monitoring wells which were installed as part of this Phase Two ESA drilling program and those which had been installed at the Phase Two Property as part of historical previous investigations. The wells selected for monitoring/sampling were purged during the drilling program. Static groundwater levels were measured in the monitoring wells prior to disturbance of the water column on the day of sampling. Groundwater samples were collected using a peristaltic pump using low-flow procedures and were submitted for laboratory analysis for the CPCs.

An elevation survey of the boreholes and groundwater monitoring wells was completed and was referenced to a temporary benchmark, the top of spindle of a fire hydrant located to the southeast of the Phase Two Property.

The following sections provide further detailed information regarding the investigation methodology completed as part of the Phase Two ESA.

ii. Drilling

The drilling field program was completed on June 24, 2020 under full time supervision of Lopers & Associates personnel. Seven boreholes were drilled for the Phase Two ESA by the drilling subcontractor George Downing Estate Drilling, located at 410 Principale Rue, Grenville-Sur-la-

Rouge, Quebec, JOV 1B0. The drill rig used for the Phase Two ESA was a truck mounted CME drill, equipped with hollow stem augers and stainless-steel split spoons. One of the seven boreholes was advanced to auger or cone refusal on suspected bedrock.

Samples were collected using stainless steel split spoons from the near surface to the full depth of drilling. Split spoon samples, collected in 0.6 m segments, were recovered at continuous 0.76 m intervals; the additional 0.16 m between split spoon samples was over-drilled to provide undisturbed field measurement of geotechnical parameters (blow counts) and to prevent cave in materials from stratigraphic units above the intended sampling intervals from being collected at unrepresentative depths during sampling.

The split spoons, which were the only media to come into contact with the soil samples, were washed using soap and water and a scrub brush between samples to minimize the potential for cross-contamination among samples. The field technician used sterile nitrile gloves, which were changed prior to the handling of each soil sample to further reduce the potential of cross-contamination. The flights of the hollow stem augers were cleaned manually following each borehole.

iii. Soil Sampling

As described above, soil samples were recovered using stainless steel split spoons.

Soil samples were initially collected in Ziploc bags for initial screening as part of sample selection. Soil samples selected for laboratory analysis were collected in dedicated clear glass jars prepared and provided by the analytical laboratory. Soil samples collected for BTEXs/VOCs and the F1 range of PHCs analysis were collected using a dedicated graduated syringe provided by the laboratory and placed directly into a glass vial with a known quantity of methanol preservative. Analytes and associated preservatives were specified on each jar/vial by the laboratory. Each jar/vial sample set was provided with a unique sample identifier, project number and date of sampling in the field.

Detailed soil descriptions of the stratigraphy for each borehole/monitoring are included on the borehole logs provided in Appendix C.

Based on the observations of soil samples collected during the Phase Two ESA field program, there were five stratigraphic units identified at the Phase Two Property, which include:

Asphalt

A layer of asphalt, approximately 0.10 to 0.13 m in thickness, was encountered at the ground surface in BH1-20, BH5-20, BH6-20 and BH7-20.

Silty Sand and Gravel (Fill)

A layer of silty sand and gravel fill material, ranging from approximately 0.1 to 2.2 m in thickness, was encountered from ground surface, immediately below the asphalt layer, or stratified with

the sand (fill) layer in boreholes BH2-20 through BH7-20 drilled as part of the Phase Two ESA. This material was identified to consist of silty sand and gravel, was loose to compact and generally grey. This layer was encountered at varying moisture conditions, generally moist to dry at shallow depths and was observed to be wet at approximately 1.8 m BGS in BH5-20.

Sand (Fill)

A layer of sand fill material, ranging from approximately 1.3 to 4.2 m in thickness, was encountered from near the ground surface, below a thin layer of silty sand and gravel (fill), in boreholes BH2-20 through BH4-20 drilled as part of the Phase Two ESA. This material was identified to consist of clean, poorly graded (uniform grain size) sand, was loose and brown. This layer was encountered at varying moisture conditions, generally moist to dry at shallow depths and was observed to be wet at approximately 2.4 m BGS in BH4-20.

Silty Clay

A layer of silty clay, ranging from approximately 0.6 to 3.1 m in thickness, was encountered immediately below the silty sand and gravel fill layer in all of the boreholes drilled as part of this Phase Two ESA, with the exception of BH4-20, which had a thick layer of sand fill. This material was identified to consist of silty clay, firm and was grey or brown-grey. This layer was encountered at varying moisture conditions, generally moist at shallow depths becoming wet at depths ranging from 3.2 to 3.8 m BGS.

Petroleum hydrocarbon odours, suspected to be associated with weathered heating oil were observed in BH1-20 in this unit from an approximate depth of 0.8 m BGS extending through to below the silty clay layer. Petroleum hydrocarbon odours, suspected to be associated with weathered gasoline were observed in BH3-20 in this unit from an approximate depth of 2.3 m BGS extending through to below the silty clay layer.

Silty Sand and Gravel (Till)

A layer of silty sand and gravel, at least 0.8 to 3.5 m in thickness, was encountered below the silty clay layer and right above refusal in BH1-20; actual auger refusal was only encountered in BH1-20, as deeper drilling was not required to investigate the contaminants of potential concern in the boreholes drilled as part of this Phase Two ESA. This material consisted of grey silty sand and gravel with some clay was compact and wet. Where present, this material was found or suspected to be underlain by bedrock.

Petroleum hydrocarbon odours, suspected to be associated with weathered heating oil were observed in BH1-20 in this unit extending from the silty clay layer above to an approximate depth of 5.2 m BGS. Petroleum hydrocarbon odours, suspected to be associated with weathered gasoline were observed in BH3-20 in this unit extending from the silty clay layer above to an approximate depth of 5.2 m BGS.

iv. Field Screening Measurements

Initial field screening of the soil samples consisted of visual and olfactory observations made at the time of sample collection during the drilling program.

Additional field screening of the soil samples was completed using an RKI Instruments Model Eagle-2 combustible gas detector ("RKI Eagle"). The RKI Eagle used for soil sample screening as part of this Phase Two ESA was obtained from Maxim Environmental and Safety Inc. and was calibrated by Maxim on June 23, 2020. The RKI Eagle is capable of measuring combustible vapours at concentrations ranging from 0 parts per million (PPM) to 50% of the lower explosive limit (LEL). The RKI Eagle is also capable of measuring VOC vapours at concentrations ranging from 0 ppm to 1000 ppm. Additional equipment and calibration information for the RKI Eagle is provided on the certificate of calibration included in Appendix D.

Where soil samples were selected in a borehole within an APEC and the SAP identified proposed soil analysis in that borehole, the field screening was used as follows to select the appropriate sample for laboratory analysis.

- 1. Select sample with evidence of visual and/or olfactory indications of suspected contamination, such as staining, PHC odours or deleterious fill material.
- 2. Select sample with most significant elevated soil vapour concentration.
- 3. Select sample based on stratigraphy and/or moisture content, as certain CPCs are generally expected to be found in these defined conditions (i.e. fill material at shallow depths or PHC impacts near the groundwater table interface).
- v. Groundwater: Monitoring Well Installation

Installation of monitoring wells in BH1-20, BH3-20 and BH5-20 were completed by George Downing Estate Drilling. The wells were installed using slotted PVC No. 10 monitoring well screens, which were 51 mm in diameter; these screens were installed at depths intended to straddle the shallow groundwater table in each of the aforementioned boreholes. Well screens were 3.0 m in length in all three of the monitoring wells installed as part of this Phase Two ESA. The monitoring wells were extended to approximately 0.1 m below the surface grade with PVC riser, also 51 mm in diameter. A threaded PVC end cap was installed at the base of the screen to prevent sediment infiltration, while a J-Plug was installed at the top of the riser to present surface influence.

The annular space in each monitoring well was backfilled with clean silica sand to approximately 0.3 m above the monitoring well screens. A layer of bentonite chips was then used to make a hydraulic seal above the sand pack to near the ground surface. The monitoring wells were completed with aluminum flushmount protective casings, which were backfilled with sand to allow drainage of any surface water which may infiltrate into the casings.

Development of each of the monitoring wells was completed using dedicated Waterra low density polyethylene (LDPE) tubing and a Waterra footvalve. The monitoring wells were

developed on June 24, 2020 by purging the wells dry at least three times. The wells were left to stabilize for a period of six days prior to groundwater sampling.

vi. Groundwater: Field Measurement of Water Quality Parameters

Measurements of the groundwater quality field parameters were completed to determine stabilization of these parameters prior to sampling. These measurements were completed using Horiba U-52 groundwater quality measurement device ("Horiba"). The Horiba used for groundwater quality parameter stabilization measurements as part of this Phase Two ESA was obtained from Maxim Environmental and Safety Inc. and was calibrated on June 23, 2020. The Horiba is capable of measuring temperature, pH, conductivity, turbidity, dissolved oxygen and oxidation reduction potential. Additional equipment and calibration information for the Horiba is provided on the certificate of calibration included in Appendix D.

Field measurement of water quality parameters were collected at regular intervals (0 L, 0.5 well volumes, 1 well volume, 2 well volumes, etc.) during purging of the monitoring wells prior to sampling. The Horiba was placed in a flow-through cell and water quality parameters were measured until they were found to stabilize to within approximately 10% of the previous measurements prior to sample collection.

vii. Groundwater: Sampling

Groundwater sampling was completed on June 30, 2020 (six days after well installation). Groundwater samples were collected from monitoring wells BH1-20, BH3-20 and BH5-20, which were installed as part of this Phase Two ESA. Monitoring wells installed in MW-6 and MW-8, which were previously installed at the Phase Two Property within the APECs and in close proximity to APEC #1 / #2 were also sampled as part of this Phase Two ESA; it is suspected that these monitoring wells have their screens set in the overburden to straddle the shallow aquifer.

Stabilized groundwater levels were measured in each of the groundwater monitoring wells prior to disturbance of the water column prior to sampling. The dedicated Waterra LDPE tubing and footvalve was removed from each of the monitoring wells and 6 m Waterra LDPE tubing was placed in each of the monitoring wells. The LDPE tubing was connected to a dedicated length of silicon tubing, run through a peristaltic pump set to low flow (approximately 0.2-0.5 L/minute) during purging and sampling while monitoring groundwater level to minimize the drop in head. The monitoring wells were purged on the day of sampling while water quality parameters were measured as noted above.

Groundwater samples were collected in dedicated amber glass bottles and vials or plastic bottles prepared and provided by the analytical laboratory. Analytes and associated preservatives were specified on each bottle by the laboratory. Each bottle sample set was provided with a unique sample identifier, project number and date of sampling in the field. Samples for PHCs, BTEXs, VOCs, PAHs and general chemistry were unfiltered, while metals samples were field filtered using a dedicated 0.45 µm Waterra filter for each sample.

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The field technician changed dedicated sterile nitrile gloves prior to initiating work at each monitoring well and changed gloves prior to sample collection to minimize the potential for cross-contamination.

viii. Sediment: Sampling

There were no natural surface water bodies at the Phase Two Property, and as such no sediment sampling was completed as part of the Phase Two ESA.

ix. Analytical Testing

Soil and groundwater analytical testing was conducted by Paracel Laboratories Ltd. (Paracel). Paracel is accredited by the Canadian Association for Laboratory Accreditation Inc. (CALA) and the National Institute of Standards and Technology (NIST), Standard Services Division, National Voluntary Laboratory Accreditation Program (NVLAP) for specific environmental and IAQ tests listed in the Scopes of Accreditation registered with each association. For the scope of accreditation under CALA Membership Number 1262, Paracel is accredited for analysis including, but not limited to, metals, organics, conventionals, bacteria, mold, and asbestos in various matrices.

x. Residue Management Procedures

Excess soil cuttings from drilling and monitoring well installations were containerized in steel 205 L drums, which were stored in the in the southeast portion of the Property. These drums were marked with a wax crayon indicating the origin location(s) of the cuttings containerized within each.

Groundwater from well development and purging was initially placed in a graduated plastic bucket for volume measurements and then was transferred to a dedicated plastic 205 L drum, which was stored in the southeast portion of the Property. This drum was marked with a wax crayon indicating the origin location(s) of the water containerized within.

Fluids from equipment cleaning and decontamination were containerized within the purge water drum.

xi. Elevation Surveying

An elevation survey was completed of the boreholes/monitoring wells drilled as part of the Phase Two ESA as well as the two existing monitoring wells at the Phase Two Property. The boreholes/monitoring wells were surveyed relative to a temporary benchmark of the top of spindle of the City of Ottawa fire hydrant located at the southwest corner of the Ridgewood Avenue and Dupont Street intersection; this benchmark was assigned a reference elevation of 100.000 m ("Site Datum") for the purposes of this Phase Two ESA. The reference elevations of each borehole/monitoring well are provided on the borehole logs in Appendix C.

xii. Quality Assurance and Quality Control Measures

Soil samples were collected in dedicated clear glass jars prepared and provided by the analytical laboratory. Soil samples collected for BTEXs/VOCs and the F1 range of PHCs analysis were collected using dedicated graduated syringes provided by the laboratory and placed directly into a glass vial with methanol preservative. Analytes and associated preservatives were specified on each jar/vial by the laboratory. Each jar/vial sample set was provided with a unique sample identifier, project number and date of sampling in the field.

Groundwater samples were collected in dedicated amber glass bottles and vials or plastic bottles prepared and provided by the analytical laboratory. Analytes and associated preservatives were specified on each bottle by the laboratory. Each bottle sample set was provided with a unique sample identifier, project number and date of sampling in the field.

Following sample collection, the soil and groundwater samples were stored in an ice pack chilled cooler to minimize volatilization and begin the cooling process on the day of sampling. On each day of sample collection, following completion of the fieldwork, samples were delivered directly to the analytical laboratory. Standard chain of custody procedures were used to maintain a custody record of soil and groundwater samples between the field technician and the analytical laboratory.

The split spoons, which were the only media to come into contact with the soil samples, were washed using soap and water and a scrub brush between samples to minimize the potential for cross contamination among samples. The field technician used sterile nitrile gloves, which were changed prior to the handling of each soil sample to prevent cross-contamination. The field technician changed dedicated sterile nitrile gloves prior to initiating work at each monitoring well and changed gloves prior to groundwater sample collection to minimize the potential for cross-contamination.

A trip blank water sample for VOCs was submitted for laboratory analysis from the groundwater sampling event completed on June 30, 2020. No detectable VOC concentrations were reported in the trip blank water sample.

The soil sample (BH11-20-SS5) was submitted to the laboratory as a blind field duplicate sample of BH1-20-SS5). The ratio of soil duplicate results to original sample results was generally 1 to 25%, which meets the required ratio. The soil duplicate ratios for PAHs and PHCs observed had higher degrees of variability, with ratios ranging from 33 to 120%; it should be noted that where exceedances of the site condition standards were observed, they were present in both samples and that the sample results for these parameters are comparable. Additionally, the high degree of heterogeneity in soil samples can attribute to higher levels of variability in analytical ratios. These samples were analyzed for PHCs, VOCs (including BTEXs), PAHs and metals & inorganics, which provide a blind quality assurance and quality control QA/QC validation for all soil parameters analyzed as part of this Phase Two ESA.

The groundwater sample (BH13-20) was submitted to the laboratory as a blind field duplicate sample of BH3-20. The ratio of groundwater duplicate results to original sample results was generally 1 to 5 or 18% which meets the required ratio. The groundwater duplicate ratios of PAH parameters were found to range from 5 to 120%; however, the instance of higher variability was with individual parameters which were detected at very low concentrations and close to the laboratory method detection limits. The PAH groundwater sample results are generally comparable. The groundwater sample duplicate comparison results for PHC-F1 and benzene, ethylbenzene, toluene and xylenes were between 2% and 7%, which indicates good reliability. The comparison results in the F2 and F3 ranges of PHCs, however, display a high degree of variability. The laboratory was asked to verify the results of the two samples and stated that the extract which was rerun confirmed the original results. Given the magnitude of the PHC-F2 results in the BH3-20 sample, the explanation may be that it contained droplets of product while the dissolved phase concentrations were in good agreement. Given that exceedances of the site condition standards for PHCs were detected in both samples, the variability of these sample results does not affect the validity of the conclusions with respect to these results. These samples were analyzed for PHCs, VOCs (including BTEXs), PAHs and metals & inorganics, which provide a blind quality assurance and quality control QA/QC validation for all groundwater parameters analyzed as part of this Phase Two ESA.

No equipment blank of groundwater was required since the groundwater samples were collected using dedicated tubing.

6. Review and Evaluation

i. Geology

Based on the observations of soil samples collected during the Phase Two ESA field program, there were five stratigraphic units identified at the Phase Two Property, which include:

Asphalt

A layer of asphalt, approximately 0.10 to 0.13 m in thickness, was encountered at the ground surface in BH1-20, BH5-20, BH6-20 and BH7-20.

Silty Sand and Gravel (Fill)

A layer of silty sand and gravel fill material, ranging from approximately 0.1 to 2.2 m in thickness, was encountered from ground surface, immediately below the asphalt layer, or stratified with the sand (fill) layer in boreholes BH2-20 through BH7-20 drilled as part of the Phase Two ESA. This material was identified to consist of silty sand and gravel, was loose to compact and generally grey. This layer was encountered at varying moisture conditions, generally moist to dry at shallow depths and was observed to be wet at approximately 1.8 m BGS in BH5-20.

Sand (Fill)

A layer of sand fill material, ranging from approximately 1.3 to 4.2 m in thickness, was encountered from near the ground surface, below a thin layer of silty sand and gravel (fill), in boreholes BH2-20 through BH4-20 drilled as part of the Phase Two ESA. This material was identified to consist of clean, poorly graded (uniform grain size) sand, was loose and brown. This layer was encountered at varying moisture conditions, generally moist to dry at shallow depths and was observed to be wet at approximately 2.4 m BGS in BH4-20.

Silty Clay

A layer of silty clay, ranging from approximately 0.6 to 3.1 m in thickness, was encountered immediately below the silty sand and gravel fill layer in all of the boreholes drilled as part of this Phase Two ESA, with the exception of BH4-20, which had a thick layer of sand fill. This material was identified to consist of silty clay, firm and was grey or brown-grey. This layer was encountered at varying moisture conditions, generally moist at shallow depths becoming wet at depths ranging from 3.2 to 3.8 m BGS.

Petroleum hydrocarbon odours, suspected to be associated with weathered heating oil were observed in BH1-20 in this unit from an approximate depth of 0.8 m BGS extending through to below the silty clay layer. Petroleum hydrocarbon odours, suspected to be associated with weathered gasoline were observed in BH3-20 in this unit from an approximate depth of 2.3 m BGS extending through to below the silty clay layer.

Silty Sand and Gravel (Till)

A layer of silty sand and gravel, at least 0.8 to 3.5 m in thickness, was encountered below the silty clay layer and right above refusal in BH1-20; practical refusal was only encountered in BH1-20, as deeper drilling was not required to investigate the contaminants of potential concern in the boreholes drilled as part of this Phase Two ESA. This material consisted of grey silty sand and gravel with some clay was compact and wet. Where present, this material was found or suspected to be underlain by suspected bedrock.

Petroleum hydrocarbon odours, suspected to be associated with weathered heating oil were observed in BH1-20 extending from the silty clay layer above this unit to an approximate depth of 5.2 m BGS. Petroleum hydrocarbon odours, suspected to be associated with weathered gasoline were observed in BH3-20 extending from the silty clay layer above to an approximate depth of 5.2 m BGS.

Aquifer

The shallow (unconfined) aquifer is the aquifer of interest based on the nature of APECs and PCAs identified for the Phase Two Property. It is present in several geological units, including the native silty clay and silty sand and gravel (till) layers, as well as in the sand and/or silty sand and gravel fill layers (where present in the former areas of excavation of backfilling).

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Based on moisture contents observed in the soil samples collected as part of this Phase Two ESA it is expected that seasonal and annual variability affect the groundwater table elevation in the shallow aquifer.

ii. Groundwater and Elevations and Flow Direction

Based on the nature of the primary CPCs identified for groundwater at the Phase Two Property (including light non-aqueous phase liquids (LNAPLs)), the screened intervals for the groundwater monitoring wells installed as part of this Phase Two ESA were selected to straddle the shallow groundwater table within the overburden. Based on previous investigations, it was suspected that existing monitoring wells located within the APECs at the Phase Two Property had monitoring well screens that are also installed within the overburden and would be expected to straddle the shallow groundwater table, and are thus in same aquifer as the 2020 monitoring wells and could be used for supplemental sampling as part of this Phase Two ESA.

The boreholes/monitoring wells were surveyed relative to a temporary benchmark of the top of spindle of the City of Ottawa fire hydrant located at the southwest corner of the Ridgewood Avenue and Dupont Street intersection, southeast of the Phase Two Property; this benchmark was assigned a reference elevation of 100.000 m ("Site Datum") for the purposes of this Phase Two ESA.

The shallow groundwater aquifer was present within the overburden on central, central-south and southeast portions of the Phase Two Property. Given the general consistency in depth of the groundwater table in different geological units at the Phase Two Property, it is suspected that the same shallow aquifer exists across these units and can be used for a determination of groundwater flow direction and hydraulic gradient. Monitoring well construction details are presented in Table 1 below.

Monitoring Well	Ground Surface Elevation (m RSD)	Top of Piezometer Elevation (m RSD)	Screen Elevation (m RSD)	Sand Pack Elevation (m RSD)	Bentonite Seal (m RSD)
BH1-20	100.93	100.80	95.45 – 98.50	95.45 – 98.80	98.80 - 100.63
BH3-20	100.15	99.91	94.61 – 97.66	94.61 – 97.96	97.96 - 99.85
BH5-20	100.39	100.31	95.01 – 98.06	95.01 – 98.36	98.36 - 100.09
MW-6	99.73	99.66	93.74 - unknown	unknown	unknown
MW-8	99.89	99.86	93.89 - unknown	unknown	unknown

Table 1: Monitoring Well Construction Deta
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m RSD – metres Referenced to Site Datum

On June 30, 2020, following a period of six days for stabilization after drilling and developing the monitoring wells, the groundwater levels were measured and are presented in Table 2 below. The groundwater table was measured at depths ranging between 3.15 and 3.90 m BGS on June 30, 2020.

Monitoring Well	Ground Surface Elevation (m RSD)	Top of Piezometer Elevation (m RSD)	Depth to Groundwater (m below TOP)	Groundwater Table Elevation (m RSD)	Depth to Groundwater (m BGS)
BH1-20	100.93	100.80	3.46	97.34	3.59
BH3-20	100.15	99.91	3.66	96.25	3.90
BH5-20	100.39	100.31	3.10	97.21	3.18
MW-6	99.73	99.66	3.80	95.86	3.87
MW-8	99.89	99.86	3.12	96.74	3.15

Table 2: Groundwater Table Elevations Measured on June 30, 2020

m RSD – meters Referenced to Site Datum

m BGS – metres below Ground Surface.

Three groundwater monitoring well water table elevations are required to triangulate groundwater elevations and determine an approximate groundwater flow direction. The groundwater table elevations in BH1-20, BH3-20 and BH5-20 were used for a determination of groundwater flow direction, while MW-8 was used to verify the accuracy of the model. Based on the measured groundwater table elevations in these monitoring wells, the local groundwater flow direction on the central and southeast portion of the Phase Two Property is towards the southeast. This interpreted local groundwater flow direction is reasonable based on the regional topography; however, it is expected that regional groundwater flow is toward the west in the direction of the Phase Two Property. The water table elevation in MW-8 was treated as an outlier, however, this measure elevation indicates that there is local influence in groundwater flow based on the lower elevation of the adjacent property to the east of the Phase Two Property.

No observations or indications of free product were observed in any of the monitoring wells accessed as part of this Phase Two ESA, as measured with an interface probe during water level measurements, and through observations of the purge water during development and sampling of the monitoring wells. A petroleum hydrocarbon odour, suspected to consist primarily of gasoline, was observed in the groundwater sample collected from BH3-20. Slight to moderate petroleum hydrocarbon odours were observed in the purge water recovered from BH1-20, MW-6 and MW-8

The underground utility corridors associated with the storm and sanitary sewers (exiting the southeast corner of the Phase Two Property) and east water service (accessing the Property on the southeast portion of the Property) do have the potential to affect contaminant distribution and transport, as they would create preferential pathways for lateral migration in the areas of historically identified contaminated soil and groundwater. Based on the depth to groundwater observed in the monitoring wells as part of this investigation, observed between 3.15 and 3.9 m BGS, the potential exists for migration of contaminants through underground utility service trenches (generally approximately 2 to 3 m BGS) during periods of seasonally high groundwater table elevations.

iii. Groundwater: Hydraulic Gradients

The horizontal hydraulic gradient was determined by plotting groundwater contours interpreted from groundwater elevations presented in Table 2 and then by dividing the difference in hydraulic head by the lateral separation distance in the groundwater contours. Based on the measured groundwater elevations in BH1-20, BH3-20 and BH5-20 the horizontal hydraulic gradient at the Phase Two Property is approximately 0.02 m/m.

iv. Course Grained Soil Texture

A substantial layer of native glacial till, consisting of silty sand and gravel, which would be classified as coarse grained soil, is present underlaying a silty clay unit to full depth to bedrock on the south and central portions of the Phase Two Property while sand and gravel fill is present near surface elsewhere at the Property. It is interpreted that greater than 1/3 of the Phase Two Property has coarse grained soil. For the purposes of this Phase Two ESA, the soil conditions are considered to be coarse grained, which provides a more conservative comparison to the MECP site condition standards than the fine-grained values.

v. Soil Field Screening

Initial field screening of the soil samples consisted of visual and olfactory observations made at the time of sample collection during the drilling program. Petroleum hydrocarbon odours, suspected to be associated with weathered heating oil were observed in BH1-20 from an approximate depth of 0.8 m BGS extending to an approximate depth of 5.2 m BGS. Petroleum hydrocarbon odours, suspected to be associated with weathered gasoline were observed in BH3-20 from an approximate depth of 2.3 m BGS extending to an approximate depth of 5.2 m BGS.

Additional field screening of the soil samples was completed using an RKI Eagle gas detector. A combustible soil vapour screening concentration of 700 ppm was encountered in soil sample BH3-20-SS6, collected from a depth of 3.8 to 4.4 m BGS; this elevated concentration was suspected to be indicative of PHC contamination. Combustible soil vapour screening concentrations in the other soil samples were found to range from 0 to 55 ppm, which is low and generally not considered indicative of significant PHC contamination.

vi. Soil Quality

Location and Depth of Soil Samples

The following soil samples, which were collected from the boreholes drilled as part of this Phase Two ESA, were submitted for laboratory analysis.

Sample Location	Sample ID	Sample Depth (m BGS)	Analytical Parameters
BH1-20	BH1-20-SS5	3.1 – 3.7	PHCs, VOCs, PAHs, Metals & Inorganics
Duplicate of BH1-20	BH11-20-SS5	3.1 – 3.7	PHCs, VOCs, PAHs, Metals & Inorganics
BH3-20	BH3-20-SS6	3.8 - 4.4	PHCs, BTEXs, PAHs
BH4-20	BH4-20-SS6	3.8 - 4.4	PHCs, BTEXs, PAHs, Metals & Inorganics
BH5-20	BH5-20-SS4	2.3 – 2.9	PHCs, BTEXs
BH5-20	BH5-20-SS7	4.6 - 5.2	PHCs, VOCs

Table 3: Soil Samples Selected for Laboratory Analysis

Comparison of Soil Analytical Results to Applicable Site Conditions Standards

The analytical soil results were compared to the full depth generic site condition standards, with non-potable groundwater, course textured soil, for residential property use, as specified in Table 3 of the MECP Soil, Ground Water and Sediment Standards for Use Under Part XV.1 of the Environmental Protection Act, April 15, 2011.

The aforementioned soil samples selected for laboratory analysis were submitted to Paracel under chain of custody No. 54317 on June 24, 2020. The laboratory certificate of analysis (Paracel Report # 2026367) is provided in Appendix E. The following samples had exceedance concentrations reported compared to ('vs.') their respective site condition standards.

- BH1-20-SS5 and BH11-20-SS5 (Duplicate of BH1-20-SS5), collected from a depth of approximately 3.1-3.7 m BGS, had reported concentrations of PHC F2 range (909 µg/g and 306 µg/g vs. 98 µg/g), Methylnaphthalene (7.61 µg/g and 2.26 µg/g vs. 0.99 µg/g) and reported concentrations of vanadium (101 µg/g and 104 µg/g vs. 86 µg/g). These samples also had respective cobalt concentrations of 20.1 µg/g and 22.5 µg/g compared to the site condition standard of 22 µg/g; since the average concentration of cobalt in these samples is less than the site condition standard, the marginal exceedance in the duplicate standard is not considered to exceed the site condition standard.
- BH3-20-SS6, collected from a depth of approximately 3.8-4.4 m BGS, had reported concentrations of PHC F1 range (117 µg/g vs. 55 µg/g), PHC F2 range (110 µg/g vs. 98 µg/g), benzene (3.02 µg/g vs. 0.21 µg/g), ethylbenzene (59 µg/g vs. 2 µg/g), toluene (73.5 µg/g vs.

2.3 μ g/g) and xylenes (276 μ g/g vs. 3.1 μ g/g). Additionally, PAH exceedances from the same soil sample included Methylnaphthalene (1.95 μ g/g vs. 0.99 μ g/g) and Naphthalene (1.69 μ g/g vs. 0.6 μ g/g).

All of the other analyzed soil samples were in compliance with the site condition standards. A full summary of the soil analytical results and comparison to the applicable site condition standards are presented in Table 1: Soil Analytical Results following the text of this report.

Comparison of TCLP Analytical Results to O.Reg. 558/00

A waste characterization sample was selected for laboratory analysis of toxicity characteristic leaching procedure (TCLP) analysis for flashpoint, leachate metals & inorganics, leachate VOCs and leachate organics (PAHs and polychlorinated biphenyls (PCBs)). This sample was comprised of a composite of worst-case samples, as determined by field screening parameters, from BH1-20 and BH3-20.

The aforementioned composite soil sample selected for TCLP laboratory analysis was submitted to Paracel under chain of custody No. 54317 on June 24, 2020. The laboratory certificate of analysis (Paracel Report # 2026368) is provided in Appendix E.

This composite sample was compared to the criteria specified in schedule IV of O.Reg. 558/00 and no measured parameter exceeded the toxicity criteria. Based on the analytical results and field screening, if excess soil generated from redevelopment of the Site cannot be reused as clean fill at an appropriate receiving site, it can be treated as solid non-hazardous waste.

A full summary of the soil analytical results and comparison to the Schedule IV of O.Reg. 558/00 standards are presented in Table 2: TCLP Analytical Results following the text of this report.

Contaminants of Concern

The presence of a former retail fuel outlet and automotive service garage were identified on the southeast portion of the Phase One Property (APEC #1 / #2). The contaminants of potential concern associated with retail fuelling are petroleum hydrocarbons (PHCs) and benzene, toluene, ethylbenzene and xylenes (BTEXs), and metals, since this was an older facility and lead was historically present in gasoline. Based on historical soil analysis in this area of the Property, PAH and VOCs are also considered contaminants of potential concern associated with the former automotive garage operations. The practice of backfilling following demolition activities was identified on the central-south portion of the Phase One Property (APEC #3). The contaminants of potential concern commonly found in poor environmental quality backfill are PHCs/BTEXs, polycyclic aromatic hydrocarbons (PAHs) and metals. Based on the operations observed at the active automotive service garage (APEC #4), the contaminants of concern associated with this activity are PHCs and BTEXs.

The contaminants of concern for a particular sample were based on the relative location and depth of the sample, visual and/or olfactory observations and combustible vapour screening concentrations.

Contaminants Related to Chemical and Biological Transformations

Contaminants related to chemical and biological transformations were not suspected to be present at the Phase Two Property and were not identified as part of the Phase Two ESA soil analysis.

Soil Serving as a Source of Contaminant Mass Contributing to Groundwater

Based on the analytical results, there may be soil that serves as a source of contaminant mass contributing to groundwater at the Phase Two Property. Soil contamination, namely PHCs and BTEXs were encountered at the southeast portion of the Phase Two Property (APEC #1 – former retail fuel outlet) and in the central-south portion of the Phase Two Property (APEC #3 – placement of fill material of unknown quality). There are detectable concentrations of PHCs and BTEXs in these areas of the Phase Two Property and it is suspected that soil serving as a source of contaminant mass is contributing to groundwater quality.

Light or Dense Non-Aqueous Phase Liquids

The analytical soil results indicate the potential presence of light non-aqueous phase liquids (LNAPLs) at the Phase Two Property, given that PHCs and BTEXs were identified in excess of the site condition standards. It should be noted that the concentrations of PHCs and BTEXs which exceed the site condition standards in the soil are not themselves indicative of the suspected presence of LNAPL free product at the Phase Two Property.

The analytical soil results do not indicate the suspected presence of dense non-aqueous phase liquids at the Phase to Property.

vii. Groundwater Quality

Locations and Sample Depth Interval of Groundwater Samples

The groundwater samples were collected using a peristaltic pump with tubing lowered to between the top and approximate (vertical) center of the water column within each monitoring well and withdrawing the water at low flow rates. The groundwater sample locations, screen depths and parameters analyzed are presented in Table 4 below.

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Sample Location	Groundwater Level (m RSD)	Screen Depth (m RSD)	Analytical Parameters
BH1-20	97.34	95.45 – 98.50	PHCs, VOCs, PAHs, Metals & Inorganics
BH3-20	96.25	94.61 – 97.66	PHCs, VOCs, PAHs, Metals & Inorganics
BH13-20 (Duplicate of BH3-20)	96.25	94.61 – 97.66	PHCs, VOCs, PAHs, Metals & Inorganics
BH5-20	97.21	95.01 – 98.06	PHCs, BTEXs, PAHs
MW-6	95.86	93.74 - unknown	PHCs, BTEXs
MW-8	96.74	93.89 - unknown	PHCs, BTEXs

Table 4: Groundwater Samples Selected for Laboratory Analysis

m RSD – metres Referenced to Site Datum

Field Filtering

Samples for PHCs, BTEXs, VOCs, PAHs and general chemistry were unfiltered, while metals samples were field filtered using a dedicated 0.45 µm Waterra filter for each sample.

