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Proposed High-Rise Residential Development 1200 Maritime Way

Serviceability & Stormwater Management Report

Proposed High-Rise Residential Development 1200 Maritime Way

Serviceability and Stormwater Management Report

Prepared for:

Claridge Homes

Prepared By:

NOVATECH Suite 200, 240 Michael Cowpland Drive Ottawa, Ontario K2M 1P6

January 25, 2021

Novatech File: 120144 Ref No. R-2021-012



January 25, 2021

City of Ottawa Planning, Infrastructure and Economic Development Department Planning and Infrastructure Approvals Branch 110 Laurier Avenue West, 4th Floor Ottawa ON, K1P 1J1

Attention: Ms. Laurel McCreight, MCIP, RPP

Dear Ms.:

Reference: 1200 Maritime Way - Claridge Development Serviceability and Stormwater Management Report

Enclosed is the Serviceability and Stormwater Management Report for the proposed 1200 Maritime Way development located along the Highway 417, Kanata Avenue and Maritime Way in the City of Ottawa. This report is submitted in support of the zoning amendment/site plan control applications and outlines how the site will be serviced with public infrastructure.

Trusting this report is adequate for your purposes. Should you have any questions, or require additional information, please contact me.

Yours truly,

NOVATECH 7 Marcon

Greg MacDonald, P. Eng. Director, Land Development and Public Sector Infrastructure

cc: Vincent, Denomme, Claridge Homes

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1.0 INTRODUCTION

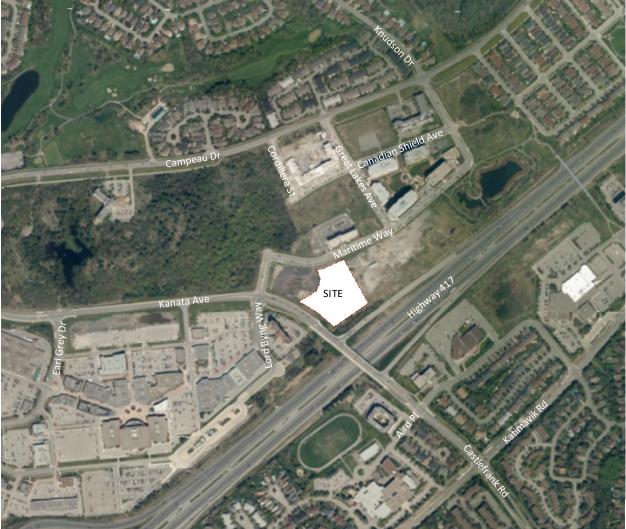
This Serviceability Study has been prepared in support of Zoning By-Law Amendment and Site Plan Control applications for the Claridge lands located at 1200 Maritime Way, as shown in **Figure 1 – Key Plan of Subject Site**. The subject site is currently occupied by a vacant land. The proposed redevelopment will include a total of 632 dwelling units and 662 parking spaces.

The subject site has an approximate area of 1.28 hectares, and is surrounded by the following:

- Maritime Way and Townplace Suites by Marriott hotel to the north;
- Highway 417 to the south;
- Vacant land to the east; and
- A retirement residence to the west.

The most recent aerial view of the subject site is provided in Figure 1.

Figure 1: Key Plan of Subject Site



1.1 **Proposed Development**

The subject site is located within one of the Zone AM10 of the City of Ottawa. The implemented zoning for the property permits the proposed land uses. However, a Zoning By-Law Amendment is required to permit certain attributes of the proposed development, such as building height and FSI.

The proposed development will be constructed in one phase as shown in **Figure 2 – Phasing Plan** and as described below.

- Phase 1 (East/West Towers):
 - East Tower: 28-storey high-rise including 7-storey podium with 300 dwellings;
 - West Tower: 30-storey high-rise including 7-storey podium with 332 dwellings.

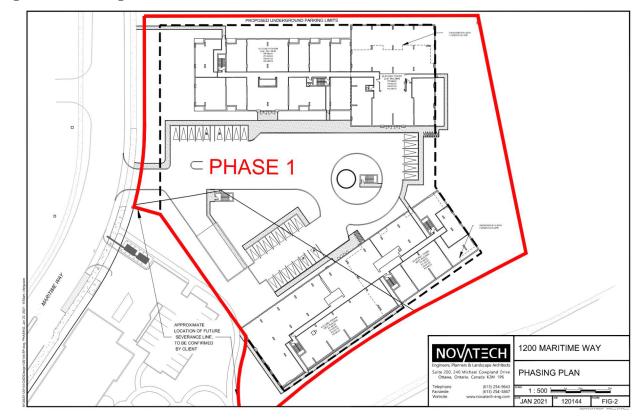


Figure 2 Phasing Plan

In total, the proposed development will consist of 632 apartment dwellings. The entire site will include 662 parking spaces for residents (632 inside) and visitors (30 outside) and will be accessed via full-movement driveway to Maritime Way. Phase 1 of the development is anticipated to be built out by 2028.

A copy of the Site Plan is included in **Appendix A – Site Plan**.

2.0 SANITARY SEWER

The development will be serviced by the existing 825mm diameter sanitary sewer on Maritime Way, as shown on the general plan of services.

The service will be a 375mm diameter sanitary sewer to Maritime Way.

The proposed development flows are based on the City of Ottawa Sewer Design Guidelines and are provided below.

2.1 **Proposed Sanitary Flows from Development Site**

Proposed sanitary flows are summarized in **Table 2.1 – Proposed Sanitary Flows** with detailed calculations below. Development statistics, e.g. apartment and townhouses are summarized in **Table 2.2 – Development Statistics**. A Phasing Plan is shown in **Figure 2 – Phasing Plan**.

Table 2.1 Proposed Sanitary Flows

Phase	Peak Sanitary Flow (L/sec)	
East Tower	5.52	
West Tower	6.02	
Total	11.54	

Table 2.2 Development Statistics

Building Component	Area (ha)	1 Bdr (x1.4)	2 Bdr (x2.1)	Total
East Tower				
Tower (incl. Podium)	-	177	123	300
Total	+/- 0.64	177	123	300
West Tower				
Tower (incl. Podium)	-	204	128	332
Total	+/- 0.64	204	128	332
Grand Total	1.28	381	251	632

Sanitary Flows East Tower

Area = 1.28 ha

Tower (incl. Podium): 247.8 + 258.3 = 507 people

Sanitary flows are calculated below using the City's new Sewer Design Criteria.

Population = 507 people Peak Factor = $1 + \frac{14}{(4 + (P/1000)^{1/2})} \times 0.80 = 3.23$ (using entire population of 1,062) Area = 1.23 ha

 $Q_{\text{Phases 1}} = (507)(280)(3.23) + (0.64)(0.33) = 5.52 \text{ L/sec}$ 86,400

Therefore, the total peak sanitary flow for **East Tower is 5.52 L/sec.**

Sanitary Flows West Tower

Area = 1.28 ha

Tower (incl. Podium): 285.6 + 268.8 = 555 people

Sanitary flows are calculated below using the City's new Sewer Design Criteria.

Population = 555 people Peak Factor = $1 + \frac{14}{(4 + (P/1000)^{1/2})} \times 0.80 = 3.23$ (using entire population of 1,062) Area = 0.52 ha

 $Q_{Phases 2} = (555)(280)(3.23) + (0.64)(0.33) = 6.02 L/sec$ 86,400

Therefore, the total peak sanitary flow for **West Tower is 6.02 L/sec.**

Furthermore, the total peak sanitary flow for all Phase 1 is 11.54 L/sec.

The development of the adjacent site at 1250 Maritime Way (Timberwalk Retirement Home) accounted for flows of 3.86 L/s from the future development at 1200 Maritime Way. Because of an increase in flows over and above the flows allocated to the site (e.g. 2.84 L/s per the Robinson Consultants letter dated March 27, 1996) an analysis of the downstream sewer system was completed by J.L. Richards allowing for the additional flows of 4.35 L/sec (e.g. 7.19 L/s development flows for 1250 Maritime Way and future 1200 Maritme Way – 2.84 L/s allocated flows). Both the Robinson letter and the JLR analysis is included in **Appendix B – Sanitary Sewer Design Downstream Capacity.**

The calculated flow for the current proposed development at 1200 Maritime Way is 11.54 L/sec. This flow was input into the same spreadsheet analysis used for 1250 Maritime Way to assess the impact on the downstream sewer. To provide a comparison to the initial assessment completed by JLR, the same flow parameters were used, as follows:

- Per capita flow 350 L/cap/day
- Harmon Factor = 1.0
- Infiltration = 0.28 L/sec/ha
- Commercial/Retail = 50,000 L/ha/day

The resulting analysis shows an increase in peak sanitary flow in the truck sewer of 6.48 L/sec, explained as follows:

490.60 L/s	Peak Sanitary Flow in Trunk Sewer including proposed 1200 Maritime Way
<u>480.26 L/s</u>	Peak Sanitary Flow in Trunk Sewer under current conditions
10.34 L/s	Difference
<u>3.86 L/s</u>	Less portion of 1200 Maritime Way included in original analysis
6.48 L/s	Additional Sanitary Flows to be Added to System

The total additional flows of 6.48 L/sec represents 1.35 % of the total flow. Given the available capacity in the trunk sewer the impacts are negligible. Further, to be consistent with the original analysis completed for 1250 Maritime Way, the previous sanitary flow parameters were used.

Using current standards, e.g. 280 L/cap/day and Harmon 0.80 would further reduce the flows going into the sanitary sewer.

3.0 STORM SEWER AND STORMWATER MANAGEMENT

As part of this development, stormwater will be controlled on-site and discharged via a 375mm dia. service that will connect to the existing 1650mm dia. storm sewer on Maritime Way as shown on the General Plan of Services.

The site is fairly flat overall and the majority of storm runoff from the site is self-contained with some being conveyed overland towards the neighboring properties.

3.1 Storm Water Management Criteria

Stormwater management (SWM) design criteria for the proposed development were established by the City of Ottawa Sewer Design Guidelines (October 2012); Kanata Town Centre, Central Business District, Stormwater Management Report (J.L. Richards, January 1999) and Servicing Brief (Revised) – Kanata Town Centre Central Business District Subdivision, Technical Memorandum (J.L. Richards, June 13, 2012). The SWM design criteria are as follows:

- Control post-development peak flows up-to and including the 100-year storm event to the allowable release rate. Provide on-site water quantity control for all flow in excess of the allowable release rate. The allowable release rate is to be determined by applying the following parameters to the site area:
 - A runoff coefficient of 0.8
 - A time of concentration of 20 minutes
 - A 5-year intensity using the City of Ottawa Intensity-Duration-Frequency (IDF) curves
- Minimize the impact on the downstream receiving watercourses by minimizing the potential erosion and volume of sediment entering the watercourses both on a temporary basis (during construction) and on a permanent basis.
- Provide guidelines to ensure that site preparation and construction is in accordance with the current Best Management Practices for Erosion and Sediment Control.

3.2 Hydrologic and Hydraulic Modelling

The allowable release rate for the 1.28 ha site was determined to be 199.99 L/s based on the SWM criteria provided by the City of Ottawa.

The rational method was used to estimate post-development peak flows (quantity control targets) and determine approximate storage requirements for the site. The storage requirements for the site were determined for each tower of the development.

The post-development drainage areas were delineated based on the proposed development grading. Refer to **Drawing 120144-GR** for the proposed site grading and **Drawing 120144-SWM** for the drainage areas. The storage requirements are based on meeting the allowable release rate generated for the site.

The site will be graded such that flows in excess of the 100-year storm event will be conveyed overland to Maritime Way.

<u>Design Storms</u>

The design storms are based on City of Ottawa design storms. Design storms were used for the 5 and 100-year return periods (i.e. storm events).

Model Parameters

Post-development catchments were modelled based on the proposed site plan and grading as shown on **Drawing 120144-SWM**. All the sub-catchments are assumed to be 100% impervious with exception to the grassed areas not over underground parking (A-1, part A-2, A-3, part A-4, part A-5 and part A-6) which are 0% impervious. The building roofs were assumed to have no depression storage.

A summary of the allowable release rate, post-development parameters and output for the 5 and 100-year storm events are provided in **Appendix C – Stormwater Management Calculations**.

3.3 Water Quantity Control

On-site stormwater management will be implemented to control post-development stormwater discharge to the allowable release rate of 199.99 L/s and will be achieved using internal stormwater tanks that will be pumped to the storm sewers on Maritime Way.

Runoff from the grassed areas (Sub-catchments A-1, A-2, A-3, A-4, A-5 and A-6) will be uncontrolled and will drain towards to Maritime Way. The total uncontrolled flows from the site in the 100-year event will be 87.81 L/s which requires the remaining areas of the site to be controlled to 112.18 L/s in order to meet the allowable release rate.

The remaining 112.18 L/s of allowable release rate was divided between the development phases using area-weighting as shown in **Table 3.1**.

Phase	Drainage Area (ha)	Allowable Release Rate (L/s)
East Tower (incl. CB1/2)	0.54	70.44
West Tower (incl. CB3/4 & TD)	0.32	41.74
Total	0.86	112.18

Table 3.1 Controlled Release Rates

The runoff from each tower and corresponding CBs or TD will be collected into at least one tank located within the development. The site was modeled so that the pump rate for each phase was equal to the allowable release rate for that phase. East and West towers will be pumped to the Maritime Way storm sewer. The tanks will have an emergency overflow that will connect to the ground surface. The required storage in the 100-eyar event for each phase is summarized in **Table 3.2**.

Table 3.2 Required Tank Storage for the 100-year Storm

Phase	Required Storage Volume (m ³)
East Tower	131.56
West Tower	77.96
Total	209.52

The storage provided allows for the proposed development to meet the allowable release rate of 199.99 L/s. The total release rates from the site during the 100-year storm event are provided in **Table 3.3**.

Phase	Drainage Area (ha)	Allowable Release Rate (L/s)
East Tower (incl. CB1/2)	0.54	70.44
West Tower (incl. CB3/4 & TD)	0.32	41.74
Uncontrolled	0.42	87.81
Total	1.28	199.99

Table 3.3 Overall Site Release Rate for the 100-year Storm

3.4 Water Quality Control

Runoff from the roofs, podiums, and uncontrolled grassed areas would be considered clean and will not require treatment. Additionally, the storage tanks will allow for some settling of particulates in the stored runoff from the remaining site areas. Additional water quality treatment will not be required. Erosion and sediment control measures will be implemented during all phases of construction and inspected regularly.

Cisterns from the East Tower and West Tower will discharge to the existing storm sewer on Maritime Way.

Also, there will be water quality control provided by the downstream SWM facility which has been designed to provide quantity and quality control for the proposed development (as per the Stormwater Management Study prepared by JL Richards.

The site will be graded such that flows in excess of the 100-year storm event will be conveyed overland to Maritime Way.

4.0 WATERMAIN

4.1 Domestic Water Demand

The proposed development will be serviced by the 200mm dia. watermain on Maritime Way as shown on the General Plan of Services. Shutoff valves will be provided at property lines as per City of Ottawa Specifications. The water meters will be in the basement level mechanical rooms of the buildings. Similarly, remote receptacles will be located at the surface near the entrances to the buildings on the exterior.

The services will be two (2) 200mm diameter water services to Maritime Way, with a valve in between both of them.