Comparison of Groundwater Analytical Results to Applicable Site Conditions Standards

The analytical groundwater results were compared to the full depth generic site condition standards, with non-potable groundwater, course textured soil, as specified in Table 3 of the MECP Soil, Ground Water and Sediment Standards for Use Under Part XV.1 of the Environmental Protection Act, April 15, 2011.

The groundwater samples selected for laboratory analysis were submitted to Paracel under chain of custody No. 126519 on June 30, 2020. The laboratory certificate of analysis (Paracel Report # 2027199) is provided in Appendix E. The following samples had exceedance concentrations reported compared to ('vs.') their respective site condition standards.

BH3-20 and BH13-20 (Duplicate of BH3-20), collected from a screen depth of approximately 2.5-5.5 m BGS, had reported concentrations of PHC F1 range (3,600 µg/g and 3,790 µg/g vs. 750 µg/g), PHC F2 range (52,400 µg/g and 2,260 µg/g vs. 150 µg/g), PHC F3 range (3,940 µg/g vs. 500 µg/g), benzene (19,300 µg/g and 19,700 µg/g vs. 44 µg/g), ethylbenzene (3,800 µg/g and 3,700 µg/g vs. µg/g), toluene (65,200 µg/g and 60,900 µg/g vs. 18,000 µg/g) and xylenes (27,600 µg/g and 26,600 µg/g vs. 4,200 µg/g). Lead was also reported at concentrations of 51.6 µg/g and 54.6 µg/g vs. 25 µg/g.

All of the other groundwater samples were in compliance with the site condition standards. A full summary of the groundwater analytical results and comparison to the applicable site

condition standards are presented in Table 3: Groundwater Analytical Results following the text of this report.

Contaminants of Concern

The contaminants of potential concern associated with retail fueling (APEC #1) are generally PHCs and BTEXs, and metals, since this was an older facility and lead was historically present in gasoline. Based on historical groundwater analysis at the Property, PAH and VOCs are limited contaminants of potential concern in selected areas of the Property and are suspected to have been associated with the former automotive garage operations (APEC #2). The contaminants of potential concern commonly found in poor environmental quality backfill (APEC #3) are PHCs/BTEXs, PAHs and metals. Based on the operations observed at the active automotive service garage (APEC #4), the contaminants of concern associated with this activity are PHCs and BTEXs.

The contaminants of concern for a particular sample were based on the relative location and depth of the sample, visual and/or olfactory observations of soil samples collected which could have come into contact with the groundwater table.

Contaminants Related to Chemical and Biological Transformations

Contaminants related to chemical and biological transformations were not suspected to be present at the Phase Two Property and were not identified as part of the Phase Two ESA groundwater analysis.

Soil Serving as a Source of Contaminant Mass Contributing to Groundwater

Based on the groundwater analytical results, there may be soil that serves as a source of contaminant mass contributing to groundwater at the Phase Two Property. Soil contamination, namely PHCs and BTEXs were encountered at the southeast portion of the Phase Two Property (APEC #1 – former retail fuel outlet) and in the central-south portion of the Phase Two Property (APEC #3 – placement of fill material of unknown quality). There are detectable concentrations of PHCs and BTEXs in soil in these areas of the Phase Two Property, and in the instance of APEC #1 there was identified groundwater contamination, and it is suspected that soil serving as a source of contaminant mass is contributing to groundwater quality.

Light or Dense Non-Aqueous Phase Liquids

The analytical groundwater results indicate the potential presence of light non-aqueous phase liquids (LNAPLs) at the Phase Two Property, given that PHCs and BTEXs were identified in excess of the site condition standards and at significant concentrations in the sample (and duplicate) from the monitoring well installed in BH3-20. It should be noted that the presence of measurable levels LNAPL free product were not observed at the Phase Two Property, as measured with an interface probe and with observations of the purge water recovered from the

monitoring wells. A light sheen was observed on the purge water recovered from the monitoring well installed in BH3-20.

The analytical groundwater results do not indicate the suspected presence of dense nonaqueous phase liquids at the Phase to Property.

viii. Sediment Quality

There were no natural surface water bodies at the Phase Two Property, and as such no sediment sampling was completed as part of the Phase Two ESA.

ix. Quality Assurance and Quality Control Results

The soil sample (BH11-20-SS5) was submitted to the laboratory as a blind field duplicate sample of BH1-20-SS5). These samples were analyzed for PHCs, VOCs (including BTEXs), PAHs and metals & inorganics, which provide a blind quality assurance and quality control QA/QC validation for all soil parameters analyzed as part of this Phase Two ESA.

The groundwater sample (BH13-20) was submitted to the laboratory as a blind field duplicate sample of BH3-20. These samples were analyzed for PHCs, VOCs (including BTEXs), PAHs and metals & inorganics, which provide a blind quality assurance and quality control QA/QC validation for all groundwater parameters analyzed as part of this Phase Two ESA.

The soil sample (BH11-20-SS5) was submitted to the laboratory as a blind field duplicate sample of BH1-20-SS5). The ratio of soil duplicates to samples was generally 1 to 25%, which meets the required ratio. The soil duplicate ratios for PAHs and PHCs observed had higher degrees of variability, with ratios ranging from 33 to 120%; it should be noted that where exceedances of the site condition standards were observed, they were observed in both samples and that the sample results for these parameters are comparable. Additionally, the high degree of heterogeneity in soil samples can attribute to higher levels of variability in analytical ratios. These samples were analyzed for PHCs, VOCs (including BTEXs), PAHs and metals & inorganics, which provide a blind quality assurance and quality control QA/QC validation for all soil parameters analyzed as part of this Phase Two ESA.

The groundwater sample (BH13-20) was submitted to the laboratory as a blind field duplicate sample of BH3-20. The ratio of groundwater duplicates to samples was generally 1 to 5 or 18% which meets the required ratio. The groundwater duplicate ratios of PAH parameters were found to range from 5 to 120%; however, the instance of higher variability was with individual parameters which were detected at very low concentrations and close to the laboratory method detection limits. The PAH groundwater sample results are generally comparable. The groundwater sample duplicate comparison results for PHC-F1 and benzene, ethylbenzene, toluene and xylenes were between 2% and 7%, which indicates good reliability. The comparison results in the F2 and F3 ranges of PHCs however display a high degree of variability. The laboratory was asked to verify the results of the two samples and stated that the extract which was rerun confirmed the original results. Given the magnitude of the PHC-F2 results in the BH3-

20 sample, the explanation may be that it contained droplets of product while the dissolved phase concentrations were in good agreement. Given that exceedances of the site condition standards for PHCs were detected in both samples, the variability of these sample results does not affect the validity of the conclusions with respect to these results. These samples were analyzed for PHCs, VOCs (including BTEXs), PAHs and metals & inorganics, which provide a blind quality assurance and quality control QA/QC validation for all groundwater parameters analyzed as part of this Phase Two ESA. The QA/QC duplicate sample results demonstrate that the data are reliable, appropriate and accurate in the determination of whether the phase two property meets the applicable site condition standards.

The laboratory made qualifying statements for login criteria on several sample results due to the difference in naming convention on the soil jars and vials compared to what was presented on the chain of custody. In these circumstances, the labeling on the jar was made for simplicity and no concerns are present with respect to the validity of any of the laboratory results. The laboratory noted that elevated detection limits were presented for the duplicate groundwater sample from BH3-20 due to dilution required because of high target analyte concentration; it should be noted that the original sample did not have elevated detection limits. The qualifying remarks in certificates of analysis are not expected to impact the validity of any results qualified.

All certificates of analysis were received pursuant to clause 47 (2) (b) of O.Reg. 153/04 and comply with subsection 47 (3) of O.Reg. 153/04.

The overall quality of the field data from the investigation with respect to the data quality objectives, demonstrate that decision-making was not affected, and the overall objectives of the investigation and the assessment were met.

x. Phase Two Conceptual Site Model

The presence of a former retail fuel outlet and automotive service garage on the southeast portion of the Phase One Property are a significant PCAs which represent APEC #1 / #2 for the Property. Given that reports were provided which document remnant PHC/BTEXs soil contamination and that groundwater quality was not confirmed following the completion a remediation program, further investigation is warranted. The contaminants of potential concern associated with retail fuelling are generally PHCs and BTEXs, with older facilities also having concerns associated with metals, as lead was historically present in gasoline. Based on historical soil analysis in this area of the Property, PAH and VOCs are also considered contaminants of potential concern associated with the former automotive garage operations.

The practice of backfilling following demolition activities at the Phase One Property is a significant PCA which represents APEC #3 for the Property. Given that no reports were provided with analytical data to support the environmental quality of the backfill used to fill the former suspected residential building footprint on the central-south portion of the Property, this area warrants further investigation. The contaminants of potential concern commonly found in poor environmental quality backfill are PHCs/BTEXs, PAHs and metals.

The presence of an active automotive service garage was observed on the central portion of the Phase One Property at the time of the Site Investigation. Although this garage has only been operating for a short time period (2017 to present), these operations are a PCA which represents APEC #4 for the Property. Based on the observations at this automotive garage, that contaminants of potential concern are considered to be PHCs and BTEXs.

Various underground utility services, including natural gas, electricity, water and sewers were identified at the Phase Two Property. The natural gas, water and sewer services are present in underground trenches which enter the Property from Ridgewood Avenue to the south and lead to the commercial buildings. Electricity enters the property through an underground service trench to the north of the north commercial building. The underground utility corridors associated with the storm and sanitary sewers (exiting the southeast corner of the Phase Two Property) and east water service (accessing the Property on the southeast portion of the Property) do have the potential to affect contaminant distribution and transport, as they would create preferential pathways for lateral migration in the areas of historically identified contaminated soil and groundwater.

The overburden stratigraphy of the Phase Two Property is present in five geological units, including an asphalt layer at ground surface, silty sand and gravel (fill) layer, sand (fill) layer primarily present on the southeast portion of the Property, a native silty clay layer present across the Property and a native silty sand and gravel layer, found below the silty clay across the Property.

The shallow (unconfined) aquifer is the aquifer of interest based on the nature of APECs and PCAs identified for the Phase Two Property. It is present in across several geological units, including the native silty clay and silty sand and gravel (till) layers, as well as in the sand and/or silty sand and gravel fill layers (where present in the former areas of excavation of backfilling). The aquifer is expected to have higher permeability in more porous stratigraphic units such as the silty sand and gravel and sand fill, while the shallow aquifer in the silty clay layer is expected to have low permeability and restrict the movement of groundwater and associated contaminants.

The overburden soil is underlain by interbedded shale and limestone bedrock at depths ranging from approximately 6 to 8 m BGS.

The groundwater table was measured at depths ranging between 3.15 and 3.90 m BGS The shallow groundwater aquifer was present within the overburden on central, central-south and southeast portions of the Phase Two Property. Given the general consistency in depth of the groundwater table in different geological units at the Phase Two Property, it is suspected that the same shallow aquifer exists across these units and can be used for a determination of groundwater flow direction and hydraulic gradient. The horizontal hydraulic gradient on the southeast portion of the Phase Two Property was calculated to be approximately 0.02 m/m with a localized groundwater flow direction towards the southeast.

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The proposed redevelopment of the Phase Two Property includes construction of one building with five adjoining segments ranging from seven to twenty storeys in height, with subgrade parking, commercial ground floors and residential units above.

The Phase Two Property and all other properties within 250 m of the property boundaries are supplied by Ottawa's municipal potable water supply system. The RSC does not specify agricultural use and there are no wells within 250 m of the property boundaries that are intended for use as a source of water for human consumption or agriculture. As such, the designation of non-potable groundwater setting is determined to be applicable [O.Reg. 153/04, section 35].

The Phase Two Property is not situated within or adjacent to an area of natural significance and does not include any land within 30 m of an area of natural significance. The pH of the soil was analyzed as part of this Phase Two ESA and was found to range from 7.24 to 7.51. As such, the Phase Two Property is not considered to be an environmentally sensitive area [O.Reg. 153/04, section 41].

Review of the drilling program and borehole/monitoring well logs completed as part of this Phase Two ESA and previous investigations was completed. It was determined that greater than 2/3 of the Phase Two Property has greater than 2 m of overburden soil. The Phase Two Property is not considered a shallow soil property [O.Reg. 153/04, section 43.1].

The Phase Two Property does not include and does not have any land located within 30 m of a water body. The MECP site condition standards for use within 30 m of a water body do not apply [O.Reg. 153/04, section 43.1].

The full depth generic site condition standards, with non-potable groundwater, course textured soil, for residential property use, as specified in Table 3 of the MECP Soil, Ground Water and Sediment Standards for Use Under Part XV.1 of the Environmental Protection Act, April 15, 2011 were determined to be the applicable site condition standards for the Phase Two Property as part of this Phase Two ESA.

The soil samples BH1-20-SS5 and BH11-20-SS5 (Duplicate of BH1-20-SS5), collected from a depth of approximately 3.1-3.7 m BGS, had reported concentrations of PHC F2 range (909 μ g/g and 306 μ g/g vs. 98 μ g/g), Methylnaphthalene (7.61 μ g/g and 2.26 μ g/g vs. 0.99 μ g/g) and reported concentrations of vanadium (101 μ g/g and 104 μ g/g vs. 86 μ g/g). These samples also had respective cobalt concentrations of 20.1 μ g/g and 22.5 μ g/g compared to the site condition standard of 22 μ g/g; since the average concentration of cobalt in these samples is less than the site condition standard, the marginal exceedance in the duplicate standard is not considered to exceed the site condition standard. It should be noted that based on past investigations, it has been observed that both Cobalt and Vanadium are known to exceed MECP standards in Ottawa region natural soils, particularly clay.

The soil sample BH3-20-SS6, collected from a depth of approximately 3.8-4.4 m BGS, had reported concentrations of PHC F1 range (117 μ g/g vs. 55 μ g/g), PHC F2 range (110 μ g/g vs. 98 μ g/g), benzene (3.02 μ g/g vs. 0.21 μ g/g), ethylbenzene (59 μ g/g vs. 2 μ g/g), toluene (73.5 μ g/g vs. 2.3 μ g/g) and xylenes (276 μ g/g vs. 3.1 μ g/g). Additionally, PAH exceedances from the same soil sample included Methylnaphthalene (1.95 μ g/g vs. 0.99 μ g/g) and Naphthalene (1.69 μ g/g vs. 0.6 μ g/g).

The groundwater samples BH3-20 and BH13-20 (Duplicate of BH3-20), collected from a screen depth of approximately 2.5-5.5 m BGS, had reported concentrations of PHC F1 range (3,600 μ g/g and 3,790 μ g/g vs. 750 μ g/g), PHC F2 range (52,400 μ g/g and 2,260 μ g/g vs. 150 μ g/g), PHC F3 range (3,940 μ g/g vs. 500 μ g/g), benzene (19,300 μ g/g and 19,700 μ g/g vs. 44 μ g/g), ethylbenzene (3,800 μ g/g and 3,700 μ g/g vs. μ g/g), toluene (65,200 μ g/g and 60,900 μ g/g vs. 18,000 μ g/g) and xylenes (27,600 μ g/g and 26,600 μ g/g vs. 4,200 μ g/g). Lead was also reported at concentrations of 51.6 μ g/g and 54.6 μ g/g vs. 25 μ g/g.

All of the other soil and groundwater results for the Phase Two Property are in compliance with the applicable site condition standards. The Phase Two Property is not in compliance with the site condition standards as of the certification date of June 30, 2020.

7. Conclusions

The soil samples BH1-20-SS5 and BH11-20-SS5 (Duplicate of BH1-20-SS5), collected from a depth of approximately 3.1-3.7 m BGS, had reported concentrations of PHC F2 range (909 μ g/g and 306 μ g/g vs. 98 μ g/g), Methylnaphthalene (7.61 μ g/g and 2.26 μ g/g vs. 0.99 μ g/g) and reported concentrations of vanadium (101 μ g/g and 104 μ g/g vs. 86 μ g/g). These samples also had respective cobalt concentrations of 20.1 μ g/g and 22.5 μ g/g compared to the site condition standard of 22 μ g/g; since the average concentration of cobalt in these samples is less than the site condition standard, the marginal exceedance in the duplicate standard is not considered to exceed the site condition standard. It should be noted that based on past investigations, it has been observed that both Cobalt and Vanadium are known to exceed MECP standards in Ottawa region natural soils, particularly clay.

The soil sample BH3-20-SS6, collected from a depth of approximately 3.8-4.4 m BGS, had reported concentrations of PHC F1 range (117 μ g/g vs. 55 μ g/g), PHC F2 range (110 μ g/g vs. 98 μ g/g), benzene (3.02 μ g/g vs. 0.21 μ g/g), ethylbenzene (59 μ g/g vs. 2 μ g/g), toluene (73.5 μ g/g vs. 2.3 μ g/g) and xylenes (276 μ g/g vs. 3.1 μ g/g). Additionally, PAH exceedances from the same soil sample included Methylnaphthalene (1.95 μ g/g vs. 0.99 μ g/g) and Naphthalene (1.69 μ g/g vs. 0.6 μ g/g).

The groundwater samples BH3-20 and BH13-20 (Duplicate of BH3-20), collected from a screen depth of approximately 2.5-5.5 m BGS, had reported concentrations of PHC F1 range (3,600 μ g/g and 3,790 μ g/g vs. 750 μ g/g), PHC F2 range (52,400 μ g/g and 2,260 μ g/g vs. 150 μ g/g), PHC F3 range (3,940 μ g/g vs. 500 μ g/g), benzene (19,300 μ g/g and 19,700 μ g/g vs. 44 μ g/g), ethylbenzene (3,800 μ g/g and 3,700 μ g/g vs. μ g/g), toluene (65,200 μ g/g and 60,900 μ g/g vs. 18,000 μ g/g) and xylenes (27,600 μ g/g and 26,600 μ g/g vs. 4,200 μ g/g). Lead was also reported at concentrations of 51.6 μ g/g and 54.6 μ g/g vs. 25 μ g/g.

All of the other soil and groundwater results for the Phase Two Property are in compliance with the applicable site condition standards. The Phase Two Property is not in compliance with the site condition standards as of the certification date of June 30, 2020.

It is suspected that remnant soil and/or groundwater contamination may be present near the east Property limits of the Phase Two Property based on historical sampling data, however, this could not be confirmed as part of this Phase Two ESA due to physical impediments (fencing) during the drilling program. Additional investigation and confirmation of soil and groundwater quality in this area of the Property is recommended at the time of excavation for site redevelopment. It should be noted that the proposed redevelopment includes excavation for at least two to three levels of underground parking, which is expected to be sufficient for remediation of the aforementioned environmental contamination at the Phase Two Property.

LOPERS & ASSOCIATES

An environmental remediation program, including the bulk removal and off-site disposal of soil and groundwater in excess of the site condition standards is recommended for the Phase Two Property. Given the scope and timeline for the proposed redevelopment and the requirements for specialized construction techniques to complete remediation of the Phase Two Property to meet the site condition standards, it is recommended that remediation be completed in conjunction with redevelopment of the Property.

Further delineation and confirmation of remediation sampling will be required prior to the completion of an environmental remediation and program and confirmation of compliance with the site condition standards; however, these tasks can be completed at the time decommissioning and demolition of existing structures at the Phase Two Property. The submission of a record of site condition would be required in the event of a change of zoning of the Phase Two Property; however, these tasks can be completed at the time decommissioning and demolition of existing structures at the Phase Two Property; however, these tasks can be completed at the time decommissioning and demolition of existing structures at the Phase Two Property. The Phase Two ESA could be then updated at that time to show compliance with site condition standards.

Preparation of a soil management plan in accordance with O.Reg. 406/19 will be required as part of management of excess soil generated as part of construction activities. It is recommended that a remedial action plan be prepared to develop a strategy for remediation, including soil and groundwater management, during redevelopment.

i. Signatures

The Qualified Person for this study is Mr. Luke Lopers, P. Eng. Mr. Lopers has been a Professional Engineer, registered in Ontario since 2012 and has been working on environmental site assessments since 2006. Mr. Lopers has been an author, project manager and/or peer reviewer for hundreds of Phase One ESAs and Phase Two ESAs as well as previously filed RSCs.

The reviewer for this study is Mr. Don Plenderleith, P.Eng. Mr. Plenderleith is a Professional Engineer registered in Ontario since 1994 and has authored and/or reviewed hundreds of Phase One and Two ESAs in Ontario and the rest of Canada. The qualifications of the assessor/Qualified Person and reviewer are included in Appendix F.

Sincerely,

Luke Lopers, P.Eng., QP_{ESA}



Don Plenderletto

Don Plenderleith, P.Eng., QP_{ESA}

8. Limitations

The findings and conclusions of this Phase Two ESA are based on the information provided and/or reviewed as part of this study.

This Phase Two ESA has been completed with the standard of care generally expected in the industry for a study of this nature.

This Phase Two ESA has been prepared for the sole use of 11684663 Canada Inc. for the purposes of a due diligence assessment of the potential liabilities which may exist at the Phase Two Property. No other party is permitted to rely on the conclusions or findings of this report without the written consent of Lopers & Associates and 11684663 Canada Inc.

Changes to the physical setting of the Phase Two Property, Phase One Study Area and applicable regulations governing Phase One and Two Environmental Site Assessments have the potential to influence the validity of the conclusions and opinions presented in this Phase Two ESA.

9. References

Legal Survey Plan, Fairhall, Moffatt & Woodland Limited, dated January 8, 2018.

City of Ottawa, geoOttawa mapping website, Visited June through July, 2020. <u>http://maps.ottawa.ca/geoottawa/</u>

Google Earth, Visited June through July, 2020.

"Soil, Ground Water and Sediment Standards for Use Under Part XV.1 of the Environmental Protection Act", produced by the Ontario Ministry of the Environment, dated April 15, 2011.

"Phase One Environmental Site Assessment, 729 Ridgewood Avenue, Ottawa, Ontario" dated July 27, 2020 prepared for 11684663 Canada Inc. by Lopers & Associates.

"Phase II Environmental Site Assessment, 729 Ridgewood Avenue, Ottawa, Ontario", dated January 12, 2018, completed by Pinchin Ltd. for Canadian Rental Development Services Inc.

"Verification Soil Sampling Program, 729 Ridgewood Avenue, Ottawa, Ontario", dated October 19, 2018, completed by Pinchin Ltd. for 561226 Ontario Inc.

Paracel Certificate of Analysis - Report # 2026367 - Soil Sample Submission June 24, 2020

Paracel Certificate of Analysis - Report # 2026368 - TCLP Sample Submission June 24, 2020

Paracel Certificate of Analysis – Report # 2027199 - Groundwater Sample Submission June 30, 2020

10. Appendices

- Appendix A Sampling and Analysis Plan
- Appendix B Underground Utility Locates
- Appendix C Borehole Logs
- Appendix D Certificates of Equipment Calibration
- Appendix E Laboratory Certificates of Analysis
- Appendix F Qualifications of Assessors

Figures





Tables

Table 5: Soil Analytical Results

729 Ridge	ewod Avenue,	, Ottawa,	Ontaric

			Sample Location:	BH1-20-SS5	BH11-20-SS5	BH3-20-SS6	BH4-20-SS6	BH5-20-SS4	BH5-20-SS7
			Sample Depth:	3.1-3.7 m BGS	3.1-3.7 m BGS	3.8-4.4 m BGS	3.8-4.4 m BGS	2.3-2.9 m BGS	4.6-5.2 m BGS
			Sample Date:	June 24, 2020					
			Laborartory Sample ID:	2026367-01	2026367-06	2026367-02	2026367-03	2026367-04	2026367-05
			MECP Table 3: Residential						
		Method Detection Limit	Property Use Standard						
Parameter	Units	(MDL)	Coarse Grain Soil						
Petroluem Hydrocarbons (PHCs)		7		22	24	117	ND	ND	ND
F1 PHCs (C6-C10) F2 PHCs (C10-C16)	ug/g ug/g	4	55 98	909	34 306	117	ND	ND	ND
F3 PHCs (C16-C34)	ug/g	8	300	102	25	11	ND	ND	ND
F4 PHCs (C34-C50)	ug/g	6	2800	ND	ND	ND	ND	ND	ND
F4G PHCs (gravimetric)	ug/g	50	2800	-	-	-	-	-	-
Volatile Organic Compounds (VOCs)		0.50	10	ND	ND				ND
Acetone	ug/g	0.50	16	ND		- 3.02	- ND	- ND	
Bromodichloromethane	ug/g	0.05	13	ND	ND	-	-	-	ND
Bromoform	ug/g	0.05	0.27	ND	ND	-	-	-	ND
Bromomethane	ug/g	0.05	0.05	ND	ND	-	-	-	ND
Carbon Tetrachloride	ug/g	0.05	0.05	ND	ND	-	-	-	ND
Chlorobenzene	ug/g	0.05	2.4	ND ND	ND	-	-	-	ND
Dibromochloromethane	ug/g	0.05	9.4	ND	ND	-	-	-	ND
Dichlorodifluoromethane	ug/g	0.05	16	ND	ND	-	-	-	ND
1,2-Dichlorobenzene	ug/g	0.05	3.4	ND	ND	-	-	-	ND
1,3-Dichlorobenzene	ug/g	0.05	4.8	ND	ND	-	-	-	ND
1,4-Dichlorobenzene	ug/g	0.05	0.083	ND	ND	-	-	-	ND
1.2-Dichloroethane	ug/g ug/g	0.05	0.05	ND	ND	-	-	-	ND
1,1-Dichloroethylene	ug/g	0.05	0.05	ND	ND	-	-	-	ND
cis-1,2-Dichloroethylene	ug/g	0.05	3.4	ND	ND	-	-	-	ND
trans-1,2-Dichloroethylene	ug/g	0.05	0.084	ND	ND	-	-	-	ND
1,2-Dichloropropane	ug/g	0.05	0.05	ND	ND	-	-	-	ND
trans-1,3-Dichloropropylene	ug/g ug/g	0.05		ND	ND	-	-	-	ND
1,3-Dichloropropene, total	ug/g	0.05	0.05	ND	ND	-	-	-	ND
Ethylbenzene	ug/g	0.05	2	ND	ND	59	ND	ND	ND
Ethylene dibromide (dibromoethane, 1,2-)	ug/g	0.05	0.05	ND	ND	-	-	-	ND
Hexane	ug/g	0.05	2.8	ND	ND	-	-	-	ND
Methyl Isobutyl Ketone	ug/g	0.50	10	ND	ND	-	-	-	ND
Methyl tert-butyl ether	ug/g	0.05	0.75	ND	ND	-	-	-	ND
Methylene Chloride	ug/g	0.05	0.1	ND	ND	-	-	-	ND
Styrene	ug/g	0.05	0.7	ND	ND	-	-	-	ND
1,1,1,2-Tetrachloroethane	ug/g	0.05	0.058	ND	ND	-	-	-	ND
1,1,2,2-Tetrachioroethane	ug/g	0.05	0.05	ND ND	ND	-	-	-	ND
Toluene	ug/g	0.05	2.3	ND	ND	73.5	ND	ND	ND
1,1,1-Trichloroethane	ug/g	0.05	0.38	ND	ND	-	-	-	ND
1,1,2-Trichloroethane	ug/g	0.05	0.05	ND	ND	-	-	-	ND
Trichloroethylene	ug/g	0.05	0.061	ND	ND	-	-	-	ND
Vinyl Chloride	ug/g	0.05	4	ND ND		-	-	-	
m/p-Xylene	ug/g	0.05	NV	ND	0.08	196	ND	ND	ND
o-Xylene	ug/g	0.05	NV	ND	ND	80.3	ND	ND	ND
Xylenes, total	ug/g	0.05	3.1	ND	0.08	276	ND	ND	ND
Polycyclic Aromatic Hydrocarbons	ug/g	0.02	7.0	0.07	0.05	ND	ND		
Acenaphthylene	ug/g	0.02	0.15	0.11	0.03	ND	ND	-	-
Anthracene	ug/g	0.02	0.67	ND	ND	ND	ND	-	-
Benzo[a]anthracene	ug/g	0.02	0.5	ND	ND	ND	ND	-	-
Benzo[a]pyrene	ug/g	0.02	0.3	ND	ND	ND	ND	-	-
Benzo[b]fluoranthene	ug/g	0.02	0.78	ND ND	ND ND	ND	ND	-	-
Benzo[k]fluoranthene	ug/g	0.02	0.78	ND	ND	ND	ND	-	-
Chrysene	ug/g	0.02	7	ND	ND	ND	ND	-	-
Dibenzo[a,h]anthracene	ug/g	0.02	0.1	ND	ND	ND	ND	-	-
Fluoranthene	ug/g	0.02	0.69	ND 0.20	ND	ND	ND	-	-
Indeno[1 2 3-cd]ovrene	ug/g	0.02	0.38	0.26 ND	U.I ND		ND	-	-
1-Methylnaphthalene	ug/g	0.02	0.99	3.02	0.89	0.58	ND	-	-
2-Methylnaphthalene	ug/g	0.02	0.99	4.58	1.38	1.37	ND	-	-
Methylnaphthalene (1&2)	ug/g	0.04	0.99	7.61	2.26	1.95	ND	-	-
Naphthalene	ug/g	0.01	U.6	0.5	0.26	1.69 ND	ND ND	-	-
Pyrene	ug/g	0.02	78	ND	ND	ND	ND	-	-
Metals		ł							
Boron, available	ug/g	0.5	1.5	ND	ND	-	ND	-	-
Chromium (VI)	ug/g	0.2	8	0.3	0.3	-	ND	-	-
Antimony	ug/g ug/g	1.0	7.5	ND	ND	-	ND	-	-
Arsenic	ug/g	1.0	18	3.9	4	-	3.7	-	-
Barium	ug/g	1.0	390	284	327	-	46.3	-	-
Beryllium	ug/g	0.5	4	0.7	0.9	-	ND	-	-
Boron	ug/g	5.0	120	5.8 ND	7.2 ND	-	6.5 ND	-	-
Chromium	ug/g	5.0	160	104	126	-	13.1	-	-
Cobalt	ug/g	1.0	22	20.1	22.5	-	5.6	-	-
Copper	ug/g	5.0	140	45.5	49	-	12.4	-	-
Lead	ug/g	1.0	120	6.1	6.8	-	5.2	-	-
iviolybdenum Nickel	ug/g	1.0	6.9 100	ND 56 1		-	ND	-	-
Selenium	ug/g	1.0	2.4	ND	55.4 ND	-	ND	-	-
Silver	ug/g	0.3	20	ND	ND	-	ND	-	-
Thallium	ug/g	1.0	1	ND	ND	-	ND	-	-
Uranium Vene diver	ug/g	1.0	23	ND	ND	-	ND	-	-
vanadium Zinc	ug/g	10.0 20 0	80 340	101 02 7	104	-	23.5 ND	-	-
General Inorganics	ч <u>6</u> /б	1 20.0	1 570	JL.1	114		שא		
SAR	N/A	0.01	5	0.76	0.81	-	1.12	-	-
Conductivity	uS/cm	5	700	636	670	-	627	-	-
Cyanide, free	ug/g	0.03	0.051	ND	ND	-	ND	-	-
μu	pH Units	0.05	INV NV	/.14	/.33	-	7.4	-	-

NV - No value listed in MECP site condition standards

- - Not Analyzed

ND - Not detected above laboratory method detection limits

Exceeds MECP site condition standards

Table 6: TCLP Analytical Results 729 Ridgewood Avenue, Ottawa, Ontario

			Sample ID:	TCLP
			Laborartory Sample ID:	2026368-01
			Sample Date:	June 24, 2020
		Method	Reg 558	
		Detection Limit	Schedule IV	
Parameter	Units	(MDL)		
Physical Characteristics	•			
Flashpoint	°C			>70
TCLP Leachate Inorganics		•		
Fluoride	mg/L	0.05	150	0.32
Nitrate as N	mg/L	1	1000	ND
Nitrite as N	mg/L	1	1000	ND
Cyanide, free	mg/L	0.02	20	ND
TCLP Leachate Metals				
Mercury	mg/L	0.005	0.1	ND
Arsenic	mg/L	0.05	2.5	ND
Barium	mg/L	0.05	100	0.97
Boron	mg/L	0.05	500	0.05
Cadmium	mg/L	0.01	0.5	ND
Chromium	mg/L	0.05	5	ND
Lead	mg/L	0.05	5	ND
Selenium	mg/L	0.05	1	ND
Silver	mg/L	0.05	5	ND
Uranium	mg/L	0.05	10	ND
TCLP Leachate Volatiles			•	
Benzene	mg/L	0.005	0.5	ND
Carbon Tetrachloride	mg/L	0.005	0.5	ND
Chlorobenzene	mg/L	0.004	8	ND
Chloroform	mg/L	0.006	10	ND
1,2-Dichlorobenzene	mg/L	0.004	20	ND
1,4-Dichlorobenzene	mg/L	0.004	0.5	ND
1,2-Dichloroethane	mg/L	0.005	0.5	ND
1,1-Dichloroethylene	mg/L	0.006	1.4	ND
Methyl Ethyl Ketone (2-Butanone)	mg/L	0.30	200	ND
Methylene Chloride	mg/L	0.04	5	ND
Tetrachloroethylene	mg/L	0.005	3	ND
Trichloroethylene	mg/L	0.004	5	ND
Vinyl Chloride	mg/L	0.005	0.2	ND
TCLP Leachate Organics			•	
Benzo[a]pyrene	mg/L	0.0001	0.13	ND
Benzo[a]pyrene	mg/L	0.0001	0.001	ND
Nitrobenzene	mg/L	0.001	2	ND
Hexachloroethane	mg/L	0.001	3	ND
Hexachlorobenzene	mg/L	0.050	0.13	ND
Hexachlorobutadiene	mg/L	0.001	-	ND
2,3,4,6-Tetrachlorophenol	mg/L	0.002	10	ND
2,4,5-Trichlorophenol	mg/L	0.001	400	ND
2,4,6-Trichlorophenol	mg/L	0.001	0.5	ND
2,4-Dichlorophenol	mg/L	0.001	90	ND
2-Methylphenol	mg/L	0.001	200	ND
3/4-Methylphenol	mg/L	0.001	200	ND
Pentachlorophenol	mg/L	0.005	6	ND
PCBs, total	mg/L	0.003	0.3	ND
,	0/ =			.=

ND - Not detected above laboratory method detection limits

Table 7: Groundwater Analytical Results

729 Ridgewood Avenue, Ottawa, Ontario

			Sample Location:	Bill of	DUC OF	BH13-20	DUE -5			
				BH1-20	BH3-20	Duplicate of BH3-20	BH5-20	MW-6	MW-8	Trip Blank
			Sample Date: Laborartory Sample ID:	June 30, 2020 2027199-01	June 30, 2020 2027199-02	June 30, 2020 2027199-04	June 30, 2020 2027199-03	June 30, 2020 2027199-05	June 30, 2020 2027199-06	June 30, 2020 2027199-07
		Mothod	· · ·							
		Detection Limit	MECP Table 3 Standards							
Parameter Petroluem Hydrocarbons (PHCs)	Units	(MDL)	Coarse Grain Soil							
F1 PHCs (C6-C10)	ug/L	25	750	123	3600	3790	ND	ND	ND	-
F2 PHCs (C10-C16) F3 PHCs (C16-C34)	ug/L ug/L	100 100	150 500	ND ND	52400 3940	2260 ND	ND ND	ND ND	ND ND	-
F4 PHCs (C34-C50)	ug/L	100	500	ND	ND	ND	ND	ND	ND	-
Volatile Organic Compounds (VOCs Acetone) ug/L	5.0	130000	ND	ND (2500)	ND (2500)	-	-	-	ND
Benzene	ug/L	0.5	44	ND	19300	19700	-	-	-	ND
Bromodichloromethane Bromoform	ug/L ug/L	0.5 0.5	85000 380	ND ND	ND (250) ND (250)	ND (250) ND (250)	-	-	-	ND ND
Bromomethane	ug/L	0.5	5.6	ND	ND (250)	ND (250)	-	-	-	ND
Chlorobenzene	ug/L ug/L	0.2	630	ND	ND (100) ND (250)	ND (100) ND (250)	-	-	-	ND
Chloroform	ug/L	0.5	2.4	ND	ND (250)	ND (250)	-	-	-	ND
Dichlorodifluoromethane	ug/L ug/L	1.0	4400	ND	ND (250) ND (500)	ND (250) ND (500)	-	-	-	ND
1,2-Dichlorobenzene	ug/L	0.5	4600	ND	ND (250)	ND (250)	-	-	-	ND
1,3-Dichlorobenzene	ug/L ug/L	0.5	8	ND	ND (250) ND (250)	ND (250) ND (250)	-	-	-	ND
1,1-Dichloroethane	ug/L	0.5	320	ND	ND (250)	ND (250)	-	-	-	ND
1,2-Dichloroethane 1,1-Dichloroethylene	ug/L ug/L	0.5	1.6 1.6	ND ND	ND (250) ND (250)	ND (250) ND (250)	-	-	-	ND ND
cis-1,2-Dichloroethylene	ug/L	0.5	1.6	ND	ND (250)	ND (250)	-	-	-	ND
trans-1,2-Dichloroethylene	ug/L	0.5	1.6 16	ND ND	ND (250)	ND (250)	-	-	-	ND ND
cis-1,3-Dichloropropylene	ug/L	0.5	NV	ND	ND (250)	ND (250)	-	-	-	ND
trans-1,3-Dichloropropylene	ug/L	0.5	NV E 2	ND	ND (250)	ND (250)	-	-	-	ND
Ethylbenzene	ug/L ug/L	0.5	2300	ND	עאו (250) <u>3800</u>	עאו (250) 3700	-	-	-	ND
Ethylene dibromide (dibromoethane, 1,	ug/L	0.2	0.25	ND	ND (100)	ND (100)	-	-	-	ND
Hexane Methyl Ethyl Ketone (2-Butanone)	ug/L ug/L	1.0 5.0	51 470000	ND ND	ND (500) ND (2500)	ND (500) ND (2500)	-	-	-	ND ND
Methyl Isobutyl Ketone	ug/L	5.0	140000	ND	ND (2500)	ND (2500)	-	-	-	ND
Methyl tert-butyl ether Methylene Chloride	ug/L	2.0 5.0	190 610	ND ND	ND (1000) ND (2500)	ND (1000) ND (2500)	-	-	-	ND ND
Styrene	ug/L	0.5	1300	ND	ND (2500)	ND (250)	-	-	-	ND
1,1,1,2-Tetrachloroethane	ug/L	0.5	3.3	ND	ND (250)	ND (250)	-	-	-	ND
Tetrachloroethylene	ug/L ug/L	0.5	1.6	ND	ND (250) ND (250)	ND (250) ND (250)	-	-	-	ND
Toluene	ug/L	0.5	18000	ND	65200	60900	-	-	-	ND
1,1,1-Trichloroethane	ug/L ug/L	0.5	4.7	ND ND	ND (250) ND (250)	ND (250) ND (250)	-	-	-	ND ND
Trichloroethylene	ug/L	0.5	1.6	ND	ND (250)	ND (250)	-	-	-	ND
Trichlorofluoromethane Vinyl Chloride	ug/L ug/L	1.0 0.5	2500 0.5	ND ND	ND (500) ND (250)	ND (500) ND (250)	-	-	-	ND ND
m/p-Xylene	ug/L	0.5	NV	ND	19200	18200	-	-	-	ND
o-Xylene Xylenes, total	ug/L ug/L	0.5 0.5	NV 4200	ND ND	8400 27600	8320 26600	-	-	-	ND ND
Polycyclic Aromatic Hydrocarbons	~8/ L	0.0			27000	20000		I		no
Acenaphthene Acenaphthylene	ug/L	0.05	600 1.8	0.12	0.29	0.21 ND	ND ND	-	-	-
Anthracene	ug/L	0.01	2.4	ND	0.04	0.03	ND	-	-	-
Benzo[a]anthracene	ug/L	0.01	4.7	0.02	ND	ND	ND	-	-	-
Benzo[b]fluoranthene	ug/L	0.01	0.75	ND	ND	ND	ND	-	-	-
Benzo[g,h,i]perylene	ug/L	0.05	0.2	ND	ND	ND	ND	-	-	-
Chrysene	ug/L ug/L	0.05	1	ND	ND	ND	ND	-	-	-
Dibenzo[a,h]anthracene	ug/L	0.05	0.52	ND	ND	ND	ND	-	-	-
Fluoranthene Fluorene	ug/L ug/L	0.01 0.05	130 400	0.21 0.28	0.04 0.41	0.01 0.27	0.19 ND	-	-	-
Indeno[1,2,3-cd]pyrene	ug/L	0.05	0.2	ND	ND	ND	ND	-	-	-
1-Methylnaphthalene 2-Methylnaphthalene	ug/L ug/L	0.05 0.05	1800 1800	18.2 10.4	50.8 87.3	37.8 79.7	ND ND	-	-	-
Methylnaphthalene (1&2)	ug/L	0.10	1800	28.6	138	118	ND	-	-	-
Naphthalene Phenanthrene	ug/L ug/L	0.05 0.05	1400 580	11.3 0.69	419 0.34	392 0.19	0.08 0.43	-	-	-
Pyrene	ug/L	0.01	68	0.4	0.07	0.03	0.37	-	-	-
Metals Mercury	ug/L	0.1	0.29	ND	ND	ND	-	-	-	-
Antimony	ug/L	0.5	20000	ND	ND	ND	-	-	-	-
Arsenic Barium	ug/L	1	1900	1 874	1 1880	1 1880	-	-	-	-
Beryllium	ug/L	0.5	67	ND	ND	ND	-	-	-	-
Boron	ug/L	10	45000	27 ND	25	26	-	-	-	-
Chromium	ug/L	1	810	ND	ND	ND	-	-	-	-
Chromium (VI) Cabalt	ug/L	10	140	ND	ND	ND	-	-	-	-
Copper	ug/L ug/L	0.5	ьь 87	11.3 1.2	6.5 1	6.8 1.2	-	-	-	-
Lead	ug/L	0.1	25	ND	51.2	54.6	-	-	-	-
iviolybdenum Nickel	ug/L ug/L	0.5	9200 490	1.2 23	1.4 12	1.4 13	-	-	-	-
Selenium	ug/L	1	63	ND	3	3	-	-	-	-
Silver Sodium	ug/L	0.1	1.5	ND 174000	ND 202000	ND 209000	-	-	-	-
Thallium	ug/L	0.1	510	ND	ND	ND	-	-	-	-
Uranium Vanadium	ug/L	0.1	420	9.2	11.2	11.2	-	-	-	-
Zinc	ug/L ug/L	5	1100	1.2 ND	1.1 7	I.2 ND	-	-	-	-
General Inorganics		-				10				
cyanide, tree pH	ug/L pH Units	2 0.1	66 NV	ND 7	ND 7.2	ND 7.2	-	-	-	-
Chloride	mg/L	1	2300	918	833	828	-	-	-	-

NV - No value listed in MECP site condition standards

- - Not Analyzed

ND - Not detected above laboratory method detection limits

ND(250) - Not detected above elevated laboratory method detection limits due to high analyte concentrations. Elevated MDL listed in "()" Exceeds MECP site condition standards

Appendix A

Sampling and Analysis Plan
Sampling and Analysis Plan

729 Ridgewood Avenue Ottawa, Ontario

Prepared for: 11684663 Canada Inc.



6/22/2020

Table of Contents

1.	Bac	Background1				
2.	Pla	nning Site Investigation - Specific Objectives	.2			
3.	Und	derground Utility Service Locates	.2			
4.	Pla	nning Site Investigation - Specific Requirements	.3			
	i.	Media for Investigation	.3			
	ii.	Locations and Depths for Sampling	.3			
	iii.	Parameters for Laboratory Analysis.	.3			
5.	Qua	ality Assurance and Quality Control	.4			
5	.1	Field Equipment Decontamination	.4			
5	.2	Trip Blanks	.5			
5	.3	Field Duplicates	.5			
5	.4	Equipment Calibration	.5			
5	.5	Data Quality Objectives	.5			
6.	Sta	ndard Operating Procedures	.6			
6	.1	Borehole Drilling	.6			
6	.2	Soil Sampling	.6			
6	.3	Field Soil Screening Measurements	.6			
6	.4	Monitoring Well Installation	.7			
6	.5	Elevation Survey	.7			
6	.6	Monitoring Well Development;	.7			
6	5.7	Field Measurement of Water Quality Indicators	.8			
6	.8	Groundwater Sampling	.8			

1. Background

Lopers & Associates (Lopers) was retained by 11684663 Canada Inc. (Brigil) to complete a Phase Two Environmental Site Assessment (Phase Two ESA) of the commercial property with Civic address No. 729 Ridgewood Avenue, Ottawa, Ontario ("Phase Two Property", "Property" or "Site").

Lopers has previously completed a Phase One Environmental Site Assessment (Phase One ESA) for Brigil at the Property. The Phase One ESA identified the presence of four potentially contaminating activities (PCAs) at the Property which were interpreted to represent areas of potential environmental concern (APECs).

The presence of a former retail fuel outlet and automotive service garage on the southeast portion of the Phase One Property are a significant potentially contaminating activities (PCAs) which represents areas of potential environmental concern (APECs) for the Property. Given that previous reports were provided which document remnant petroleum hydrocarbon (PHC) and benzene, toluene, ethylbenzene and xylenes (BTEX) soil contamination and that groundwater quality was not confirmed following the completion of a remediation program, further investigation is warranted. The contaminants of potential concern associated with retail fuelling are generally PHCs and BTEXs, and metals as this was an older facility and lead was historically present in gasoline. Based on historical soil analysis in this area of the Property, polycyclic aromatic hydrocarbons (PAH) and volatile organic compounds (VOCs) are also considered contaminants of potential concern associated with the former automotive garage operations.

The practice of backfilling following demolition activities at the Phase One Property is also a significant PCA which represents an APEC for the Property. Given that no reports were provided with analytical data to support the environmental quality of the backfill used to fill the former residential building footprint on the central-south portion of the Property, this area warrants further investigation. The contaminants of potential concern commonly found in poor environmental quality backfill are PHCs/BTEXs, PAHs and metals.

The presence of an active automotive service garage was observed during the Site walk over on the central portion of the Phase One Property at the time of the Site Investigation. Although this garage has only been operating for a short time period (2017 to present), these operations are a PCA which represents an APEC for the Property. Based on the observations at this automotive garage, that contaminants of potential concern are considered to be PHCs and BTEXs.

The scope of work for the Phase Two ESA includes drilling 5 boreholes at the Phase Two Property.

Three of the boreholes will be instrumented with groundwater monitoring wells with screens installed in the overburden. The two existing groundwater monitoring wells at the Phase Two Property, which were installed as part of historical investigations, may also be accessed and sampled to supplement the groundwater quality assessment.

In the event that additional contaminants of APECs are identified during the drilling or sampling fieldwork, additional scope of work will be discussed with Brigil to complete the Phase Two ESA.

Planning Site Investigation - Specific Objectives

The following are the specific objectives for planning a site investigation of the Phase Two Environmental Site Assessment, as defined in O.Reg. 153/04.

1. To plan an investigation that will achieve the general objectives of a Phase Two Environmental Site Assessment,

i. through the use of an appropriate and complete information base concerning the Phase Two Property, and

ii. through the conduct of an investigation based both on information obtained before the Phase Two Environmental Site Assessment begins and on the incorporation of information obtained during the Phase Two Environmental Site Assessment.

2. To develop a sampling and analysis plan that will adequately assess all areas of the Phase Two Property where contaminants may be present in land or water on, in or under the Property.

3. To develop a quality assurance program that is designed to effectively limit errors and bias in sampling and analysis through implementation of assessment and control measures that will ensure data are useful, appropriate and accurate in the determination of whether the Phase Two Property, or any record of site condition (RSC) property within it, meets applicable site condition standards and any standards specified in a risk assessment.

3. Underground Utility Service Locates

Prior to completing the Phase Two ESA field investigation activities, public underground locates will be coordinated through Ontario One Call. Privately owned underground services and infrastructure are present at the Phase Two Property, as such, private locates were undertaken by USL-1 Underground Service Locators Inc.

The locations of the proposed boreholes will be reviewed in relation to the public underground locates and locations will be modified accordingly if conflicts exist between any location or if the location is in close proximity to an active underground service.

A copy of the public and private underground locates will be retained by Lopers' field personnel during all excavation components of the fieldwork.

Planning Site Investigation - Specific Requirements

The qualified person has ensured the following requirements were met in planning a site investigation. The Phase One conceptual site model for the Phase One Environmental Site Assessment report was used in conjunction with other information in determining:

i. Media for Investigation

Soil and groundwater sampling and analysis for the purpose of assessing environmental quality will be completed as part of the Phase Two ESA.

There are no surface water bodies at the Phase Two Property, as such, sediment and surface water quality sampling and analysis will not be completed as part of this Phase Two ESA.

ii. Locations and Depths for Sampling

A total of three borehole locations have been proposed to provide coverage of the APECs identified at the Phase Two Property. Boreholes will be located in the southeast portion of the Property to assess APEC #1 /2. One borehole has been proposed in the central-south portion of the Property to assess APEC #3. One borehole has been proposed in the central portion of the Phase Two Property to assess APEC #4.

Sampling depths will include as a minimum, collection of samples in 0.6 m intervals from the ground surface to native soil conditions within the groundwater table. Borehole/monitoring wells depths are proposed to be drilled to approximately 4 m to 6 m BGS to intercept the groundwater table in APECs were groundwater quality assessment is required.

iii. Parameters for Laboratory Analysis.

The parameters for laboratory analysis will be selected based on the contaminants of potential concern for each APEC as well as the field screening observations.

The contaminants of potential concern associated with retail fueling and a former automotive service garage (APEC #1 / 2) are generally PHCs and BTEXs, with older facilities also having concerns associated with metals, as lead was historically present in gasoline. Based on historical

soil analysis in this area of the Property, PAH and VOCs are also considered contaminants of potential concern associated with the former automotive garage operations.

The contaminants of potential concern commonly found in poor environmental quality backfill (APEC #3) are PHCs/BTEXs, PAHs and metals.

Based on the observations at the active automotive garage (APEC #4), the contaminants of potential concern are considered to be PHCs and BTEXs.

The contaminants of concern for a particular sample will be based on the relative location and depth of the sample, visual and/or olfactory observations and combustible vapour screening concentrations.

Information obtained after the completion of the phase one environmental site assessment shall be used to modify the investigation, as appropriate.

5. Quality Assurance and Quality Control

The qualified person has ensured that there is a quality assurance and quality control program, data quality objectives, standard operating procedures and a description of any physical impediments that interfere with or limit the ability to conduct sampling and analysis.

The quality assurance and quality control program includes the following requirements:

5.1 Field Equipment Decontamination

All non-dedicated sampling and monitoring equipment must be cleaned following each use.

The split spoons, which are the only media to come into contact with the soil samples, will be washed using soap and water and a scrub brush between samples to minimize the potential for cross-contamination among samples. The field technician will use sterile nitrile gloves, which are to be changed prior to the handling of each soil sample to further reduce the potential of cross-contamination. The flights of the hollow stem augers are to be cleaned manually following each borehole.

Water level monitoring equipment, including water level meters and interface probes will be decontaminated with an environmentally safe cleaning solution and rinsed with deionized water between water level readings to prevent cross contamination.

The field technician will change dedicated sterile nitrile gloves prior to initiating work at each monitoring well and change gloves prior to sample collection to minimize the potential for cross-contamination.

5.2 Trip Blanks

Since groundwater samples are to be analyzed for benzene, toluene, ethylbenzene and xylenes (BTEXs), which are components of volatile organic compounds (VOCs), one trip blank sample shall be submitted for laboratory analysis with each laboratory submission of groundwater samples.

5.3 Field Duplicates

Sufficient field duplicate samples shall be collected in each medium (soil and groundwater) being sampled, so that at least one field duplicate sample can be submitted for laboratory analysis for every ten samples submitted for laboratory analysis.

At least one field duplicate sample shall be submitted for laboratory analysis for every ten samples submitted for laboratory analysis.

One field duplicate will be submitted from each medium sampled for each parameter suite analyzed as part of this Phase Two ESA.

5.4 Equipment Calibration

Field screening of the soil samples will be completed using an RKI Instruments Model Eagle-2 combustible gas detector ("RKI Eagle"). The RKI Eagle used for soil sample screening as part of this Phase Two ESA will be obtained from Maxim Environmental and Safety Inc. and will be calibrated prior to use.

Measurements of the groundwater quality field parameters will be completed to determine stabilization of these parameters prior to sampling. These measurements will be completed using Horiba U-52 groundwater quality measurement device ("Horiba"). The Horiba used for groundwater quality parameter stabilization measurements as part of this Phase Two ESA will be obtained from Maxim Environmental and Safety Inc. and will be calibrated prior to use.

5.5 Data Quality Objectives

The data quality objectives for all types of field data collected during the Phase Two Environmental Site Assessment field investigation that set the level of uncertainty in environmental data shall be such that,

- (a) the decision-making is not affected; and
- (b) the overall objectives of the investigation are met.

6. Standard Operating Procedures

Standard operating procedures were developed for all of the following field investigation methods used in the field investigation.

6.1 Borehole Drilling

The drilling field program will be completed under full time supervision of Lopers & Associates personnel. The drilling subcontractor retained for the Phase Two ESA is George Downing Estate Drilling, located at 410 Principale Rue, Grenville-Sur-la-Rouge, Quebec, JOV 1B0. The drill rig used for the Phase Two ESA will be a track mounted CME drill, equipped with hollow stem augers and stainless steel split spoons. Operation of the drilling equipment is the responsibility of the drilling subcontractor, who is trained and competent in the operation of this equipment.

The field technician logs the drilling and recovery of soil samples from each borehole, noting the soil type, physical and environmental characteristics at each borehole location on the field borehole logs.

6.2 Soil Sampling

Samples are to be collected from auger cuttings at the ground surface for surficial samples (0-0.6 m below ground surface (m BGS)) and then using split spoons for subsequent samples. Split spoon samples are generally not collected from surficial depths, as poor recovery of loose packed fill material does not yield sufficient volume of samples required for field screening or laboratory analysis. Split spoon samples, collected in 0.6 m segments, are to be recovered at continuous 0.76 m intervals; the additional 0.16 m between split spoon samples will be overdrilled to provide undisturbed field measurement of geotechnical parameters (blow counts) and to prevent cave in materials from stratigraphic units above the intended sampling intervals from being collected at unrepresentative depths during sampling.

Soil samples are initially collected in Ziploc bags for initial screening as part of sample selection. Soil samples selected for laboratory analysis are collected in dedicated clear glass jars prepared and provided by the analytical laboratory. Soil samples collected for BTEXs/VOCs and the F1 range of PHCs analysis are collected using a dedicated graduated syringe provided by the laboratory and placed directly into a glass vial with methanol preservative. Analytes and associated preservatives are specified on each jar/vial by the laboratory. Each jar/vial sample set is provided with a unique sample identifier, project number and date of sampling in the field.

6.3 Field Soil Screening Measurements

Initial field screening of the soil samples will consist of visual and olfactory observations made at the time of sample collection during the drilling program.

Additional field screening of the soil samples will be completed using an RKI Instruments Model Eagle-2 combustible gas detector ("RKI Eagle"). The RKI Eagle is capable of measuring combustible vapours at concentrations ranging from 0 parts per million (PPM) to 50% of the lower explosive limit (LEL). The RKI Eagle is also capable of measuring VOC vapours at concentrations ranging from 0 ppm.

6.4 Monitoring Well Installation

Installation of monitoring wells in selected boreholes is to be completed by George Downing Estate Drilling, who is a licensed well driller in accordance with O.Reg. 903. The wells will be installed using slotted PVC No. 10 monitoring well screens, which are 51 mm in diameter; these screens are to be installed at the base of each of the aforementioned boreholes, directly above the bedrock surface. Well screens can range from 1.5 m to 4.5 m in length. The monitoring wells are extended to approximately 1.0 m above the surface grade with PVC riser, also 51 mm in diameter. A threaded PVC end cap should be installed at the base of the screen to prevent sediment infiltration, while a J-Plug is installed at the top of the riser to present surface influence.

The annular space in each monitoring well is to be backfill with clean silica sand to approximately 0.3 m above the monitoring well screens. A layer of bentonite chips is then used to make a hydraulic seal above the sand pack to near the ground surface. The monitoring wells are to be completed with steel stickup protective casings, which were backfilled with sand to provide stability to the casing and PVC riser.

6.5 Elevation Survey

An elevation survey of all boreholes and monitoring wells will be conducted following the completion of the drilling program. A fixed temporary benchmark should be used as a reference elevation; the top of the spindle of a fire hydrant is preferred for this purpose as geodetic elevations can be obtained for these points. The reference benchmark should be assigned a field site datum of 100.00 m for the purposes of the elevation survey. The ground surface elevation of all boreholes should be surveyed. The top of piezometer of each monitoring well should also be surveyed; this allows for higher accuracy in the interpretation of groundwater elevations.

6.6 Monitoring Well Development;

Groundwater monitoring wells will be developed on the day of drilling using LDPE tubing and a footvalve. At least three and up to ten well volumes will be removed from the monitoring wells in order to remove as much sediment as possible from the wells. In cases where the monitoring well goes dry prior to purging three well volumes, the well should be purged dry a minimum of three times, waiting at least one hour between purging events. The LDPE tubing should be removed from the monitoring wells following well development.

6.7 Field Measurement of Water Quality Indicators

Field measurement of water quality parameters were collected at regular intervals (0 L, 0.5 well volumes, 1 well volume, 2 well volumes, etc.) during purging of the monitoring wells prior to sampling. The Horiba was placed in a flow-through cell and water quality parameters were measured until they were found to stabilize to within approximately 10% of the previous measurements prior to sample collection.

6.8 Groundwater Sampling

Follow a period of stabilization after drilling and monitoring well development (1 week recommended), static groundwater elevations are measured relative to the top of piezometer at each groundwater monitoring well on the day of sampling, prior to disturbance of the water column.

Following static groundwater elevation measurements, 6 mm LDPE tubing is placed in each of the monitoring wells. The LDPE tubing is connected to silicon tubing, run through a peristaltic pump set to low flow (approximately 0.2-0.5 L/minute) during purging and sampling. The peristaltic pump is used to avoid mixture of sediment into the groundwater column and prevent volatilization during sample collection. The monitoring wells are purged on the day of sampling while water quality parameters were measured and stabilize as noted above.

Groundwater samples are collected in dedicated amber glass bottles and vials or plastic bottles prepared and provided by the analytical laboratory. Analytes and associated preservatives are specified on each bottle by the laboratory. Each bottle sample set will be provided with a unique sample identifier, project number and date of sampling in the field. Samples for PHCs, BTEXs, VOCs, PAHs and general chemistry are unfiltered, while metals samples are to be field filtered using a dedicated 0.45 µm filter for each sample.

LOPERS & ASSOCIATES

Appendix B

Underground Utility Locates





UNDERGROUND @ SERVICE LOCATORS INC.

USL-1 UNDERGROUND SERVICE LOCATORS INC.

100 - 1704 CARLING AVE. - OTTAWA, ON - K2H 1H3 613-226-8750 - WWW.USL-1.COM

COVER SHEET

DATE: JUNE 19/20 TO: LUKE-

RE: 753 RIOGEWOO AVE. PAGES (INCLUDING COVER): 24

FROM: MATT MOREAU 613-218-7751 - MATTM@USL-1.COM

IF YOU DID NOT RECEIVE ALL OF THE PAGES FOR THIS REPORT, OR IF ANY PART OF IT IS UNCLEAR, PLEASE CONTACT ME. THANK YOU AND HAVE A GREAT DAY!



DATE: JUNE 19/20

CLIENT: LOPERS \$	JOB LOCATION: 753 RIDGELOOD	WORK :	BHS
ASSOC.	AUE.		

PUBLIC UTILITY LOCATE REPORT

D
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1
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UTILITY	LOCATED BY	MARKED / CLEAR
BELL, CAS, HYDRO	PROMARIC	MARKED
WATER, SEWER	CITY	CLEATZ
ROLORS, TELUS	CLI	CLOAR
STREET LIGHTS	BLACK & MAC	CLEAR.
NOTES: BELL & CAS NARKED		
		<i>4</i>

PRIVATE UTILITY LOCATE REPORT

UTILITY	MARKED / CLEAR or N/A	UTILITY	MARKED / CLEAR or N/A
HYDRO / ELECTRICAL	MARKOD	STORM SEWER	CLEAT
COMMS / FOC	CLEAR	SANITARY SEWER	L
GAS / PROPANE / FUEL	L L	STEAM / TUNNELS	N/A-
WATER	MATRICO	OTHER	
NOTES:			

AS-BUILT OR UTILITY PLANS PROVIDED? YES / (NO) - WORK AREA MARKED? YES

/ NO

USL-1 UNDERGROUND SERVICE LOCATORS INC.

100-1704 CARLING AVE. - OTTAWA, ON - K2H1H3 - 613-226-8750 - WWW.USL-1.COM

Sara S

From: Sent: To: Subject: solutions@on1call.com Monday, June 1, 2020 1:08 PM Locates Request 2020234213



LOCATE REQUEST CONFIRMATION

TICKET #: 2020234213

REQUEST PRIORITY: STANDARD

REQUEST TYPE: REGULAR

WORK TO BEGIN DATE: 06/08/2020

Update of Ticket #

Project #

Transmit date: 06/01/2020 01:03:13 PM

REQUESTOR'S CONTRACT INFORMATION	
Contractor ID#: 202	Company Phone #: (613) 226-8750
Contact Name: Sara Staniszewski	Cell #:
Alternate Contact Name: JACQUES DESJARDINS	Fax #: (613) 226-8677
Company name: U S L	Email: locates@usl-1.com
Address: 1704 Carling	Alternate Contact #:

DIG INFORMATION		
Region/County: OTTAWA	Type of work: BORE HOLES	Mark & Fax: NO
Community:	Max Depth: 100.00 FT	Area is not marked: NO
City: OTTAWA	Machine Dig: YES	Area is marked: YES
Address: 729, RIDGEWOOD AVE To 753	Hand Dig: NO	Site Meet Req.: NO
	Directional Drilling: NO	Work being done for: Lopers and Associates
Intersecting Street 1: SPRINGLAND DR	Public Property: YES	
Intersecting Street 2: RIVERSIDE DR	Private Property: YES	

DETAILED DESGRIPTION OF WORK	REMARKS	
CORLOT=U Drilling throughout property. Clear entir e property and to sidewalk along Ridgewood Ave. In cludes property known as 753 Ridgewood.	Civic # 729, 753.	

MEMBERS NOTIFIED: The following owners of underground infrastructure in the area of your excavation site have been notified.

1

lviember neme	Station Code	Infilial Status
HYDRO OTTAWA (HOT1)	HOT1	Notification sent
PROMARK FOR ENBRIDGE GAS (ENOE01)	ENOE01	Notification sent
TELUS (TELUSON3)	TELUSON3	Notification sent
CLI FOR ROGERS (ROGOTT01)	ROGOTT01	Notification sent
CITY OF OTTAWA WATER/SEWER (OTWAWS01)	OTWAWS01	Notification sent
BLACK AND MC DONALD FOR CITY OF OTTAWA STREET LIGHTS (OTWASL01)	OTWASL01	Notification sent
PROMARK FOR BELL CANADA (BCOE01)	BCOE01	Notification sent

MAP SELECTION: Map Selection provided by the excavator through Ontario One Call's map tool or through agent interpretation by



IMPORTANT INFORMATION: Please read.

Defining "NC" - Non-Compliant

- Non-compliant members have not met their obligations under section 5 of the Ontario Underground Infrastructure Notification Act.ON1Call has notified these members to ensure they are aware of your excavation. In this circumstance, should the member not respond, the excavator should contact the member directly to obtain their locates or request a status. ON1Call will not be provided with a locate status from the member regarding this ticket and therefore, cannot provide further information at this time. For locate status contact information please refer to our website.

You have a valid locate when...

- You have reviewed your locate request information for accuracy. CONTACT Ontario One Call (ON1Call) IMMEDIATELY if changes are needed and obtain a corrected locate request confirmation.

- You have obtained locates or clearances from all ON1Call members listed in this ticket before beginning your dig.

You've met your obligations when...

- In addition to this locate request, you have DIRECTLY contacted all owners of infrastructure who ARE NOT current members of ON1Call (such as owned buried infrastructure on private property), as well as arranged for contract locates for your private lines on your private property - where applicable. For a list of locate status contacts visit www.on1call.com.

You respect the marks and instructions provided by the locators and dig with care; the marks and locator instructions MUST MATCH.
 You have obtained any necessary permits from the municipality in whichyou are excavating.

What does "Cleared" mean in the "Initial Status" section?

1. The information that you have provided about your dig will not affect that member's underground infrastructure and they have provided you with a

clearance, if anything about your excavation changes, please ensure that you update your ticket immediately.

What are the images under "Map Selection":

1. A drawing created by an excavator directly within Ontario One Call's web ticket tool, this is expected to be an accurate rendition of the dig site, and it is the excavator's responsibility to ensure the location matches the information they provide under the 'Dig Location' section OR;

2. A drawing created by an Ontario One Call agent, this drawing is based on a verbal description by phone of the area by the excavator. Agents may create drawings that are larger than the proposed dig to minimize risk of interpretation. It is the excavator's responsibility to review these map selections for accuracy. Changes can be made by the excavator through the web ticket tool, to learn how visit www.on1call.com/contractors.

3. All drawings dictate which members are notified.

2020234213_HOT1

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	_ 2020234	213_HOT1		Page 4 of	10
Promark	Auxiliary L	ocate Sheet	Union Gas Emergency # 1-877-969-0999	: :	
	Fax: 613-723-9277	Tall free: 1-800-371-1	Email 8866		
Utilities 🗆 Bell 🗖 Gas 🗣 Hydro Ottawa 🗖	Street Lighting	Date Located:	Request #	2020234243	
Located: DBlink DPeel Fibre D		mm/dd/uuue 06/12/20	120	1020204210	1 •
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operator during work operations. If sketch	and markings do	not coincide, the E	xcavator must obtai	<u>n a new locate</u>	ð.
This form revised March 2020	White-Excavator		Yellow-Office	LAC	FORM





This form revised March 2020

		2020234	1213_HOT1		Page 7	of 10
Promark 1		Auxiliary L	ocate Sheet	Union Gas Emergenc 1-877-969-0999	¥ #	
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operator during work o	perations. If sketch a	nd markings do	not coincide, the E	xcavator must ol	tain a new loc	ate.