Estimated domestic water demands for the development are provided below with a detailed breakdown per phase:

Watermain Flows East Tower

Average Day Demand = 2.05 L/sec Maximum Day Demand (x2.5) = 5.13 L/sec Peak Hour Demand (x2.2) = 11.28 L/sec

Watermain Flows West Tower

Average Day Demand = 2.25 L/sec Maximum Day Demand (x2.5) = 5.63 L/sec Peak Hour Demand (x2.2) = 12.38 L/sec

4.2 Fire Demand

An estimate of the water required to meet firefighting demands is described below.

Section 4.2.11 of the City of Ottawa Water Design Guidelines reads:

"When calculating the fire flow requirements and affected pipe sizing, designers shall use the method developed by the Fire Underwriters Survey", and

"The requirements for levels of fire protection on private property are covered in Section 7.2.11 of the Ontario Building Code."

The Fire Underwriters Survey is used to assess the performance of the water distribution system on a "City Block" basis rather than an individual building basis. The Ontario Building Code governs the assessment of fire demand for individual buildings.

Section 7.2.11.1 of the Ontario Building Code states that the design, construction, installation and testing of fire service mains and water service pipe combined with fire service mains shall be in conformance with NFPA 24.

NFPA 24 is the standard for the "Installation of Private Fire Service Mains and their Appurtenances". Chapter 13 of NFPA 24 discusses sizing the private service fire mains for fire

protection systems which shall be approved by the authority having jurisdiction, considering the following factors:

- Construction and Occupancy of the Building
- Fire Flow and Pressure of the Water Required
- Adequacy of the Water Supply

It is expected that any future building on the site will be sprinklered per Section 3.2.2.45 of the OBC. Section 3.2.5.7 of the OBC requires that an adequate water supply for fire fighting be provided to each building, and references Appendix A of the OBC. Sentence 3 of Section A 3.2.5.7 of the OBC (Appendix A) states that NFPA 13 be used for determining both sprinkler and hose stream demands for a sprinklered building.

The design of the sprinkler system is completed by a Fire Protection Engineer, or typically computed by the sprinkler contractor and approved by the Fire Protection Engineer. This process involves detailed hydraulic calculations based on building layout, pipe runs, head losses, fire pump requirements, etc. At this stage in the planning and site design process, these details are not available. Therefore, this report will confirm the maximum anticipated sprinkler and hose stream demands as per NFPA 13.

Section 11.2.3 of the NFPA 13, "Water Demand Requirements – Hydraulic Calculations Methods" was used to estimate the sprinkler and hose stream demands. Figure 11.2.3.1.1 – Area/Density Curves confirms the sprinkler demand, assuming Ordinary 1 construction. Table 11.2.3.1.2 confirms the hose stream allowance and water supply demand requirements, assuming ordinary hazard construction.

For Ordinary 1 type construction, design is based on a density of 0.15 gpm (US), and a maximum area of sprinkler operation limited to 1500 ft² (139 m²). As per NFPA 13 Figure 11.2.3.1.1, the maximum anticipated sprinkler demand is 225 gpm (US). As per NFPA 13 Table 11.2.3.1.2, the maximum total combined inside and outside hose demand is 250 gpm (US) with a duration of 60-90 minutes.

Based on the calculations above, the total estimated sprinkler and hose demand for the development is 475 gpm (US). However, because the development has not been finalized to-date, it is recommended to add a 50% contingency. Therefore, a sprinkler demand of 713 gpm (US), 2700L/min, should be anticipated at this stage. Refer to **Appendix E – Fire Demand Calculations.**

Boundary conditions are requested from the City of Ottawa using a fire demand calculated using the **Fire Underwriters Insurance** procedure. This method is used by municipalities to assess their systems on a more global basis and results in a more conservative fire demand for individual sites, as compared to Building Code calculations. The estimated fire demand using FUS for each of the phases is provided in **Table 4.1 – Calculated Fire Demand.** Detailed calculations are included in **Appendix D – Fire Demand.**

Phase	Fire Demand (L/min)	
East Tower	6000	
West Tower	5000	

Table 4.1 Calculated Fire Demand

5.0 CONCLUSIONS

Based on the foregoing, report conclusions are:

- Adequate sanitary sewer capacity is available on Maritime Way and in the downstream system to the trunk sewer.
- On site stormwater management will be implemented to control post-development flows to that value calculated using a tc of 20 minutes, run-off coefficient of 0.80 and 5-year storm. This will be implemented through construction of cisterns in the underground parking structure as summarized below. Uncontrolled flow from grass areas will drain overland to Maritime Way.

Phase	Cistern Volume (m ³)	Discharge (L/s)	Street Sewer
East Tower	131.56	70.44	Maritime Way
West Tower	77.96	41.74	Maritime Way
1	Uncontrolled	87.81	Maritime Way
Total	209.52	199.99	

• Adequate water services are available on Maritime Way for domestic demand. It is expected that adequate water supply is available for firefighting which will be confirmed once boundary conditions are received from the City. Calculated fire demand ranged from 5000 L/min to 6,000 L/min. The buildings will be equipped with fire pumps and sprinklers.

NOVATECH

Prepared by:

agmine Southie

Jazmine Gauthier, B.A.Sc. Project Manager | Land Development

Reviewed by:



Greg MacDonald, P.Eng. Director | Land Development and Public Sector Infrastructure

APPENDIX A

Site Plan



KEY PLAN

ZONE AM10			
PROVISION Min Lot Width Min Lot Area Max Building Height Min Front Yard Setback Min Corner Side Yard Setback	REQUIRED no minimum no minimum 67m no mininum no minimum		PROVIDED +/- 69.65 m +/- 12 808 m ² +/- 93.5 m 3.05 m / 3.09 m 16.74 m
Min FSI Min Interior Side Yard Setback	2 no minimum		+/- 4.88 14.70 m / 15.13 m
SITE AREA : SITE COVERAGE :		T ()	+/- 12 808 sq.m. (To be confirmed by surveyor) +/- 2 207 m ² (East Tower) +/- 1 968 m ² (West Tower)
GROUND PARKING AREA : LANDSCAPED AREA (EXCLUDING PARKING) :		i otal =	+/- 4 175 m² = 32.6 % +/- 2 298 m² = 17.9% +/- 6 335 m² = 49.5 %

RENTAL - EAST TOWER

PROPOSED GROSS FLOOR AREA : BASEMENT G.F.A. : GROUND FLOOR G.F.A. : RENTAL FLOORS G.F.A. (2nd to 30th floor) : PRIVATE AMENITY AREA (G.F.A.) : COMMUNAL AMENITY AREA : NUMBER OF FLOORS AND BUILDING HEIGHT DWELLING UNITS : PARKING STALLS : PROVIDED BICYCLE STALLS :

+/- 21 964 m²
+/- 0m²
+/- 635 m²
+/- 21 329 m²
+/- 1 953 m²
+ /- 925 m²
28 FLOORS + MECH. / +/- 87.50m
300
315 (300 INSIDE / 15 VISITORS OUTSIDE)
150 (142 INSIDE / 8 OUTSIDE)

NUMBER OF SUITES REQUIRED TO BE BARRIER-FREE : 300 UNITS = 45 UNITS HAVE TO BE BARRIER-FREE THEY WILL BE DISTRIBUTED BETWEEN THE 28 FLOORS

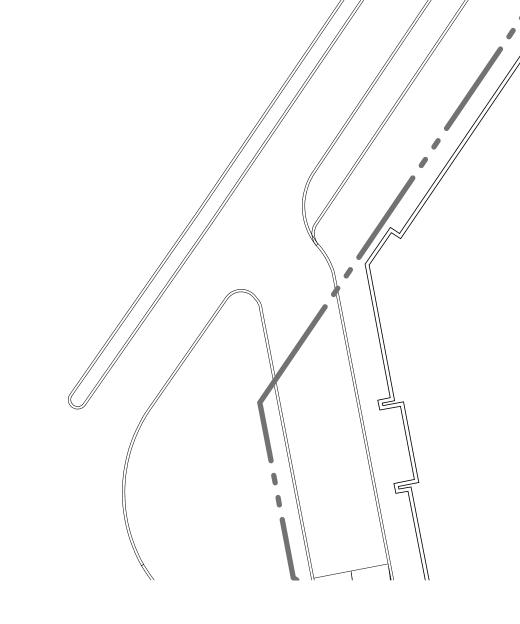
RENTAL - WEST TOWER

PROPOSED GROSS FLOOR AREA : BASEMENT G.F.A. : GROUND FLOOR G.F.A. : RENTAL FLOORS G.F.A. (2nd to 30th floor) : PRIVATE AMENITY AREA (G.F.A.) : COMMUNAL AMENITY AREA : NUMBER OF FLOORS AND BUILDING HEIGHT DWELLING UNITS : PARKING STALLS : PROVIDED BICYCLE STALLS :

+/- 30 179 m² +/- 0m² +/- 375 m² +/- 29 804 m² +/- 2 247 m² + /- 1 045 m² 30 FLOORS + MECH. / +/- 93.50m 332 347 (332 INSIDE / 15 VISITORS OUTSIDE) 166 (159 INSIDE / 7 OUTSIDE)

NUMBER OF SUITES REQUIRED TO BE BARRIER-FREE 332 UNITS = 50 UNITS HAVE TO BE BARRIER-FREE THEY WILL BE DISTRIBUTED BETWEEN THE 30 FLOORS

- FOR EXISTING SITE CONDITIONS, SEE SURVEY PLAN BY
- ANNIS, O'SULLIVAN, VOLLEBEKK LTD., SUBMITTED SEPARATELY;
- FOR NEW GRADES AND SITE SERVICES, SEE CIVIL ENGINEERING PLAN BY
- NOVATECH ENGINEERING CONSULTANTS, SUBMITTED SEPARATELY;
- FOR PROPOSED VEGETATION AND LANDSCAPE INFORMATION, SEE LANDSCAPE PLAN BY JAMES B. LENNOX & ASSOCIATES, SUBMITTED SEPARATELY.



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MAγ

MARITIME + PARKING LIMIT +

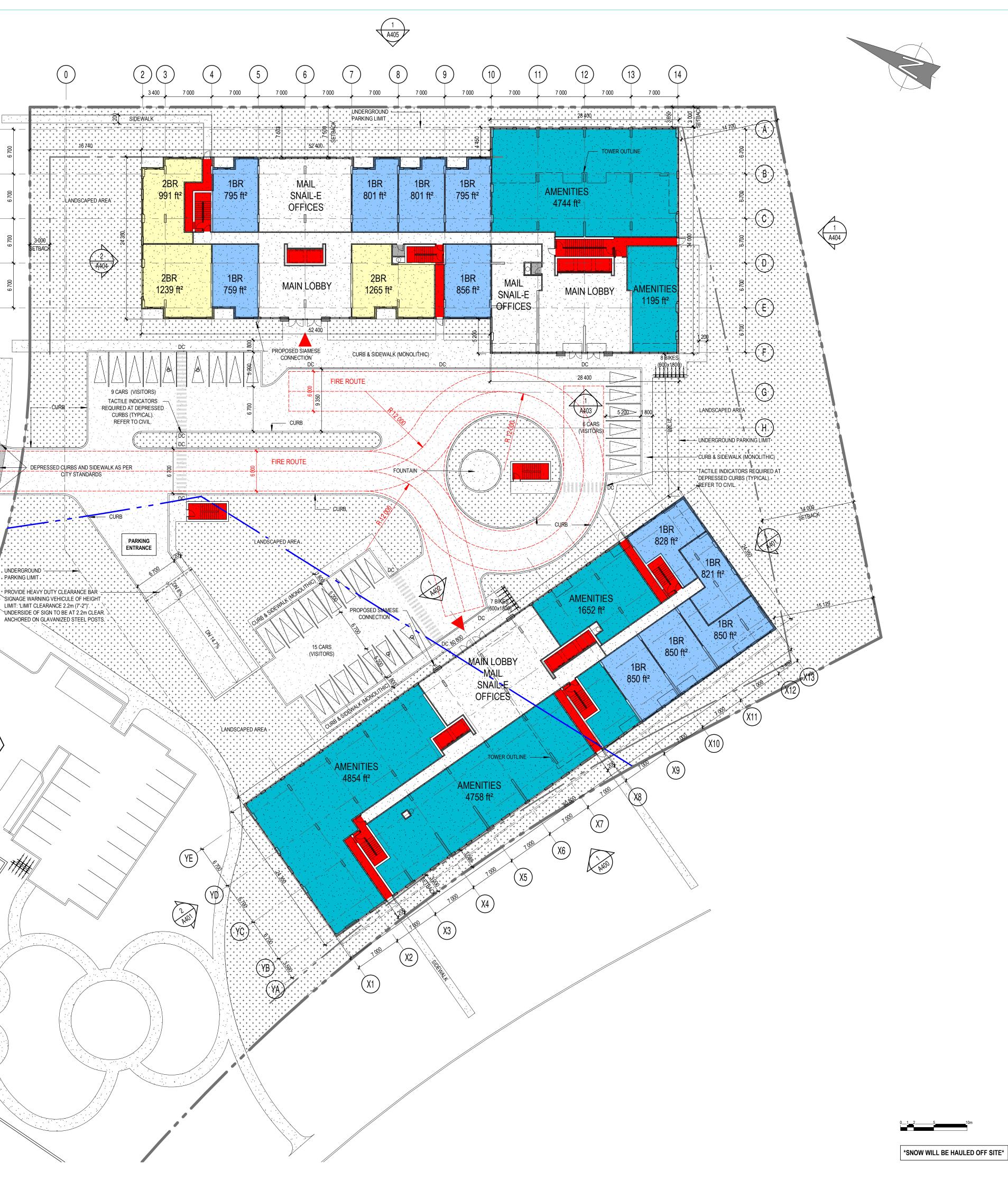
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(B)

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NOTES GÉNÉRALES General Notes

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 divergences entre ces documents et ceux des autres professionnels. / The architect must be notified of all errors, omissions and discrepancies between these documents and those of the others professionnals.
- Les dimensions sur ces documents doivent être lues et non
 mesurées. / The dimensions on these documents must be read and not measured.

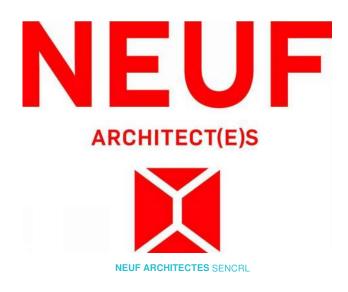
STRUCTURE Structural Goodeve Structural Inc. 18-77, Auriga Drive, Ottawa ON K2E 7Z7 T 613 226 4558 goodevestructural.ca

ARCHITECTURE DE PAYSAGE Landscape Architect James B. Lennox & Associates 3332, Carling Avenue, Ottawa ON K2H 5A8 T 613 722 5168 jbla.ca

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ARCHITECTES Architect **NEUF architect(e)S** SENCRL 630, boul. René-Lévesque O. 32e étages, Montréal QC H3B 1S6 T 514 847 1117 NEUFarchitectes.com

SCEAU / Seal



CLIENT Client



OUVRAGE Project **1200 MARITIME WAY** (KANATA RENTAL)

EMPLACEMENT Location OTTAWA

RÉVISION Revision

NO PROJET No. 12371.00

NO	RÉVISION	DATE (aa-mm-jj)
А	FOR COMMENTS	2020.05.28
В	FOR COMMENTS	2020.06.05
С	FOR COMMENTS	2020.07.23
D	IN PROGRESS	2020.09.16
Е	SITE PLAN COORDINATION	2020.12.08
F	SITE PLAN COORDINATION	2020.12.16

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SITE PLAN AT	_
GROUND FLO	OK LEVEL



APPENDIX B

Sanitary Sewer Design Downstream Capacity

MEMORANDUM



J.L. Richards & Associates Limited 864 Lady Ellen Place Ottawa, ON Canada K1Z 5M2 Tel: 613 728 3571 Fax: 613 728 6012

Page 1 of 2

To: Greg MacDonald, P.Eng. Novatech Engineering Consultants Ltd.