This form revised March 2020

	2020234	4213_HOT1		Page 8 o	r <u> 10 </u>
Promark	Auxiliary L	ocate Sheet	Union Gas Emergeno 1-877-969-0999	:y #	
	Fах: 613-723-9277	Tall free: 1-800-371	Email -B866		
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This form revised March 2020

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	Utilities 🜒 Bell 🗖 Ga	s 🗆 Hydro Ottawa 🗆 Hydro One	Date Located:	Request #
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Street Light Image with the second services within the located area have not been marked- check with service/property owner. North N. East E. West W. South S. Any privately owned services within the located area have not been marked- check with service/property owner. A copy of this Auxillary Locate Sheet(s) and the Primary Locate Sheet must be on site and in the hands of the machine operator during work operations. If sketch and markings do not coincide, the Excavator must obtain a new locate.	Street Light Cable - SL -		Ridaow	and Avenue
North N. East E. West W. South S. Any privately owned services within the located area have not been marked- check with service/property owner. A copy of this Auxiliary Locate Sheet(s) and the Primary Locate Sheet must be on site and in the hands of the machine operator during work operations. If sketch and markings do not coincide, the Excavator must obtain a new locate.	Street Light		INUGEW	
West W. THIS FORM VALID ONLY WITH Primary Locate Form. This sketch is not to scale. South S. Any privately owned services within the located area have not been marked- check with service/property owner. A copy of this Auxillary Locate Sheet(s) and the Primary Locate Sheet must be on site and in the hands of the machine operator during work operations. If sketch and markings do not coincide, the Excavator must obtain a new locate.	Norrin N. East F			
South S. [Any privately owned services within the located area have not been marked- check with service/property owner. A copy of this Auxillary Locate Sheet(s) and the Primary Locate Sheet must be on site and in the hands of the machine operator during work operations. If sketch and markings do not coincide, the Excavator must obtain a new locate.	West W.	THIS FORM VALID ONLY W	TH Primary Locate Form. Th	is sketch is not to scale.
a copy of uns Auxiliary Locale. Sneeds) and the Primary Locale Sneet must be on site and in the hands of the machine operator during work operations. If sketch and markings do not coincide, the Excavator must obtain a new locale.	South S.	Any privately owned services within the loca	ated area have not been marke	d- check with service/property owner.
promore working store operations in exclusion and alternarys to not contrarts, the Excepted of Hust Orden a new locate.	A COPY OF THIS AUXILIARY	Locale Sheel(s) and the Primary Loc parations of ekotch and exactinge do	ate Sheet must be on sile a not coincide the Evenuete	and in the hands of the machine or must obtain a now locato
his form revised March 2020 White-Excavator Yellow-Office LAC FORM	This form revised March	2020 White-Excevator	Yellow	Office LAC FORM





ENBRIDGE GAS INC.

Thank you for calling for a locate prior to starting your project.

Please note Enbridge Gas Inc has changed the locate validity period for station codes **ENOE01** and **EN2OE01** and this completed locate is valid for a period of **60 days** from the completion date on the Primary Locate Sheet.

You must adhere to the following:

- You must follow all STOP letters associated with your locate if provided in your locate package.
- You should always review the Primary and all the Auxiliary Sheets of your locate package and understand the validity period for all utilities / infrastructure owners.
- It is the responsibility of Excavators to protect and preserve the original yellow paint markings. White paint can be used to preserve/maintain the markings but should be place beside or at the top / bottom of the original markings ensuring not to replace the yellow paint.

When winter conditions exist, such as snow, pink paint and stakes or flags can be used.

Please be aware new gas services or mains can be installed after this locate was completed. Newly buried gas plant flags will be installed as visual identifier if this occurs.



If flags are present, please contact Enbridge Gas Damage Prevention at 1-866-922-3622

For station code – **ENOE01** or *Legacy Enbridge Gas Distribution* please refer to the Third Party Requirements in the Vicinity of Natural Gas Facilities must always be followed.

https://www.enbridgegas.com/~/media/Extranet-Pages/Safety/Before-you-dig/Third-Party-Requirements-in-the-Vicinity-of-Natural-Gas-Facilities

For station code EN2OE01 or Legacy Union Gas please refer to

https://www.uniongas.com/about-us/safety/safe-digging-practices

Thank you



February 9 2015

To all Excavators:

Bell locates are now valid for the life of the excavation project and will not automatically be relocated every 60 days.

Please note the following for the above to apply:

- a) Construction within the located area begins within 60 days of the "locate completed" date on the original ticket.
- b) The construction company named on the locate remains active on the site.

Bell expects excavators will protect and preserve the paint marks put down on the original locate ticket. If markings are removed due to weather or excavation work the excavator is expected to recreate the markings based on the tie-in measurements provided on the original locate ticket.

If an excavator would like their markings freshened up they can contact Promark (the Bell Canada Locate Service Provider in this area) directly to arrange for them to place fresh markings on the ground however this will be at the excavators expense. Promark can be reached at 613-723-9888.

The locate will be considered officially expired one day after the final day of construction.

Thank you,

Bell Canada

Service Request Details

Source: Contractor Cr Priority: Rep Status: RESOLVED Location Information Address: RIDGEWOOD AVE Between Streets: SPRINGLAND DR / RIVERSIDE DR Description:	eated By: Ga Maxpusr orted By: Initiated: 2020-Jun-01 1:0 Range: 729-753	5 PM Unit:	
Priority: Rep Status: RESOLVED Location Information Address: RIDGEWOOD AVE Between Streets: SPRINGLAND DR / RIVERSIDE DR Description:	orted By: Initiated: 2020-Jun-01 1:0 Range: 729-753	5 PM Unit:	
Status: RESOLVED Location Information Address: RIDGEWOOD AVE Between Streets: SPRINGLAND DR / RIVERSIDE DR Description:	Initiated: 2020-Jun-01 1:09	5 PM Unit:	
ocation Information Address: RIDGEWOOD AVE Between Streets: SPRINGLAND DR / RIVERSIDE DR Description:	Range: 729-753	Unit:	
ocation Information Address: RIDGEWOOD AVE Between Streets: SPRINGLAND DR / RIVERSIDE DR Description:	Range: 729-753	Unit:	
Address: RIDGEWOOD AVE Between Streets: SPRINGLAND DR / RIVERSIDE DR Description:	Range: 729-753	Unit:	
Between Streets: SPRINGLAND DR / RIVERSIDE DR Description:			_
Description:		Municipality: OC)
Street Range: 729-753			•
Intersect 1:SPRINGLAND DR			
Intersect 2:RIVERSIDE DR			
Door Numbers:-			
Municipality:		· ·	
The work area is clear of undergroun	d water and sewer pipes ow	ned by The City of	Ottawa if
the excavation is not in the road. The	service pipes within the pr	operty are privately	owned by
the property owner and are not the re	sponsibility of The City of	Ottawa. Attached is	an
ArcView.	• • •		
Please note: City of Ottawa locates a	re valid for sixty (60) days.	S'il-vous-plaît not	ez: les
localisations de la ville d'Ottawa sont	t valables pendant soixante	e (60) jours.	
Requestor Information		****	
Name: Sara Staniszewski	Phones	- "	
Address: 1704 CARLING AVE	Res:	Cell:	
City: OTTAWA	Bus: 6132268750	Ext:	
Postal Code: K2A1C7 Unit:	Fax: 6132268677	****	
Call Back & Other Assignments		*****	
Responsibilities			
Service Request Work Orde	er # Work Order		
Request Details			
Request Details Start Date: Appointment Time:	Service:	ESD	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
Request Details Start Date: Appointment Time: Finish Date: 2020-Jun-04	Service: Classification: LOCATES -	ESD PROVIDE	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
Request Details Start Date: Appointment Time: Finish Date: 2020-Jun-04 Amount Charge to Customer:	Service: Classification: LOCATES - Category:	ESD PROVIDE	
Request Details Start Date: Appointment Time: Finish Date: 2020-Jun-04 Amount Charge to Customer:	Service: Classification: LOCATES - Category:	ESD PROVIDE	



ECANADIAN	RS INC.	RO Primary L	GERS .ocate Sheet		ON 1 Call Ticket # : 2020234213	
Ph: (905)479-!	5674 Email: ontario	Gcanadianlocators	.com			
Contractor / Excavat	or :		Contact Name : Sara Stani	szewski		
fel : 513-226-8750	Alt. Phone :	Email:	Com		<u>,</u>	
Received Date :	Excavation Date :	Revised Excavation I	Date: Type of Work :	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		
ocate Address :	20 1001 0 2020 ess:		City / Municipal	City / Municipality :		
learest Intersection		nan ar an	Lorinney on	IRALU	ner andre and an and a second and an and a second and and a second and a second and a second and a second and a	
lethod of Field Mari	s RIVERSIDE DR King: □ Paint □ St	akes 🗖 Flaos				
aller's Remarks (Ac	Idilional Info) :					
ncludes prope	rtý known as 753 Rl	dgewood.//Civic 🖡	729, 753.			
tilities Marked : Coaxial Plant	Fibre Optics Plant			This locat which are	e has multiple work areas greater than 100 m apart :	
otal Length :	Total Length :	anaran ana amin'ny tanàna mandritry amin'ny tanàna amin'ny tanàna amin'ny tanàna amin'ny tanàna amin'ny tanàna				
m.	l m l	<u> </u>				
	This locate	e is for ROGERS	plant/infrastr	nemre ONLY!		
		Apply sticker	here if requ	i en el		
CAUTION : Lo	cate is VOID after @	30 days l				
CAUTION : Ha Auxiliary Loc area or nature For all cut cal	and dig within one ate Sheet(s) contai e of work requires (ble, please call : 1-800-265-950	(1) meter or 3.28 ns all known RO a new locate.)1	feet of marking GERS infrastruc Locator's Comm ROGERS CABLE	s. The Located cture. Any char ents : 25 CLEAR IN LOCA	Area defined on the nges to excavation	
Locator's Name : (P) Courtney Stodd	lease Print) lard IStart Time :	End Time :				
Jun 4 2020 A copy of th	5:30 PM is Primary Locate Shee	5:40 PM et and Auxiliary Local	te Sheet(s) must be	e on site and in the	hands of the machine	
operator	during work operations	s. Should sketch and	a markings not coin	icide, a new locale	Page 1 of 2	



Sara S

From:	agt_comm@irth.com
Sent:	Monday, June 1, 2020 2:20 PM
То:	Locates
Subject:	Ticket 2020234213 - TELUS Facility Locate Request: Results/Information
Attachments:	2020234213.PNG; 2020234213_1.PNG

To:U S LAttn: Sara StaniszewskiVoice:6132268750Fax:6132268677Re:TELUS Facility Locate Request: Results/Information

This is an important Safety Message from TELUS Communications Company regarding your Provincial One-Call Locate Request PLEASE REVIEW THIS ENTIRE MESSAGE! We are responding to your request to locate TELUS underground facilities in the specific area of excavation listed on this One-Call ticket. Your locate request has been reviewed and its status is explained below*:

Ticket: 2020234213 County: ONTARIO Place: OTTAWA Address: 729 to 753 RIDGEWOOD AVE

TELUSON3:

Upon review of the information and the work area specified on this locate request, we will not be marking TELUS lines at this time for the following reason: Additional information below *NOTE* TELUS N/R, OUR RECORDS SHOW NO TELUS FACILITIES LOCATED AT THE LOCATION LISTED ON YOUR LOCATE REQUEST.

If you have any questions or concerns regarding this response, please contact the Cable Locate Support Centre for AB,BC,SK, MB & ON, by calling, 1-800-980-0030. Any damage to TELUS facilities MUST be reported as a "Dig Up" to your Provincial One-Call Center ASAP. We ask, that while excavating, you dig with caution. Thank You for using your local Dial / Click Before You Dig process!

1

This message was generated by an automated system. Please do not reply to this email.

Ont Call #	2020234213	City of Ottawa Street Light Lo	ocate	
Date Requested	06/01/2020 1:03:33 PM	Dispatcher: Melissa Dowdell Phone: 612 525 1225	<u>,</u>	Black&McDonal
Company	USL	Instructions	• • • • • • • • • • • • • • • • • • •	
Name	SARA STANISZEWSKI	729 to 753, RIDGEWOOD	AVE F-H DOILLING THROHOMOU	T DEADEDTLA OF EAD
Phone	(613)-226-8750 ext.	ENTIRE PROPERTY AND T	O SIDEWALK ALONG RIDC	EWOOD AVE.
FAX	(613)-226-8677 ext.	INCLUDES PROPERTY KN	OWN AS 753 RIDGEWOOD	.NO_PLAN:613737
ite Contact	JACQUES DESJARDINS			
Phone				
		LOCATOD CKETCH		
		LUCATOR SKETCH		N
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the second second				
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		Cloar		
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			· · · · · · · · · · · · · · · · · · ·	
	No	o Citv of Ottawa Street		
				in the second
	Li	ght assets in dig area		
		· · · · · · · · · · · · · · · · · · ·		
				• • • • •
		and a second state of the		
	mund Street Light Cable	-0H- Overhead/Aprial Wires		Source/Transforme
X Street	Light	Globe/Decorative Light	0	Hydro Pole
ocator Notes/	Comments:		*****	*******
			· · ·	· · · ·
ocate is valid fo	or 60 days. If sketch is differe	ent from markings, location or nature of work chang	les, a Date Located	06/03/2020
new locate mu	ist be requested. Hand dig v	within 1m (3.28fi) on either side of markings. Depth ed plant varies	1 of Time of dea	
	11UU	en hietir Autos.	• • • • • • • • • • • • • • • • • • •	
			· · · · ·	

a la nature du travail necessite un nouveau reperage. Creuser a la main un metre (3.28 pieds) du repere. La profondeur des installation varie d'un endroit a l'autre.

Page 2

of 2

USL-1 DISCLAIMER - FORM 101

- It is our Clients responsibility to fully read and understand this document, prior to any ground disturbance taking place. Should any questions or clarifications be required, contact USL-1 before commencing work
- Locate is VOID after 30 days from the date the locate was completed. Contact USL-1 for remarks and/or new ticket requests, with a minimum notice of 5 business days
- If the scope of work, locate area, or site information changes, contact USL-1 before continuing work. In certain instances, a new ticket request may be required
- Any work within 1.5 metres laterally of a marked utility, must be hand dug or daylighted. Utility depths vary, as does the
 accuracy of the locate equipment, and therefore depths are typically not provided and should not be used for excavation
 purposes. Depth of utilities should also be verified by hand digging or daylighting. The best information is provided at the
 time of the locate, however the accuracy of field markings can vary with regard to equipment accuracy and external
 interference
- If the paint markings or flags on site differ from that of the sketch provided, please contact USL-1 before commencing work. If possible, the issue will be clarified by USL-1 and/or a site meet may be requested with the appropriate parties
- The "Excavator" is responsible for keeping a current copy of the locates on site, with the operators and in/on the excavation equipment AT ALL TIMES
- It is the "Excavator/Contractor's" responsibility to read ALL locate sheets, both public and private, to ensure they understand what potential hazards or buried utilities exist in their work area
- Special purpose locates such as sewer sondeing, locate surveys, tunnel identification, conduit identification, ground fault
 detections, ground penetrating radar, well cap location, concrete scanning, or anything else that requires use of more than
 Radiodetection equipment, must be identified at the time of the original locate request. Should a USL-1 locator identify
 any special needs services during a normal Private utility locate, the client will be notified for the appropriate course of
 action
- Not all buried utilities can be traced. In many instances, water and sewer lines, irrigation systems, grounding cables, fibre optic cables, heating cables, protection cables, and communication cables may not be traceable. Typically, sewer lines will be painted and lined up directionally from manhole to manhole where possible. It may not be possible to detect bends in the sewer lines between manholes. If tracer wires have been buried with the utility, they will be used to locate the buried utility where possible. If a buried utility cannot be traced, it will be noted on the USL-1 report. USL-1 is not liable for damage to untraceable utilities
- Public utility locators have maps, plans and as-built diagrams for reference to work from. Private utility locators, for the most part, do not. USL-1 will attempt to locate any Private utilities on a site, using as-built plans provided to them.
 Building access is mandatory and must be arranged by our client. Any conduits or utilities noted entering or exiting a building will be traced if possible, as well as any other visible utilities observed on site. It is the responsibility of the contractor to provide any and all buried utility information and site contacts that they have. There is no guarantee that USL-1 can find all buried utilities if the property owner does not have records or information regarding their own buried utilities
- USL- 1 cannot be held liable for damage to Private water and/or sewer laterals unless building access is granted, and the utility is locatable
- Thick snow and ice, frozen manhole lids, live traffic, parked cars, construction debris and activities etc, are all factors that can interfere with USL-1's ability to perform Private utility locates. USL-1 cannot guaranty location of all buried utilities when such factors impede the locate process. It is the contractor's responsibility to ensure that the work areas are safe and accessible for locates, prior to USL-1's arrival to site
- USL-1 as a Private utility locator, is not permitted to locate Publicly owned utilities. In some cases, Public utilities may be noted on a sketch, but are FOR REFERENCE ONLY, and under no circumstances shall be used for excavation purposes. It is the contractor's responsibility to verify any Public utilities noted on the USL-1 sketch by referring to the Public utility locate sheets for physical LOCATION AND ACCURACY. USL-1 DOES NOT ASSUME LIABILTY FOR PUBLIC LOCATE INNACCURACIES
- If the proposed work area is on Private property, it does NOT mean that all buried utilities are Private. Regardless of
 where you are digging, and what the proposed depth of excavation is, it is the law to notify Ontario One Call (or InfoExcavation in Quebec) to obtain Public utility locates
- NCC PROPERTY assuming the contractor has been issued a Land Access Permit from the NCC, it is typically indicated
 within the permit that it is the contractor's responsibility to contact NCC for utility locates of their buried utilities

USL-1 - January 2016
Appendix C

Borehole Logs

Lopers & Associates 30 Lansfield Way Ottawa, Ontario K2G3V8



Lopers & Associates 30 Lansfield Way Ottawa. Ontario K2G3V8

	004003 Cana	ida Inc.			PROJECT N	NAME Phase Two Environme	ental Site Assessment
PROJECT N	UMBER LOI	P20-002B			PROJECT L	OCATION 729 Ridgewood A	venue, Ottawa, Ontario
DATE STARTED 6-24-20 COMPLETED 6-24-20					GROUND ELEV	ATION 99.63 m HO	_E SIZE 20 cm
ORILLING C	ONTRACTOR	George D	owning	Estate Drilling	GROUND WAT	ER LEVELS:	
ORILLING M	ETHOD Tra	ck Mounted	CME 5	5	AFTER DRIL	LING	
OGGED BY	L. Lopers		CHE	CKED BY D. Plenderleith			
NOTES Sit	e Datum = 10	0.00 m Top	of Spin	dle of Fire Hydrant SE of Pro	perty		
TAL						VOC Concentration	
표 눈없	SE L	_ ∐⊿	₽				
		NNM NAT/	₽8	MATERIAL DES	SCRIPTION	20 40 60 80	WELL DIAGRAM
			19_1			⊕ LEL (%)	
ر ي ا		N EN				20 40 60 80	
			ЬЙЙ	0.1 Silty Sand and gravel	(Fill). Grey,	99.50	
]∬ s`	S 2-5-5-6	Vapor = 0		Sand (Fill). Brown. loo	ose. moist.		
7/\					,		
- <u>-</u> -		1					
- - - ≬ s	S 4-6-50-50) / am an = 0					
-//\ *		vapor – u		1 /		08.26	
		-	i yiy	Silty Sand and gravel	(Fill). Grey,	96.20	
-			6914	compact, moist.			
_ V s	S 8-9-11-9		691				
2//) (20)	vapor = 0	$\beta $				
		-	6014	23		97 33	
				Silty Clay. Brown/grey	/, firm, moist.		
_lV s	S 3-8-10-50						
	+ (18)	vapor = 0					
3		-		2.9 Silty Sand and Gravel	(Till). Grey,	96.73	
				compact, wet.	() ,		
V s	S 3-8-12-10			Wet at 3.05 m BGS.		5	
	(20)	vapor = 5					
		-					
		1					
<u> </u>	S 3-4-4-4						
- /\ e	o (8)	Vapor = 0					
				4.4		95.21:	

Lopers & Associates 30 Lansfield Way Ottawa, Ontario K2G3V8



Lopers & Associates 30 Lansfield Way Ottawa, Ontario K2G3V8

BH4-20



Lopers & Associates 30 Lansfield Way Ottawa, Ontario K2G3V8



Lopers & Associates 30 Lansfield Way Ottawa, Ontario K2G3V8

BH6-20



Lopers & Associates 30 Lansfield Way Ottawa, Ontario K2G3V8

BH7-20



Appendix D

Certificates of Equipment Calibration



ENVIRONMENTAL AND SAFETY INC. "Exceptional Customer Service!"

Certificate of Calibration

HORIBA U-52 Serial Number $\forall H3HOGFIF$ has been calibrated per the Manufacturers published instructions, using NIST traceable solutions and

4.00, 7.00, 10.0

2, 2-Point pH

pH 4.0 Lot #9GD684 Exp05/21

pH 7.0 Lot# 9GK721 Exp.11/21 pH 10.0 Lot#9GD483 Exp. 04/21

une 25,

2020

zero checked Cond.Standard Lot#9GC1263 Exp. 04/2021

Cond.

4.49uS/cm

StableCal Standard, 100 NTU Lot#A9151 Exp.06/21

Turb,

0, 100 NTU

Zero checked

\$162 mg/L@ 22 75 DegC 240mV

Sodium Sulfite Zero

DO

Oakton Zero Oxygen **ORP** Test Solution Lot# 639901

240 mV

Solution

ORP

Lot # 4318 Exp 06/2024

Solutions ref. to NIST SRM's

Calibrated

RENTALS, SALES, SERVICE, SUPPORT

9 - 170 AMBASSADOR DR., MISSISSAUGA, ONTARIO L5T 2H9 PHONE: (905) 670-1304 TOLL FREE: (888) 285-2324 E-MAIL: SALES@MAXIMENVIRONMENTAL.COM

9 - 148 COLONNADE RD., OTTAWA, ONTARIO K2E 7R4 PHONE: (613) 224-4747 TOLL FREE: (888) 285-2324 E-MAIL: SALES@MAXIMENVIRONMENTAL.COM

WWW.MAXIMENVIRONMENTAL.COM

MAXIM ENVIRONMENTAL AND SAFETY INC.

Phone:

(613)224-4747

148 Colonnade Rd, UNIT # 9 Nepean, Ontario, K2E 7R4

CERTIFICATE OF CALIBRATION

The RKI Instruments Model EAGLE-2 as listed below has been inspected and calibrated following the Manufacturer's published specifications and methods.

Instrument Model: EAGLE-2		Serial Number: E2F80/ Date of Calil		bration: June 23/2020		
<u>SENSOR</u>	CALIBRATION GAS STANDARD	CALIBRATION GAS CONCENTRATION	READING PRIOR TO ADJUSTMENT	INSTRUMENT SPAN SETTING	<u>ALARM LEVEL</u> <u>SETTINGS</u>	
Combustible	Hexane lot # 1189063	15% LEL	1650 ppm	15% LEL "Methane Elimina	10 & 50% LEL tion" Mode	
Combustible	Methane lot # 1013148	50% LEL	<500 PPM	Verification Only		
		"Methar		"Methane Eliminat	ion" Mode	
Combustible	Hexane lot # 1189063	15% LEL	1650 ppm	15% LEL "Methane Respon s	10 & 50% LEL se Enabled'' Mode	
VOC	Isobutylene lot # 1278604	100 PPM	100 PPM	100 PPM	400 & 1000 PPM	

The calibration gas standard used is considered to be a certified standard and is traceable to the National Institute of Standards and Technology (NIST). Certificate of Analysis is available upon request.

The instrument indicated above is now certified to be operating within the Manufacturer's specifications. This does not preclude the requirement for regular maintenance and pre-use sensor response checks in order to ensure continued complete and accurate operating condition.

Certified: ______

Appendix E

Laboratory Certificates of Analysis



RELIABLE.

300 - 2319 St. Laurent Blvd Ottawa, ON, K1G 4J8 1-800-749-1947 www.paracellabs.com

Certificate of Analysis

Lopers & Associates

30 Lansfield Way Ottawa, ONT K2G 3V8 Attn: Luke Lopers

Client PO: LOP20-002B Project: LOP20-002B Custody: 54317

Revised Report

Report Date: 7-Jul-2020 Order Date: 24-Jun-2020

Order #: 2026367

This Certificate of Analysis contains analytical data applicable to the following samples as submitted:

Paracel ID	Client ID
2026367-01	BH1-20-SS5
2026367-02	BH3-20-SS6
2026367-03	BH4-20-SS6
2026367-04	BH5-20-SS4
2026367-05	BH5-20-SS7
2026367-06	BH11-20-SS5

Approved By:

Dale Robertson, BSc Laboratory Director

Any use of these results implies your agreement that our total liability in connection with this work, however arising, shall be limited to the amount paid by you for this work, and that our employees or agents shall not under any circumstances be liable to you in connection with this work.



Analysis Summary Table

Report Date: 07-Jul-2020 Order Date: 24-Jun-2020

Project Description: LOP20-002B

Analysis	Method Reference/Description	Extraction Date	Analysis Date
Boron, available	MOE (HWE), EPA 200.8 - ICP-MS	3-Jul-20	3-Jul-20
BTEX by P&T GC-MS	EPA 8260 - P&T GC-MS	26-Jun-20	29-Jun-20
Chromium, hexavalent - soil	MOE E3056 - Extraction, colourimetric	25-Jun-20	27-Jun-20
Conductivity	MOE E3138 - probe @25 °C, water ext	30-Jun-20	30-Jun-20
Cyanide, free	MOE E3015 - Auto Colour, water extraction	25-Jun-20	26-Jun-20
Mercury by CVAA	EPA 7471B - CVAA, digestion	30-Jun-20	30-Jun-20
pH, soil	EPA 150.1 - pH probe @ 25 °C, CaCl buffered ext.	25-Jun-20	26-Jun-20
PHC F1	CWS Tier 1 - P&T GC-FID	26-Jun-20	26-Jun-20
PHCs F2 to F4	CWS Tier 1 - GC-FID, extraction	25-Jun-20	30-Jun-20
REG 153: Metals by ICP/MS, soil	EPA 6020 - Digestion - ICP-MS	29-Jun-20	7-Jul-20
REG 153: PAHs by GC-MS	EPA 8270 - GC-MS, extraction	25-Jun-20	29-Jun-20
REG 153: VOCs by P&T GC/MS	EPA 8260 - P&T GC-MS	26-Jun-20	26-Jun-20
SAR	Calculated	2-Jul-20	2-Jul-20
Solids, %	Gravimetric, calculation	26-Jun-20	27-Jun-20



Certificate of Analysis Client: Lopers & Associates

Client PO: LOP20-002B

Order #: 2026367

Report Date: 07-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B

	Client ID:	BH1-20-SS5	BH3-20-SS6	BH4-20-SS6	BH5-20-SS4
	Sample Date:	24-Jun-20 09:00	24-Jun-20 09:00	24-Jun-20 09:00	24-Jun-20 09:00
	Sample ID:	2020307-01 Soil	2020307-02 Soil	2020307-03 Soil	2020307-04 Soil
Physical Characteristics	MDE/Onits				
% Solids	0.1 % by Wt.	71.5	92.5	87.1	79.5
General Inorganics			•	1	
SAR	0.01 N/A	0.76	-	1.12	-
Conductivity	5 uS/cm	636	-	627	-
Cyanide, free	0.03 ug/g dry	<0.03	-	<0.03	-
рН	0.05 pH Units	7.14	-	7.40	-
Metals					
Antimony	1.0 ug/g dry	<1.0	-	<1.0	-
Arsenic	1.0 ug/g dry	3.9	-	3.7	-
Barium	1.0 ug/g dry	284	-	46.3	-
Beryllium	0.5 ug/g dry	0.7	-	<0.5	-
Boron	5.0 ug/g dry	5.8	-	6.5	-
Boron, available	0.5 ug/g dry	<0.5	-	<0.5	-
Cadmium	0.5 ug/g dry	<0.5	-	<0.5	-
Chromium	5.0 ug/g dry	104	-	13.1	-
Chromium (VI)	0.2 ug/g dry	0.3	-	<0.2	-
Cobalt	1.0 ug/g dry	20.1	-	5.6	-
Copper	5.0 ug/g dry	45.5	-	12.4	-
Lead	1.0 ug/g dry	6.1	-	5.2	-
Mercury	0.1 ug/g dry	<0.1	-	<0.1	-
Molybdenum	1.0 ug/g dry	<1.0	-	<1.0	-
Nickel	5.0 ug/g dry	56.1	-	10.6	-
Selenium	1.0 ug/g dry	<1.0	-	<1.0	-
Silver	0.3 ug/g dry	<0.3	-	<0.3	-
Thallium	1.0 ug/g dry	<1.0	-	<1.0	-
Uranium	1.0 ug/g dry	<1.0	-	<1.0	-
Vanadium	10.0 ug/g dry	101	-	23.5	-
Zinc	20.0 ug/g dry	92.7	-	<20.0	-
Volatiles				-	
Acetone	0.50 ug/g dry	<0.50	-	-	-
Benzene	0.02 ug/g dry	<0.02	-	-	-
Bromodichloromethane	0.05 ug/g dry	<0.05	-	-	-
Bromoform	0.05 ug/g dry	<0.05	-	-	-
Bromomethane	0.05 ug/g dry	<0.05	-	-	-
Carbon Tetrachloride	0.05 ug/g dry	<0.05	-	-	-



Order #: 2026367

Report Date: 07-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B

	Client ID:	BH1-20-SS5	BH3-20-SS6	BH4-20-SS6	BH5-20-SS4
	Sample Date:	24-Jun-20 09:00 2026367-01	24-Jun-20 09:00 2026367-02	24-Jun-20 09:00 2026367-03	24-Jun-20 09:00 2026367-04
]	MDL/Units	Soil	Soil	Soil	Soil
Chlorobenzene	0.05 ug/g dry	<0.05	-	-	-
Chloroform	0.05 ug/g dry	<0.05	-	-	-
Dibromochloromethane	0.05 ug/g dry	<0.05	-	-	-
Dichlorodifluoromethane	0.05 ug/g dry	<0.05	-	-	-
1,2-Dichlorobenzene	0.05 ug/g dry	<0.05	-	-	-
1,3-Dichlorobenzene	0.05 ug/g dry	<0.05	-	-	-
1,4-Dichlorobenzene	0.05 ug/g dry	<0.05	-	-	-
1,1-Dichloroethane	0.05 ug/g dry	<0.05	-	-	-
1,2-Dichloroethane	0.05 ug/g dry	<0.05	-	-	-
1,1-Dichloroethylene	0.05 ug/g dry	<0.05	-	-	-
cis-1,2-Dichloroethylene	0.05 ug/g dry	<0.05	-	-	-
trans-1,2-Dichloroethylene	0.05 ug/g dry	<0.05	-	-	-
1,2-Dichloropropane	0.05 ug/g dry	<0.05	-	-	-
cis-1,3-Dichloropropylene	0.05 ug/g dry	<0.05	-	-	-
trans-1,3-Dichloropropylene	0.05 ug/g dry	<0.05	-	-	-
1,3-Dichloropropene, total	0.05 ug/g dry	<0.05	-	-	-
Ethylbenzene	0.05 ug/g dry	<0.05	-	-	-
Ethylene dibromide (dibromoethane, 1,2-)	0.05 ug/g dry	<0.05	-	-	-
Hexane	0.05 ug/g dry	<0.05	-	-	-
Methyl Ethyl Ketone (2-Butanone)	0.50 ug/g dry	<0.50	-	-	-
Methyl Isobutyl Ketone	0.50 ug/g dry	<0.50	-	-	-
Methyl tert-butyl ether	0.05 ug/g dry	<0.05	-	-	-
Methylene Chloride	0.05 ug/g dry	<0.05	-	-	-
Styrene	0.05 ug/g dry	<0.05	-	-	-
1,1,1,2-Tetrachloroethane	0.05 ug/g dry	<0.05	-	-	-
1,1,2,2-Tetrachloroethane	0.05 ug/g dry	<0.05	-	-	-
Tetrachloroethylene	0.05 ug/g dry	<0.05	-	-	-
Toluene	0.05 ug/g dry	<0.05	-	-	-
1,1,1-Trichloroethane	0.05 ug/g dry	<0.05	-	-	-
1,1,2-Trichloroethane	0.05 ug/g dry	<0.05	-	-	-
Trichloroethylene	0.05 ug/g dry	<0.05	-	-	-
Trichlorofluoromethane	0.05 ug/g dry	<0.05	-	-	-
Vinyl chloride	0.02 ug/g dry	<0.02	-	-	-
m,p-Xylenes	0.05 ug/g dry	<0.05	-	-	-
o-Xylene	0.05 ug/g dry	<0.05	-	-	-



Order #: 2026367

Report Date: 07-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B

	Client ID:	BH1-20-SS5	BH3-20-SS6	BH4-20-SS6	BH5-20-SS4
	Sample Date:	24-Jun-20 09:00	24-Jun-20 09:00	24-Jun-20 09:00	24-Jun-20 09:00
	Sample ID:	2026367-01 Soil	2026367-02 Soil	2026367-03 Soil	2026367-04 Soil
Yulenes total	0.05 ug/g drv	<0.05	001	001	001
4-Bromofluorobenzene	Surrogate	98.3%	-	-	-
Dibromofluoromethane	Surrogate	116%	-	-	-
Toluene-d8	Surrogate	104%	-	-	-
Benzene	0.02 ug/g dry	-	3.02	<0.02	<0.02
Ethylbenzene	0.05 ug/g dry	-	59.0	<0.05	<0.05
Toluene	0.05 ug/g dry	-	73.5	<0.05	<0.05
m,p-Xylenes	0.05 ug/g dry	-	196	<0.05	<0.05
o-Xylene	0.05 ug/g dry	-	80.3	<0.05	<0.05
Xylenes, total	0.05 ug/g dry	-	276	<0.05	<0.05
Toluene-d8	Surrogate	-	100%	104%	102%
Hydrocarbons					
F1 PHCs (C6-C10)	7 ug/g dry	22	117	<7	<7
F2 PHCs (C10-C16)	4 ug/g dry	909	110	<4	<4
F3 PHCs (C16-C34)	8 ug/g dry	102	11	<8	<8
F4 PHCs (C34-C50)	6 ug/g dry	<6	<6	<6	<6
Semi-Volatiles					
Acenaphthene	0.02 ug/g dry	0.07	<0.02	<0.02	-
Acenaphthylene	0.02 ug/g dry	0.11	<0.02	<0.02	-
Anthracene	0.02 ug/g dry	<0.02	<0.02	<0.02	-
Benzo [a] anthracene	0.02 ug/g dry	<0.02	<0.02	<0.02	-
Benzo [a] pyrene	0.02 ug/g dry	<0.02	<0.02	<0.02	-
Benzo [b] fluoranthene	0.02 ug/g dry	<0.02	<0.02	<0.02	-
Benzo [g,h,i] perylene	0.02 ug/g dry	<0.02	<0.02	<0.02	-
Benzo [k] fluoranthene	0.02 ug/g dry	<0.02	<0.02	<0.02	-
Chrysene	0.02 ug/g dry	<0.02	<0.02	<0.02	-
Dibenzo [a,h] anthracene	0.02 ug/g dry	<0.02	<0.02	<0.02	-
Fluoranthene	0.02 ug/g dry	<0.02	<0.02	<0.02	-
Fluorene	0.02 ug/g dry	0.26	<0.02	<0.02	-
Indeno [1,2,3-cd] pyrene	0.02 ug/g dry	<0.02	<0.02	<0.02	-
1-Methylnaphthalene	0.02 ug/g dry	3.02	0.58	<0.02	-
2-Methylnaphthalene	0.02 ug/g dry	4.58	1.37	<0.02	-
Methylnaphthalene (1&2)	0.04 ug/g dry	7.61	1.95	<0.04	-
Naphthalene	0.01 ug/g dry	0.50	1.69	<0.01	-
Phenanthrene	0.02 ug/g dry	0.13	0.03	<0.02	-
Pyrene	0.02 ug/g dry	<0.02	<0.02	<0.02	-
2-Fluorobiphenyl	Surrogate	67.4%	91.3%	88.2%	-



Certificate of Analysis Client: Lopers & Associates

Report Date: 07-Jul-2020 Order Date: 24-Jun-2020

Project Description: LOP20-002B

	Client ID: BH1-20-SS5		BH3-20-SS6	BH4-20-SS6	BH5-20-SS4	
	Sample Date:		24-Jun-20 09:00	24-Jun-20 09:00	24-Jun-20 09:00	
	Sample ID:	2026367-01	2026367-02	2026367-03	2026367-04	
	MDL/Units	Soil	Soil	Soil	Soil	
Terphenyl-d14	Surrogate	75.7%	97.2%	116%	-	



Order #: 2026367

Report Date: 07-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B

	Client ID:	BH5-20-SS7	BH11-20-SS5	-	-
	Sample Date:	24-Jun-20 09:00	24-Jun-20 09:00	-	-
	Sample ID:	2026367-05	2026367-06	-	-
Physical Characteristics	MDL/Units	3011	3011	-	-
% Solido	0.1 % by Wt.	02.8	70.6		
General Inorganics	· · ·	32.0	10.0	-	
SAR	0.01 N/A	-	0.81	-	-
Conductivity	5 uS/cm	-	670	-	-
Cyanide, free	0.03 ug/g dry	-	<0.03	-	-
рН	0.05 pH Units	-	7.33	-	-
Metals	· · ·				
Antimony	1.0 ug/g dry	-	<1.0	-	-
Arsenic	1.0 ug/g dry	-	4.0	-	-
Barium	1.0 ug/g dry	-	327	-	-
Beryllium	0.5 ug/g dry	-	0.9	-	-
Boron	5.0 ug/g dry	-	7.2	-	-
Boron, available	0.5 ug/g dry	-	<0.5	-	-
Cadmium	0.5 ug/g dry	-	<0.5	-	-
Chromium	5.0 ug/g dry	-	126	-	-
Chromium (VI)	0.2 ug/g dry	-	0.3	-	-
Cobalt	1.0 ug/g dry	-	22.5	-	-
Copper	5.0 ug/g dry	-	49.0	-	-
Lead	1.0 ug/g dry	-	6.8	-	-
Mercury	0.1 ug/g dry	-	<0.1	-	-
Molybdenum	1.0 ug/g dry	-	<1.0	-	-
Nickel	5.0 ug/g dry	-	65.4	-	-
Selenium	1.0 ug/g dry	-	<1.0	-	-
Silver	0.3 ug/g dry	-	<0.3	-	-
Thallium	1.0 ug/g dry	-	<1.0	-	-
Uranium	1.0 ug/g dry	-	<1.0	-	-
Vanadium	10.0 ug/g dry	-	104	-	-
Zinc	20.0 ug/g dry	-	114	-	-
Volatiles				I	
Acetone	0.50 ug/g dry	<0.50	<0.50	-	-
Benzene	0.02 ug/g dry	<0.02	<0.02	-	-
Bromodichloromethane	0.05 ug/g dry	<0.05	<0.05	-	-
Bromoform	0.05 ug/g dry	<0.05	<0.05	-	-
Bromomethane	0.05 ug/g dry	<0.05	<0.05	-	-
Carbon Tetrachloride	0.05 ug/g dry	<0.05	<0.05	-	-



Order #: 2026367

Report Date: 07-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B

	Client ID:	BH5-20-SS7	BH11-20-SS5	-	-
	Sample Date:	24-Jun-20 09:00	24-Jun-20 09:00	-	-
	Sample ID:	2026367-05	2026367-06	-	-
Chlanchanzana	MDL/Units	Soli	50II	-	-
Chloroform	0.05 ug/g dry	<0.05	<0.05	-	-
Dikremeskleremethere	0.05 ug/g dry	<0.05	<0.05	-	-
	0.05 ug/g dry	<0.05	<0.05	-	-
Dichlorodifluoromethane	0.05 ug/g dry	<0.05	<0.05	-	-
1,2-Dichlorobenzene		<0.05	<0.05	-	-
1,3-Dichlorobenzene	0.05 ug/g dry	<0.05	<0.05	-	-
1,4-Dichlorobenzene	0.05 ug/g dry	<0.05	<0.05	-	-
1,1-Dichloroethane		<0.05	<0.05	-	-
1,2-Dichloroethane	0.05 ug/g dry	<0.05	<0.05	-	-
1,1-Dichloroethylene	0.05 ug/g dry	<0.05	<0.05	-	-
cis-1,2-Dichloroethylene	0.05 ug/g dry	<0.05	<0.05	-	-
trans-1,2-Dichloroethylene	0.05 ug/g dry	<0.05	<0.05	-	-
1,2-Dichloropropane	0.05 ug/g dry	<0.05	<0.05	-	-
cis-1,3-Dichloropropylene	0.05 ug/g dry	<0.05	<0.05	-	-
trans-1,3-Dichloropropylene	0.05 ug/g dry	<0.05	<0.05	-	-
1,3-Dichloropropene, total	0.05 ug/g dry	<0.05	<0.05	-	-
Ethylbenzene	0.05 ug/g dry	<0.05	<0.05	-	-
Ethylene dibromide (dibromoethane, 1	0.05 ug/g dry	<0.05	<0.05	-	-
Hexane	0.05 ug/g dry	<0.05	<0.05	-	-
Methyl Ethyl Ketone (2-Butanone)	0.50 ug/g dry	<0.50	<0.50	-	-
Methyl Isobutyl Ketone	0.50 ug/g dry	<0.50	<0.50	-	-
Methyl tert-butyl ether	0.05 ug/g dry	<0.05	<0.05	-	-
Methylene Chloride	0.05 ug/g dry	<0.05	<0.05	-	-
Styrene	0.05 ug/g dry	<0.05	<0.05	-	-
1,1,1,2-Tetrachloroethane	0.05 ug/g dry	<0.05	<0.05	-	-
1,1,2,2-Tetrachloroethane	0.05 ug/g dry	<0.05	<0.05	-	-
Tetrachloroethylene	0.05 ug/g dry	<0.05	<0.05	-	-
Toluene	0.05 ug/g dry	<0.05	<0.05	-	-
1,1,1-Trichloroethane	0.05 ug/g dry	<0.05	<0.05	-	-
1,1,2-Trichloroethane	0.05 ug/g dry	<0.05	<0.05	-	-
Trichloroethylene	0.05 ug/g dry	<0.05	<0.05	-	-
Trichlorofluoromethane	0.05 ug/g dry	<0.05	<0.05	-	-
Vinyl chloride	0.02 ug/g dry	<0.02	<0.02	-	-
m,p-Xylenes	0.05 ug/g dry	<0.05	0.08	-	-
o-Xylene	0.05 ug/g dry	<0.05	<0.05	-	-



Order #: 2026367

Report Date: 07-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B

	Client ID: Sample Date: Sample ID: MDL/Units	BH5-20-SS7 24-Jun-20 09:00 2026367-05 Soil	BH11-20-SS5 24-Jun-20 09:00 2026367-06 Soil	- - - -	- - - -
Xylenes, total	0.05 ug/g dry	<0.05	0.08	-	-
4-Bromofluorobenzene	Surrogate	101%	101%	-	-
Dibromofluoromethane	Surrogate	110%	111%	-	-
Toluene-d8	Surrogate	103%	102%	-	-
Hydrocarbons					
F1 PHCs (C6-C10)	7 ug/g dry	<7	34	-	-
F2 PHCs (C10-C16)	4 ug/g dry	<4	306	-	-
F3 PHCs (C16-C34)	8 ug/g dry	<8	25	-	-
F4 PHCs (C34-C50)	6 ug/g dry	<6	<6	-	-
Semi-Volatiles					
Acenaphthene	0.02 ug/g dry	-	0.05	-	-
Acenaphthylene	0.02 ug/g dry	-	0.03	-	-
Anthracene	0.02 ug/g dry	-	<0.02	-	-
Benzo [a] anthracene	0.02 ug/g dry	-	<0.02	-	-
Benzo [a] pyrene	0.02 ug/g dry	-	<0.02	-	-
Benzo [b] fluoranthene	0.02 ug/g dry	-	<0.02	-	-
Benzo [g,h,i] perylene	0.02 ug/g dry	-	<0.02	-	-
Benzo [k] fluoranthene	0.02 ug/g dry	-	<0.02	-	-
Chrysene	0.02 ug/g dry	-	<0.02	-	-
Dibenzo [a,h] anthracene	0.02 ug/g dry	-	<0.02	-	-
Fluoranthene	0.02 ug/g dry	-	<0.02	-	-
Fluorene	0.02 ug/g dry	-	0.10	-	-
Indeno [1,2,3-cd] pyrene	0.02 ug/g dry	-	<0.02	-	-
1-Methylnaphthalene	0.02 ug/g dry	-	0.89	-	-
2-Methylnaphthalene	0.02 ug/g dry	-	1.38	-	-
Methylnaphthalene (1&2)	0.04 ug/g dry	-	2.26	-	-
Naphthalene	0.01 ug/g dry	-	0.26	-	-
Phenanthrene	0.02 ug/g dry	-	0.07	-	-
Pyrene	0.02 ug/g dry	-	<0.02	-	-
2-Fluorobiphenyl	Surrogate	-	79.7%	-	-
Terphenyl-d14	Surrogate	-	75.2%	-	-



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Method Quality Control: Blank

Report Date: 07-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B

		Reporting		Source		%REC		RPD	
Analyte	Result	Limit	Units	Result	%REC	Limit	RPD	Limit	Notes
General Inorganics									
Conductivity	ND	5	uS/cm						
Cyanide, free	ND	0.03	ug/g						
Hydrocarbons									
F1 PHCs (C6-C10)	ND	7	ug/g						
F2 PHCs (C10-C16)	ND	4	ug/g						
F3 PHCs (C16-C34)	ND	8	ug/g						
F4 PHCs (C34-C50)	ND	6	ug/g						
Metals									
Antimony	ND	1.0	ug/g						
Arsenic	ND	1.0	ug/g						
Barium	ND	1.0	ug/g						
Beryllium	ND	0.5	ug/g						
Boron, available	ND	0.5	ug/g						
Boron	ND	5.0	ug/g						
Cadmum		0.5	ug/g						
Chromium		0.2 5.0	ug/g						
Cobalt	ND	1.0	ug/g ug/g						
Copper	ND	5.0	ua/a						
Lead	ND	1.0	ug/g						
Mercury	ND	0.1	ug/g						
Molybdenum	ND	1.0	ug/g						
Nickel	ND	5.0	ug/g						
Selenium	ND	1.0	ug/g						
Silver	ND	0.3	ug/g						
Thallium	ND	1.0	ug/g						
	ND	1.0	ug/g						
Zinc		10.0	ug/g						
Semi-Volatiles	ND	20.0	uy/y						
Acononthene	ND	0.02	ua/a						
		0.02	ug/g						
Anthracene	ND	0.02	ug/g						
Benzo [a] anthracene	ND	0.02	ug/g						
Benzo [a] pyrene	ND	0.02	ug/g						
Benzo [b] fluoranthene	ND	0.02	ug/g						
Benzo [g,h,i] perylene	ND	0.02	ug/g						
Benzo [k] fluoranthene	ND	0.02	ug/g						
Chrysene	ND	0.02	ug/g						
Dibenzo [a,h] anthracene	ND	0.02	ug/g						
	ND	0.02	ug/g						
Fluorene		0.02	ug/g						
1-Methylpanhthalene		0.02	ug/g						
2-Methylnaphthalene	ND	0.02	ug/g						
Methylnaphthalene (1&2)	ND	0.04	ug/g						
Naphthalene	ND	0.01	ug/g						
Phenanthrene	ND	0.02	ug/g						
Pyrene	ND	0.02	ug/g						
Surrogate: 2-Fluorobiphenyl	1.30		ug/g		97.5	50-140			
Surrogate: Terphenyl-d14	1.31		ug/g		97.9	50-140			
Volatiles									
Acetone	ND	0.50	ug/q						
Benzene	ND	0.02	ug/g						
Bromodichloromethane	ND	0.05	ug/g						
Bromoform	ND	0.05	ug/g						
Bromomethane	ND	0.05	ug/g						



Order #: 2026367

Report Date: 07-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B

Method Quality Control: Blank

American		Reporting		Source		%REC		RPD	
Analyte	Result	Limit	Units	Result	%REC	Limit	RPD	Limit	Notes
Carbon Tetrachloride	ND	0.05	ug/g						
Chlorobenzene	ND	0.05	ug/g						
Chloroform	ND	0.05	ug/g						
Dibromochloromethane	ND	0.05	ug/g						
Dichlorodifluoromethane	ND	0.05	ug/g						
1,2-Dichlorobenzene	ND	0.05	ug/g						
1,3-Dichlorobenzene	ND	0.05	ug/g						
1,4-Dichlorobenzene	ND	0.05	ug/g						
1,1-Dichloroethane	ND	0.05	ug/g						
1,2-Dichloroethane	ND	0.05	ug/g						
1,1-Dichloroethylene	ND	0.05	ug/g						
cis-1,2-Dichloroethylene	ND	0.05	ug/g						
trans-1,2-Dichloroethylene	ND	0.05	ug/g						
1,2-Dichloropropane	ND	0.05	ug/g						
cis-1,3-Dichloropropylene	ND	0.05	ug/g						
trans-1,3-Dichloropropylene	ND	0.05	ug/g						
1,3-Dichloropropene, total	ND	0.05	ug/g						
Ethylbenzene	ND	0.05	ug/g						
Ethylene dibromide (dibromoethane, 1,2	ND	0.05	ug/g						
Hexane	ND	0.05	ug/g						
Methyl Ethyl Ketone (2-Butanone)	ND	0.50	ug/g						
Methyl Isobutyl Ketone	ND	0.50	ug/g						
Methyl tert-butyl ether	ND	0.05	ug/g						
Methylene Chloride	ND	0.05	ug/g						
Styrene	ND	0.05	ug/g						
1,1,1,2- letrachloroethane	ND	0.05	ug/g						
1,1,2,2- letrachloroethane	ND	0.05	ug/g						
	ND	0.05	ug/g						
	ND	0.05	ug/g						
	ND	0.05	ug/g						
		0.05	ug/g						
		0.05	ug/g						
Vinul ablarida		0.05	ug/g						
		0.02	ug/g						
III,p-Aylenes		0.05	ug/g						
0-Aylenee total		0.05	ug/g						
Ayleries, total	ND 9.40	0.05	ug/g		105	50 140			
Surrogate: 4-Bromoliuoroberizerie	0.40		ug/g		105	50-140			
Surrogale. Dibromonuorometriane	9.45		ug/g		110	50-140			
Surrogate: Toluene-d8	6.94		ug/g		86.7	50-140			
Benzene	ND	0.02	ug/g						
Ethylbenzene	ND	0.05	ug/g						
Ioluene	ND	0.05	ug/g						
m,p-xyienes	ND	0.05	ug/g						
	ND	0.05	ug/g						
Aylenes, total		0.05	ug/g		06.7	50 4 40			
Surrogate: 10/uene-av	6.94		ug/g		80.7	50-140			



Method Quality Control: Duplicate

Report Date: 07-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B

		Reporting		Source		%REC		RPD	
Analyte	Result	Limit	Units	Result	%REC	Limit	RPD	Limit	Notes
General Inorganics									
Conductivity	626	5	uS/cm	627			0.2	5	
Cyanide, free	ND	0.03	ug/g dry	ND			NC	35	
рН	7.37	0.05	pH Units	7.38			0.1	2.3	
Hydrocarbons									
- F1 PHCs (C6-C10)	30	7	ua/a dry	22			30.6	40	
$F_2 PHC_{s} (C10_{-}C16)$		1	ug/g dry				NC	30	
$F_3 PHC_8 (C16-C34)$		-	ug/g dry				NC	30	
F4 PHCs (C34-C50)		6	ug/g dry				NC	30	
Motals	ND	0	ug/g ury	ND			NO	50	
Antinonia	1.0	1.0					NO	20	
Anumony	1.0	1.0	ug/g ary	ND 0.4			NC	30	
Alsellic	Z.Z	1.0	ug/g dry	2.1				30	
Bandlium	12.J	1.0	ug/g ury				9.0 NC	30	
Derymum Beren, sveileble	ND	0.5	ug/g dry				NC	30	
Boron	ND	0.5	ug/g dry				NC	30	
Bolon		5.0	ug/g dry				NC	30	
		0.5	ug/g dry	ND			NC	30	
Chromium (VI)	ND 8.0	0.2	ug/g ary				NC 6.0	35	
Chromum	0.9	5.0	ug/g dry	0.4			0.0 NC	30	
Copart	3.7	1.0	ug/g dry	3.0			NC	30	
Copper	8.3	5.0	ug/g ary	8.5			NC 6.2	30	
Lead	2.3	1.0	ug/g dry				0.2	30	
Melubdonum	ND	0.1	ug/g dry				NC	30	
Niekol	ND 6.2	1.0	ug/g dry				20	30	
Solonium	0.2	5.0 1.0	ug/g ury	0.4			2.0 NC	30	
Silver		1.0	ug/g dry				NC	30	
Sliver	ND	0.3	ug/g dry				NC	30	
	ND	1.0	ug/g dry				NC	30	
Venedium	ND 17.4	1.0	ug/g dry					30	
Zino	17.4 ND	10.0	ug/g dry				7.Z	30	
	ND	20.0	ug/g ury	ND			NC	30	
Physical Characteristics									
% Solids	78.6	0.1	% by Wt.	68.7			13.5	25	
Semi-Volatiles									
Acenaphthene	ND	0.02	ug/g dry	ND			NC	40	
Acenaphthylene	ND	0.02	ug/g dry	ND			NC	40	
Anthracene	ND	0.02	ug/g dry	ND			NC	40	
Benzo [a] anthracene	ND	0.02	ug/g dry	ND			NC	40	
Benzo [a] pyrene	ND	0.02	ug/g dry	ND			NC	40	
Benzo [b] fluoranthene	ND	0.02	ug/g dry	ND			NC	40	
Benzo [g,h,i] perylene	ND	0.02	ug/g dry	ND			NC	40	
Benzo [k] fluoranthene	ND	0.02	ug/g dry	ND			NC	40	
Chrysene	ND	0.02	ug/g dry	ND			NC	40	
Dibenzo [a,h] anthracene	ND	0.02	ug/g dry	ND			NC	40	
Fluoranthene	ND	0.02	ug/g dry	ND			NC	40	
Fluorene	ND	0.02	ug/g dry	ND			NC	40	
Indeno [1,2,3-cd] pyrene	ND	0.02	ug/g dry	ND			NC	40	
1-Methylnaphthalene	ND	0.02	ug/g dry	ND			NC	40	
2-Methylnaphthalene	ND	0.02	ug/g dry	ND			NC	40	
Naphthalene	ND	0.01	ug/g dry	ND			NC	40	
Phenanthrene	ND	0.02	ug/g dry	ND			NC	40	
Pyrene	ND	0.02	ug/g dry	ND			NC	40	
Surrogate: 2-Fluorobiphenyl	1.07		ug/g dry		67.2	50-140			
Surrogate: Terphenyl-d14	1.36		ug/g dry		85.5	50-140			
Volatiles									
Acetone	ND	0.50	ug/g dry	ND			NC	50	



Method Quality Control: Duplicate

Report Date: 07-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B

		Reporting		Source		%REC		RPD	
Analyte	Result	Limit	Units	Result	%REC	Limit	RPD	Limit	Notes
Benzene	ND	0.02	ug/g dry	ND			NC	50	
Bromodichloromethane	ND	0.05	ug/g dry	ND			NC	50	
Bromoform	ND	0.05	ug/g dry	ND			NC	50	
Bromomethane	ND	0.05	ug/g dry	ND			NC	50	
Carbon Tetrachloride	ND	0.05	ua/a drv	ND			NC	50	
Chlorobenzene	ND	0.05	ua/a drv	ND			NC	50	
Chloroform	ND	0.05	ua/a drv	ND			NC	50	
Dibromochloromethane	ND	0.05	ua/a drv	ND			NC	50	
Dichlorodifluoromethane	ND	0.05	ua/a drv	ND			NC	50	
1,2-Dichlorobenzene	ND	0.05	ug/g dry	ND			NC	50	
1.3-Dichlorobenzene	ND	0.05	ua/a drv	ND			NC	50	
1.4-Dichlorobenzene	ND	0.05	ua/a drv	ND			NC	50	
1,1-Dichloroethane	ND	0.05	ug/g dry	ND			NC	50	
1.2-Dichloroethane	ND	0.05	ua/a drv	ND			NC	50	
1.1-Dichloroethylene	ND	0.05	ua/a drv	ND			NC	50	
cis-1,2-Dichloroethylene	ND	0.05	ug/g dry	ND			NC	50	
trans-1,2-Dichloroethylene	ND	0.05	ug/g dry	ND			NC	50	
1,2-Dichloropropane	ND	0.05	ug/g dry	ND			NC	50	
cis-1,3-Dichloropropylene	ND	0.05	ug/g dry	ND			NC	50	
trans-1,3-Dichloropropylene	ND	0.05	ug/g dry	ND			NC	50	
Ethylbenzene	ND	0.05	ug/g dry	ND			NC	50	
Ethylene dibromide (dibromoethane, 1,2	ND	0.05	ug/g dry	ND			NC	50	
Hexane	ND	0.05	ug/g dry	ND			NC	50	
Methyl Ethyl Ketone (2-Butanone)	ND	0.50	ug/g dry	ND			NC	50	
Methyl Isobutyl Ketone	ND	0.50	ug/g dry	ND			NC	50	
Methyl tert-butyl ether	ND	0.05	ug/g dry	ND			NC	50	
Methylene Chloride	ND	0.05	ug/g dry	ND			NC	50	
Styrene	ND	0.05	ug/g dry	ND			NC	50	
1,1,1,2-Tetrachloroethane	ND	0.05	ug/g dry	ND			NC	50	
1,1,2,2-Tetrachloroethane	ND	0.05	ug/g dry	ND			NC	50	
Tetrachloroethylene	ND	0.05	ug/g dry	ND			NC	50	
Toluene	ND	0.05	ug/g dry	ND			NC	50	
1,1,1-Trichloroethane	ND	0.05	ug/g dry	ND			NC	50	
1,1,2-Trichloroethane	ND	0.05	ug/g dry	ND			NC	50	
Trichloroethylene	ND	0.05	ug/g dry	ND			NC	50	
Trichlorofluoromethane	ND	0.05	ug/g dry	ND			NC	50	
Vinyl chloride	ND	0.02	ug/g dry	ND			NC	50	
m,p-Xylenes	ND	0.05	ug/g dry	ND			NC	50	
o-Xylene	ND	0.05	ug/g dry	ND			NC	50	
Surrogate: 4-Bromofluorobenzene	11.1		ug/g dry		99.1	50-140			
Surrogate: Dibromofluoromethane	12.4		ug/g dry		111	50-140			
Surrogate: Toluene-d8	11.5		ug/g dry		103	50-140			
Benzene	ND	0.02	ug/g dry	ND			NC	50	
Ethylbenzene	ND	0.05	ug/g dry	ND			NC	50	
Toluene	ND	0.05	ug/g dry	ND			NC	50	
m,p-Xylenes	ND	0.05	ug/g dry	ND			NC	50	
o-Xylene	ND	0.05	ug/g dry	ND			NC	50	
Surrogate: Toluene-d8	11.5		ug/g dry		103	50-140			



Analyte

General Inorganics

Method Quality Control: Spike

							Report	Jale. 07-Jui-2	.04
							Order Da	ate: 24-Jun-20	02
						Projec	t Descript	ion: LOP20-0	02
Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes	
0.210	0.03	ug/g	ND	70.1	70-130				

Cyanide, free	0.210	0.03	ug/g	ND	70.1	70-130	
Hydrocarbons							
F1 PHCs (C6-C10)	187	7	ua/a	ND	93.6	80-120	
F2 PHCs (C10-C16)	119	4	ua/a	ND	129	60-140	
F3 PHCs (C16-C34)	292	8	ua/a	ND	129	60-140	
F4 PHCs (C34-C50)	164	6	ua/a	ND	115	60-140	
Metals			3.5				
Antimony	38.0	1.0	ua/a		77 7	70 130	
Arcenic	30.9 44 Q	1.0	ug/g		98.1	70-130	
Rorium	44.9	1.0	ug/g	16	00.1	70-130	
Bendlium	40.0	0.5	ug/g	4.0 ND	04.2	70-130	
Berginum Boron, available	40.3	0.5	ug/g		92.4	70-130	
Boron, available	3.73	0.5	ug/g		74.5	70-122	
	42.0	5.0	ug/g		02.9	70-130	
	43.1	0.5	ug/g	ND	86.2	70-130	
	0.1	0.2	ug/g	ND	74.5	70-130	
Chromium	47.9	5.0	ug/g	ND	89.1	70-130	
Cobalt	44.9	1.0	ug/g	1.5	86.9	70-130	
Copper	45.1	5.0	ug/g	ND	83.5	70-130	
Lead	41.9	1.0	ug/g	ND	82.1	70-130	
Mercury	1.42	0.1	ug/g	ND	94.9	70-130	
Molybdenum	42.4	1.0	ug/g	ND	84.7	70-130	
Nickel	45.2	5.0	ug/g	ND	85.3	70-130	
Selenium	44.2	1.0	ug/g	ND	88.2	70-130	
Silver	51.6	0.3	ug/g	ND	103	70-130	
Thallium	46.2	1.0	ug/g	ND	92.4	70-130	
Uranium	44.1	1.0	ug/g	ND	87.8	70-130	
Vanadium	49.2	10.0	ug/g	ND	85.5	70-130	QM-07
Zinc	46.6	20.0	ug/g	ND	83.6	70-130	
Semi-Volatiles							
Acenaphthene	0.141	0.02	ug/g	ND	71.2	50-140	
Acenaphthylene	0.132	0.02	ug/g	ND	66.4	50-140	
Anthracene	0.145	0.02	ug/g	ND	73.2	50-140	
Benzo [a] anthracene	0.131	0.02	ug/g	ND	66.0	50-140	
Benzo [a] pyrene	0.136	0.02	ug/g	ND	68.5	50-140	
Benzo [b] fluoranthene	0.155	0.02	ug/g	ND	77.9	50-140	
Benzo [g,h,i] perylene	0.129	0.02	ug/g	ND	65.2	50-140	
Benzo [k] fluoranthene	0.149	0.02	uq/q	ND	74.8	50-140	
Chrysene	0.148	0.02	ug/g	ND	74.4	50-140	
Dibenzo [a.h] anthracene	0.118	0.02	ua/a	ND	59.4	50-140	
Fluoranthene	0.142	0.02	uq/q	ND	71.6	50-140	
Fluorene	0.138	0.02	ua/a	ND	69.7	50-140	
Indeno [1.2.3-cd] pyrene	0.122	0.02	ua/a	ND	61.5	50-140	
1-Methylnaphthalene	0 138	0.02	ua/a	ND	69.4	50-140	
2-Methylnaphthalene	0 158	0.02	ua/a	ND	79 7	50-140	
Naphthalene	0 162	0.01	nu/u	ND	81.3	50-140	
Phenanthrene	0.102	0.02	nu\u ~a,a	ND	70.4	50-140	
Pyrene	0 144	0.02	~9,9	ND	72 7	50-140	
Surrogate: 2-Eluorobinhenvl	0 959	0.02	~9 [,] 9		60.3	50-140	
	0.000		~9′9				

Report Date: 07-Jul-2020

20

2B



Method Quality Control: Spike

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Surrogate: Terphenyl-d14	1.23		ug/g		77.4	50-140			
Volatiles									
Acetone	11.2	0.50	ug/g	ND	112	50-140			
Benzene	4.57	0.02	ug/g	ND	114	60-130			
Bromodichloromethane	4.45	0.05	ug/g	ND	111	60-130			
Bromoform	4.54	0.05	ug/g	ND	113	60-130			
Bromomethane	3.15	0.05	ug/g	ND	78.7	50-140			
Carbon Tetrachloride	4.29	0.05	ug/g	ND	107	60-130			
Chlorobenzene	4.27	0.05	ug/g	ND	107	60-130			
Chloroform	4.66	0.05	ug/g	ND	116	60-130			
Dibromochloromethane	4.62	0.05	ug/g	ND	116	60-130			
Dichlorodifluoromethane	2.44	0.05	ug/g	ND	60.9	50-140			
1,2-Dichlorobenzene	4.35	0.05	ug/g	ND	109	60-130			
1,3-Dichlorobenzene	4.30	0.05	ug/g	ND	108	60-130			
1,4-Dichlorobenzene	4.14	0.05	ug/g	ND	104	60-130			
1,1-Dichloroethane	5.00	0.05	uq/q	ND	125	60-130			
1,2-Dichloroethane	4.88	0.05	uq/q	ND	122	60-130			
1,1-Dichloroethylene	3.74	0.05	uq/q	ND	93.4	60-130			
cis-1,2-Dichloroethylene	4.67	0.05	uq/q	ND	117	60-130			
trans-1.2-Dichloroethylene	3.59	0.05	ua/a	ND	89.7	60-130			
1,2-Dichloropropane	4.79	0.05	uq/q	ND	120	60-130			
cis-1.3-Dichloropropylene	5.01	0.05	ua/a	ND	125	60-130			
trans-1.3-Dichloropropylene	5.12	0.05	ua/a	ND	128	60-130			
Ethylbenzene	4.41	0.05	ua/a	ND	110	60-130			
Ethylene dibromide (dibromoethane, 1.2	4.73	0.05	ua/a	ND	118	60-130			
Hexane	3.33	0.05	ua/a	ND	83.1	60-130			
Methyl Ethyl Ketone (2-Butanone)	11.9	0.50	ua/a	ND	119	50-140			
Methyl Isobutyl Ketone	12.8	0.50	ua/a	ND	128	50-140			
Methyl tert-butyl ether	13.0	0.05	ua/a	ND	130	50-140			
Methylene Chloride	4.82	0.05	ua/a	ND	121	60-130			
Styrene	4.33	0.05	ua/a	ND	108	60-130			
1.1.1.2-Tetrachloroethane	4.58	0.05	ua/a	ND	114	60-130			
1.1.2.2-Tetrachloroethane	5.00	0.05	ua/a	ND	125	60-130			
Tetrachloroethylene	3.70	0.05	ua/a	ND	92.5	60-130			
Toluene	3.82	0.05	ua/a	ND	95.5	60-130			
1.1.1-Trichloroethane	4.80	0.05	ua/a	ND	120	60-130			
1.1.2-Trichloroethane	4.87	0.05	ua/a	ND	122	60-130			
Trichloroethylene	4.09	0.05	ua/a	ND	102	60-130			
Trichlorofluoromethane	3.96	0.05	ua/a	ND	99.0	50-140			
Vinvl chloride	2.47	0.02	ua/a	ND	61.9	50-140			
m.p-Xvlenes	8.65	0.05	ua/a	ND	108	60-130			
o-Xvlene	4.60	0.05	ua/a	ND	115	60-130			
Surrogate: 4-Bromofluorobenzene	7 77				97.1	50-140			
Surrogate: Dibromofluoromethane	10.4		uq/q		130	50-140			
Surrogate: Toluene-d8	7.57		ug/g		94.7	50-140			
Benzene	4.57	0.02	ug/g	ND	114	60-130			
Ethylbenzene	4.41	0.05	ug/g	ND	110	60-130			
Toluene	3.82	0.05	ug/g	ND	95.5	60-130			
m,p-Xylenes	8.65	0.05	ug/g	ND	108	60-130			
o-Xylene	4.60	0.05	ug/g	ND	115	60-130			

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Order #: 2026367

Report Date: 07-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B



Report Date: 07-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B

Method Quality Control: Spike

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Surrogate: Toluene-d8	7.57		ug/g		94.7	50-140			



Login Qualifiers :

Container(s) - Labeled improperly/insufficient information - Date reads 23 on Jar Applies to samples: BH3-20-SS6, BH4-20-SS6

Container(s) - Labeled improperly/insufficient information - Date reads 23 on Vial Applies to samples: BH5-20-SS4

Container(s) - Bottle and COC sample ID don't match - ID missing -20- on jars Applies to samples: BH1-20-SS5, BH3-20-SS6, BH4-20-SS6, BH5-20-SS7, BH11-20-SS5

QC Qualifiers :

QM-07: The spike recovery was outside acceptance limits for the MS and/or MSD. The batch was accepted based on other acceptable QC.