Date:	August 18, 2017
Job No.:	15712-015.1
CC:	Lucie Dalrymple, P.Eng. J.L. Richards & Associates Ltd.

From: Karla Ferrey, P.Eng.

Re: Kanata Town Centre Central Business District Master Design Sheet Update - Sanitary Peak Flows Block 4, Block 5 and Block west of Block 9 (Zone 122)

We understand that the City is requesting an update to the Master Sanitary Sewer Design Sheet for the Kanata Town Centre Central Business District (KTC-CBD) from JL Richards such to incorporate the proposed peak flow revision from Block 4, Block 5, and the parcel west of Block 9 (previously Robinson'96 - Zone 122). Refer to attached JLR Sanitary Drainage Plan and Robinson Consultants Figure 7.1 for locations of Block 4, Block 5 and Zone 122.

We understand that the City will ultimately decide (as the owner of the existing sewers within the KTC-CBD and downstream system) whether the proposed peak flow increase is acceptable and that if accepted, it will not require a reduction of the allowable peak flows for the remaining future development in the KTC-CBD.

As requested, we have incorporated the proposed sanitary peak flow increase associated with your following developments:

a) Proposed Block 4 - Residential development

The proposed development will result in a theoretical increase in peak flow from 3.88 L/s to 4.71 L/s at MH 513 where the Block 4 development outlets to Maritime Way. This represents a theoretical peak flow increase of 0.83 L/s from the anticipated 2012 land use (i.e., hotel use, based on 270 L/pers/day).

b) Proposed Block 5 - Residential development

The proposed development will result in a theoretical increase in peak flow from 3.52 L/s to 5.13 L/s at MH 511 where the Block 5 development outlets to Maritime Way. This represents a theoretical peak flow increase of 1.61 L/s from the anticipated 2012 land use (i.e., hotel use, based on 270 L/pers/day).

c) <u>Proposed parcel west of Block 9 (previously identified in the 1996 Robinson KTC Sanitary Design as Zone</u> <u>122) – Retirement Home – Claridge Homes</u>

The proposed development will result in a theoretical increase in peak flow from 2.84 L/s to 7.19 L/s at MH 7A where Claridge Homes development outlets to Maritime Way. This represents a theoretical peak flow increase of 3.57 L/s from the anticipated 2012 land use (i.e., Commercial use based on 2787m2 office space and Infiltration based 1.5ha). Theoretical flows for Zone 122 were taken from Robinson Consultants Sanitary Trunk Information from Table 4.7 and Figure 7.1, see attached copies.

At the most downstream MH at the intersection of Teron Rd and Campeau Dr (MH Ex. 2) shown on the attached Sanitary Sewer Design Sheet for the Kanata Village Green subdivision (prepared in 1998 by JLR), the proposed 3 developments would result in a theoretical increase in peak flow from 475.94 L/s to 480.24 L/s which corresponds to a 4.3 L/s (0.9%) peak flow increase.

Based on the available theoretical residual capacities noted in the attached updated Master Sanitary Sewer Design Sheet, the existing sanitary sewer system from the intersection of Rock Mountain Gate and Maritime Way to the intersection of Campeau Dr and Teron Rd has the capacity to accommodate the additional theoretical peak flows of Block 4, Block 5 and Zone 122. Downstream of the Campeau Drive intersection, JLR does not have on record design sheets for the City's existing downstream sanitary sewer system.

August 18, 2017 JLR No.: 15712-015.1

Page 2 of 2

Should you have any questions or require anything further, please do not hesitate to call.

J.L. RICHARDS & ASSOCIATES LIMITED

Prepared by:

Karla Ferrey, P.Eng.



CITY OF OTTAWA

KANATA TOWN CENTRE CENTRAL BUSINESS DISTRICT URBANDALE CORPORATION JLR PROJECT NO.: 15712

Commercial Flow =	50000	L/ha/d
q residential=	350	l/cap/d
q hotel =	270	l/cap/d
q retirement homes =	450	l/cap/d
i =	0.28	l/s/ha
SING. HOUSING	3.4	pers/hse
MULT. HOUSING	2.7	pers/hse
Hotel/Appartments	1.8	pers/ro
Retirement Homes	1.6	pers/ro

Manning's Coefficient (n) =

	RESIDENTIAL												COMMERCIAL / INSTITUTIONAL PLUGGED								2017 Updates to Block 4,5, West of 9 Peak Flows						
							SIDENTIAL			-							PLUGGE			+C		1	SEWER	DATA	1	CAPA	ACITY
M.H. #					JMBER OF U				r			PEAKING			UMM.	COMM.		CUMM.	PEAK EXTR.				CAPAC.				
	SING. Stacks	s Towns				Hotel/Apart.	_	POPUL.	AREA	POPUL.		FACTOR			AREA	FLOW	FLOW	FLOW	FLOW	FLOW	DIA. mm	SLOPE %	l/s	VEL. m/s	LENGTH m	Residual	% Full
FROM TO			No units	Act. pop	No units	Act. pop.	Equ. pop.	people	ha	people	ha		l/s	ha	ha	l/s	l/s	l/s	l/s	l/s						(L/s)	
Robinson - 1996 Upstream 7A								(1) 2588	(1) 28.38	2588	28.38	3.50	36.65	(1) 20.37	20.37	17.68	(1) 162.69	162.69	14.01	231.04							
								(1) 2566	(1) 20.30	2000	20.30	3.50	30.00	(1) 20.37	20.37	17.00	(1) 162.69	102.09	14.01	231.04							
Claridge Block 122 (per Robinson'96) 7A								377	0.89	377	0.89	4.00	6.11	0.005	0.005	0.004	(6) 0.83	0.83	0.25	7.19							
																	(-)			-							
MARITIME WAY 7A 507										2965	29.27	3.45	41.40		20.38	17.69		163.52	14.26	236.87	825	0.14	529.34	0.99	81.90	292.47	45%
MARITIME WAY 507 506					125	225	174	174	1.02	3139	30.29	3.43	43.56	4.91	25.29	21.95		163.52	15.92	244.95	825	0.12	500.32	0.94	119.30	255.37	49%
CORDILLERA ST. 534 533 CANADIAN SHIELD AV. 533 532					125	207	207	207	0.58	207 207	0.58 0.58	4.00 4.00	3.35 3.35		0.55	0.48			0.32	4.15 4.15	200 200	1.65 1.20	42.13 35.93	1.34 1.14	66.60 69.60	37.98 31.78	10% 12%
CANADIAN SHIELD AV. 532 531									0.33	207	0.91	4.00	3.35		0.55	0.48			0.41	4.24	200	1.20	35.93	1.14	69.60	31.69	12%
GREAT LAKES AV. 536 531					100	180	139	139	0.78	139	0.78	4.00	2.25	0.04	0.04	0.03	(5) 0.30	0.30	0.23	2.81	200	2.40	50.81	1.62	60.00	48.00	6%
GREAT LAKES AV. 531 530										346	1.69	4.00	5.61		0.59	0.51		0.30	0.64	7.05	200	3.75	63.51	2.02	80.80	56.46	11%
GREAT LAKES AV. 530 506A										346	1.69	4.00	5.61		0.59	0.51		0.30	0.64	7.05	200	1.40	38.80		85.20	31.75	18%
GREAT LAKES AV. 506A 506	$\left \right $				├				0.38	346	2.07	4.00	5.61		0.59	0.51		0.30	0.74	7.16	200	1.40	38.80	1.24	4.90	31.65	18%
MARITIME WAY 506 505	┨──┤───				176	316.8	269	269	0.57	3754	32.93	3.36	51.06	<u> </u>	25.87	22.46		163.82	16.82	254.17	825	0.12	486.76	0.91	111.00	232.59	52%
MARITIME WAY 506 505 MARITIME WAY 505 504	 		+		176	262.8	269	269	0.57	3754 3984	32.93	3.36	51.06		25.87	22.46		163.82	16.82	254.17 259.09	825	0.12	486.76	0.91	111.00	232.59	52% 53%
MARTIME WAT 505 504 501					140	202.0	200	200	0.38	3984	33.49	3.33	53.82		27.62	23.98		163.82	17.55	259.09	825	0.11	476.06	0.91	29.90	225.55	54%
CANADIAN SHIELD AV. 542 541			1		176	316.8	269	269	0.74	269	0.74	4.00	4.36						0.21	4.57	200	2.20	48.64	1.55	71.30	44.08	9%
CANADIAN SHIELD AV. 541 540					154	277.2	232	232	0.51	501	1.25	3.97	8.06	1.36	1.36	1.18			0.73	9.98	200	0.90	31.13	0.99	77.70	21.15	32%
Block 3 540			208	333			428	428	1.02	428	1.02	4.00	6.93						0.29	7.22	200	0.60	25.40	0.81	12.00	18.18	28%
CANADIAN SHIELD AV. 540 512									0.30	929	2.57	3.82	14.38		1.36	1.18			1.10	16.66	200	0.71	27.65	0.88	82.60	11.00	60%
MARITIME WAY 514 513												4.00	-								200	2.14	47.96	1.53	51.20	47.96	
MARITIME WAY (Block 4) 513 512					144	271	271	271	1.12	271	1.12	4.00	4.39						0.31	4.71	200	2.28	49.52	1.58	51.90	44.81	10%
															-				0.01		200	2.20	40.02	1.00	01.00	11.01	1070
MARITIME WAY 512 511							58	58	(2) 0.73	1258	4.42	3.73	19.02		1.36	1.18			1.62	21.82	200	3.12	57.95	1.84	49.30	36.12	38%
Block 5 511					154	301	301	301	0.92	301	0.92	4.00	4.88						0.26	5.13	200	2.00	46.38	1.48	12.20	41.25	11%
MARITIME WAY 511 510										1559	5.34	3.67	23.16		1.36	1.18			1.87	26.21	200	1.70	42.76	1.36	38.40	16.54	61%
MARITIME WAY 510 501										1559	5.34	3.67	23.16		1.36	1.18			1.87	26.21	200	2.28	49.52	1.58	11.30	23.30	53%
TRUNK EASEMENT 501 500										5543	39.09	3.20	71.92		28.98	25.16	-	163.82	19.42	280.32	825	0.10	462.89	0.87	129.00	182.57	61%
TRUNK EASEMENT 500 94										5543	39.09	3.20	71.92		28.98	25.16		163.82	19.42	280.32	020	0.10	402.00	0.07	123.00	102.07	0170
A 90 92		35						95	0.80	95	0.80	4.00	1.53						0.22	1.76	250	0.60	46.06	0.94	120.0	44.30	4%
92 94		12						32	1.19	127	1.99	4.00	2.06						0.56	2.61	250	2.20	88.20	1.80	103.0	85.58	3%
										ļ											μ		L	ļ			
94 95	+ $+$ $+$ $$				├───				. =0	5670	41.08	3.19	73.36		28.98	25.16		163.82	19.98	282.31	825	0.12	497.22	0.93	17.5		57%
95 89	┨──┤───	10			├───			27	0.52	5697	41.60	3.19	73.66		28.98	25.16		163.82	20.12	282.76	825	0.12	497.22	0.93	66.6	214.46	57%
B 85 87	19	-	1					65	1.19	65	1.19	4.00	1.05						0.33	1.38	250	0.40	37.61	0.77	116.9	36.23	4%
B 85 87 87 89		24	1					65	0.82	129	2.01	4.00	2.10						0.56	2.66	250	1.41	70.70	1.44	116.9		4%
			1																			1		1			
A 89 84		12						32	0.35	5859	43.96	3.18	75.48		28.98	25.16		163.82	20.78	285.24	825	0.12	497.22	0.93	79.0	211.98	57%
C 80 82	19							65	1.08	65	1.08	4.00	1.05						0.30	1.35	250	0.40	37.61	0.77	120.0		4%
82 84		25						68	0.83	132	1.91	4.00	2.14						0.53	2.68	250	1.20	65.18	1.33	118.5	62.51	4%
<u>├</u>	+ $+$ $+$ $$				├───				0.54	6000	46.11	0.17	77.00		20.00	05.40		460.00	01.47	007.00	μ		407.00	0.00		200.00	E00/
A 84 79	┨──┤───	14						38	0.54	6028	46.41	3.17	77.38		28.98	25.16		163.82	21.47	287.83	825	0.12	497.22	0.93	79.0	209.39	58%
D 75 76	1 1	17	+					46	0.37	46	0.37	4.00	0.74						0.10	0.85	250	0.40	37.61	0.77	57.0	36.76	2%
76 77		20						54	0.37	100	0.66	4.00	1.62						0.10	1.80	250	0.40	37.61	0.77	57.0		5%
77 79	1 1	13						35	0.63	135	1.29	4.00	2.19						0.36	2.55	250	0.40	53.66	1.09	117.7		5%
			1		† †																	2.01					
PARK EASEMENT 79 67		L							0.98	6163	48.68	3.16	78.89		28.98	25.16		163.82	22.11	289.97	825	0.12	497.22	0.93	55.0	207.25	58%
67 66		6						16	0.33	6180	49.01	3.16	79.07		28.98	25.16		163.82	22.20	290.25	825	0.12	497.22	0.93	70.0	206.98	58%

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MASTER SANITARY SEWER DESIGN SHEET Designed: L.D.

2017 Update by: KF 2017 Check by: LD

Date: August 15, 2017

ers/hse

ers/hse

pers/room

pers/room 0.013



CITY OF OTTAWA

KANATA TOWN CENTRE CENTRAL BUSINESS DISTRICT URBANDALE CORPORATION JLR PROJECT NO.: 15712

Commercial Flow =	50000	L/ł
q residential=	350	l/ca
q hotel =	270	l/ca
q retirement homes =	450	l/ca
i =	0.28	l/s/
SING. HOUSING	3.4	pe
MULT. HOUSING	2.7	ре
Hotel/Appartments	1.8	F
Retirement Homes	1.6	F

Manning's Coefficient (n) =

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Norme Norme <th< th=""><th></th><th></th><th>-</th><th>SING.</th><th>Stacks</th><th>Towns</th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th>FACTOR</th><th></th><th></th><th>AREA</th><th>FLOW</th><th>FLOW</th><th></th><th>FLOW</th><th></th><th>DIA. mm</th><th>SLOPE %</th><th></th><th>VEL. m/s</th><th>LENGTH m</th><th>Residual</th><th>% Full</th></th<>			-	SING.	Stacks	Towns								FACTOR			AREA	FLOW	FLOW		FLOW		DIA. mm	SLOPE %		VEL. m/s	LENGTH m	Residual	% Full	
····································		FROM	то				No units	Act. pop No units	Act. pop. Equ. pop.	people	ha	people	ha		l/s	ha	ha	l/s	l/s	l/s	l/s	l/s						(L/s)		
image image <th< td=""><td></td><td></td><td></td><td></td><td>10</td><td>44</td><td></td><td></td><td></td><td>70</td><td>2.50</td><td>70</td><td>2.56</td><td>4.00</td><td>4.44</td><td></td><td></td><td></td><td></td><td></td><td>0.70</td><td>4.05</td><td>050</td><td></td><td>27.64</td><td>0.77</td><td></td><td>0F 7F</td><td>E0/</td></th<>					10	44				70	2.50	70	2.56	4.00	4.44						0.70	4.05	050		27.64	0.77		0F 7F	E0/	
b b	BELLROCK DRIVE				12																								5% 7%	
····································	EASEMENT					12				52																			7%	
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Marcial Marcial <t< td=""><td>BISHOPS MILLS WAY</td><td>66</td><td>65</td><td></td><td></td><td>9</td><td></td><td></td><td></td><td>24</td><td>0.53</td><td>6374</td><td>53.43</td><td>3.15</td><td>81.22</td><td></td><td>28.98</td><td>25.16</td><td></td><td>163.82</td><td>23.44</td><td>293.64</td><td>825</td><td>0.12</td><td>497.22</td><td>0.93</td><td>62.0</td><td>203.59</td><td>59%</td></t<>	BISHOPS MILLS WAY	66	65			9				24	0.53	6374	53.43	3.15	81.22		28.98	25.16		163.82	23.44	293.64	825	0.12	497.22	0.93	62.0	203.59	59%	
Important Important <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>																														
Distant of the set of	SOUTH of HWY 417	EX.	65							(1) 7792	(1) 191.60	7792	191.60	3.06	96.63				(4) 37.72	37.72	53.65	188.00	900	0.11	600.38	0.94	50.2	412.38	31%	
Distant of the set of				_						-		4 4 4 7 4	0.45.00	0.00	400.00		00.00	05.40		004.54	77.00	404.70			000.00	0.04		405.00	770/	
Network Network <t< td=""><td>BISHOPS MILLS WAY</td><td>65</td><td>64</td><td></td><td></td><td>2</td><td></td><td></td><td></td><td>5</td><td></td><td>14171</td><td>245.03</td><td>2.80</td><td>160.92</td><td></td><td>28.98</td><td>25.16</td><td></td><td>201.54</td><td>77.08</td><td>464.70</td><td>900</td><td>0.11</td><td>600.38</td><td>0.94</td><td>17.0</td><td>135.69</td><td>77%</td></t<>	BISHOPS MILLS WAY	65	64			2				5		14171	245.03	2.80	160.92		28.98	25.16		201.54	77.08	464.70	900	0.11	600.38	0.94	17.0	135.69	77%	
Normal No No No No No<		50	60			8				22	0.50	22	0.50	4 00	0.35						0.14	0.49	200	1.40	38.80	1 24	77.0	38.31	1%	
Cambook Cambook <t< td=""><td></td><td></td><td></td><td></td><td></td><td>-</td><td></td><td></td><td></td><td></td><td>-</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>4%</td></t<>						-					-																		4%	
SHAPP H H H H																							200	0.10			100.0			
Bender Bender<	CAMBRAY LANE	58	61			5				14	0.41	14	0.41	4.00	0.22						0.11	0.33	200	0.70	27.44	0.87	74.5	27.10	1%	
Boom Boom <th< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></th<>																														
Norm Norm Norm Norm No	KETTLEBY STREET	61	64	_		25				68	0.42	162	1.95	4.00	2.63						0.55	3.17	250	0.90	56.41	1.15	105.0	53.24	6%	
Norm Norm Norm Norm No			-	_	<u> </u>			$\left \right $	├ ──┤			4.40.40	040.00	2.00	100 55	├ ───┤	00.00	05.40		201.54	77.00	400.07	μ		600.00	0.01		400.54	700/	
intende intende <t< td=""><td>BISHOPS MILLS WAY</td><td></td><td></td><td>-</td><td></td><td>_</td><td></td><td><u>├ </u></td><td><u> </u></td><td></td><td>0.69</td><td>_</td><td>_</td><td></td><td></td><td> </td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>-</td><td></td><td></td><td></td><td>78% 78%</td></t<>	BISHOPS MILLS WAY			-		_		<u>├ </u>	<u> </u>		0.69	_	_												-				78% 78%	
Norm Norm Norm Norm N		63	5/			10				21	0.08	14309	247.00	2.00	102.00		20.90	20.10		201.04	11.02	407.32	900	0.11	000.38	0.94	64.9	133.00	1070	
Image Image <th< td=""><td>TER, BUNGALOW Ph.2</td><td>51</td><td>53</td><td></td><td>48</td><td></td><td></td><td></td><td></td><td>130</td><td>0.94</td><td>130</td><td>0.94</td><td>4.00</td><td>2.10</td><td></td><td></td><td></td><td></td><td></td><td>0.26</td><td>2.36</td><td>200</td><td>0.70</td><td>27.44</td><td>0.87</td><td>122.3</td><td>25.08</td><td>9%</td></th<>	TER, BUNGALOW Ph.2	51	53		48					130	0.94	130	0.94	4.00	2.10						0.26	2.36	200	0.70	27.44	0.87	122.3	25.08	9%	
Best M					4					11		140	0.94	4.00	2.28						0.26				27.44	0.87			9%	
No. No. No. No. No.		54	55								0.27	140	1.21	4.00	2.28						0.34	2.61	200	0.70	27.44	0.87	36.7	24.82	10%	
Image: state Image: state<	BISHOPS MILLS WAY	55	56	11						37	0.81	178	2.02	4.00	2.88						0.57	3.45	250	0.40	37.61	0.77	107.1	34.16	9%	
Heat Heat <th< td=""><td></td><td>56</td><td>57</td><td>7</td><td></td><td>12</td><td></td><td></td><td></td><td>56</td><td>0.65</td><td>234</td><td>2.67</td><td>4.00</td><td>3.79</td><td></td><td></td><td></td><td></td><td></td><td>0.75</td><td>4.54</td><td>250</td><td>0.60</td><td>46.06</td><td>0.94</td><td>101.5</td><td>41.52</td><td>10%</td></th<>		56	57	7		12				56	0.65	234	2.67	4.00	3.79						0.75	4.54	250	0.60	46.06	0.94	101.5	41.52	10%	
Heat Heat <th< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>-</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></th<>										-																				
IMARC ImarC <th< td=""><td>PARK</td><td></td><td>-</td><td></td><td></td><td>_</td><td></td><td></td><td></td><td></td><td>0.37</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>-</td><td></td><td></td><td></td><td></td><td>78%</td></th<>	PARK		-			_					0.37													-					78%	
i+1 64 64 64 7 8 7 8 7 <td></td> <td>34</td> <td>33</td> <td></td> <td></td> <td>3</td> <td></td> <td></td> <td></td> <td>0</td> <td></td> <td>14013</td> <td>250.70</td> <td>2.79</td> <td>105.14</td> <td></td> <td>20.90</td> <td>25.16</td> <td></td> <td>201.54</td> <td>/0.0/</td> <td>470.51</td> <td>900</td> <td>0.11</td> <td>600.36</td> <td>0.94</td> <td>50.3</td> <td>129.07</td> <td>78%</td>		34	33			3				0		14013	250.70	2.79	105.14		20.90	25.16		201.54	/0.0/	470.51	900	0.11	600.36	0.94	50.3	129.07	78%	
i+1 i+1< i+1 i+1 i+1 i+1<	HAWKSTONE	43	44		22					59	1.19	59	1.19	4.00	0.96						0.33	1.30	250	1.00	59.46	1.21	51.0	58.17	2%	
BRENALE 64 53 74 74 74 74 74 75 75 75 7											-																		4%	
M M	ENDENVALE	45	35								0.08	81	1.36	4.00	1.31						0.38	1.69	250	0.50	42.05	0.86	39.8	40.35	4%	
1 1	BIRKENDALE DRIVE	35	36	7							-	_			-								250	0.37			93.2		7%	
Image Image <th< td=""><td></td><td></td><td>-</td><td>-</td><td></td><td></td><td></td><td></td><td></td><td></td><td>0.79</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>9%</td></th<>			-	-							0.79																		9%	
Image: Since of the s		37	33	2	-	3				15		164	3.33	4.00	2.66						0.93	3.59	250	0.40	37.61	0.77	17.9	34.02	10%	
Markan Markan<		20				10				27	0.56	14804	254 59	2 78	166.96		28.08	25.16		201.54	79.76	473.42	000	0.44	600.38	0.94	70.7	126.07	79%	
1 1	BIRKENDALE DRIVE	33	32			10				21	0.50	14004	234.33	2.70	100.30		20.30	23.10		201.34	13.10	473.42	900	0.11	000.30	0.34	12.1	120.37	1370	
11 12 13 14 15 <th< td=""><td>TEESWATER STREET</td><td>30</td><td>31</td><td></td><td></td><td>16</td><td></td><td></td><td></td><td>43</td><td>0.66</td><td>43</td><td>0.66</td><td>4.00</td><td>0.70</td><td></td><td></td><td></td><td></td><td></td><td>0.18</td><td>0.88</td><td>250</td><td>0.40</td><td>37.61</td><td>0.77</td><td>75.1</td><td>36.72</td><td>2%</td></th<>	TEESWATER STREET	30	31			16				43	0.66	43	0.66	4.00	0.70						0.18	0.88	250	0.40	37.61	0.77	75.1	36.72	2%	
Image: Single		31	32			19				51	0.41	95	1.07	4.00	1.53						0.30	1.83		0.40	37.61	0.77	77.9	35.78	5%	
Image: Single																														
N N	BIRKENDALE STREET			_	<u> </u>						0.37																		79%	
COLCHESTER SQUARE 17 16 1		18	16	+		4				11	+	14926	256.03	2.78	168.11		28.982	25.16		201.54	80.16	474.97	900	0.11	600.38	0.94	44.4	125.41	79%	
COLCHESTER SQUARE 17 16 1		40		-			1	+ + +			+		+	4 00		0.52	0.52	0.45		<u> </u>	0.15	0.60	450	0.00	14.45	0 85		13.85	4%	
COLCHESTER SQUARE 16 15 16 15 16 10 10 16 16 15 16 10											0.10	-	0.10			0.02													2%	
1114A1211<11111111111111111111111	SOLUTED LIN BRUARE	17	10					<u>}</u>			0.10		5.10		1		5.02	0.10		l			200	0.40			33.2	20.00		
1114A11211<	COLCHESTER SQUARE	16	15		1	10				27	0.56	14953	256.69	2.78	168.37		29.50	25.61		201.54	80.49	476.01	900	0.11	600.38	0.94	66.0	124.37	79%	
28 24 18 18 10 10 10 10 110						2				5		14958	256.69	2.78	168.42		29.50	25.61		201.54	80.49	476.06			600.38	0.94		124.32	79%	
28 24 18 18 10 10 10 10 110																							Ц							
24 23 12 <t< td=""><td>ELSINORE LANE</td><td></td><td></td><td>_</td><td></td><td> </td><td></td><td><u>↓ </u></td><td> </td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td> </td><td> </td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>3%</td></t<>	ELSINORE LANE			_		 		<u>↓ </u>																					3%	
LSINGE LANE 230 306 8 8 9								+																						
Income and and and and and and and an analysis of the state				-			1	+ + +								├ ──┤				<u> </u>									9% 9%	
A MA M MA					0					~~~~																			9%	
			175		1	1		<u>† </u>			0.10				2.00				1			5.00	200	0.43		2100	40.4	27.00		
	COLCHESTER SQUARE	14 A	14		1		1				1	15147	259.52	2.77	170.21		29.50	25.61		201.54	81.29	478.65	900	0.11	600.38	0.94	14.7	121.74	80%	
Church 14 4.0 0.52 0.52 0.45 0.15 0.60 15.0 1.00 15.23 0.86 35.0 14.63																														
		Church	14											4.00		0.52	0.52	0.45			0.15	0.60	150	1.00	15.23	0.86	35.0	14.63	4%	
								1				I						l	J											

MASTER SANITARY SEWER DESIGN SHEET Designed: L.D.

2017 Update by: KF 2017 Check by: LD

Date: August 15, 2017

./ha/d

- cap/d
- cap/d
- cap/d
- s/ha
- ers/hse
- ers/hse
- pers/room

0.013

pers/room



CITY OF OTTAWA

KANATA TOWN CENTRE CENTRAL BUSINESS DISTRICT URBANDALE CORPORATION JLR PROJECT NO.: 15712

Commercial Flow = 50000 L/ha/d q residential= 350 q hotel = 270 q retirement homes = 450 i= 0.28 SING. HOUSING 3.4 pers/hse MULT. HOUSING 2.7 Hotel/Appartments 1.8 **Retirement Homes** 1.6

Manning's Coefficient (n) = 0.013

																								2017 Updates		5, West of 9 P	eak Flows	CAPACITY	
1									RESIDENTIAL			1					ERCIAL / INSTIT		PLUGG	ED FLOW		R+C			SEWER	DATA		CAP	PACITY
STREET	M.H. #	ŧ					UMBER OF			1		_		-	POPUL.	Actual	CUMM.	COMM.		CUMM.		. PEAK DES.	11		CAPAC.				
			SING.	Stacks Town		t. Care		Hotel/Apar		POPUL.		1		FACTOR	FLOW	AREA	AREA	FLOW	FLOW	FLOW	FLOW	FLOW	DIA. mm	SLOPE %	//s	VEL. m/s	LENGTH m	Residual	I % Full
	FROM	TO			No units	Act. pop	No units	Act. pop.	Equ. pop.	people	ha	people	ha		l/s	ha	ha	l/s	l/s	l/s	l/s	i/s						(L/s)	
COLCHESTER SQUARE	14	11		4						11	0.16	15158	259.68	2.77	170.31		30.02	26.06		201.54	81.48	479.39	900	0.11	600.38	0.94	72.6	120.99	80%
TERON	11	10										15158	259.68	2.77	170.31		30.02	26.06		201.54	81.48	479.39	900	0.11	600.38	0.94	29.6	120.99	80%
	10	EX.									0.25	15158	259.93	2.77	170.31		30.02	26.06		201.54	81.55	479.46	900	0.11	600.38	0.94	72.3	120.92	80%
TERON	O.P.P.	EX.												4.00					0.78	0.78		0.78	100	Forcemain					
			_	+								15159	259.93	2.77	170.31		30.02	26.06		202.32	81.55	480.24	H		000.04	0.01		050.07	
TERON	EX.	EX. 2		-								15158	259.93	2.77	170.31		30.02	26.06		202.32	81.55	480.24	680	0.96	838.61	2.31	9.4	358.37	57%
			(1)																				#						
			- (')	As per Kanata	Town Centre S	anitary Trunk	Sewer Study,	revised Marc	h 27, 1996, by	Robinson Cons	ultants Inc.				-								/}───			+		<u> </u>	
]																										
			(2)	Park or open	space area.										+					+			₩						
			(3)	Equivalent po	pulation base	e on 208 roor	ns and 20 st	aff member	S.				-	<u> </u>						1			H					<u> </u>	
			(4)	Allowance for Centre Sanita			s to provide f	lexibility in f	uture develop	oment as per K	Kanata Town									-			╂───			-			
			1		-	-																						<u> </u>	1
			(5)	Additional flor laundry as pe						th bathrooms a	and												₩					<u> </u>	<u> </u>
			-	laundry as pe	r design calc	ulations for B	nock i provid	led by WSP	Coclober 20	16)												-	₭───					<u> </u>	+
			(6)							aff, dining and					1								1						
			_	laundry as pe	-		250 Maritime	e Way (Timb	perwalk Retire	ement Home)					1						_		#──						
		_	-	provided by N																₭┼───					<u> </u>	+			
																									<u> </u>	-			
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																							1	1	1			1	



MASTER SANITARY SEWER DESIGN SHEET Designed: L.D.