Sample Data Revisions

None

Work Order Revisions / Comments:

Revision 1 - This report includes updated data for metals due to preparation error

Other Report Notes:

n/a: not applicable ND: Not Detected MDL: Method Detection Limit Source Result: Data used as source for matrix and duplicate samples %REC: Percent recovery. RPD: Relative percent difference. NC: Not Calculated

Soil results are reported on a dry weight basis when the units are denoted with 'dry'. Where %Solids is reported, moisture loss includes the loss of volatile hydrocarbons.

CCME PHC additional information:

- The method for the analysis of PHCs complies with the Reference Method for the CWS PHC and is validated for use in the laboratory. All prescribed quality criteria identified in the method has been met.

- F1 range corrected for BTEX.

- F2 to F3 ranges corrected for appropriate PAHs where available.

- The gravimetric heavy hydrocarbons (F4G) are not to be added to C6 to C50 hydrocarbons.

- In the case where F4 and F4G are both reported, the greater of the two results is to be used for comparison to CWS PHC criteria.

- When reported, data for F4G has been processed using a silica gel cleanup.

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Report Date: 07-Jul-2020 Order Date: 24-Jun-2020 Project Description: LOP20-002B

PARAC Parac	el I	D: 2	026. 	367 	ent Blvd. 1G 4J8	Pa	racel ((Lab	Order I	Numb Inly)	er		Ch	ain ((Lab	Of Cus Use Oni	tody v)
LABORATORIES					llabs.com	20	26	.3(./- 3-	PVII TCL		N°.	5	4317	1
Client Name: LEPERS : ASSOCIATES		Proje	ect Ref:	LOPZO-00	2B		1		No. 1 Tagata		4	9	Pag	e_of	
Contact Name: Luke Lopers		Quot	e #:	P	an a				73.1	9		Т	urnar	ound T	ime
Address: Za la abaldlata (Harin (a)		PO #	Lo	P20-002	.B] 0	1 day			🛛 3 day
Denstricia una, citada, on		E-ma	il: 1 - 1		· · · · ·	ι.	-					2 day			Regula
Telephone: 613-327-9073			Lut	e@Lopes	s.ca						Date	Requir	ed:	1	
Regulation 153/04 Other Regulation		Matrix	Type:	S (Soil/Sed) GW ((Fround Water										
Table 1 Res/Park Med/Fine REG 558 PWQO	1	SW (Su	urface \	Vater) SS (Storm/S	anitary Sewer)					Re	equired N	d Analy	sis		
Table 2 Ind/Comm Coarse CCME MISA			P (F	aint) A (Air) O (Ot	her)	1 		Å		1	ĥ			Ν	TT
Table 3 Agri/Other SU - Sani SU - Storm			ers			٦¥		er gen		lech	e t				
Table		a	ntain	Sample	Taken	878	6	4/3	\$	122					
For RSC: U Yes U No U Other:	atrix	Volc	of Co		1	5	8	elak	8	S S		2 - 3 * 1		(1)学)	
Sample ID/Location Name	Σ	Ż	*	Date	Time	Å	-	ž		25			_	\perp	
- Df[-20-SS5	2	-	2	June 24,2020		X	Х	Х	Х				$ \rightarrow$		++
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BH9-20-356	5	-	6		2.0	X		Х	Х		794		<u>.</u>	\perp	++
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0 10HII-20-555	S		2			×	Х	Х	Х	. /			_		
· ICLY	2		2	V.						X			÷,		
0										×					
10															
Comments:															
										Metho	d of Del	ivery:	16		
Received By Dr	iver/De	epot:			Received at Lab					Verifie	d By:	VI	D		-
Relinquiched Bergert 1					14	2	E				1	2		Z	
Date/Time: Luke Lopers Uste/Time:					Date/Clime:	24	20	17	56	Date/T	ime:)G-	20	22	12/19
June 24, 2020 / 5:30 PM				°C	Temperature: 2	1,9	°C			pH Ver	ified: []	By:		



300 - 2319 St. Laurent Blvd Ottawa, ON, K1G 4J8 1-800-749-1947 www.paracellabs.com

Certificate of Analysis

Lopers & Associates	
30 Lansfield Way Ottawa, ONT K2G 3V8 Attn: Luke Lopers	
Client PO: LOP20-002B	
Project: LOP20-002B	Report Date: 6-Jul-2020
Custody: 54317	Order Date: 24-Jun-2020
	Order #: 2026368

This Certificate of Analysis contains analytical data applicable to the following samples as submitted:

Paracel IDClient IDParacel ID2026368-01TCLP

Client ID

Approved By:

6

Dale Robertson, BSc Laboratory Director

Analysis Summary Table

Report Date: 06-Jul-2020 Order Date: 24-Jun-2020

Project Description: LOP20-002B

Analysis	Method Reference/Description	Extraction Date	Analysis Date
Flashpoint	ASTM D93 - Pensky-Martens Closed Cup	29-Jun-20	29-Jun-20
REG 558 - Cyanide	MOE E3015- Auto Colour	6-Jul-20	6-Jul-20
REG 558 - Fluoride	EPA 340.2 - ISE	30-Jun-20	30-Jun-20
REG 558 - Mercury by CVAA	EPA 7470A - Cold Vapour AA	29-Jun-20	29-Jun-20
REG 558 - Metals, ICP-MS	TCLP EPA 6020 - Digestion - ICP-MS	30-Jun-20	30-Jun-20
REG 558 - NO3/NO2	EPA 300.1 - IC	30-Jun-20	30-Jun-20
REG 558 - PCBs	EPA 608 - GC-ECD	29-Jun-20	29-Jun-20
REG 558 - SVOCs	EPA 625 - GC-MS	29-Jun-20	30-Jun-20
REG 558 - VOCs	EPA 624 - P&T GC-MS	30-Jun-20	30-Jun-20
Solids, %	Gravimetric, calculation	30-Jun-20	30-Jun-20



Report Date: 06-Jul-2020 Order Date: 24-Jun-2020

Project Description: LOP20-002B

Criteria:

Summary of Exceedances

(If this page is blank then there are no exceedances)

Only those criteria that a sample exceeds will be highlighted in red

Regulatory Comparison:

Paracel Laboratories has provided regulatory guidelines on this report for informational purposes only and makes no representations or warranties that the data is accurate or reflects the current regulatory values. The user is advised to consult with the appropriate official regulations to evaluate compliance. Sample results that are highlighted have exceeded the selected regulatory limit. Calculated uncertainty estimations have not been applied for determining regulatory exceedances. Regulatory limits displayed in brackets, (), applies to medium and fine textured soils.

Client ID	Analyte	MDL / Units	Result	Reg 558 Schedule 4

PARACEL LABORATORIES LTD.

Certificate of Analysis

Report Date: 06-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B

	Client ID:	TCLP	-	-	-	
	Sample Date:	24-Jun-2020	-	-	-	Criteria:
	Sample ID:	2026368-01	-	-	-	Reg 558 Schedule 4
	Matrix:	Soil	-	-	-	
	MDL/Units					
Physical Characteristics			-	-		
% Solids	0.1 % by Wt.	91.8	-	-	-	
Flashpoint	°C	>70	-	-	-	
EPA 1311 - TCLP Leachate Inorganics	5				-	
Fluoride	0.05 mg/L	0.32	-	-	-	150 mg/L
Nitrate as N	1 mg/L	<1	-	-	-	1,000 mg/L
Nitrite as N	1 mg/L	<1	-	-	-	1,000 mg/L
Cyanide, free	0.02 mg/L	<0.02	-	-	-	20 mg/L
EPA 1311 - TCLP Leachate Metals						
Arsenic	0.05 mg/L	<0.05	-	-	-	2.5 mg/L
Barium	0.05 mg/L	0.97	-	-	-	100 mg/L
Boron	0.05 mg/L	0.05	-	-	-	500 mg/L
Cadmium	0.01 mg/L	<0.01	-	-	-	0.5 mg/L
Chromium	0.05 mg/L	<0.05	-	-	-	5 mg/L
Lead	0.05 mg/L	<0.05	-	-	-	5 mg/L
Mercury	0.005 mg/L	<0.005	-	-	-	0.1 mg/L
Selenium	0.05 mg/L	<0.05	-	-	-	1 mg/L
Silver	0.05 mg/L	<0.05	-	-	-	5 mg/L
Uranium	0.05 mg/L	<0.05	-	-	-	10 mg/L
EPA 1311 - TCLP Leachate Volatiles						
Benzene	0.005 mg/L	<0.005	-	-	-	0.5 mg/L
Carbon Tetrachloride	0.005 mg/L	<0.005	-	-	-	0.5 mg/L
Chlorobenzene	0.004 mg/L	<0.004	-	-	-	8 mg/L
Chloroform	0.006 mg/L	<0.006	-	-	-	10 mg/L
-			-	-	-	

PARACEL

Certificate of Analysis Client: Lopers & Associates

Client PO: LOP20-002B

Report Date: 06-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B

Samploine 24-00-200 - - - - Chronic Samploine Samploine Samploine -		Client ID:	TCLP	-	-	-	
Sample Bin Ministry Mathematical Sample Sa		Sample Date:	24-Jun-2020	-	-	-	Criteria:
Mathem Sol P P 1.20-blorobenzene 0.00400 0.00000 <td></td> <td>Sample ID:</td> <td>2026368-01</td> <td>-</td> <td>-</td> <td>-</td> <td>Reg 558 Schedule 4</td>		Sample ID:	2026368-01	-	-	-	Reg 558 Schedule 4
MDU/UNSMDU/UNSMDU/UNSMDU/UNSMDU/UNSMDU/UNSMDU/UNS1.2.Dichlorobenzene0.004 mll-0.004-0.0-0.0-0.0-0.0mgL1.4.Dichlorobenzene0.005 mgL-0.005-0.0-0.0-0.0-0.0mgL1.1.Dichlorothylene0.005 mgL-0.005-0.0-0.0-0.0-0.0mgL1.1.Dichlorothylene0.005 mgL-0.005-0.0-0.0-0.0-0.0mgLMthylen Choide0.03 mgL-0.005-0.0-0.0-0.0-0.0mgLMthylen Choide0.04 mgL-0.005-0.0-0.0-0.0mgL-0.0mgLTichlorothylene0.005 mgL-0.005-0.0-0.0-0.0mgL-0.0mgLTichlorothylene0.005 mgL-0.005-0.0-0.0-0.0mgL-0.0mgLDibromofucomatina3.005 mgL-0.0-0.0-0.0-0.0mgL-0.0mgLDibromofucomatina3.005 mgL-0.0-0.0-0.0-0.0mgL-0.0mgLDibromofucomatina3.005 mgL-0.0-0.0-0.0-0.0mgL-0.0mgLDibromofucomatina3.005 mgL-0.0-0.0-0.0-0.0mgL-0.0mgLDibromofucomatina3.005 mgL-0.0-0.0-0.0-0.0mgL-0.0mgLDibromofucomatina-0.05 mgL-0.0-0.0-0.0-0.0mgL-0.		Matrix:	Soil	-	-	-	
1.2-Dichlorobenzene0.004 mgL<0004000mgL1.4-Dichlorobenzene0.004 mgL<0.005		MDL/Units					
1.4-Dichlorodehane0.004 mgl-0.004-0.005mgl1.2-Dichlorodehane0.005 mgl-0.005-0.00 <t< td=""><td>1,2-Dichlorobenzene</td><td>0.004 mg/L</td><td><0.004</td><td>-</td><td>-</td><td>-</td><td>20 mg/L</td></t<>	1,2-Dichlorobenzene	0.004 mg/L	<0.004	-	-	-	20 mg/L
1.2.Dichloroethane0.005 mgl.4.0.05.	1,4-Dichlorobenzene	0.004 mg/L	<0.004	-	-	-	0.5 mg/L
1.1.Dicklorendly0.408 mind0.4000.4000.400m.d.Metyl Edyt Koton (2-Batton)0.300 mind0.400	1,2-Dichloroethane	0.005 mg/L	<0.005	-	-	-	0.5 mg/L
Methylethylethylethylethylethylethylethyl	1,1-Dichloroethylene	0.006 mg/L	<0.006	-	-	-	1.4 mg/L
Methylene choloride0.4 mg/d0.4 mg/dmg/dTetachloredtylene0.005 mg/d0.005 mg/d0.01 mg/d0.01 mg/dTichloredtylene0.004 mg/d0.004 mg/d0.01 mg/d0.01 mg/dVindrode0.004 mg/d0.01 mg/d0.01 mg/d0.01 mg/dVindrode0.007 mg/d0.01 mg/d0.01 mg/d0.01 mg/dAbromotorobargene0.007 mg/d0.01 mg/d0.01 mg/d0.01 mg/dDibonduromethane0.01 mg/d0.01 mg/d0.01 mg/d0.01 mg/dTotareds0.01 mg/d0.01 mg/d0.01 mg/d0.01 mg/dPatrettrettrettrettrettrettrettrettrettre	Methyl Ethyl Ketone (2-Butanone)	0.30 mg/L	<0.30	-	-	-	200 mg/L
Tetachlorothylene0.005 mg/l.0005 mg/l.0006 mg/	Methylene Chloride	0.04 mg/L	<0.04	-	-	-	5 mg/L
Trichorothylene1.004 m/dl<	Tetrachloroethylene	0.005 mg/L	<0.005	-	-	-	3 mg/L
Viny choirde0.00 mg/L	Trichloroethylene	0.004 mg/L	<0.004	-	-	-	5 mg/L
4-BromofluorobenzeneSurogate115%	Vinyl chloride	0.005 mg/L	<0.005	-	-	-	0.2 mg/L
DibronofluoromethaneSurrogate112%IIIIIIToluene-d8Surrogate101%II	4-Bromofluorobenzene	Surrogate	115%	-	-	-	
Toluene48Surgade101%EPA131 - TCLP Leachtad Organist2,4-Dintroduene0.001 mg/L0.001 mg/L0.01mg/LBenzo [a pyrene mail0.001 mg/L0.010.01mg/LBenzo [a pyrene mail0.001 mg/L0.010.01mg/LNitobenzene0.001 mg/L0.0010.010.01mg/LHexachoroethane0.001 mg/L0.010.010.01mg/LHexachoroethane0.001 mg/L0.0010.010.01mg/L1,2,4,6-Terchoroethane0.001 mg/L0.010.010.01mg/L2,3,4,6-Terchoroethane0.001 mg/L0.010.010.01mg/L2,4,6-Terchoroethane0.001 mg/L0.010.010.01mg/L2,4,6-Terchoroethane0.001 mg/L0.010.010.01mg/L2,4,6-Terchoroethane0.001 mg/L0.010.010.01mg/L2,4,6-Terchoroethane0.010.010.010.01mg/L2,4,6-Terchoroethane0.010.010.010.01mg/L2,4,6-Terchoroethane0.010.010.010.01mg/L2,4,6-Terchoroethane0.010.010.010.01mg/L2,4,6-Terchoroethane0.010.010.010.01mg/L2,4,6-Terchoroethane0.010.010.010.01mg/L2,4-Dettoroethane0.010.010.010.01mg/L2,4-Dettoroethane </td <td>Dibromofluoromethane</td> <td>Surrogate</td> <td>112%</td> <td>-</td> <td>-</td> <td>-</td> <td></td>	Dibromofluoromethane	Surrogate	112%	-	-	-	
EPA 131 - TCL P Leachate Organis 2,4-Dinitrobluene 0.001 mg/L <0.001	Toluene-d8	Surrogate	101%	-	-	-	
2,4-Dinitrodulene0.001 mg/L.<001	EPA 1311 - TCLP Leachate Organics						-
Benzo [a] pyrene0.001 mg/L	2,4-Dinitrotoluene	0.001 mg/L	<0.001	-	-	-	0.13 mg/L
Nitobenzene0.001 mg/L<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001<0.001	Benzo [a] pyrene	0.001 mg/L	<0.001	-	-	-	0.001 mg/L
Hexachlorodethane0.001 mg/L<0.001 mg/L<0.00	Nitrobenzene	0.001 mg/L	<0.001	-	-	-	2 mg/L
Hexachlorobenzene0.050 mg/L<0.0500.13mg/LHexachlorobtadiene0.001 mg/L<0.001	Hexachloroethane	0.001 mg/L	<0.001	-	-	-	3 mg/L
Hexachlorobutadiene0.001 mg/L<0.001<1<1<1<10<10mg/L2,3,4,6-Tetrachlorophenol0.002 mg/L<0.002	Hexachlorobenzene	0.050 mg/L	<0.050	-	-	-	0.13 mg/L
2,3,4,6-Tetrachlorophenol 0.002 mg/L <0.002 mg/L <0.002 mg/L <0.001 mg/L <0.0	Hexachlorobutadiene	0.001 mg/L	<0.001	-	-	-	
2,4,5-Trichlorophenol 0.001 mg/L <0.001	2,3,4,6-Tetrachlorophenol	0.002 mg/L	<0.002	-	-	-	10 mg/L
2,4,6-Trichlorophenol 0.001 mg/L <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001 <0.001	2,4,5-Trichlorophenol	0.001 mg/L	<0.001	-	-	-	400 mg/L
2,4-Dichlorophenol 0.001 mg/L <0.001 - - - 90 mg/L 2-Methylphenol 0.001 mg/L <0.001	2,4,6-Trichlorophenol	0.001 mg/L	<0.001	-	-	-	0.5 mg/L
2-Methylphenol 0.001 mg/L <0.001 200 mg/L	2,4-Dichlorophenol	0.001 mg/L	<0.001	-	-	-	90 mg/L
	2-Methylphenol	0.001 mg/L	<0.001	-	-	-	200 mg/L

PARACEL LABORATORIES LTD.

Certificate of Analysis

Client: Lopers & Associates

Client PO: LOP20-002B

Report Date: 06-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B

	Client ID:	TCLP	-	-	-	
	Sample Date:	24-Jun-2020	-	-	-	Criteria:
	Sample ID:	2026368-01	-	-	-	Reg 558 Schedule 4
	Matrix:	Soil	-	-	-	
	MDL/Units					
3/4-Methylphenol	0.001 mg/L	<0.001	-	-	-	200 mg/L
Pentachlorophenol	0.005 mg/L	<0.005	-	-	-	6 mg/L
2,4,6-Tribromophenol	Surrogate	50.0%	-	-	-	
2-Fluorobiphenyl	Surrogate	66.4%	-	-	-	
2-Fluorophenol	Surrogate	12.8% [2]	-	-	-	
Terphenyl-d14	Surrogate	94.8%	-	-	-	
PCBs, total	0.003 mg/L	<0.003	-	-	-	0.3 mg/L
Decachlorobiphenyl	Surrogate	90.0%	-	-	-	
PARACEL LABORATORIES LTD.

Certificate of Analysis

Client: Lopers & Associates

Client PO: LOP20-002B

Method Quality Control: Blank

[Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
EPA	1311 - TCLP Leachate Inorganics									
	Fluoride	ND	0.05	mg/L						
	Nitrate as N	ND	1	mg/L						
	Nitrite as N	ND	1	mg/L						
	Cyanide, free	ND	0.02	mg/L						
EPA	1311 - TCLP Leachate Metals			-						
	Arsenic	ND	0.05	mg/L						
	Barium	ND	0.05	mg/L						
	Boron	ND	0.05	mg/L						
	Cadmium	ND	0.01	mg/L						
	Chromium	ND	0.05	mg/L						
	Lead	ND	0.05	mg/L						
	Mercury	ND	0.005	mg/L						
	Selenium	ND	0.05	mg/L						
	Silver	ND	0.05	mg/L						
	Uranium	ND	0.05	mg/L						
EPA	1311 - TCLP Leachate Organics									
	2,4-Dinitrotoluene	ND	0.001	mg/L						
	Benzo [a] pyrene	ND	0.001	mg/L						
	Nitrobenzene	ND	0.001	mg/L						
	Hexachloroethane	ND	0.001	mg/L						
	Hexachlorobenzene	ND	0.050	mg/L						
	Hexachlorobutadiene	ND	0.001	mg/L						
	2,3,4,6-Tetrachlorophenol	ND	0.002	mg/L						
	2,4,5-Trichlorophenol	ND	0.001	mg/L						
	2,4,6-Trichlorophenol	ND	0.001	mg/L						
	2,4-Dichlorophenol	ND	0.001	mg/L						
	2-Methylphenol	ND	0.001	mg/L						
	3/4-Methylphenol	ND	0.001	mg/L						
	Pentachlorophenol	ND	0.005	mg/L						
	Surrogate: 2,4,6-Tribromophenol	0.019		mg/L		46.7	40-150			
	Surrogate: 2-Fluorobiphenyl	0.013		mg/L		62.8	40-150			
	Surrogate: 2-Fluorophenol	0.0053		mg/L		13.2	40-150		S-GC	
	Surrogate: Terphenyl-d14	0.021		mg/L		107	40-150			
	PCBs, total	ND	0.003	mg/L						
	Surrogate: Decachlorobiphenyl	0.0080		mg/L		80.4	62-138			
EPA	1311 - TCLP Leachate Volatiles			-						
	Benzene	ND	0.005	ma/l						
	Carbon Tetrachloride	ND	0.005	mg/L						
	Chlorobenzene	ND	0.004	mg/L						
			0.004	ing/L						

Order #: 2026368

Report Date: 06-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B



Certificate of Analysis Client: Lopers & Associates

Client PO: LOP20-002B

Method Quality Control: Blank

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Chloroform	ND	0.006	mg/L						
1,2-Dichlorobenzene	ND	0.004	mg/L						
1,4-Dichlorobenzene	ND	0.004	mg/L						
1,2-Dichloroethane	ND	0.005	mg/L						
1,1-Dichloroethylene	ND	0.006	mg/L						
Methyl Ethyl Ketone (2-Butanone)	ND	0.30	mg/L						
Methylene Chloride	ND	0.04	mg/L						
Tetrachloroethylene	ND	0.005	mg/L						
Trichloroethylene	ND	0.004	mg/L						
Vinyl chloride	ND	0.005	mg/L						
Surrogate: 4-Bromofluorobenzene	0.851		mg/L		124	83-134			
Surrogate: Dibromofluoromethane	0.796		mg/L		116	78-124			
Surrogate: Toluene-d8	0.767		mg/L		111	76-118			

Order #: 2026368

Report Date: 06-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B

PARACEL LABORATORIES LTD.

Certificate of Analysis

Client: Lopers & Associates

Client PO: LOP20-002B

Method Quality Control: Duplicate

		Reporting		Source		%REC		RPD	
Analyte	Result	Limit	Units	Result	%REC	Limit	RPD	Limit	Notes
EPA 1311 - TCLP Leachate Inorganics									
Fluoride	0.32	0.05	ma/L	0.32			0.4	20	
Nitrate as N	ND	1	mg/L	ND			NC	20	
Nitrite as N	ND	1	mg/L	ND			NC	20	
Cyanide, free	ND	0.02	mg/L	ND			NC	20	
EPA 1311 - TCLP Leachate Metals			U						
Arsenic	ND	0.05	ma/L	ND			NC	29	
Barium	0.960	0.05	mg/L	0.969			0.9	34	
Boron	0.055	0.05	mg/L	0.055			1.4	33	
Cadmium	ND	0.01	mg/L	ND			NC	33	
Chromium	ND	0.05	mg/L	ND			NC	32	
Lead	ND	0.05	mg/L	ND			NC	32	
Mercury	ND	0.005	mg/L	ND			NC	30	
Selenium	ND	0.05	mg/L	ND			NC	28	
Silver	ND	0.05	mg/L	ND			NC	28	
Uranium	ND	0.05	mg/L	ND			NC	27	
EPA 1311 - TCLP Leachate Organics									
PCBs, total	ND	0.003	mg/L	ND			NC	30	
Surrogate: Decachlorobiphenyl	0.0090		mg/L		89.7	62-138			
EPA 1311 - TCLP Leachate Volatiles									
Benzene	ND	0.005	mg/L	ND			NC	25	
Carbon Tetrachloride	ND	0.005	mg/L	ND			NC	25	
Chlorobenzene	ND	0.004	mg/L	ND			NC	25	
Chloroform	ND	0.006	mg/L	ND			NC	25	
1,2-Dichlorobenzene	ND	0.004	mg/L	ND			NC	25	
1,4-Dichlorobenzene	ND	0.004	mg/L	ND			NC	25	
1,2-Dichloroethane	ND	0.005	mg/L	ND			NC	25	
1,1-Dichloroethylene	ND	0.006	mg/L	ND			NC	25	
Methyl Ethyl Ketone (2-Butanone)	ND	0.30	mg/L	ND			NC	25	
Methylene Chloride	ND	0.04	mg/L	ND			NC	25	
Tetrachloroethylene	ND	0.005	mg/L	ND			NC	25	
Trichloroethylene	ND	0.004	mg/L	ND			NC	25	
Vinyl chloride	ND	0.005	mg/L	ND			NC	25	
Surrogate: 4-Bromofluorobenzene	0.788		mg/L		114	83-134			
Surrogate: Dibromofluoromethane	0.730		mg/L		106	78-124			
Surrogate: Toluene-d8	0.690		mg/L		100	76-118			
Physical Characteristics									
% Solids	77.6	0.1	% by Wt.	75.9			2.2	25	

Report Date: 06-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B

PARACEL

Certificate of Analysis Client: Lopers & Associates

Client PO: LOP20-002B

Method Quality Control: Spike

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
EPA 1311 - TCLP Leachate Inorganics									
Fluoride	0.71	0.05	mg/L	0.32	78.3	70-130			
Nitrate as N	10	1	mg/L	ND	99.6	81-112			
Nitrite as N	10	1	mg/L	ND	100	76-107			
Cyanide, free	0.042	0.02	mg/L	ND	84.4	60-136			
EPA 1311 - TCLP Leachate Metals									
Arsenic	52.0	0.05	mg/L	0.450	103	83-119			
Barium	155	0.05	mg/L	96.9	117	83-116			QM-07
Boron	44.4	0.05	mg/L	5.45	78.0	71-128			
Cadmium	47.0	0.01	mg/L	0.050	93.9	78-119			
Chromium	59.5	0.05	mg/L	0.460	118	80-124			
Lead	47.5	0.05	mg/L	2.63	89.8	77-126			
Mercury	0.0305	0.005	mg/L	ND	102	70-130			
Selenium	44.8	0.05	mg/L	0.864	87.8	81-125			
Silver	49.1	0.05	mg/L	ND	98.2	70-128			
Uranium	48.7	0.05	mg/L	0.765	95.8	70-131			
EPA 1311 - TCLP Leachate Organics									
2,4-Dinitrotoluene	0.006	0.001	mg/L	ND	63.9	50-140			
Benzo [a] pyrene	0.009	0.001	mg/L	ND	93.1	50-140			
Nitrobenzene	0.013	0.001	mg/L	ND	129	50-140			
Hexachloroethane	0.011	0.001	mg/L	ND	109	50-140			
Hexachlorobutadiene	0.006	0.001	mg/L	ND	57.3	50-140			
2,3,4,6-Tetrachlorophenol	0.006	0.002	mg/L	ND	60.6	51-140			
2,4,5-Trichlorophenol	0.007	0.001	mg/L	ND	66.6	50-140			
2,4,6-Trichlorophenol	0.006	0.001	mg/L	ND	64.9	50-140			
2,4-Dichlorophenol	0.009	0.001	mg/L	ND	94.3	50-140			
2-Methylphenol	0.006	0.001	mg/L	ND	58.0	50-140			
3/4-Methylphenol	0.006	0.001	mg/L	ND	64.0	50-140			
Pentachlorophenol	0.005	0.005	mg/L	ND	50.6	50-140			
Surrogate: 2,4,6-Tribromophenol	0.021		mg/L		53.4	40-150			
Surrogate: 2-Fluorobiphenyl	0.014		mg/L		67.6	40-150			
Surrogate: 2-Fluorophenol	0.0030		mg/L		7.55	40-150			S-GC
Surrogate: Terphenyl-d14	0.018		mg/L		87.7	40-150			
PCBs, total	0.035	0.003	mg/L	ND	87.0	86-145			
Surrogate: Decachlorobiphenyl	0.0085		mg/L		84.8	62-138			

Order #: 2026368

Report Date: 06-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B

PARACEL LABORATORIES LTD.

Certificate of Analysis

Client: Lopers & Associates

Client PO: LOP20-002B

Method Quality Control: Spike

Analyte Re	esult	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
EPA 1311 - TCLP Leachate Volatiles									
Benzene 0.3	314	0.005	mg/L	ND	91.3	55-141			
Carbon Tetrachloride 0.3	391	0.005	mg/L	ND	114	49-149			
Chlorobenzene 0.3	340	0.004	mg/L	ND	98.8	64-137			
Chloroform 0.3	397	0.006	mg/L	ND	115	58-138			
1,2-Dichlorobenzene 0.4	409	0.004	mg/L	ND	119	60-150			
1,4-Dichlorobenzene 0.3	330	0.004	mg/L	ND	96.0	63-132			
1,2-Dichloroethane 0.3	383	0.005	mg/L	ND	111	50-140			
1,1-Dichloroethylene 0.4	404	0.006	mg/L	ND	118	43-153			
Methyl Ethyl Ketone (2-Butanone) 1.	.00	0.30	mg/L	ND	117	26-153			
Methylene Chloride 0.3	357	0.04	mg/L	ND	104	58-149			
Tetrachloroethylene 0.4	415	0.005	mg/L	ND	121	51-145			
Trichloroethylene 0.4	411	0.004	mg/L	ND	120	52-135			
Vinyl chloride 0.3	351	0.005	mg/L	ND	102	31-159			
Surrogate: 4-Bromofluorobenzene 0.7	740		mg/L		107	83-134			
Surrogate: Dibromofluoromethane 0.6	655		mg/L		95.2	78-124			
Surrogate: Toluene-d8 0.6	628		mg/L		91.4	76-118			

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Order #: 2026368

Report Date: 06-Jul-2020

Order Date: 24-Jun-2020

Project Description: LOP20-002B



Certificate of Analysis Client: Lopers & Associates

Client PO: LOP20-002B

2 : Surrogate recovery outside of control limits. The data was accepted based on valid recovery of the remaining surrogate.

QC Qualifiers :

- QM-07 : The spike recovery was outside acceptance limits for the MS and/or MSD. The batch was accepted based on other acceptable QC.
- S-GC: Surrogate recovery outside of control limits. The data was accepted based on valid recovery of the remaining surrogate.

Sample Data Revisions

None

Work Order Revisions / Comments:

None

Other Report Notes:

n/a: not applicable ND: Not Detected MDL: Method Detection Limit Source Result: Data used as source for matrix and duplicate samples %REC: Percent recovery. RPD: Relative percent difference. NC: Not Calculated Soil/Solid results are reported on a dry weight basis unless otherwise indicated

Where %Solids is reported, moisture loss includes the loss of volatile hydrocarbons.

Any use of these results implies your agreement that our total liability in connection with this work, however arising, shall be limited to the amount paid by you for this work, and that our employees or agents shall not under any circumstances be liable to you in connection with this work.

Order #: 2026368

Report Date: 06-Jul-2020 Order Date: 24-Jun-2020 Project Description: LOP20-002B

GPARA	<u> </u>	Parac	el II): 2(0263	68	ent Blvd. (1G 4J8	Pa	racel C	order N Use Or	lumbe nly)	er R 11		Ch	ain C (Lab l	If Cus	tody ()
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Address:				PO #:	LC	P20-002	ß] 0	1 day			□ 3 day
30 Lonsfield Way	Offacua	,00		E-mail	1:									2 day			Regular
Telephone: 613-327-90	073			12	Luk	e@Lopes	s.Ca						Date	Requir	ed:		
Regulation 153/04	OtherR	Regulation		Aatrix 1	fvpe:	S (Soil/Sed.) GW (G	Ground Water)					Re	ouiro	d Anab	cle		
Table 1 Res/Park Med/Fine	REG SS8	PWQ0]	5W (Su	rface V	Vater) SS (Storm/Sa	anitary Sewer)			<u>.</u>			d d	a Parkany	313		
Table 2 Ind/Comm Coarse Coarse	CCME	🗆 MISA			P (P	aint) A (Air) O (Ot	her)	_		S.		36					
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RELIABLE.

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Certificate of Analysis

Lopers & Associates

30 Lansfield Way Ottawa, ON K2G 3V8 Attn: Luke Lopers

Client PO: LOP20-002B Project: LOP20-002B Custody: 126519

Report Date: 9-Jul-2020 Order Date: 30-Jun-2020

Order #: 2027199

This Certificate of Analysis contains analytical data applicable to the following samples as submitted:

Paracel ID	Client ID
2027199-01	BH1-20
2027199-02	BH3-20
2027199-03	BH5-20
2027199-04	BH13-20
2027199-05	MW-6
2027199-06	MW-8
2027199-07	Trip Blank

Approved By:

Dale Robertson, BSc Laboratory Director

Any use of these results implies your agreement that our total liability in connection with this work, however arising, shall be limited to the amount paid by you for this work, and that our employees or agents shall not under any circumstances be liable to you in connection with this work.