2017 Update by: KF 2017 Check by: LD

Date: August 15, 2017

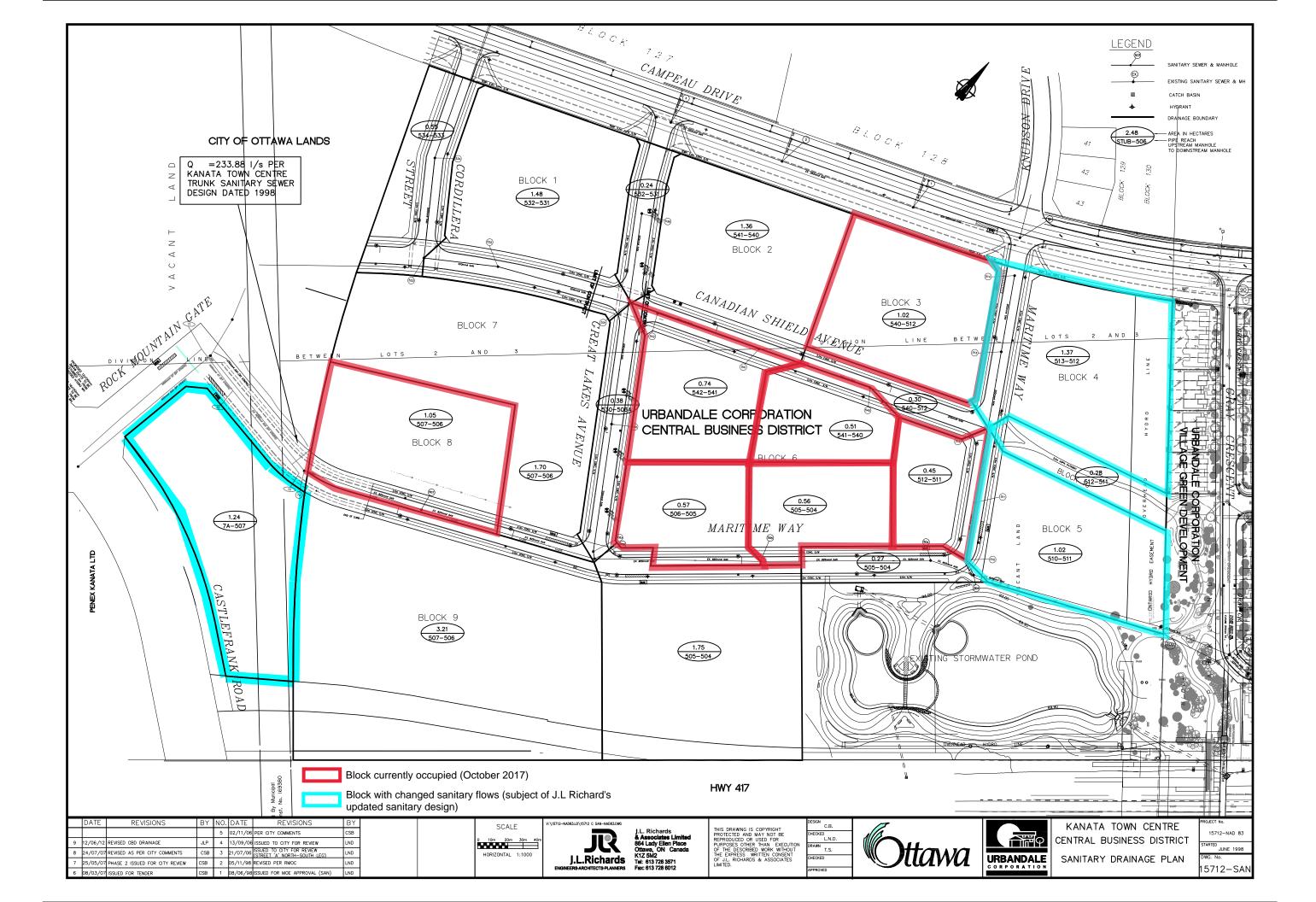
l/cap/d

l/cap/d

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pers/hse

pers/room pers/room



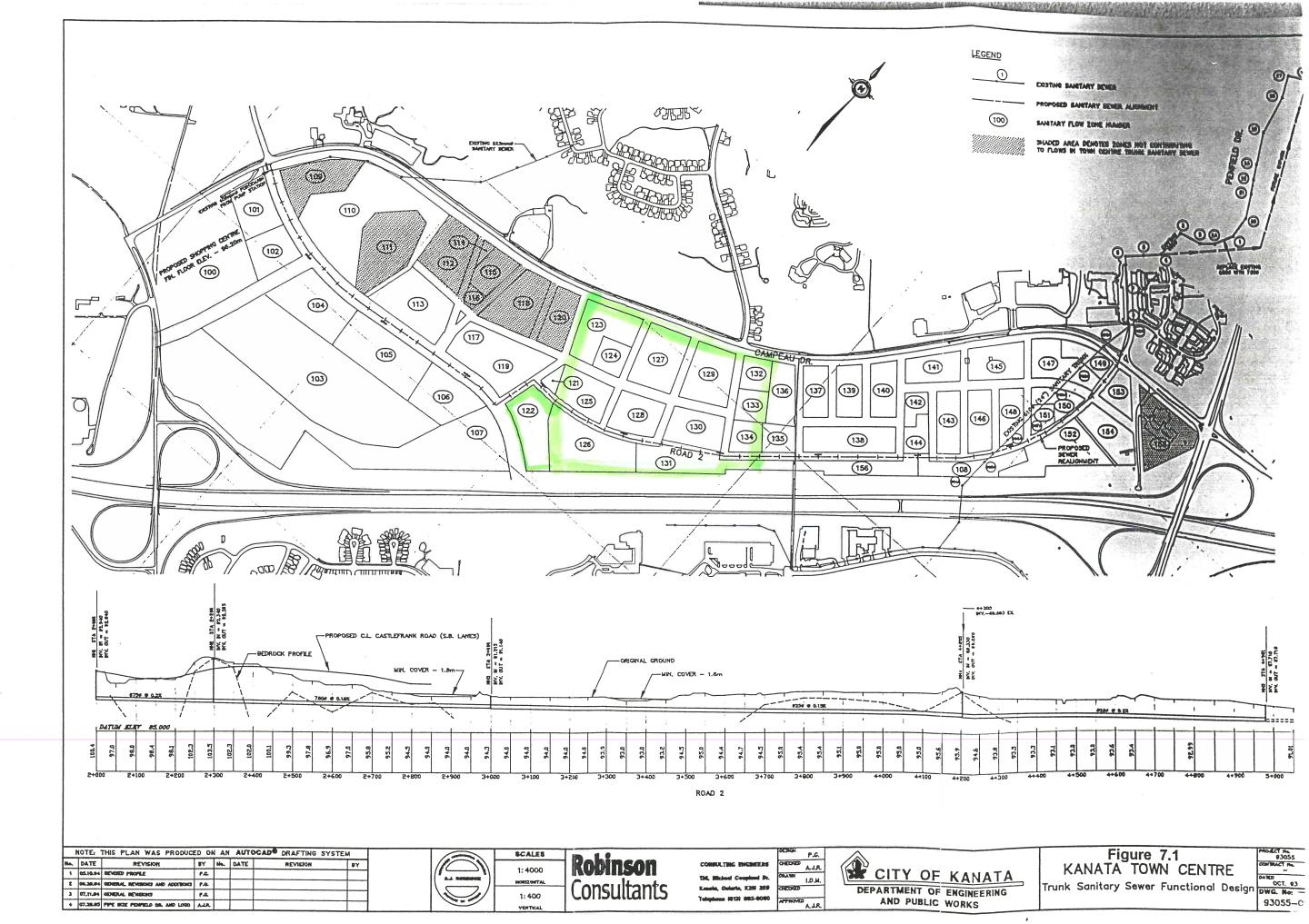


Table 4.7 - Revised as per RMOC Letter Dated March 27, 1996 KANATA TOWN CENTRE SANITARY TRUNK SEWER STUDY

Ultimate Development Flows Worksheet

Average Persons per Dwelling Unit

3.19

, s.

61.7

					Stn. Qp=	163 2 beds/r			q (res)= q (ret)= q (com)= q (hot)= Peaking fact 3.8		I/s x m2 I/s x m2 I/s x bed off & hot=	5000 5000 225 1.5	cu. m/ca l/1000m2 l/1000m2 l/bed x da	2 x day 2 x day	
					=		l/s/ha			persons/dw					
one	Area	Resident	ial Units	5	Retail		Office		Special Gen.		Peaking	Qp	Qi	Qtot	Cummu
		Low	Med	High	GLA (m2)	Emp.	Area (m2)	Emp.	Hotel Rooms	Emp.	Factor	(l/s)	(l/s)	(l/s)	Qtot (1/s
			0												
112	1.6		100		2230	47	5574	200							
111	2.2	1. A.S					Sec. Sec.							e e Pri	1
109	2.2		33				and the second		200	88					1 · · ·
115	0.8						1394	50	Sec. Same						
116	0.20													lise an	100.0
114	0.10														
118	1.7			50			9755	350							- 15g
120	1.1		87					1. A.	A Second		- markers		100.00		1.25
100	7.40		0 din 880.7 i	1000000000708.co	16908	386					4.00	1.47	2.07	3.54	166.6
101	1.30				4041	87					4.00	0.35	0.36	0.71	167.3
102	0.80				1579	34					4.00	0.14	0.22	0.36	167.7
104	1.50			168	10080	217					4.00	6.86	0.42	7.28	174.9
110	8.20		300	.00	10000	~					3.68	16.98	2.30	19.28	193.
103	13.30				74459	1603					3.68	6.46	3.72	10.19	203.9
105	2.10			90	8826	190					3.64	3.68	0.59	4.27	203.3
105	1.50	,		50	3298	71					3.64	0.29	0.42	0.71	208.7
117	0.04				3290	71					3.64	0.29	0.42	0.01	208.
	2.60		- 1	100	2230	47	34838	1250			3.60	6.42	0.01	7.15	
119				100	2230	4/	34030	1200	100	88		0.42			215.
107	9.10								100	88	3.60		2.55	3.33	218.
113	2.10			300	2230	47	16722	600			3.50	10.99	0.59	11.58	229.
121	0.10						19509	700			3.50	1.69	0.03	1.72	231.
122	1.50				_		27870	1000			3.50	2.42	0.42	2.84	233.
123	1.70	Concession of the local division of the loca	72	50			1394	50			3.45	5.48	0.48	5.95	239.3
124	0.60							-			3.45	0.00	0.17	0.17	239.
125	1.40										3.45	0.00	0.39	0.39	239.
126	2.80										3.45	0.00	0.78	0.78	240.6
127	1.80		80				4181	150			3.41	4.56	0.50	5.07	245.2
128	1.20		36				4181	150			3.39	2.24	0.34	2.58	247.6
129	1.70		70				6968	250			3.37	4.23	0.48	4.71	251.
130	1.10						11148	400			3.37	0.97	0.31	1.28	253.2
131	2.00										3.37	0.00	0.56	0.56	253.
132	0.60		40			-					3.35	2.06	0.17	2.23	255.
133	0.60										3.35	0.00	0.17	0.17	255.
134	0.70		Surger Street of the local division of the l				4181	150	-		3.35	0.36	0.20	0.56	256.
135	0.60		36								3.34	1.85	0.17	2.02	258.
136	1.00		18								3.33	0.92	0.28	1.20	259.
137	0.80	10	18								3.32	1.43	0.22	1.65	260.
138	1.50		93								3.29	4.71	0.42	5.13	265.
139	0.80	18	8								3.28	1.31	0.22	1.54	266.
156	1.10	"I	37								3.27	1.86	0.31	2.17	268.
140	0.90	8	27								3.26	1.75	0.25	2.01	270.
141	1.00	l vl	59								3.24	2.94	0.28	3.22	273.
142	0.50		59								3.24	0.00	0.14	0.14	273.
	0.50		24		1						3.24	1.69	0.14	1.86	275.
144			34								3.23	1.98	0.17	2.29	275.
143	1.10	10	30											4.88	281.
145	1.30		92								3.19	4.52	0.36	4.88	281.
146	1.00	16	19				1				3.18	1.71	0.28		285.
108	1.20		34								3.17	1.66	0.34	2.00	
148	1.00	8	18								3.17	1.27	0.28	1.55	286.
150	0.70		11								3.16	0.54	0.20	0.73	287.
151	0.30										3.16	0.00	0.08	0.08	287.
152	2.00		. 1								3.16	0.00	0.56	0.56	287.
	1.20	1 I	66								3.15	3.20	0.34	3.53	291.
154		I					3177	114			3.15	0.28	0.50	0.78	291.
	1.80														294.
154	1.80 1.30		49								3.13	2.36	0.36	2.73	
154 155			49	100							3.13	2.36 2.78	0.36	3.00	296.
154 155 147	1.30		49	100	1858	39								2.73 3.00 0.33	

Combined Down Stream Flow

425.64

J.L.RICHARDS & ASSOCIATES LIMITED, Consulting Engineers, Architects & Planners

q (res) = 350 q (com) = 50,000 q (inst) = 50,000

I = 0.280

l/cap/day l/ha/day l/ha/day

l/s/ha

1. 1. 1. 1. 1.

× × ×,

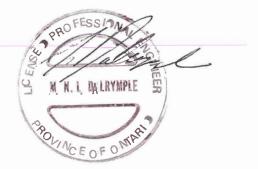
SANITARY SEWER DESIGN SHEET

KANATA TOWN CENTRE (RESIDENTIAL)

CITY OF KANATA

= Singles, Townhouses, Ter. Bungalows =	0.280 3.8	l/s/ha pers / unit	(low & medium	donaity				TOWN						Designed by:	L∗N.D.
Stacked Townhouses / Apartments =	2.2	pers / unit	、 (high density)	density)		U			AL) RPORATIO	N				Checked by.	M-F.S-
Stacked Townhouses / Apartments =	80	units / ha	(high density)												
STREET		.H. #	Singles &	UNITS Stacked	AREA	CUMMU POPUL.	AREA	Peaking Factor	POPUL. FLOW	INFIL. FLOW	PEAK FLOW	DIA	Slope	SEWER DATA CAPAC.	VEL.
	FROM	то	Townhouses	Townhouses	ha	peop.	ha	4.00	I/s	I/s	1/5	mm	%	I/s	m/s
A	90 92	92: 94	37 13		0.80	141 190	0.80	4.00	2.28	0.22	2.50 3.64	250 250	0.60 2.20	46.06 88.20	(
	94 95	95 89	10		66.80 0.52	4831 4869	68.79 69.31	3.26 3.26	63.77 64.21	19.26 19.41	270.61 271.20	825 825	0.12 0.12	497.22 497.22	(
В	85 87	87 89	19		1.19 0.82	72	1.19	4.00 4.00	1.17 2.77	0.33 0.56	1.50	250 250	0.40	37.61	(
A	89	84	26		0.35	171 5085	2.01 71.67	3.24	66.71	20.07	3.33 274.35	825	1.41	70.70	
c	80	82	20		1.08	76	1.08	4.00	1.23	0.30	1.53	250	0.12	497.22 37.61	
, , , , , , , , , , , , , , , , , , ,	82	84	28		0.83	182	1.91	4.00	2.96	0.53	3.49	250	1.20	65.18	
A	84	79	14		0.54	5321	74.12	3.22	69.40	20.75	277.74	825	0.12	497.22	
D	75 76	76 77	19 20		0.37 0.29	72 148	0.37 0.66	4.00 4.00	1.17 2.40	0.10 0.18	1.27 2.59	250 250	0.40	37.61 37.61	
	77	79	14		0.63	201	1.29	4.00	3.26	0.36	3.62	250	0.81	53.66	
PARK EASEMENT	79 67	67 66	6		0.98 0.33	5522 5545	76.39 76.72	3.20 3.20	71.69 71.95	21.39 21.48	280.66 281.01	825 825	0.12 0.12	497.22 497.22	
BELLROCK DRIVE	70	73	26		2.56	99	2.56	4.00	1.60	0.72	2.32	250	0.40	37.61	
EASEMENT	73 74	74 62	10		0.54 0.31	137 137	3.10 3.41	4.00 4.00	2.22 2.22	0.87 0.95	3.08 3.17	250 250	0.40 0.40	37.61 37.61	
	62	66	25		0.48	232	3.89	4.00	3.76	1.09	4.85	250	0.77	52.18	
BISHOPS MILLS WAY	66 EX	65	9		0.53	5811	81.14 191.60	3.18	74.95	22.72	285.25	825	0.12	497.22	
SOUTH of HWY 417 BISHOPS MILLS WAY	EX. 65	64	2		191.60	13610	272.74	3.06 2.82	96.63 155.52	53.65 76.37	188.16 457.35	900	0.11	600.38	
EDENVALE DRIVE	59	60	8		0.50	30	0.50	4.00	0.49	0.14	0.63	200	0.11	600.38	
KETTLEBY STREET	60	61	24	-	0.62	122	1.12	4.00	1.97	0.31	2.28	250	1.40 0.40	38.80 37.61	(
CAMBRAY LANE	58	61	8		0.41	30	0.41	4.00	0.49	0.11	0.61	200	0.70	27.44	
KETTLEBY STREET	61	64	25		0.42	247	1.95	4.00	4.00	0.55	4.55	250	0.90	56.41	
BISHOPS MILLS WAY	64 63	63 57	3 10		0.68	13869 13907	274.69 275.37	2.81 2.81	158.01 158.38	76.91 77.10	460.38 460.94	900 900	0.11	600.38 600.38	
TER. BUNGALOW Ph.2	51	53	48		0.94	182	0.94	4.00	2.96	0.26	3.22	200	0.70	27.44	
	53 54	54 55	4		0.27	198 198	0.94 1.21	4.00 4.00	3.20 3.20	0.26 0.34	3.47 3.54	200 200	0.70 0.70	27.44 27.44	
BISHOPS MILLS WAY	55 56	56 57	11 19		0.81 0.65	239 312	2.02 2.67	4.00 4.00	3.88 5.05	0.57 0.75	4.44 5.80	250 250	0.40 0.60	37. ⁶ 1 46.06	
PARK	57 34	34 33	1		0.37	14222	278.41	2.80 2.80	161.40	77.95 77.95	464.82	900	0.11	600.38	
HAWKSTONE	43	44	3 16		1.19	14234	278.41	4.00	161.51 0.99	0.33	464.93 1.32	900 250	0.11	600.38 59.46	
ENDENVALE	44 45	45	8		0.09	91	1.28	4.00	1.48	0.36	1.84	250 250 250	0.50	42.05	
BIRKENDALE DRIVE	35 36	36 37	7 13		1.18	118	2.54	4.00 4.00	1.91 2.71	0.71	2.62 3.64	250 250	0.37	36.18 36.09	
	37	33	2		0.00	175		4.00	2.83	0.93	3.76	250	0.40	37.61	
BIRKENDALE DRIVE	33	32	13		0.56	14458	282.30	2.79	163.66	79.04	468.16	900	0.11	600.38	
TEESWATER STREET	30 31	31 32	18 19		0.66 0.41	68 141	0.66 1.07	4.00 4.00	1.11 2.28	0.18 0.30	1.29 2.58	250 250	0.40 0.40	37.61 37.61	
BIRKENDALE STREET	32	18	4		0.37	14614	283.74	2.79	165.14	79.45	470.05	900	0.11	600.38	
COMMERCIAL PLAZA	18	16	6		0.50	14636	283.74	2.79	165.36	79.45	470.27	900	0.11	600.38	
COLCHESTER SQUARE	19 17	16			0.52 0.10	0	0.52 0 62	1.50 4.00	0.45 0.45	0.15 0.17	0.60 0.62	150 250	0.90 0.40	14.45 37.61	
COLCHESTER SQUARE	16 15	15 14 A	10 2		0.56	14674 14682	284 92 284 92	2.79 2.79	166.17 166.25	79.78 79.78		900	0.11	600.38 600.38	(
ELSINORE LANE	39	28	22		0.53	84	0.53	4.00	1.35	0.15	1.50	250	1.00	59.46	
	28 24	24 23	14 12		1.47 0.14	137	2.00	4.00	2.22 2.96	0.56 0.60	2.78	250 250	0.40	37.61	
ELSINORE LANE ENDENVALE DRIVE	23 306	306 14 A	8		0.24 0.45	213 213	2.38 2.83	4.00 4.00	3.45 3.45	0.67 0.79	4.11	250 250	0.44	39.41 41.68	
COLCHESTER SQUARE	14 A	14				14895	287.75	2.78	167.82	80.57	473.85	900	0.11	600.38	
	Church	14			0.52	0		1.50	0.45	0.15		150	1.00	15.23	
COLCHESTER SQUARE	14	11	4		0.16	14910		2.78	168.87	80.76		900	0.11	600.38	
TERON	11 10	10 EX.			0.25	14910 14910	288.43 288.68	2.78 2.78	168.87 168.87	80.76 80.83		900 900	0.11 0.11	600.38 600.38	
TERON	OPP.	EX.									0.78	100	Forcemain		
TERON	EX.	EX									475.94	680	0.96	838.61	:

'EL. n/s	LENGTH m
0.94	120.0
1 80	103.0
0.93 0.93	17.5 66.6
0.77 1.44	116.9 116.7
0.93	79.0
0.77 1.33	120.0 118.5
0.93	79.0
0.77	57.0
0.77 1.09	78.4 117.7
0.93 0.93	55.0 70.0
0.77	87.2
0.77 0.77 1.06	60.3 39.9 100.5
0.93	62.0
0.94	50.2
0.94	17.0
1.24 0.77	77.0 103.6
0.87	74.5
1.15	105.0
0.94 0.94	13.0 64.9
0.87 0.87	122.3 13.6
0.87	36.7
0.77 0.94	36.7 107.1 101.5
0.94 0.94	53.5 50.3
1.21 0.85	51.0 29.0
0.86	39.8
0.74 0.74 0.77	93.2 77.1 17.9
0.94	72.7
0.77 0.77	75.1 77.9
0.9 <i>4</i> 0.94	44.4
0.8.2	26.5
0.77 0.94	33.2 66.0
0.94	25.8
1.21 0.77	56.7 43.0
0.77 0.80	34.0 48.8
0.85	46.4
0.91	14.7
0.86	35.0 72.6
0.94 0.94 0.94	29.6
2.31	9.4



Karla Ferrey

From: Sent: To: Subject: Attachments: Lucie Dalrymple August 1, 2017 9:43 AM Karla Ferrey FW: Kanata Town Centre - Sanitary Flows 1088 San Drainage.pdf; 1136 San Drainage.pdf

...here it is

Lucie Dalrymple, P.Eng. Associate Senior Civil Engineer

J.L. Richards & Associates Limited 864 Lady Ellen Place, Ottawa, ON K1Z 5M2 Tel: 613-728-3571 Fax: 613-728-6012

J.L. Richards & Associates Limited ENGINEERS · ARCHITECTS · PLANNERS



From: Matthew Hrehoriak [mailto:m.hrehoriak@novatech-eng.com] Sent: July 31, 2017 10:16 AM To: Lucie Dalrymple Subject: RE: Kanata Town Centre - Sanitary Flows

Hi Lucie,

The sanitary info for the block 4 and 5 developments are as follows:

1088 Maritime Way (Block 4)

San service connection between SANMH 512-513 San Drainage Area = 1.121 ha No. Units = 144 Population = 271

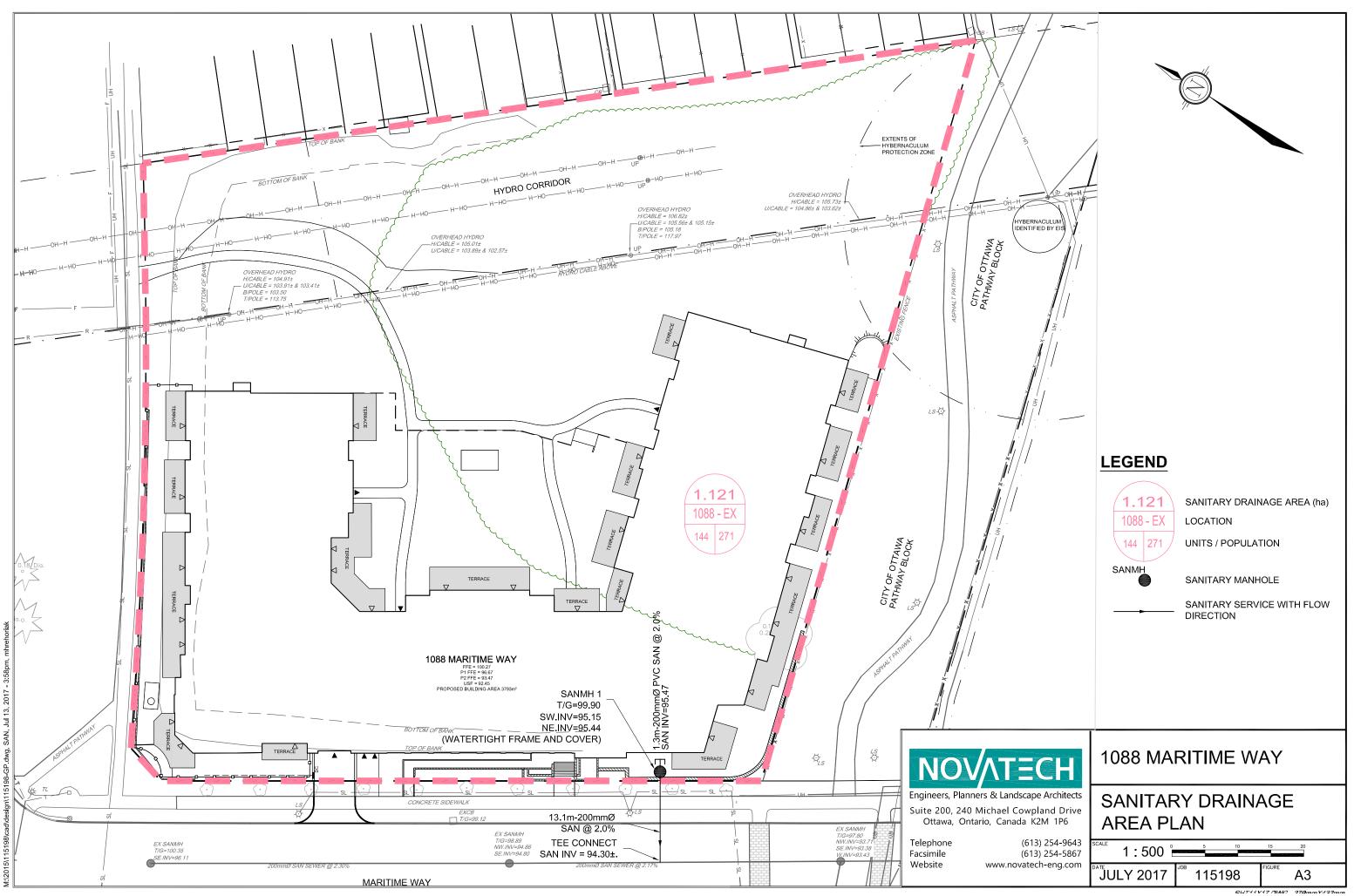
1136 Maritime Way (Block 5)

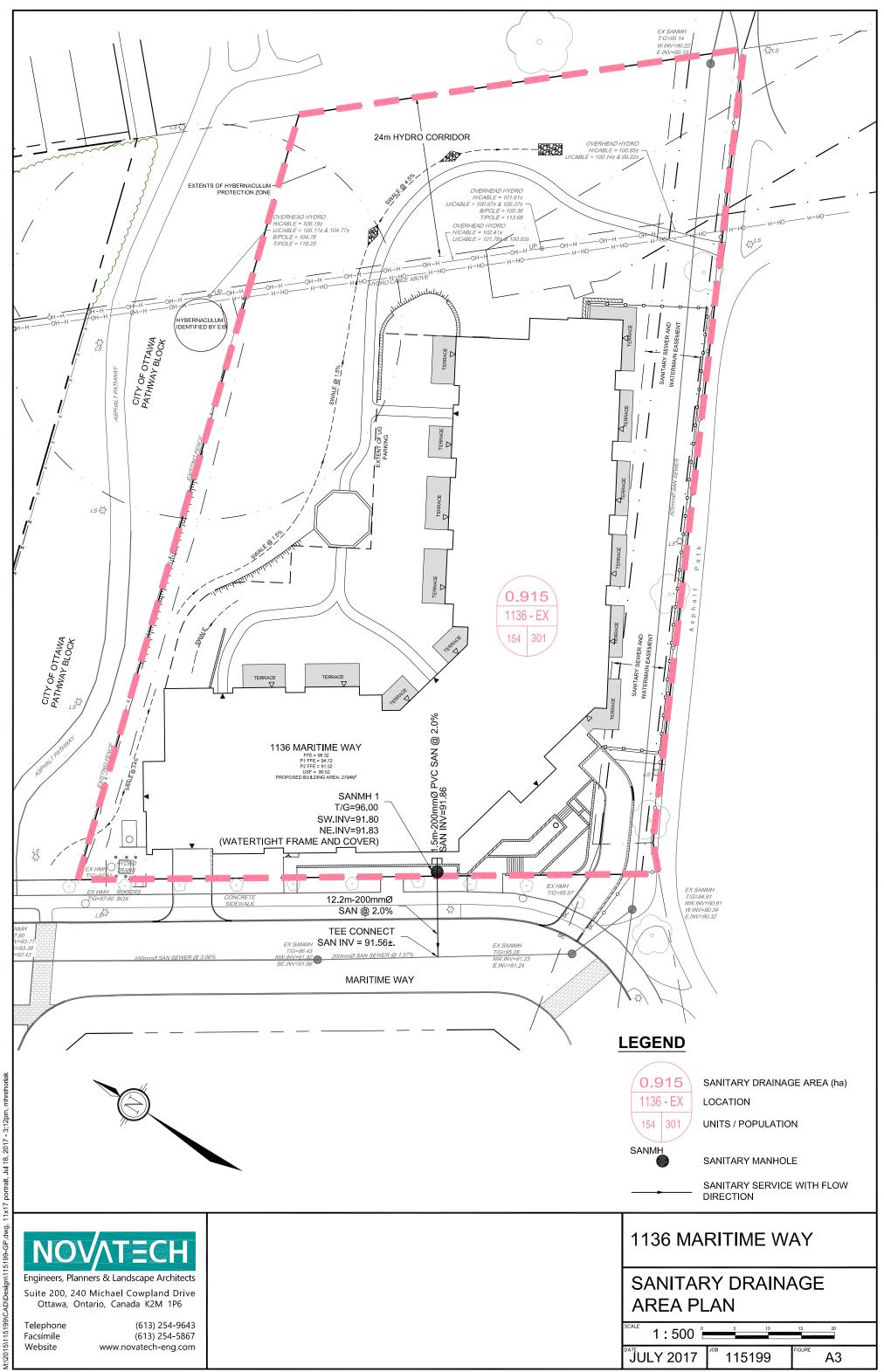
San service connection between SANMH 510-511 San Drainage Area = 0.915 ha No. Units = 154 Population = 301

Regards,

Matthew Hrehoriak, B.Eng., EIT

NOVATECH Engineers, Planners & Landscape Architects 240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 x 273 | Fax: 613.254.5867 The information contained in this email message is confidential and is for exclusive use of the addressee.