Analysis Summary Table

Report Date: 09-Jul-2020 Order Date: 30-Jun-2020

Project Description: LOP20-002B

Analysis	Method Reference/Description	Extraction Date	Analysis Date
Anions	EPA 300.1 - IC	2-Jul-20	2-Jul-20
BTEX by P&T GC-MS	EPA 624 - P&T GC-MS	2-Jul-20	2-Jul-20
Chromium, hexavalent - water	MOE E3056 - colourimetric	6-Jul-20	6-Jul-20
Cyanide, free	MOE E3015 - Auto Colour	6-Jul-20	6-Jul-20
Mercury by CVAA	EPA 245.2 - Cold Vapour AA	3-Jul-20	3-Jul-20
Metals, ICP-MS	EPA 200.8 - ICP-MS	30-Jun-20	30-Jun-20
рН	EPA 150.1 - pH probe @25 °C	6-Jul-20	6-Jul-20
PHC F1	CWS Tier 1 - P&T GC-FID	30-Jun-20	2-Jul-20
PHCs F2 to F4	CWS Tier 1 - GC-FID, extraction	7-Jul-20	7-Jul-20
REG 153: PAHs by GC-MS	EPA 625 - GC-MS, extraction	7-Jul-20	7-Jul-20
REG 153: VOCs by P&T GC/MS	EPA 624 - P&T GC-MS	30-Jun-20	2-Jul-20



Certificate of Analysis Client: Lopers & Associates

Client PO: LOP20-002B

Order #: 2027199

Report Date: 09-Jul-2020

Order Date: 30-Jun-2020

Project Description: LOP20-002B

	Client ID: Sample Date: Sample ID: MDL/Units	BH1-20 30-Jun-20 09:00 2027199-01 Water	BH3-20 30-Jun-20 09:00 2027199-02 Water	BH5-20 30-Jun-20 09:00 2027199-03 Water	BH13-20 30-Jun-20 09:00 2027199-04 Water
General Inorganics			•	ł	
Cyanide, free	2 ug/L	<2	<2	-	<2
рН	0.1 pH Units	7.0	7.2	-	7.2
Anions					
Chloride	1 mg/L	918	833	-	828
Metals					
Mercury	0.1 ug/L	<0.1	<0.1	-	<0.1
Antimony	0.5 ug/L	<0.5	<0.5	-	<0.5
Arsenic	1 ug/L	1	1	-	1
Barium	1 ug/L	874	1880	-	1880
Beryllium	0.5 ug/L	<0.5	<0.5	-	<0.5
Boron	10 ug/L	27	25	-	26
Cadmium	0.1 ug/L	<0.1	<0.1	-	<0.1
Chromium	1 ug/L	<1	<1	-	<1
Chromium (VI)	10 ug/L	<10	<10	-	<10
Cobalt	0.5 ug/L	11.3	6.5	-	6.8
Copper	0.5 ug/L	1.2	1.0	-	1.2
Lead	0.1 ug/L	<0.1	51.2	-	54.6
Molybdenum	0.5 ug/L	1.2	1.4	-	1.4
Nickel	1 ug/L	23	12	-	13
Selenium	1 ug/L	<1	3	-	3
Silver	0.1 ug/L	<0.1	<0.1	-	<0.1
Sodium	200 ug/L	174000	202000	-	209000
Thallium	0.1 ug/L	<0.1	<0.1	-	<0.1
Uranium	0.1 ug/L	9.2	11.2	-	11.2
Vanadium	0.5 ug/L	1.2	1.1	-	1.2
Zinc	5 ug/L	<5	7	-	<5
Volatiles					
Acetone	5.0 ug/L	<5.0	<2500 [1]	-	<2500 [1]
Benzene	0.5 ug/L	<0.5	19300 [1]	-	19700 [1]
Bromodichloromethane	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
Bromoform	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
Bromomethane	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
Carbon Tetrachloride	0.2 ug/L	<0.2	<100 [1]	-	<100 [1]
Chlorobenzene	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
Chloroform	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]



Certificate of Analysis Client: Lopers & Associates

Client PO: LOP20-002B

Order #: 2027199

Report Date: 09-Jul-2020

Order Date: 30-Jun-2020

Project Description: LOP20-002B

	Client ID: Sample Date: Sample ID:	BH1-20 30-Jun-20 09:00 2027199-01	BH3-20 30-Jun-20 09:00 2027199-02	BH5-20 30-Jun-20 09:00 2027199-03	BH13-20 30-Jun-20 09:00 2027199-04
Dibromochloromethane	0.5 µg/l			vvaler	
Dichlorodifluoromethane	1.0 µg/l	<0.5	<230 [1]	-	<230 [1]
	0.5 µg/l	<1.0	<500 [1]	-	<500 [1]
	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
	0.5 µg/l	<0.5	<250 [1]	-	<250 [1]
	0.5 µg/l	<0.5	<250 [1]	-	<250 [1]
	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
cis-1,2-Dichloroethylene	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
trans-1,2-Dichloroethylene	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
1,2-Dichloropropane	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
cis-1,3-Dichloropropylene	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
trans-1,3-Dichloropropylene	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
1,3-Dichloropropene, total	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
Ethylbenzene	0.5 ug/L	<0.5	3800 [1]	-	3700 [1]
Ethylene dibromide (dibromoethane, 1,2-)	0.2 ug/L	<0.2	<100 [1]	-	<100 [1]
Hexane	1.0 ug/L	<1.0	<500 [1]	-	<500 [1]
Methyl Ethyl Ketone (2-Butanone)	5.0 ug/L	<5.0	<2500 [1]	-	<2500 [1]
Methyl Isobutyl Ketone	5.0 ug/L	<5.0	<2500 [1]	-	<2500 [1]
Methyl tert-butyl ether	2.0 ug/L	<2.0	<1000 [1]	-	<1000 [1]
Methylene Chloride	5.0 ug/L	<5.0	<2500 [1]	-	<2500 [1]
Styrene	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
1,1,1,2-Tetrachloroethane	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
1,1,2,2-Tetrachloroethane	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
Tetrachloroethylene	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
Toluene	0.5 ug/L	<0.5	65200 [1]	-	60900 [1]
1,1,1-Trichloroethane	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
1,1,2-Trichloroethane	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
Trichloroethylene	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
Trichlorofluoromethane	1.0 ug/L	<1.0	<500 [1]	-	<500 [1]
Vinyl chloride	0.5 ug/L	<0.5	<250 [1]	-	<250 [1]
m,p-Xylenes	0.5 ug/L	<0.5	19200 [1]	-	18200 [1]
o-Xylene	0.5 ug/L	<0.5	8400 [1]	-	8320 [1]
Xylenes, total	0.5 ug/L	<0.5	27600 [1]	-	26600 [1]
4-Bromofluorobenzene	Surrogate	90.7%	118% [1]	-	119% [1]
Dibromofluoromethane	Surrogate	102%	91.9% [1]	-	93.1% [1]



Order #: 2027199

Report Date: 09-Jul-2020

Order Date: 30-Jun-2020

Project Description: LOP20-002B

	Client ID:	BH1-20	BH3-20	BH5-20	BH13-20
	Sample Date:	30-Jun-20 09:00	30-Jun-20 09:00	30-Jun-20 09:00	30-Jun-20 09:00
	Sample ID:	2027199-01	2027199-02	2027199-03	2027199-04
Toluene-d8	MDL/Units Surrogate	106%	108% [1]	-	106% [1]
Benzene	0.5 ug/L	-	-	<0.5	-
Ethvlbenzene	0.5 ug/L		_	<0.5	_
Toluene	0.5 ug/L	_	-	<0.5	_
m,p-Xylenes	0.5 ug/L	_	_	<0.5	_
o-Xylene	0.5 ug/L	-	-	<0.5	-
Xylenes, total	0.5 ug/L	_	-	<0.5	-
Toluene-d8	Surrogate	-	-	98.9%	-
Hydrocarbons	++		ł		
F1 PHCs (C6-C10)	25 ug/L	123	3600	<25	3790
F2 PHCs (C10-C16)	100 ug/L	<100	52400	<100	2260
F3 PHCs (C16-C34)	100 ug/L	<100	3940	<100	<100
F4 PHCs (C34-C50)	100 ug/L	<100	<1000 [1]	<100	<100
Semi-Volatiles					•
Acenaphthene	0.05 ug/L	0.12	0.29	<0.05	0.21
Acenaphthylene	0.05 ug/L	0.06	0.08	<0.05	<0.05
Anthracene	0.01 ug/L	<0.01	0.04	<0.01	0.03
Benzo [a] anthracene	0.01 ug/L	0.02	<0.01	<0.01	<0.01
Benzo [a] pyrene	0.01 ug/L	<0.01	<0.01	<0.01	<0.01
Benzo [b] fluoranthene	0.05 ug/L	<0.05	<0.05	<0.05	<0.05
Benzo [g,h,i] perylene	0.05 ug/L	<0.05	<0.05	<0.05	<0.05
Benzo [k] fluoranthene	0.05 ug/L	<0.05	<0.05	<0.05	<0.05
Chrysene	0.05 ug/L	<0.05	<0.05	<0.05	<0.05
Dibenzo [a,h] anthracene	0.05 ug/L	<0.05	<0.05	<0.05	<0.05
Fluoranthene	0.01 ug/L	0.21	0.04	0.19	0.01
Fluorene	0.05 ug/L	0.28	0.41	<0.05	0.27
Indeno [1,2,3-cd] pyrene	0.05 ug/L	<0.05	<0.05	<0.05	<0.05
1-Methylnaphthalene	0.05 ug/L	18.2	50.8	<0.05	37.8
2-Methylnaphthalene	0.05 ug/L	10.4	87.3	<0.05	79.7
Methylnaphthalene (1&2)	0.10 ug/L	28.6	138	<0.10	118
Naphthalene	0.05 ug/L	11.3	419	0.08	392
Phenanthrene	0.05 ug/L	0.69	0.34	0.43	0.19
Pyrene	0.01 ug/L	0.40	0.07	0.37	0.03
2-Fluorobiphenyl	Surrogate	90.9%	75.2%	90.3%	88.0%
Terphenyl-d14	Surrogate	98.8%	114%	95.5%	103%



Order #: 2027199

Report Date: 09-Jul-2020

Order Date: 30-Jun-2020

Project Description: LOP20-002B

Sample Date	30-Jun-20 09:00	30-Jun-20 09:00	29-Jun-20 09:00	-
Sample ID:	2027199-05	2027199-06	2027199-07	-
MDL/Units	Water	Water	Water	-
5.0		1		
5.0 ug/L	-	-	<5.0	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
0.2 ug/L	-	-	<0.2	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
1.0 ug/L	-	-	<1.0	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
0.2 ug/L	-	-	<0.2	-
1.0 ug/L	-	-	<1.0	-
5.0 ug/L	-	-	<5.0	-
5.0 ug/L	-	-	<5.0	-
2.0 ug/L	-	-	<2.0	-
5.0 ug/L	-	-	<5.0	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
0.5 ug/L	-	-	<0.5	-
	Sample Date: Sample ID: MDL/Units 5.0 ug/L 0.5 ug/L <t< td=""><td>Sample Date 30-Jun-20 09:00 2027199-05 Water MDL/Units Water 5.0 ug/L </td><td>Sample Date: 30-Jun-20 09:00 2027199-05 MDL/Units Water 2027199-05 Water Water Water 5.0 ug/L - - 0.5 ug/L - - <t< td=""><td>Sample Date: 30-Jun-20 09:00 2027199-06 Water 204-Jun-20 09:00 2027199-06 Water 204-Jun-20 09:00 2027199-06 Water 5.0 ug/L - - <5.0</td> 0.5 ug/L - <0.5</t<></td> 0.5 ug/L - <0.5</t<>	Sample Date 30-Jun-20 09:00 2027199-05 Water MDL/Units Water 5.0 ug/L	Sample Date: 30-Jun-20 09:00 2027199-05 MDL/Units Water 2027199-05 Water Water Water 5.0 ug/L - - 0.5 ug/L - - <t< td=""><td>Sample Date: 30-Jun-20 09:00 2027199-06 Water 204-Jun-20 09:00 2027199-06 Water 204-Jun-20 09:00 2027199-06 Water 5.0 ug/L - - <5.0</td> 0.5 ug/L - <0.5</t<>	Sample Date: 30-Jun-20 09:00 2027199-06 Water 204-Jun-20 09:00 2027199-06 Water 204-Jun-20 09:00 2027199-06 Water 5.0 ug/L - - <5.0



Order #: 2027199

Report Date: 09-Jul-2020

Order Date: 30-Jun-2020

Project Description: LOP20-002B

	Client ID: Sample Date: Sample ID:	MW-6 30-Jun-20 09:00 2027199-05	MW-8 30-Jun-20 09:00 2027199-06	Trip Blank 29-Jun-20 09:00 2027199-07	- -
[MDL/Units	vvater	vvater	Water	-
1,1,1-Trichloroethane	0.5 ug/L	-	-	<0.5	-
1,1,2-Trichloroethane	0.5 ug/L	-	-	<0.5	-
Trichloroethylene	0.5 ug/L	-	-	<0.5	-
Trichlorofluoromethane	1.0 ug/L	-	-	<1.0	-
Vinyl chloride	0.5 ug/L	-	-	<0.5	-
m,p-Xylenes	0.5 ug/L	-	-	<0.5	-
o-Xylene	0.5 ug/L	-	-	<0.5	-
Xylenes, total	0.5 ug/L	-	-	<0.5	-
4-Bromofluorobenzene	Surrogate	-	-	116%	-
Dibromofluoromethane	Surrogate	-	-	99.4%	-
Toluene-d8	Surrogate	-	-	108%	-
Benzene	0.5 ug/L	<0.5	<0.5	-	-
Ethylbenzene	0.5 ug/L	<0.5	<0.5	-	-
Toluene	0.5 ug/L	<0.5	<0.5	-	-
m,p-Xylenes	0.5 ug/L	<0.5	<0.5	-	-
o-Xylene	0.5 ug/L	<0.5	<0.5	-	-
Xylenes, total	0.5 ug/L	<0.5	<0.5	-	-
Toluene-d8	Surrogate	95.0%	101%	-	-
Hydrocarbons					
F1 PHCs (C6-C10)	25 ug/L	<25	<25	-	-
F2 PHCs (C10-C16)	100 ug/L	<100	<100	-	-
F3 PHCs (C16-C34)	100 ug/L	<100	<100	-	-
F4 PHCs (C34-C50)	100 ug/L	<100	<100	-	-



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20-002B

Method Quality Control: Blank

Report Date: 09-Jul-2020

Order Date: 30-Jun-2020

Project Description: LOP20-002B

		Reporting		Source		%REC		RPD	
Analyte	Result	Limit	Units	Result	%REC	Limit	RPD	Limit	Notes
•									
Anions									
Chloride	ND	1	mg/L						
General Inorganics									
Cyanide, free	ND	2	ug/L						
Hydrocarbons									
F1 PHCs (C6-C10)	ND	25	ug/L						
F2 PHCs (C10-C16)	ND	100	ug/L						
F3 PHCs (C16-C34)	ND	100	ug/L						
F4 PHCs (C34-C50)	ND	100	ug/L						
Metals									
Mercury	ND	0.1	ug/L						
Antimony	ND	0.5	ug/L						
Arsenic	ND	1	ug/L						
Barium	ND	1	ug/L						
Beryllium	ND	0.5	ug/L						
Boron	ND	10	ug/L						
	ND	0.1	ug/L						
Chromium		10	ug/L						
Cobalt		0.5	ug/L						
Copper	ND	0.5	ug/L						
Lead	ND	0.1	ug/L						
Molvbdenum	ND	0.5	ug/L						
Nickel	ND	1	ug/L						
Selenium	ND	1	ug/L						
Silver	ND	0.1	ug/L						
Sodium	ND	200	ug/L						
Thallium	ND	0.1	ug/L						
Uranium	ND	0.1	ug/L						
Vanadium	ND	0.5	ug/L						
Zinc	ND	5	ug/L						
Semi-Volatiles									
Acenaphthene	ND	0.05	ug/L						
Acenaphthylene	ND	0.05	ug/L						
Anthracene	ND	0.01	ug/L						
Benzo [a] anthracene	ND	0.01	ug/L						
Benzo [a] pyrene	ND	0.01	ug/L						
Benzo [b] fluorantnene	ND	0.05	ug/L						
Benzo [g,n,i] perviene	ND	0.05	ug/L						
Chrisepe		0.05	ug/L						
Dibenzo [a b] anthracene		0.05	ug/L						
Fluoranthene	ND	0.00	ug/L						
Fluorene	ND	0.05	ug/L						
Indeno [1,2,3-cd] pyrene	ND	0.05	ug/L						
1-Methylnaphthalene	ND	0.05	ug/L						
2-Methylnaphthalene	ND	0.05	ug/L						
Methylnaphthalene (1&2)	ND	0.10	ug/L						
Naphthalene	ND	0.05	ug/L						
Phenanthrene	ND	0.05	ug/L						
Pyrene	ND	0.01	ug/L						
Surrogate: 2-Huorobiphenyl	19.0		ug/L		95.0	50-140			
Surrogate: Ierphenyl-d14	22.8		ug/L		114	50-140			
Volatiles									
Acetone	ND	5.0	ug/L						
Benzene	ND	0.5	ug/L						
Bromodichloromethane	ND	0.5	ug/L						
Bromoform	ND	0.5	ua/L						



Order #: 2027199

Report Date: 09-Jul-2020

Order Date: 30-Jun-2020

Project Description: LOP20-002B

Method Quality Control: Blank

Analyte	Result	Reporting Limit	Units	Source Result	%RFC	%REC	RPD	RPD Limit	Notes
	ND	0.5		rtoodit					
Corbon Totrophlorido		0.5	ug/L						
Chlorohonzono		0.2	ug/L						
Chloroform		0.5	ug/L						
Dibromochloromothono	ND	0.5	ug/L						
Diplomochioromethane	ND	0.5	ug/L						
	ND	1.0	ug/L						
1,2-Dichlorobenzene	ND	0.5	ug/L						
	ND	0.5	ug/L						
	ND	0.5	ug/L						
1, 1-Dichloroethane	ND	0.5	ug/L						
1,2-Dichloroethane	ND	0.5	ug/L						
	ND	0.5	ug/L						
cis-1,2-Dichloroethylene	ND	0.5	ug/L						
trans-1,2-Dichloroethylene	ND	0.5	ug/L						
1,2-Dichloropropane	ND	0.5	ug/L						
cis-1,3-Dichloropropylene	ND	0.5	ug/L						
trans-1,3-Dichloropropylene	ND	0.5	ug/L						
1,3-Dichloropropene, total	ND	0.5	ug/L						
Ethylbenzene	ND	0.5	ug/L						
Ethylene dibromide (dibromoethane, 1,2	ND	0.2	ug/L						
Hexane	ND	1.0	ug/L						
Methyl Ethyl Ketone (2-Butanone)	ND	5.0	ug/L						
Methyl Isobutyl Ketone	ND	5.0	ug/L						
Methyl tert-butyl ether	ND	2.0	ug/L						
Methylene Chloride	ND	5.0	ug/L						
Styrene	ND	0.5	ug/L						
1,1,1,2-Tetrachloroethane	ND	0.5	ug/L						
1,1,2,2-Tetrachloroethane	ND	0.5	ug/L						
Tetrachloroethylene	ND	0.5	ug/L						
Toluene	ND	0.5	ug/L						
1,1,1-Trichloroethane	ND	0.5	ug/L						
1,1,2-Trichloroethane	ND	0.5	ug/L						
Trichloroethylene	ND	0.5	ug/L						
Trichlorofluoromethane	ND	1.0	ug/L						
Vinyl chloride	ND	0.5	ug/L						
m,p-Xylenes	ND	0.5	ug/L						
o-Xylene	ND	0.5	ug/L						
Xylenes, total	ND	0.5	ug/L						
Surrogate: 4-Bromofluorobenzene	95.5		ug/L		119	50-140			
Surrogate: Dibromofluoromethane	90.2		ug/L		113	50-140			
Surrogate: Toluene-d8	79.1		ug/L		98.9	50-140			
Benzene	ND	0.5	ug/L						
Ethylbenzene	ND	0.5	ug/L						
Toluene	ND	0.5	ug/L						
m,p-Xylenes	ND	0.5	ug/L						
o-Xylene	ND	0.5	ug/L						
Xylenes, total	ND	0.5	ug/L						
Surrogate: Toluene-d8	79.1		ug/L		98.9	50-140			
			-						



Analyte

Method Quality Control: Duplicate

						Report Date: 09-Jul-2020						
						Order Date: 30-Jun-2020						
						Project Description: LOP20-002B						
Result	Reporting	Linita	Source	0/ DEC	%REC	חסס	RPD Limit	Notos				
Result	Linit	Units	Result	%REC	LITTIL	RED	LIITIIL	NOLES				
17.0	1	mg/L	17.0			0.3	10					

Anions							
Chloride	17.0	1	mg/L	17.0	0.3	10	
General Inorganics			U				
Cvanide, free	ND	2	ua/L	ND	NC	20	
pH	8.0	0.1	pH Units	8.0	0.1	3.3	
Hvdrocarbons							
		25	ug/l	ND	NC	30	
	ND	20	ug/L	ND	NC	30	
metals							
Mercury	ND	0.1	ug/L	ND	NC	20	
Antimony	ND	0.5	ug/L	ND	NC	20	
Arsenic	ND	1	ug/L	ND	NC	20	
Barium	47.3	1	ug/L	48.0	1.6	20	
Beryllium	ND	0.5	ug/L	ND	NC	20	
Bololi	14 I ND	10	ug/L	142 ND	0.0 NC	20	
Cadmium Chromium ()/I)		10	ug/L		NC	20	
Chromium		10	ug/L		NC	20	
Cobalt	ND	0.5	ug/L		NC	20	
Copper	0.82	0.5	ug/L	0.85	36	20	
Lead	ND	0.0	ug/L	ND	NC	20	
Molvbdenum	4.14	0.5	ug/L	4.19	1.2	20	
Nickel	3.0	1	ua/L	3.1	3.8	20	
Selenium	ND	1	ug/L	ND	NC	20	
Silver	ND	0.1	ug/L	ND	NC	20	
Sodium	32100	200	ug/L	32300	0.9	20	
Thallium	0.10	0.1	ug/L	0.11	2.3	20	
Uranium	3.5	0.1	ug/L	3.3	3.3	20	
Vanadium	0.64	0.5	ug/L	0.65	1.8	20	
Zinc	ND	5	ug/L	ND	NC	20	
Volatiles							
Acetone	ND	5.0	ug/L	ND	NC	30	
Benzene	ND	0.5	ug/L	ND	NC	30	
Bromodichloromethane	ND	0.5	ug/L	ND	NC	30	
Bromoform	ND	0.5	ug/L	ND	NC	30	
Bromomethane	ND	0.5	ug/L	ND	NC	30	
Carbon Tetrachloride	ND	0.2	ug/L	ND	NC	30	
Chlorobenzene	ND	0.5	ug/L	ND	NC	30	
Chloroform	ND	0.5	ug/L	ND	NC	30	
Dibromochloromethane	ND	0.5	ug/L	ND	NC	30	
	ND	1.0	ug/L	ND	NC	30	
1,2-Dichlorobenzene	ND	0.5	ug/L		NC	30	
		0.5	ug/L		NC	30	
1 1-Dichloroethane		0.5	ug/L		NC	30	
1 2-Dichloroethane	ND	0.5	ug/L	ND	NC	30	
1 1-Dichloroethylene	ND	0.5	ug/L	ND	NC	30	
cis-1 2-Dichloroethylene	ND	0.5	ug/L	ND	NC	30	
trans-1.2-Dichloroethylene	ND	0.5	ug/L	ND	NC	30	
1,2-Dichloropropane	ND	0.5	ug/L	ND	NC	30	
cis-1,3-Dichloropropylene	ND	0.5	ug/L	ND	NC	30	
trans-1,3-Dichloropropylene	ND	0.5	ug/L	ND	NC	30	
Ethylbenzene	ND	0.5	ug/L	ND	NC	30	
Ethylene dibromide (dibromoethane, 1,2-	ND	0.2	ug/L	ND	NC	30	
Hexane	ND	1.0	ug/L	ND	NC	30	
Methyl Ethyl Ketone (2-Butanone)	ND	5.0	ug/L	ND	NC	30	
Methyl Isobutyl Ketone	ND	5.0	ug/L	ND	NC	30	
Methyl tert-butyl ether	ND	2.0	ug/L	ND	NC	30	

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Order #: 2027199



Order #: 2027199

Report Date: 09-Jul-2020

Order Date: 30-Jun-2020

Project Description: LOP20-002B

Method Quality Control: Duplicate

Araba	_	Reporting		Source		%REC		RPD	
Analyte	Result	Limit	Units	Result	%REC	Limit	RPD	Limit	Notes
Methylene Chloride	ND	5.0	ug/L	ND			NC	30	
Styrene	ND	0.5	ug/L	ND			NC	30	
1,1,1,2-Tetrachloroethane	ND	0.5	ug/L	ND			NC	30	
1,1,2,2-Tetrachloroethane	ND	0.5	ug/L	ND			NC	30	
Tetrachloroethylene	ND	0.5	ug/L	ND			NC	30	
Toluene	ND	0.5	ug/L	ND			NC	30	
1,1,1-Trichloroethane	ND	0.5	ug/L	ND			NC	30	
1,1,2-Trichloroethane	ND	0.5	ug/L	ND			NC	30	
Trichloroethylene	ND	0.5	ug/L	ND			NC	30	
Trichlorofluoromethane	ND	1.0	ug/L	ND			NC	30	
Vinyl chloride	ND	0.5	ug/L	ND			NC	30	
m,p-Xylenes	ND	0.5	ug/L	ND			NC	30	
o-Xylene	ND	0.5	ug/L	ND			NC	30	
Surrogate: 4-Bromofluorobenzene	94.1		ug/L		118	50-140			
Surrogate: Dibromofluoromethane	83.4		ug/L		104	50-140			
Surrogate: Toluene-d8	71.4		ug/L		89.3	50-140			
Benzene	ND	0.5	ug/L	ND			NC	30	
Ethylbenzene	ND	0.5	ug/L	ND			NC	30	
Toluene	ND	0.5	ug/L	ND			NC	30	
m,p-Xylenes	ND	0.5	ug/L	ND			NC	30	
o-Xylene	ND	0.5	ug/L	ND			NC	30	
Surrogate: Toluene-d8	71.4		ug/L		89.3	50-140			



2-Methylnaphthalene

Naphthalene

Method Quality Control: Spike

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Anions									
Chloride	26.5	1	mg/L	17.0	95.0	77-123			
General Inorganics									
Cvanide free	25.2	2	ua/l	ND	83.9	70-130			
Hydrocarbons	20.2	-	~g, _		0010	10100			
	1000	25			00.7	60 117			
F1 PHCs (C6-C10)	1990	25	ug/L	ND	99.7	00-117			
F2 PHCs (C10-C10)	1070	100	ug/L		105	60 140			
F3 PHCs (C10-C34)	4000	100	ug/L		100	60 140			
F4 PRCs (C34-C30)	2090	100	ug/L	ND	109	00-140			
	0.40	0.4				70.400			
Mercury	3.48	0.1	ug/L	ND	116	70-130			
Antimony	49.5	0.5	ug/L	ND	98.3	80-120			
Arsenic	56.4	1	ug/L	ND	112	80-120			
Barium	102	1	ug/L	48.0	108	80-120			
Beryllium	54.7	0.5	ug/L	ND	109	80-120			
Boron	49	10	ug/L	ND	97.2	80-120			
	53.0	0.1	ug/L	ND	106	80-120			
Chromium (VI)	191	10	ug/L	ND	95.5	70-130			
Chromium	59.4	1	ug/L	ND	118	80-120			
Cobalt	53.0	0.5	ug/L	ND	105	80-120			
Copper	52.9	0.5	ug/L	0.85	104	80-120			
Lead	45.7	0.1	ug/L	ND	91.4	80-120			
Molybdenum	54.4	0.5	ug/L	4.19	100	80-120			
Nickel	56.4	1	ug/L	3.1	107	80-120			
Selenium	51.2	1	ug/L	ND	102	80-120			
Silver	50.9	0.1	ug/L	ND	102	80-120			
Sodium	40400	200	ug/L	32300	81.0	80-120			
Inallium	50.8	0.1	ug/L	0.11	101	80-120			
Uranium	44.6	0.1	ug/L	3.3	82.4	80-120			
	60.0	0.5	ug/L	0.65	119	80-120			
	53	5	ug/L	ND	99.6	80-120			
Semi-volatiles									
Acenaphthene	4.21	0.05	ug/L	ND	84.3	50-140			
Acenaphthylene	3.32	0.05	ug/L	ND	66.3	50-140			
Anthracene	4.02	0.01	ug/L	ND	80.5	50-140			
Benzo [a] anthracene	4.51	0.01	ug/L	ND	90.2	50-140			
Benzo [a] pyrene	5.22	0.01	ug/L	ND	104	50-140			
Benzo [b] fluoranthene	5.79	0.05	ug/L	ND	116	50-140			
Benzo [g,h,i] perylene	5.11	0.05	ug/L	ND	102	50-140			
Benzo [k] fluoranthene	5.43	0.05	ug/L	ND	109	50-140			
Chrysene	5.22	0.05	ug/L	ND	104	50-140			
Dibenzo [a,h] anthracene	5.17	0.05	ug/L	ND	103	50-140			
- Huoranthene	4.22	0.01	ug/L	ND	84.3	50-140			
Fluorene	4.00	0.05	ug/L	ND	79.9	50-140			
Indeno [1,2,3-cd] pyrene	5.15	0.05	ug/L	ND	103	50-140			
1-Methylnaphthalene	5.35	0.05	ug/L	ND	107	50-140			

Report Date: 09-Jul-2020

Order Date: 30-Jun-2020

Project Description: LOP20-002B

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ug/L

ug/L

ND

ND

119

106

50-140

50-140

5.93

5.29

0.05

0.05



Method Quality Control: Spike

Report Date: 09-Jul-2020

Order Date: 30-Jun-2020

Project Description: LOP20-002B

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Phenanthrene	4.26	0.05	ug/L	ND	85.2	50-140			
Pyrene	4.27	0.01	ug/L	ND	85.4	50-140			
Surrogate: 2-Fluorobiphenyl	20.3		ug/L		101	50-140			
Surrogate: Terphenyl-d14	20.9		ug/L		105	50-140			
Volatiles									
Acetone	126	5.0	ug/L	ND	126	50-140			
Benzene	44.5	0.5	ug/L	ND	111	60-130			
Bromodichloromethane	46.6	0.5	ug/L	ND	117	60-130			
Bromoform	46.9	0.5	ug/L	ND	117	60-130			
Bromomethane	47.5	0.5	ug/L	ND	119	50-140			
Carbon Tetrachloride	48.1	0.2	ug/L	ND	120	60-130			
Chlorobenzene	44.4	0.5	ug/L	ND	111	60-130			
Chloroform	47.5	0.5	ug/L	ND	119	60-130			
Dibromochloromethane	42.2	0.5	ug/L	ND	106	60-130			
Dichlorodifluoromethane	46.7	1.0	ug/L	ND	117	50-140			
1,2-Dichlorobenzene	49.8	0.5	ug/L	ND	124	60-130			
1,3-Dichlorobenzene	40.1	0.5	ug/L	ND	100	60-130			
1,4-Dichlorobenzene	43.7	0.5	ug/L	ND	109	60-130			
1,1-Dichloroethane	45.8	0.5	ug/L	ND	115	60-130			
1,2-Dichloroethane	44.2	0.5	ug/L	ND	111	60-130			
1,1-Dichloroethylene	37.0	0.5	ug/L	ND	92.5	60-130			
cis-1,2-Dichloroethylene	49.2	0.5	ug/L	ND	123	60-130			
trans-1,2-Dichloroethylene	34.8	0.5	ug/L	ND	87.1	60-130			
1,2-Dichloropropane	39.2	0.5	ug/L	ND	97.9	60-130			
cis-1,3-Dichloropropylene	42.3	0.5	ug/L	ND	106	60-130			
trans-1,3-Dichloropropylene	46.6	0.5	ug/L	ND	116	60-130			
Ethylbenzene	43.9	0.5	ug/L	ND	110	60-130			
Ethylene dibromide (dibromoethane, 1,2	40.5	0.2	ug/L	ND	101	60-130			
Hexane	44.0	1.0	ug/L	ND	110	60-130			
Methyl Ethyl Ketone (2-Butanone)	126	5.0	ug/L	ND	126	50-140			
Methyl Isobutyl Ketone	104	5.0	ug/L	ND	104	50-140			
Methyl tert-butyl ether	114	2.0	ug/L	ND	114	50-140			
Methylene Chloride	43.8	5.0	ug/L	ND	110	60-130			
Styrene	43.5	0.5	ug/L	ND	109	60-130			
1,1,1,2-Tetrachloroethane	44.5	0.5	ug/L	ND	111	60-130			
1,1,2,2-Tetrachloroethane	43.3	0.5	ug/L	ND	108	60-130			
Tetrachloroethylene	34.7	0.5	ug/L	ND	86.7	60-130			
Toluene	45.9	0.5	ug/L	ND	115	60-130			
1,1,1-Trichloroethane	41.8	0.5	ug/L	ND	105	60-130			
1,1,2-Trichloroethane	46.7	0.5	ug/L	ND	117	60-130			
Trichloroethylene	42.5	0.5	ug/L	ND	106	60-130			
Trichlorofluoromethane	43.6	1.0	ug/L	ND	109	60-130			
Vinyl chloride	49.0	0.5	ug/L	ND	122	50-140			
m,p-Xylenes	79.2	0.5	ug/L	ND	98.9	60-130			
o-Xylene	44.0	0.5	ug/L	ND	110	60-130			
Surrogate: 4-Bromofluorobenzene	86.1		ug/L		108	50-140			
Surrogate: Dibromofluoromethane	85.9		ug/L		107	50-140			
Surrogate: Toluene-d8	69.3		ug/L		86.6	50-140			
Benzene	44.5	0.5	ug/L	ND	111	60-130			
Ethylbenzene	43.9	0.5	ug/L	ND	110	60-130			



Report Date: 09-Jul-2020

Order Date: 30-Jun-2020

Project Description: LOP20-002B

Method Quality Control: Spike

Analyte	Result	Reporting Limit	Units	Source Result	%REC	%REC Limit	RPD	RPD Limit	Notes
Toluene	45.9	0.5	ug/L	ND	115	60-130			
m,p-Xylenes	79.2	0.5	ug/L	ND	98.9	60-130			
o-Xylene	44.0	0.5	ug/L	ND	110	60-130			
Surrogate: Toluene-d8	69.3		ug/L		86.6	50-140			



Sample Qualifiers :

1: Elevated detection limit due to dilution required because of high target analyte concentration.