SHT11X17.DWG - 279mmX432mm

SANITARY SEWER DESIGN SHEET 1250 Maritime Way Timberwalk Retirement Home Developer: Claridge Homes

Date: 31-Jul-17

Designed: CMS Checked: GJM

Locatio	n		RESIDENTIAL									INSTITUTIONAL COMMECIAL								OTHER													PIPE					
			1 Be	droom	2 Be	droom	Тс	otal (R	esidenti	al)		Assist	ed Care		Con	venience	Store		Staff		Be	eauty Sal	lon	1	aundry			Dining			Infilt.	Total			\square		Full	
ID	ID From To	Units	Pop.	Pop. Units Pop.		o. Pop				Units / Bed	Pop.	Peak Factor	Flow (L/s)			Flow (L/s)	Pop.	Peak Factor	Flow (L/s)	Stations	Peak Factor	Flow (L/s)	Machines	Peak Factor	Flow (L/s)	Seats	Peak Factor	Flow (L/s)	Total Area (ha)		Flow (L/s)	Size (mm)	Slope (%)	Length (m)	Capacity (I/s)	Flow Vel. (m/s)	Q/Q _{fu} (%)	
Part A (current application)	BLD-1	MH101	92	129.0	8	17.0	0 146.	0	4.0	2.37	54	60.0	1.5	0.47	50	1.5	0.004	20	1.5	0.10	2	1.5	0.02	6	1.5	0.13	55	1.5	0.11	0.48	0.13	3.33	200	2.00	2.5	48.4	1.49	6.9%
Part A (current application)	MH101	TEE-1	0	0.0	0	0.0	146.	0	4.0	2.37	0	0.0	1.5	0.47	0	1.5	0.004	0	1.5	0.10	0	1.5	0.02	0	1.5	0.13	0	1.5	0.11	0.00	0.13	3.33	200	2.00	13.4	48.4	1.49	6.9%
		-																	-					•					-									
Part B (future application)	BLD-2	MH103	0	0.0	110	231.	.0 231.	0	4.0 ;	3.74	0	0.0	1.5	0.00	0	1.5	0.000	0	1.5	0.00	0	1.5	0.00	0	1.5	0.00	0	1.5	0.00	0.41	0.11	3.86	200	2.00	2.5	48.4	1.49	8.0%
Part B (future application)	MH103	TEE-2	0	0.0	0	0.0	231.	0	4.0 3	3.74	0	0.0	1.5	0.00	0	1.5	0.000	0	1.5	0.00	0	1.5	0.00	0	1.5	0.00	0	1.5	0.00	0.00	0.11	3.86	200	2.00	13.4	48.4	1.49	8.0%
	1			1		1					E									1			1	•	1	1		1	•		1		<u>.</u>					
TOTAL (Parts A + B)	-	-	92	129.0	118	248.	.0 377.	0	4.0	6.11	54	60.0	1.5	0.47	50	1.5	0.004	20	1.5	0.10	2	1.5	0.02	6	1.5	0.13	55	1.5	0.11	0.89	0.25	7.18	200	2.00	2.5	48.4	1.49	14.89
Design Parameters: Residential Institutional Commercial Staff Beauty Salon Laundy Dining Infiltration	45) 275 650 1200 115	0 L/cap/da 0 L/bed/da 5 L/m ² pe 5 L/cap/da 0 L/day pe 0 L/day pe 5 L/seat/d 3 L/s/ha	ay day ay er station er machir		Peakin Reside Instituti Commo Other	ntial onal		on Equ	uation (m	nax 4, mi		1.40 2.10	Jnit: Assisted 1 Bedroo 2 Bedroo Studio	m																								
lotes: . The harmon peaking factor . Residential flows were used . Institutional flow used for as	for senior a	apartment	s (350 L/	cap/day.	Harmon	Peaking	Factor)	ed Oct	ober 12th	n, 2016																												

Institutional now used for assisted care units (450 L/bed/day, Peaking Factor = 1.5)
 Future building assumed to be a 10 storey building comprised of 110 2 bedroom units



END OF J.L RICHARDS MEMORANDUM

Matthew Linton

From: Sent: To: Cc: Subject: Mike Traub <mike.traub@claridgehomes.com> July-13-17 1:12 PM Matthew Linton Pascal Vendette; Conrad Stang Re: FW: 1250 Maritime Way - Sanitary

Hi Matt,

There will be two stations in the hair salon and about 55 seats in the main dining room.

Let me know if you have any further questions.

Thanks,

Mike

On Wed, Jul 12, 2017 at 4:40 PM, Matthew Linton <m.linton@novatech-eng.com> wrote:

Pascal/Mike,

Could we obtain some clarifications on the following below? This is for city comments as they are stating our assumed values seem low.

Thanks,

Matthew Linton, CAD Technologist

NOVATECH Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 Ext: 207 | Fax: 613.254.5867

The information contained in this email message is confidential and is for exclusive use of the addressee.

From: Conrad Stang Sent: July-12-17 3:55 PM

Matthew Linton

From: Sent: To: Subject: Pascal Vendette <pascal@neufarchitectes.com> May-01-17 1:39 PM Matthew Linton RE: Unit Counts - Timberwalk (Maritime Way)

2nd floor : 28 assisted care units 3rd floor : 26 assisted care units 4th to 7th floor : 25 units 6 suites 17 1br 2 br



PASCAL VENDETTE

Technologue senior en architecture Senior Architectural Technologist T 514 847 1117 #269 F 514 847 2287 C 514 833 6005 630, boul. René-Lévesque O. 32° étage, Montréal (QC) H3B 1S6 47 Clarence Street, suite 406, Ottawa (ON) K1N 9K1 **NEUF ARCHITECTES** SENCRL

Politiques de transmission et de confidentialité de NEUF architect(e)s NEUF architect(e)s transmission and confidentiality policy

De : Matthew Linton [mailto:m.linton@novatech-eng.com] Envoyé : 1 mai 2017 11:21 À : Pascal Vendette <pascal@neufarchitectes.com> Objet : Unit Counts - Timberwalk (Maritime Way)

Pascal,

Can you please send us over either the calculated dwelling units (I see you have the dwelling units on drawing A050 however we need to know 1 bedroom, 2 bedroom, etc.) or floor plans for each of the floors for the retirement home?

Thanks,

Matthew Linton, CAD Technologist

NOVATECH Engineers, Planners & Landscape Architects 240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 Ext: 207 | Fax: 613.254.5867 The information contained in this email message is confidential and is for exclusive use of the addressee.

Matthew Linton

From:	Pascal Vendette <pascal@neufarchitectes.com></pascal@neufarchitectes.com>
Sent:	July-13-17 9:49 AM
To:	Matthew Linton
Cc:	mike.traub@claridgehomes.com; Conrad Stang
Subject:	RE: 1250 Maritime Way - Sanitary
Follow Up Flag:	Follow up
Flag Status:	Completed

Hi Matthew. Sorry for the dealy ... it's crazy here.

Here is my response to item #1.

There are commercial washers-dryers in the basement ...

- 2 gas heater tumble dryers 75 lbs
- 1 high-performance washer extractor 65 lbs
- 1 cabinet hardmount washer extractor 20 lbs

... and residential type washer-dryer (one of each) on floors 4 to 7

Mike can you please take care of items #2 & #3.

Best regards,



PASCAL VENDETTE

Technologue senior en architecture Senior Architectural Technologist T 514 847 1117 #269 F 514 847 2287 C 514 833 6005 630, boul. René-Lévesque O. 32° étage, Montréal (QC) H3B 1S6 47 Clarence Street, suite 406, Ottawa (ON) K1N 9K1 **NEUF ARCHITECTES** SENCRL

Politiques de transmission et de confidentialité de NEUF architect(e)s NEUF architect(e)s transmission and confidentiality policy

De : Matthew Linton [mailto:m.linton@novatech-eng.com]
Envoyé : 13 juillet 2017 09:38
À : Pascal Vendette <pascal@neufarchitectes.com>
Cc : mike.traub@claridgehomes.com; Conrad Stang <c.stang@novatech-eng.com>
Objet : RE: 1250 Maritime Way - Sanitary

Pascal,

Can we have some clarification on this?

SANITARY SEWER DESIGN SHEET 1250 Maritime Way Timberwalk Retirement Home Developer: Claridge Homes

Date: 30-Nov-17

Designed: CMS Revised: JDM Checked: GJM

Location	n				RE	SIDENT	ΓIAL			IN IN	ISTITU	JTIONA	L	CC	MMEC	IAL	OTHER												INFILTR	ATION		PIPE					
			1 Bec	lroom	2 Bec	droom	Tota	l (Resider	ntial)		Assist	ed Care		Conv	enience Store			Staff		Beauty Salon			L	aundry			Dining			Infilt.	Total					Full	
ID	From	То	Units	Pop.	Units	Pop.	Pop.	Peak Factor	Flow (L/s)	Units / Bed	Pop.	Peak Factor	Flow (L/s)	Area (m2)	Peak Factor	Flow (L/s)	Pop.	Peak Factor	Flow (L/s)	Stations	Peak Factor	Flow (L/s)	Machines	Peak Factor	Flow (L/s)	Seats	Peak Factor	Flow (L/s)	Total Area (ha)	Flow (L/s)	Flow (L/s)	Size (mm)	Slope (%)	Length (m)	Capacity (I/s) 55.8 56.2 48.4 41.9	Flow Q/ Vel. (° (m/s)	
art A (current application)	BLD1	MH4	92	129.0	8	17.0	146.0	4.0	2.37	54	60.0	1.5	0.47	100	1.5	0.009	20	1.5	0.10	2	1.5	0.02	6	1.5	0.13	55	1.5	0.11	0.48	0.13	3.33	200	2.66	9.6	55.8	1.72 6.	
art A (current application)	MH4	MH2	0	0.0	0	0.0	146.0	4.0	2.37	0	0.0	1.5	0.47	0	1.5	0.009	0	1.5	0.10	0	1.5	0.02	0	1.5	0.13	0	1.5	0.11	0.00	0.13	3.33	200	2.70	27.8	56.2	1.73 5.	
														•					1		, ,																
art B (future application)	FUT-BLD2	MH2	0	0.0	110	231.0	231.0	4.0	3.74	0	0.0	1.5	0.00	0	1.5	0.000	0	1.5	0.00	0	1.5	0.00	0	1.5	0.00	0	1.5	0.00	0.41	0.11	3.86	200	2.00	2.5	48.4	1.49 8.	
										-							-												-								
TOTAL (Parts A + B)	MH2	EX MH	92	129.0	118	248.0	377.0	4.0	6.11	54	60.0	1.5	0.47	100	1.5	0.009	20	1.5	0.10	2	1.5	0.02	6	1.5	0.13	55	1.5	0.11	0.89	0.25	7.19	200	1.50	13.8	41.9	1.29 17	
sign Parameters: idential itutional nmercial f uty Salon ndy ng rg rration	450 5 275 650 1200 115) L/cap/da) L/bed/da 5 L/m ² per 5 L/cap/da) L/day per 5 L/seat/d 3 L/s/ha	ay day ay er station er machin		Peaking Resident Institutio Commer Other	nal cial		Equation ((max 4, n		1.40 2.10	Unit: Assisted 1 Bedroo 2 Bedroo Studio	m	-			-												-		(PROFES	Ň	ENGINGER H		

Restutential nows were used for senior apartments (350 D/cap/day, namon Peaking r.)
 Institutional flow used for assisted care units (450 L/bed/day, Peaking Factor = 1.5)
 Future building assumed to be a 10 storey building comprised of 110 2 bedroom units