Sample Data Revisions

None

Work Order Revisions / Comments:

None

Other Report Notes:

n/a: not applicable ND: Not Detected MDL: Method Detection Limit Source Result: Data used as source for matrix and duplicate samples %REC: Percent recovery. RPD: Relative percent difference. NC: Not Calculated

CCME PHC additional information:

- The method for the analysis of PHCs complies with the Reference Method for the CWS PHC and is validated for use in the laboratory. All prescribed quality criteria identified in the method has been met.

- F1 range corrected for BTEX.

- F2 to F3 ranges corrected for appropriate PAHs where available.

- The gravimetric heavy hydrocarbons (F4G) are not to be added to C6 to C50 hydrocarbons.

- In the case where F4 and F4G are both reported, the greater of the two results is to be used for comparison to CWS PHC criteria.

- When reported, data for F4G has been processed using a silica gel cleanup.

OTTAWA • MISSISSAUGA • HAMILTON • CALGARY • KINGSTON • LONDON • NIAGARA • WINDSOR • RICHMOND HILL

Report Date: 09-Jul-2020 Order Date: 30-Jun-2020

Project Description: LOP20-002B

GPARACEL			Para	acel ID: 2027199				Paracel Order Number (Lab Use Only)				per	Chain Of Custody (Lab Use Only)				dy	
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Contact Name: Luke Lopers				Quote	#:								+		Turna	around	Time	
Address:	ottawa 0	N		PO #:	Lo	P20-0028	5] 1 day				3 day
30 Lansfield Way				E-mail	:	1								l 2 day			A	Regular
Telephone: 613 - 327 - 907 3				Luke@Lopers.ca									Date Required:					
Regulation 153/04 Other Regulation					ype:	S (Soil/Sed.) GW (Ground Water)		100.00									
□ Table 1 🖾 Res/Park □ Med/Fine □ REG 558 □ PWQO					rface V	Vater) SS (Storm/S	anitary Sewer)						Required Analysis					
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LOPERS & ASSOCIATES

Appendix F

Qualifications of Assessors



PROFILE

Mr. Lopers is an environmental engineer with over 12 years of experience in environmental engineering specializing in due diligence investigations. Mr. Lopers has extensive experience in Phase I and II Environmental Site Assessments; environmental remediation, and investigations; record of site condition submissions; asset inventory, designated substance surveys and abatement projects; environmental expertise on legal issues; and coordination of various monitoring programs (groundwater, surface water, air).

Mr. Lopers has participated in various Property Condition and Building Envelope mandates at various residential and commercial properties throughout Ontario.

Mr. Lopers has a strong commitment to health and safety, having experience leading a regional health and safety committee as a certified employee representative. Mr. Lopers has extensive training including OSHA 40-hour HAZWOPER, ASP Health and Safety on Construction Sites in Quebec, Ontario Working at Heights, Emergency First Aid/CPR and WHMIS.

CONTACT

EMAIL: Luke@Lopers.ca

LUKE LOPERS Principal LOPERS & ASSOCIATES

EDUCATION

University of Waterloo, B.A.Sc., Honours Environmental Engineering Management Science Option Designation - 2002 - 2008

PROFESSIONAL EXPERIENCE

Lopers & Associates, Principal, Project Manager, Senior Environmental Engineer

Ottawa, Ontario - 2020–Present

Responsible for the management, coordination, supervision, completion and delivery of Phase I/1 and II/2 Environmental Site Assessments, Environmental Remediation Programs, Environmental litigation support, Designated Substance Surveys, scope of work development, cost estimates and proposals

GHD Limited, Project Manager, Senior Environmental Engineer Ottawa, Ontario - 2013–2020

Responsible for the management, senior technical review, coordination, supervision, completion and delivery of Phase I/1 and II/2 Environmental Site Assessments, Environmental Remediation Programs, Environmental litigation support, Designated Substance Surveys, scope of work development, cost estimates and proposals Office Safety Captain and Joint Health and Safety Committee team leader

Paterson Group Inc., Project Manager, Environmental Engineer Ottawa, Ontario - 2009–2013

Responsible for supervision, completion and review for Phase I/1 and II/2 Environmental Site Assessments, Environmental Remediation Programs, Designated Substance Surveys

NEXT Environmental Inc., Site Investigation Staff

Burnaby, British Columbia - 2008–2009 Responsible for fieldwork and reporting for Stage/Phase I and II Environmental Site Assessments, Environmental Remediation Programs

PROFESSIONAL DESIGNATIONS

Licensed Professional Engineer (P.Eng.) with Professional Engineers Ontario (PEO) since 2012

Qualified Person (QP), Environmental Site Assessments with Ontario Ministry of the Environment, Conservation and Parks

PROJECT EXPERIENCE

Environmental Site Assessments

Project Engineer/Manager Phase 1 Environmental Site Assessment | Various Clients | Ontario, Quebec and British Columbia | 2006-2020

Project Engineer/Manager Phase Two Environmental Site Assessments | Various Clients | Various Locations | 2008-2020

Project Manager Phase One, Phase Two Environmental Site Assessments, Environmental Delineation Quality Assurance Program | Costco Wholesale | Ottawa, ON | 2014-2019

Environmental Remediation Programs

Project Engineer Underground Fuel Storage Tank Removals and Environmental Remediation Programs in Vicinity of Active Underground Services | Ottawa, ON | 2010, 2012 Project Engineer/Manager for Phase I Environmental Site Assessments in support of acquisition/divestiture/regulatory requirements for various properties in Ontario, Quebec and British Columbia, including the following:

- Canadian Tire Retail Store and Gas Bar, CTR 417 2560 Princess Street, Kingston, Ontario
- Former Automotive Dealership and Service Garage, North Vancouver, British Columbia
- Former Philips Cable Plant, Brockville, Ontario
- Former Cornwall Cotton Mill, Cornwall, Ontario
- Retail Fuel Outlet and Automotive Service Garage, Ottawa, Ontario
- Jack Garland Airport Land, North Bay, Ontario
- Various Commercial/Residential Properties, Ontario and British Columbia
- Various Residential Properties, Ontario, Quebec and British Columbia
- Rochester Heights (811, 818 Gladstone Avenue), Ottawa, Ontario

Project Engineer/Manager for the following field investigation and/or regulatory reporting requirements for Phase II ESAs and other Site Investigations:

- Proposed Canadian Tire Development, CTR 693P Terry Fox Drive at Eagleson Road, Stittsville, Ontario
- Former Retail/Private Fuel Outlets, Ottawa/North Bay/Vancouver, Canada
- Operational/Former Industrial Facilities, Ottawa/Cornwall/Sarnia/Brockville/Gananoque, Ontario
- Existing Dry Cleaning Facilities, Ottawa/Arnprior, Ontario
 - Automotive Service Garages, Ottawa/Vancouver, Canada
- Various Commercial/Residential Properties, Eastern Ontario
- Tetrachloroethylene Groundwater Plume, Commercial Property, Ottawa, Ontario
- Rochester Heights (811, 818 Gladstone Avenue), Ottawa, Ontario

Project Manager for the completion of a Phase One ESA for the potential acquisition of a commercial property. Upon discovery of APECs at the Site and significant data gaps in previous investigations, completed a Phase Two ESA to evaluate soil and groundwater quality at the Site. Further oversight of original owner's environmental consultants was completed to ensure adequate delineation and characterization of a dNAPL groundwater plume at the Site, present at significant depths in shale bedrock, which originated as a result of a former on-Site dry-cleaning operation.

Project Engineer for removal of underground heating oil storage tanks adjacent to residential buildings. Completed excavation supervision of contaminated soil around and below active underground services, including hydro, water and natural gas infrastructure at residential properties. Activities included oversight of removal of petroleum, impacted soil, and field screening and collection of confirmatory soil and groundwater samples for petroleum hydrocarbon analysis. Prepared Phase I, II and III Environmental Site Assessment reports. Project Engineer Retail Fuel Outlet Decommissioning and Remediation | Ottawa, ON | 2012

Project Engineer/Manager Former Fuel Outlet Investigation and Remediation | Merrickville, ON | 2016-2017

Record of Site Conditions

Project Manager/Engineer Residential Redevelopment | Environmental Remediation Program and Record of Site Condition Submission | Ottawa | 2015

Project Manager/Engineer Industrial Development | Environmental Assessment and Record of Site Condition Submission | Township of Edwardsburgh/Cardinal | 2015

Excess Soil Management

Project Engineer/Manager Management of Excess Soil | CTREL, Brigil, Ottawa Community Housing Corporation | Ottawa and Pembroke, Ontario | 2016, 2018

Designated Substance Surveys

Project Manager

Designated Substance Surveys and Hazardous Building Materials Assessment | Ottawa, Pembroke, Southeastern Ontario | 2010-2020

Environmental Litigation Support

Project Manager, Field Engineer, Expert Witness Ottawa, Ontario | 2014-2020 Project Engineer for UST removal and confirmatory soil sampling at former ESSO gas station in Ottawa, Ontario. Activities included oversight of removal of USTs and product lines, oversight of removal of petroleum-impacted soil and groundwater encountered and backfilling operations, and field screening and collection of confirmatory soil and groundwater samples for petroleum hydrocarbon analysis.

Project Engineer for confirmatory soil and groundwater sampling following UST removal at former Shell gas station. Activities included oversight of removal of petroleum-impacted soil, pumping of groundwater encountered and backfilling operations, and field screening and collection of confirmatory soil and groundwater samples for petroleum hydrocarbon analysis. Additional borehole/monitoring well drilling also completed.

Project Manager for delineation of soil contamination and groundwater sampling for a former automotive garage and gas station property in Ottawa, Ontario. Presented and implemented remedial action plan to remediate on-Site contamination. Directed staff in collection of post remediation confirmatory soil and groundwater samples for contaminants of concern. Prepared remediation closure report and record of site condition supporting documentation for submission to the Ministry of the Environment and Climate Change.

Project Manager for environmental assessments for a proposed industrial business park, in an existing industrial area within the Township of Edwardsburgh/Cardinal, Ontario. Prepared environmental assessment reports and record of site condition supporting documentation for submission to the Ministry of the Environment and Climate Change.

Project Engineer/Manager for sampling, analytical testing, development of soil management plans and monitoring during removal of excess soil generated as part of construction activities, including the following properties/facilities:

- Rochester Heights (811, 818 Gladstone Avenue), Ottawa, Ontario
- Residential redevelopment, 121 Parkdale Avenue, Ottawa, Ontario
- CTR 079, 1104 Pembroke Street East, Pembroke, Ontario
- CTR 297, 2010 Ogilvie Road, Ottawa, Ontario

Project Manager for asbestos containing material (ACM) surveys, designated substance surveys (DSSs), Hazardous Building Materials Assessments (HBMAs) or mould assessments at the following sites:

- DSSs at various municipal facilities for the City of Pembroke, Pembroke, Ontario. Preparation of Asbestos Management Plan.
- HBMAs at various institutional buildings for the Catholic District School Board of Eastern Ontario, Southeastern Ontario.
- DSSs and ACM surveys at various residential, buildings (dwellings and apartment buildings) for private residential clients, Ottawa, Ontario.
- DSS and abatement oversight during demolition, residential buildings (townhouses) for Ottawa Community Housing Corporation, 818 Gladstone Avenue, Ottawa, Ontario.

Project Manager, Field Engineer and Expert Witness for a fuel spill, remediation program, groundwater monitoring program and litigation review for redevelopment of a residential property adjacent to a central heating plant at an institutional facility.

Education

BEng Geological Engineering, École Polytechnique de Montreal, Montreal, Quebec, 1990

MSc Geophysics, University of British Columbia, Vancouver, British Columbia, 1983

BSc Geophysics, Honours, University of British Columbia, Vancouver, British Columbia, 1980

Certifications

Registered as PMP with Project Management Institute since 2012, requalified in 2018

Qualified Person (QP) for Environmental Site Assessments with Ontario Ministry of Environment and Conservation and Parks

Professional Affiliations

Licensed as P.Eng. with the Professional Engineers of Ontario (PEO) since 1994

Licensed as Ing. with l'Ordre des ingénieurs du Québec (OIQ), 1992

Licensed as P.Eng. with NAPEG (NWT and Nunavut), since 2009.

Licensed as P.Eng with Engineers Yukon since 2018

Federal Clearance Level

Secret ID # 95251065

DON PLENDERLEITH

Senior Environmental Engineer and Project Manager

PROFESSIONAL SUMMARY

Mr. Plenderleith has been an environmental engineer for 30 years. From 1990 to 2000 he worked at specialty firms in Montreal and Ottawa where he gained field and reporting experience in site assessment and remediation of retail fuel outlets and railway yards. In 1991 and 1992 he worked on a CIDA sponsored project to assess additional water resource potential in two provinces in Indonesia. He worked for Golder for 19 years on projects in Ottawa, the North and overseas.

His expertise covers all steps in contaminated site management: Phase I, II and III environmental site assessments (ESAs), risk assessments, remedial options evaluations, remedial action plans, tender plans and specifications, remediation project oversight, long-term monitoring and project closure. He has largely concentrated on federal sites since 2002 and was Golder's initial point of contact on the Environmental Standing Offer Agreement with PSPC in the National Capital over that time.

Don led Golder's national client service team for Federal government and was responsible to Golder's management for maintaining strong relations with the federal government. Locally, he provided project management and technical direction of a variety of environmental projects from the Ottawa office. Don mentored several junior professionals. His site portfolio included: military bases, Northern sites, navigational sites, correctional facilities, research labs, commercial buildings and Canadian embassies abroad. On several multi-year projects (Kingston Penitentiary and Connaught Ranges landfill) he directed all steps of site management from initial investigations, through to site closure.

Don is equally experienced at providing strategic and portfolio-level assistance to clients as well as site-specific level work. He has written contaminated sites management plans for several federal Departments. He helped to develop components of the FCSAP project manager's tool kit and has trained federal project managers in its use. He has provided program-level assistance to the FCSAP Secretariat for funding demand forecasting and long-term strategy and risk management. For nine years he led a multi-disciplinary team that performed contaminated site liability peer reviews for the Office of the Auditor General of Canada.

Don completed his engineering degree in French and is licensed to practice in Quebec. He frequently coordinates the French language component at bilingual meetings and workshops.

PROJECT EXPERIENCE – STANDING OFFER MANAGER

Public Services and Procurement Canada, National Capital Region, Environmental Engineering Standing Offer (2002-2019). Don managed Golder's Environmental Standing Offer Agreement (SOA) with PSPC in the National Capital Region from 2002 to 2019. He was the first point of contact with PSPC for new call-ups. He formed project teams from the approved resources and reviewed the work plans under each call-up. He was responsible and accountable for Golder's overall project performance to PSPC.

PROJECT EXPERIENCE – SENIOR PROJECT MANAGER

Environmental Site Assessment, Remediation Planning and Implementation for the Pittsburgh Institution and Kingston Penitentiary, Kingston, Ontario from 2007 Phase I, II, and III and to 2015 - Don was the Senior Project Manager and project reviewer for the **Remediation at Pittsburgh** Phase I, II and III of contaminated sites on two similar projects at these federal **Institution and Kingston** penitentiaries. Don performed project management and provided technical Penitentiary for PSPC/CSC direction during the full suite of services from site assessment through to near Kingston, Ontario remediation. Federal project management tools, and FCSAP technical tools (GOST) were used to assist with procedural compliance. Don assisted PSPC with the tender specification for both remediation projects and performed on-site supervision during the fast-track remediation work at Pittsburgh. Don also performed senior review of the draft and final reports.

Peer Review and Liability Review of US Steel Site in Hamilton Harbour for PSPC and Transport Canada (July-August 2016)

Contaminated Site Don h Reporting and Review for relate Department of National EcoN Defence Ottawa, Ontario, which

Canada

Don was the Senior Project Manager for a Peer Review of reports pertaining to the US Steel site on Hamilton Harbour that the Hamilton Port Authority (HPA) was considering purchasing. TC requested the peer review and liability review in its oversight role over the HPA. Don brought a senior expert in at steel industry at Golder onto the project team. With his input some important gaps in the previous site assessments, management plans and liability estimates were identified to TC.

Don has managed several projects for DND's Director General Environment, related to the financial reporting of DND's contaminated sites. He managed the EcoNet validation project in 2006, in which the systems and procedures by which site cost and liability information are input to DND's Contaminated Site database, Econet. Several of DND's major projects being run out of headquarters were reviewed in that exercise. In 2008 he assisted DND by producing the 2008 update of their Contaminated Sites Management Plan (CSMP) for Treasury Board submission. Nine divisional CSMPs were reviewed, summarized and incorporated into the departmental CSMP.

PROGRAM LEVEL WORK – FEDERAL CONTAMINATED SITES

Project Management Tools for Contaminated Sites, Ottawa, Ontario, Canada Mr. Plenderleith developed two of the FCSAP Project Management Tools: Status Reporting and Project Risk Management. He has provided training in the tools to federal project managers country-wide. He has delivered training sessions at RPIC National Contaminated Sites workshops on several occasions on the PM Tools, the Sustainable Development Tool (SDAT), and Guidance Tool for Selection of Technologies Tools (GOST).

Assistance to FCSAP for program-level Risk Management, PWGSC/ECCC Ottawa, Ontario Don has led a team at Golder that provided assistance to the FCSAP Secretariat from 2013 to 2019 in the areas of cost projections for funding demand estimates. He devised a method of projecting the costs of unassessed sites based on closure costs of similar sites. This tool was used to estimate the funding demand for FCSAP Phase III and past Phase III. Don assisted the Secretariat with Long-Term Strategic planning for FSCAP post 2020 when the 15-year program is due to sunset.

Secondments to Federal Departments Mr. Plenderleith has been seconded from Golder to the Department of Foreign Affairs and International Trade (now Global Affairs Canada "GAC") on three occasions to develop their Contaminated Sites Management Plans and to fill in while GAC was staffing their full-time environmental engineer position. Through these secondments he has developed a greater understanding of the role of federal custodians in managing their programs.

PROJECT EXPERIENCE – NORTHERN SITES

Mr. Plenderleith was the project director of Golder's DEW Line Monitoring **DEW Line Site Monitoring,** contract with DND from four years 2015 to 2019. He was responsible for overall **Baffin Region, DND** program quality and liaison with the client and management of Inuit (2015-19)subcontractors. The project was multi-disciplinary, involving geotechnical and environmental components. Mr. Plenderleith has developed a very positive working relationship with the hamlet of Qikiqtarjuag and the Inuit staff from that community, many of whom have returned to work with Golder every year. All Inuit Participation Targets were exceeded. **Tundra Mine Remediation** Don was the Senior project director for Golder's Remediation Monitoring of Monitoring PSPC/INAC Tundra Mine (NWT) for PSPC and INAC. This project is multi-disciplinary (2016 - 2018)

Tundra Mine (NWT) for PSPC and INAC. This project is multi-disciplinary involving surface water and groundwater environmental monitoring and aquatic monitoring for the final stages of the remediation of Tundra Mine. Don has reviewed the monthly and annual monitoring reports produced for the Water Licence. His earlier experience with the RAP for Tundra has been valuable on this project.

Remedial Options Review and Remedial Action Planning Former Water Tanker Base, Inuvik Airport, NWT 2010-12 From 2010 to 2012, Mr. Plenderleith was the technical director for the Phase III ESA detailed site assessment and remediation planning of the former Water Tanker Base at the Inuvik Airport in NWT. The work included determining the contaminants of concern, delineation of contaminated soil and seasonal groundwater areas, and assessing remedial options. The remedial action plan reviewed chemical oxidation and removal & disposal options within the constraints of northern work season, and the distance to a disposal facility. Descriptions, costs, advantages and limitations were provided for several options. GNWT performed the remediation with own forces.

SITE SERVICING AND STORMWATER MANAGEMENT BRIEF – MOONEY'S BAY - 729 RIDGEWOOD AVENUE, OTTAWA, ON

Appendix F City of Ottawa Servicing Study Checklist

Appendix F CITY OF OTTAWA SERVICING STUDY CHECKLIST



APPLICANT'S STUDY AND PLAN IDENTIFICATION LIST - Draft

Legend: **S** indicates that the study or plan is required with application submission.

A indicates that the study or plan may be required to satisfy a condition of approval/draft approval.

For information and guidance on preparing required studies and plans refer to:

http://ottawa.ca/en/development-application-review-process-0/guide-preparing-studies-and-plans

S/A	Number of copies	ENG	ENGINEERING									
S		1. Site Servicing Plan	 Assessment of Adequacy of Public Services / Site Servicing Study / Brief 	S								
S		3. Grade Control and Drainage Plan	4. Geotechnical Study / Slope Stability Study	S								
		5. Composite Utility Plan	6. Groundwater Impact Study									
		7. Servicing Options Report	8. Wellhead Protection Study									
S		 Community Transportation Study and / or Transportation Impact Study / Brief 	10. Erosion and Sediment Control Plan / Brief	S								
S		11.Storm water Management Report / Brief	12.Hydro geological and Terrain Analysis									
		13.Hydraulic Water main Analysis	14.Noise / Vibration Study	S								
		15.Roadway Modification Design Plan	16.Confederation Line Proximity Study									

S/A	Number of copies	PLANNING	/ DESIGN / SURVEY	S/A	Number of copies
		17.Draft Plan of Subdivision	18.Plan Showing Layout of Parking Garage	S	
		19.Draft Plan of Condominium	20.Planning Rationale	S	
S		21.Site Plan	22.Minimum Distance Separation (MDS)		
		23.Concept Plan Showing Proposed Land Uses and Landscaping	24.Agrology and Soil Capability Study		
		25.Concept Plan Showing Ultimate Use of Land	26.Cultural Heritage Impact Statement		
S		27.Landscape Plan	28.Archaeological Resource Assessment Requirements: S (site plan) A (subdivision, condo)		
S		29.Survey Plan	30.Shadow Analysis		
S		31.Architectural Building Elevation Drawings (dimensioned)	32.Design Brief (includes the Design Review Panel Submission Requirements)		
		33.Wind Analysis			

S/A	Number of copies	ENVIRONMENTAL		S/A	Number of copies
s		34.Phase 1 Environmental Site Assessment	35.Impact Assessment of Adjacent Waste Disposal/Former Landfill Site		
Α		36.Phase 2 Environmental Site Assessment (depends on the outcome of Phase 1)	37.Assessment of Landform Features		
Α		38.Record of Site Condition	39.Mineral Resource Impact Assessment		
S		40.Tree Conservation Report	41.Environmental Impact Statement / Impact Assessment of Endangered Species		
		42.Mine Hazard Study / Abandoned Pit or Quarry Study	43.Integrated Environmental Review (Draft, as part of Planning Rationale)		

Number of copies

*Reports require 3 copies; Plans require 3 copies + Digital versions of all submissions

Meeting Date: TBD

Application Type: TBD

File Lead (Assigned Planner): Kelby Lodoen Unseth

*Preliminary Assessment: 1 2 3 4 5 5

Site Address (Municipal Address): 729 Ridgewood Ave.

*One (1) indicates that considerable major revisions are required before a planning application is submitted, while five (5) suggests that proposal appears to meet the City's key land use policies and guidelines. This assessment is purely advisory and does not consider technical aspects of the proposal or in any way guarantee application approval.

It is important to note that the need for additional studies and plans may result during application review. If following the submission of your application, it is determined that material that is not identified in this checklist is required to achieve complete application status, in accordance with the Planning Act and Official Plan requirements, the Planning, Infrastructure and Economic Development Department will notify you of outstanding material required within the required 30 day period. Mandatory pre-application consultation will not shorten the City's standard processing timelines, or guarantee that an application will be approved. It is intended to help educate and inform the applicant about submission requirements as well as municipal processes, policies, and key issues in advance of submitting a formal development application. This list is valid for one year following the meeting date. If the application is not submitted within this timeframe the applicant must again pre-consult with the Planning, Infrastructure and Economic Development Department.

 110 Laurier Avenue West, Ottawa ON K1P 1J1
 Mail code: 01-14
 Visit us: Ottawa.ca/planning

 110, av. Laurier Ouest, Ottawa (Ontario) K1P 1J1
 Courrier interne : 01-14
 Visitez-nous : Ottawa.ca/urbanisme

SITE SERVICING AND STORMWATER MANAGEMENT BRIEF – MOONEY'S BAY - 729 RIDGEWOOD AVENUE, OTTAWA, ON

Appendix G Background Correspondence

Appendix G BACKGROUND CORRESPONDENCE





Planning, Infrastructure and Economic Development Department Services de la planification, de l'infrastructure et du développement économique

Site Plan Pre- Application Consultation Notes

Date: Wednesday , May 6, 2020.
Site Location: 729 Ridgewood Avenue
Type of Development: ⊠ Residential (□ townhomes, □ stacked, □ singles,
□ apartments), □ Office Space, ⊠ Commercial, ⊠ Retail, □ Institutional,
□ Industrial, Other: N/A
Project Manager: Sharif Golam
Assigned Planner: Kelby Lodoen Unseth

Infrastructure

Water

Water District Plan No: 368-025 Existing public services:

• Ridgewood Avenue – 305mm Cl



Watermain Frontage Fees to be paid (\$190.00 per metre)
Ves
No

- Existing on-site water service must be shown on the plans. The existing on-site water services will be blanked at the watermain if it will not be reused.
- Service areas with a basic demand greater than 50 m³/day shall be connected with a minimum of two water services, separated by an isolation valve, to avoid creation of vulnerable service area.
- A water meter sizing questionnaire [water card] will have to be completed prior to receiving a water permit (water card will be provided post approval)

Boundary conditions:

Civil consultant must request boundary conditions from the City's assigned Project Manager prior to first submission.

- Water boundary condition requests must include the location of the service(s) and the expected loads required by the proposed developments. Please provide all the following information:
 - Location of service(s)
 - Type of development and the amount of fire flow required (as per FUS, 1999).
 - Average daily demand: ____ l/s.
 - Maximum daily demand: ____l/s.
 - Maximum hourly daily demand: ____ l/s.
- Fire protection (Fire demand, Hydrant Locations)
- A water meter sizing questionnaire [water card] will have to be completed prior to receiving a water permit (water card will be provided post approval)

Existing public services:

• Ridgewood Avenue – 225mm Conc.



Is a monitoring manhole required on private property? 🛛 Yes 🛛 🗆 No

• The sanitary sewer design has assumed a population density for the area. The sewer design should demonstrate that the proposed development is within that design criteria or that additional demand can be accommodated.

Storm Sewer

Existing public services:

• Ridgewood Avenue – 300mm Conc.



Stormwater Management

Quality Control:

• Rideau Valley Conservation Authority to confirm quality control requirements.

Quantity Control:

- Master Servicing Study:
 - Sawmills Creek Subwatershed Study
- Allowable Run-off Coefficient: to be calculated as per the Sawmills Creek Subwatershed Study
- Time of concentration (Tc): Tc = pre-development; maximum Tc = 10 min
• Allowable flowrate: Control the 100-year storm events to the 2-year storm event

Ministry of Environment, Conservation and Parks (MECEP)

All development applications should be considered for an Environmental Compliance Approval, under MECP regulations.

- a. Consultant determines if an approval for sewage works under Section 53 of OWRA is required. Consultant determines what type of application is required and the City's project manager confirms. (If the consultant is not clear if an ECA is required, they will work with the City to determine what is required. If unclear or there is a difference of opinion the City Project Manager will coordinate requirements with MECP).
- b. The project will be either transfer of review (standard), transfer of review (additional), direct submission, or exempt as per O. Reg. 525/98.
- c. Pre-consultation is not required if applying for standard or additional works (Schedule A of the Agreement) under Transfer Review.
- d. Pre-consultation with local District office of MECP is recommended for direct submission.
- e. Consultant completes an MECP request form for a pre-consultation. Sends request to <u>moeccottawasewage@ontario.ca</u>
- f. ECA applications are required to be submitted online through the MECP portal. A business account required to submit ECA application. For more information visit https://www.ontario.ca/page/environmental-compliance-approval

NOTE: Site Plan Approval, or Draft Approval, is required before any Ministry of the Environment and Climate Change (MOECC) application is signed

General Service Design Comments

- Ensure that the proposed drive lane entrance to the underground parking garage is protected from the major overland flow route within Ridgewood Avenue.
- The City of Ottawa requests that all new services be located within the existing service trench to minimize necessary road cuts.
- Monitoring manholes should be located within the property near the property line in an accessible location to City forces and free from obstruction (i.e. not a parking).
- Where service length is greater than 30 m between the building and the first maintenance hole / connection, a cleanout is required.
- The City of Ottawa Standard Detail Drawings should be referenced where possible for all work within the Public Right-of-Way.
- The upstream and downstream manhole top of grate and invert elevations are required for all new sewer connections.
- Services crossing the existing watermain or sewers need to clearly provide the obvert/invert elevations to demonstration minimum separation distances. A watermain crossing table may be provided.

Other

Are there are Capital Works Projects scheduled that will impact the application?
Yes No

References and Resources

- As per section 53 of the Professional Engineers Act, O. Reg 941/40, R.S.O. 1990, all documents prepared by engineers must be signed and dated on the seal.
- All required plans are to be submitted on standard A1 size sheets (594mm x 841mm) sheets, utilizing a reasonable and appropriate metric scale as per City of Ottawa Servicing and Grading Plan Requirements: title blocks are to be placed on the right of the sheets and not along the bottom. Engineering plans may be combined, but the Site Plans must be provided separately. Plans shall include the survey monument used to confirm datum. Information shall be provided to enable a non-surveyor to locate the survey monument presented by the consultant.
- All required plans & reports are to be provided in *.pdf format (at application submission and for any, and all, re-submissions)
- Please find relevant City of Ottawa Links to Preparing Studies and Plans below:

<u>https://ottawa.ca/en/city-hall/planning-and-development/information-developers/development-application-review-process/development-application-submission/guide-preparing-studies-and-plans#standards-policies-and-guidelines</u>

- To request City of Ottawa plan(s) or report information please contact the City of Ottawa Information Centre: <u>InformationCentre@ottawa.ca<mailto:InformationCentre@ottawa.ca</u>> (613) 580-2424 ext. 44455
- geoOttawa <u>http://maps.ottawa.ca/geoOttawa/</u>

SITE PLAN APPLICATION – Municipal servicing

For information on preparing required studies and plans refer to:

http://ottawa.ca/en/development-application-review-process-0/guide-preparing-studies-and-plans

	Number				Number
S/A	of	ENGINEERING		S/A	of
0,77	conios				conios
	copies			_	copies
<mark>S</mark>		1. Site Servicing Plan	2. Site Servicing Study	S	
<mark>S</mark>		 Grade Control and Drainage Plan 	4. Geotechnical Study	<mark>S</mark>	
		5. Composite Utility Plan	6. Groundwater Impact Study		
		7. Servicing Options Report	8. Wellhead Protection Study		
		 Community Transportation Study and/or Transportation Impact Study / Brief 	<mark>10. Erosion and Sediment Control</mark> Plan / Brief	<mark>S</mark>	
<mark>S</mark>		11. Storm water Management Report	12. Hydro-geological and Terrain Analysis		
		13. Water main Analysis	14. Noise / Vibration Study	<mark>S</mark>	
		15. Roadway Modification Design Plan	16. Confederation Line Proximity Study		

S/A	Number of copies	ENVIRONMENTAL		S/A	Number of copies
S		17. Phase 1 Environmental Site Assessment	 18. Impact Assessment of adjacent Waste Disposal/Former Landfill Site 		
		19. Phase 2 Environmental Site Assessment (depends on the outcome of Phase 1)	20. Assessment of Landform Features		
		21. Record of Site Condition	22. Mineral Resource Impact Assessment		
		23. Tree Conservation Report	24. Environmental Impact Statement / Impact Assessment of Endangered Species		
		25. Mine Hazard Study / Abandoned Pit or Quarry Study			

It is important to note that the need for additional studies and plans may result during application review. If following the submission of your application, it is determined that material that is not identified in this checklist is required to achieve complete application status, in accordance with the Planning Act and Official Plan requirements, City Planning will notify you of outstanding material required within the required 30 day period. Mandatory pre-application consultation will not shorten the City's standard processing timelines, or guarantee that an application will be approved. It is intended to help educate and inform the applicant about submission requirements as well as municipal processes, policies, and key issues in advance of submitting a formal development application. This list is valid for one year following the meeting date. If the application is not submitted within this timeframe the applicant must again pre-consult with the City.

Notes:

4. Geotechnical Study / Slope Stability Study – required as per Official Plan section 4.8.3. All site plan applications need to demonstrate the soils are suitable for development. A Slope Stability Study may be required with unique circumstances (Schedule K or topography may define slope stability concerns).

10. Erosion and Sediment Control Plan – required with all site plan applications as per Official Plan section 4.7.3.

11. Stormwater Management Report/Brief - required with all site plan applications as per Official Plan section 4.7.6.

14. Noise and Vibration Study – a Noise Study will be required if the noise sensitive development is proposed within 250 metres of an existing or proposed highway or a railway right-of-way, or 100 metres of an arterial or collector roadway or rapid-transit corridor. A Vibration Study will be required if the proposed development is within 75 metres of either an existing or proposed railway ROW. A Noise Study may also be required if the proposed development is adjacent to an existing or proposed stationary noise source.

SITE SERVICING AND STORMWATER MANAGEMENT BRIEF – MOONEY'S BAY - 729 RIDGEWOOD AVENUE, OTTAWA, ON

Appendix H Drawings

Appendix H DRAWINGS