M:\2016\116043\DATA\Calculations\Sewer Calcs\SAN\20171130-SAN.xlsx





LOCAT	ION		R					RESIDEN	ITIAL						COMMER	CIAL/INSTI	TUTIONAL	PLUGGE	ED FLOW	R	+ C			PROP	OSED SEWE	R			
					1	NUMBER	OF UNIT	s			INDI	/IDUAL	СОМОГ	ATIVE								PEAK EXTR.	PEAK DESIGN						
STREET	FROM MH	ТО МН		Houses	;	Extend	led Care		Hotel/Ap	t	POPUL. People	AREA ha	POPUL. People	AREA ha	PEAK FACTOR (M)	POPUL. FLOW L/S	ACTUAL AREA ha	CUMM AREA ha	COMM FLOW I/s	FLOW I/s	COMM FLOW I/s	FLOW	FLOW	LENGTH (m)	PIPE SIZE (mm)	SLOPE	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	RATIO (Q/Qfull)
			Singles	Stacks	Towns	No. Units	Act Pop	No. Units	Act. Pop	Equ. Pop		na	1 copio	na								l/s	L/S						
Robinson - 1996	Upstream	7A									2588	28.38	2588	28.38	3.496	36.65	20.370	20.370	17.68	162.69	162.69	14.02	231.04						
1250 Maritime Way	Blk 122	7A									377	0.89	377	0.89	4.000	6.11	0.005	0.005	0.004	0.83	0.83	0.25	7.19						
1200 Maritime Way	Blk 126	7A																				0.000	0.00						
Maritime Way	7A	507											2965	29.27	3.447	41.40		20.375	17.687		163.520	14.266	236.87	81.9	825	0.14	534.563	1.00	44%
Maritime Way	507	506						125	225	174	174	1.02	3139	30.29	3.426	43.56	4.910	25.285	21.949		163.520	15.92	244.95	119.3	825	0.12	534.563	0.93	46%
Cordillera Street	534	533						125	207	207	207	0.58	207	0.58	4.000	3.35	0.550	0.550	0.477			0.32	4.16	66.6	200	1.65	43.952	1.36	9%
Can. Shield Avenue Can. Shield Avenue	533 532	532 531										0.33	207 207	0.58 0.91	4.000	3.35 3.35		0.550	0.477			0.32	4.16 4.24	69.9 69.9	200 200	1.20	37.482 37.482	1.16 1.16	11% 11%
								400	100	100	100						0.040			0.000	0.000								
Great Lakes Avenue	536	531						100	180	139	139	0.78	139	0.78	4.000	2.25	0.040	0.040	0.035	0.300	0.300	0.23	2.82	60.0	200	2.40	53.008	1.63	5%
Great Lakes Avenue	531	530											346	1.69	4.000	5.61		0.590	0.512		0.300	0.644	7.06	80.8	200	3.75	66.260	2.04	11%
Great Lakes Avenue Great Lakes Avenue	530 506A	506A 506										0.38	346 346	1.69 2.07	4.000 4.000	5.61 5.61		0.590	0.512 0.512		0.300	0.644	7.06 7.16	85.2 4.9	200 200	1.40 1.40	40.486 40.486	1.25 1.25	<u>17%</u> 18%
	500	FOF						470	246.0	260	260	0.57	0754	22.02	2.250	E1.00		25.075	00.464		102.020	16.010	254.40	111.0	0.05	0.10	540 740	0.04	
Maritime Way Maritime Way	506 505	505 504						176 146	316.8 262.8	269 230	269 230	0.57	3754 3984	32.93 33.49	3.358 3.335	51.06 53.82	1.750	25.875 27.625	22.461 23.980		163.820 163.820	16.818 17.479	254.16 259.10	111.0 114.4	825 825	0.12	518.749 496.665	0.94	49% 52%
Maritime Way	504	501										0.27	3984	33.76	3.335	53.82		27.625	23.980		163.820	17.556	259.18	29.9	825	0.11	496.665	0.90	52%
Can. Shield Avenue	542	541						176	316.8	269	269	0.74	269	0.74	4.000	4.36						0.212	4.57	71.3	200	2.20	50.751	1.56	9%
Can. Shield Avenue	541	540						154	272.2	232	232	0.51	501	1.25	3.974	8.06	1.360	1.360	1.181			0.731	9.98	77.7	200	0.90	32.461	1.00	31%
	Block 3	540				208	333			428	428	1.02	428	1.02	4.000	6.94						0.286	7.22	12.0	200	0.60	26.504	0.82	27%
Can. Shield Avenue	540	512										0.3	929	2.57	3.820	14.38		1.360	1.181			1.100	16.66	82.6	200	0.71	28.831	0.89	58%
Maritime Way	514	513																						51.2	200	2.14	50.055	1.54	0%
Maritime Way (Blk 4)	513	512						144	271	271	271	1.12	271	1.12	4.000	4.39						0.314	4.70	51.9	200	2.28	51.666	1.59	9%
Maritime Way	512	511								58	58	0.73	1258	4.42	3.734	19.03		1.360	1.181			1.618	21.83	49.3	200	3.12	60.439	1.86	36%
	Block 5	511						154	301	301	301	0.92	301	0.92	4.000	4.88						0.258	5.13	12.2	200	2.00	48.390	1.49	11%
Maritime Way	511	510											1559	5.34	3.667	23.16		1.360	1.181			1.876	26.22	38.4	200	1.70	44.613	1.38	
Maritime Way	510	510											1559	5.34	3.667	23.16		1.360	1.181			1.876	26.22	38.4 11.3	200	2.28	44.613 51.666	1.38	59% 51%
Trunk Easement	501	500											5543	39.1	3.203	71.93		28.985	25.161		163.820	19.425	280.33	129.0	825	0.10	473.551	0.86	59%
Trunk Easement	500	94											5543	39.1	3.203	71.93		28.985	25.161		163.820	19.425	280.33						5070
A	90	92			35						95	0.80	95	0.80	4.000	1.54						0.228	1.77	120.0	250	0.60	48.055	0.95	4%
	92	94			12						32	1.19	127	1.99	4.000	2.06						0.568	2.63	103.0	250	2.20	92.018	1.82	3%
	94	95											5670	41.09	3.194	73.36		28.985	25.161		163.820	19.992	282.33	17.5	825	0.12	518.749	0.94	54%
	95	89			10						27	0.52	5697	41.61	3.192	73.67		28.985	25.161		163.820	20.141	282.79	66.6	825	0.12	518.749	0.94	55%



LOCAT	ION				RESIDEN	NTIAL						COMMER	CIAL/INSTI	TUTIONAL	PLUGGE	ED FLOW	R	+ C			PROP	OSED SEWE	R	
					TS	INDIN	/IDUAL	СОМОГ	LATIVE	PEAK	POPUL.	ACTUAL	симм	СОММ		сомм	PEAK EXTR.	PEAK DESIGN					FULL FLOW	
STREET	FROM MH	то мн	Houses	Extended Care		POPUL. People	AREA ha	POPUL. People	AREA ha	FACTOR (M)	FLOW L/S	AREA	AREA ha	FLOW I/s	FLOW I/s	FLOW I/s	FLOW	FLOW	LENGTH (m)	PIPE SIZE (mm)	SLOPE %	CAPACITY (L/s)	VELOCITY (m/s)	RATIO (Q/Qfull)
			Singles Stacks Tow	rns No. Units Act Pop	No. Act. Units Pop Equ. Pop												l/s	L/S						
В	85	87	19			65	1.19	65	1.19	4.000	1.05						0.340	1.39	116.9	250	0.40	39.237	0.77	40/
	87	89	24	1		65	0.82	130	2.01	4.000	2.11						0.540	2.68	116.7	250	1.41	73.667	1.45	4% 4%
																								.,.
A	89	84	12	2		32	0.35	5859	43.97	3.181	75.49		28.985	25.161		163.820	20.792	285.26	79.0	825	0.12	518.749	0.94	55%
С	80	82	19			65	1.08	65	1.08	4.000	1.05						0.308	1.36	120.0	250	0.40	39.237	0.77	3%
	82	84	25	5		67	0.83	132	1.91	4.000	2.14						0.544	2.68	118.5	250	1.20	67.960	1.34	4%
Δ.	84	79	14	1		38	0.54	6029	46.42	3.169	77.39		28.985	25.161		163.820	21 400	287.86	79.0	825	0.12	518.749	0.94	E E 0/
A	04	19		•		30	0.04	0029	40.42	3.109	11.39		20.900	23.101		103.020	21.490	207.00	19.0	020	0.12	510.749	0.94	55%
D	75	76	17	7		46	0.37	46	0.37	4.000	0.75						0.105	0.85	57.0	250	0.40	39.237	0.77	2%
	76	77	20)		54	0.29	100	0.66	4.000	1.62						0.188	1.81	78.4	250	0.40	39.237	0.77	5%
	77	79	13	3		35	0.63	135	1.29	4.000	2.19						0.368	2.56	117.7	250	0.81	55.835	1.10	5%
Park Easement	79 67	67	6			16	0.98	6164	48.69	3.160	78.89		28.985	25.161 25.161		163.820	22.099	289.97	55.0	825	0.12	518.749	0.94	56%
	07	66	0			10	0.33	6180	49.02	3.159	79.07		28.985	25.101		163.820	22.192	290.25	70.0	825	0.12	518.749	0.94	56%
BELLROCK DRIVE	70	73	12 14	1		70	2.56	70	2.56	4.000	1.13						0.728	1.86	87.2	250	0.40	39.237	0.77	5%
	73	74	12	2		32	0.54	102	3.1	4.000	1.65						0.882	2.53	80.3	250	0.40	39.237	0.77	6%
EASEMENT	74	62					0.31	102	3.41	4.000	1.65						0.970	2.62	39.9	250	0.40	39.237	0.77	7%
CAMBRAY LANE	62	66	25	5		68	0.48	170	3.89	4.000	2.75						1.107	3.86	100.5	250	0.40	39.237	0.77	10%
BISHOPS MILLS WAY	66	65	9			24	0.53	6374	53.44	3.146	81.22		28.985	25.161		163.820	23.450	293.65	62.0	825	0.12	518.749	0.94	57%
SOUTH OF HWY 7	EX.	65				7792	191.6	7792	191.6	3.061	96.63				37.720	37.720	53.648	188.00	50.2	900	0.11	626.373	0.95	30%
BISHOPS MILLS WAY	65	64	2			5		14171	245.04	2.803	160.91		28.985	25.161		201.540	77.083	464.70	17.0	900	0.11	626.373	0.95	74%
									210101	2.000			20.000	20.101		2011010		10 11 0			0	0201010	0.00	7470
EDENVALE DRIVE	59	60	8			22	0.50	22	0.50	4.000	0.36						0.141	0.50	77.0	200	1.40	40.486	1.25	1%
KETTLEBY STREET	60	61	22	2		59	0.62	81	1.12	4.000	1.31						0.315	1.63	103.6	250	0.40	39.237	0.77	4%
CAMBRAY LANE	58	61	5			14	0.41	14	0.41	4.000	0.23						0.115	0.34	74.5	200	0.70	28.628	0.88	1%
	61	64	25			60	0.42	160	1.05	4 000	264						0.640	2 10	105.0	250	0.00	59 955	1 10	F 0/
KETTLEBY STREET	61	04	25			68	0.42	163	1.95	4.000	2.64						0.549	3.19	105.0	250	0.90	58.855	1.16	5%
BISHOPS MILLS WAY	64	63	3			8	1	14342	246.99	2.798	162.55		28.985	25.161		201.540	77.632	466.88	13.0	900	0.11	626.373	0.95	75%
	63	57	10)		27	0.68	14369	247.67	2.797	162.81		28.985	25.161		201.540	77.823	467.33	64.9	900	0.11	626.373	0.95	75%
TER. BUNGALOW Ph. 2	51	53	48			130	0.94	130	0.94	4.000	2.11						0.264	2.37	122.3	200	0.70	28.628	0.88	8%
	53	54	4			11		141	0.94	4.000	2.28						0.264	2.55	13.6	200	0.70	28.628	0.88	9%
	54	55					0.27	141	1.21	4.000	2.28						0.340	2.63	36.7	200	0.70	28.628	0.88	9%
BISHOPS MILLS WAY	55 56	56 57	11 7 12	,		37 56	0.81	178	2.02	4.000	2.88 3.79						0.568	3.45 4.54	107.1	250	0.40	39.237 48.055	0.77	9%
	50	51	1 12	-		50	0.65	234	2.67	4.000	5.18						0.751	4.04	101.5	250	0.60	40.000	0.95	9%
PARK	57	34	1			3	0.37	14606	250.71	2.790	165.07		28.985	25.161		201.540	78.678	470.45	53.5	900	0.11	626.373	0.95	75%
	34	33	3			8		14614	250.71	2.790	165.15		28.985	25.161		201.540	78.678	470.53	50.3	900	0.11	626.373	0.95	75%
																								-



LOCATI	ON				RESIDEN	TIAL						COMMER	RCIAL/INSTI	TUTIONAL	PLUGG	ED FLOW	R	+ C			PROP	OSED SEWE	R	
			1		S	INDIV	IDUAL	CUMUL	ATIVE								PEAK	PEAK						
STREET	FROM MH	то мн	Houses	Extended Care	Hotel/Apt	POPUL. People	AREA ha	POPUL. People	AREA ha	PEAK FACTOR (M)	POPUL. FLOW L/S	ACTUAL AREA ha	CUMM AREA ha	COMM FLOW I/s	FLOW I/s	COMM FLOW I/s	EXTR. FLOW	DESIGN FLOW	LENGTH (m)	PIPE SIZE (mm)	SLOPE %	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	RATIO (Q/Qfull)
			Singles Stacks Towns	No. Units Act Pop	No. Act. Units Pop Equ. Pop	-	na	reopio	na								l/s	L/S						
HAWSTONE	43	44	22			59	1.19	59	1.19	4.000	0.96						0.335	1.29	51.0	250	1.00	62.039	1.22	2%
	44	45	8			22	0.09	81	1.28	4.000	1.31						0.360	1.67	29.0	250	0.50	43.868	0.87	4%
EDENVALE	45	35					0.06	81	1.34	4.000	1.31						0.377	1.69	39.8	250	0.50	43.868	0.87	4%
BIRKENDALE DRIVE	35	36	7			24	1.18	105	2.52	4.000	1.70						0.709	2.41	93.2	250	0.37	37.737	0.74	6%
	36 37	37 33	13 2 3			44 15	0.79	149 164	3.31	4.000	2.41 2.66						0.931 0.931	3.35 3.59	77.1 17.9	250	0.37	37.737	0.74	9%
	57	33	2 5			15		104	3.31	4.000	2.00						0.931	3.59	17.9	250	0.40	39.237	0.77	9%
BIRKENDALE DRIVE	33	32	10			27	0.56	14805	254.58	2.784	166.96		28.985	25.161		201.540	79.767	473.43	72.7	900	0.11	626.373	0.95	76%
TEESWATER STREET	30	31	16			43	0.66	43	0.66	4 000	0.70						0.186	0.88	75.1	250	0.40	39.237	0.77	
TEESWATER STREET	30	31	19			43 51	0.66	43 94	0.66	4.000 4.000	1.52						0.100	1.82	77.9	250 250	0.40	39.237	0.77	<u>2%</u> 5%
	01	02				01	0.11	01	1.07	1.000	1.02						0.001	1.02	11.0	200	0.10	00.201	0.11	5%
BIRKENDALE STREET	32	18	6			16	0.37	14915	256.02	2.781	168.01		28.985	25.161		201.540	80.172	474.88	44.4	900	0.11	626.373	0.95	76%
	18	16	4			11		14926	256.02	2.780	168.11		28.985	25.161		201.540	80.172	474.99	44.4	900	0.11	626.373	0.95	76%
COMMERCIAL PLAZA	19	17								4.000	0.00	0.520	0.520	0.451			0.146	0.60	26.5	150	0.90	15.073	0.83	4%
COLCHESTER SQUARE	17	16					0.10		0.10	4.000	0.00		0.520	0.451			0.174	0.63	33.2	250	0.40	39.237	0.77	2%
COLCHESTER SQUARE	16	15	10			27	0.56	14953	256.68	2.780	168.37		29.505	25.612		201.540	80.504	476.03	66.0	900	0.11	626.373	0.95	76%
	15	14A	2			5		14958	256.68	2.779	168.42		29.505	25.612		201.540	80.504	476.07	25.8	900	0.11	626.373	0.95	76%
ELSINORE LANE	39	28	32			86	0.53	86	0.53	4.000	1.39						0.149	1.54	56.7	250	1.00	62.039	1.22	2%
	28	24	18			49	1.47	135	2.00	4.000	2.19						0.563	2.75	43.0	250	0.40	39.237	0.77	7%
	24	23	12			32	0.14	167	2.14	4.000	2.71						0.602	3.31	34.0	250	0.40	39.237	0.77	8%
ELSINORE LANE	23	306	8			22	0.24	189	2.38	4.000	3.06						0.669	3.73	48.8	250	0.44	41.152	0.81	9%
ENDENVALE DRIVE	306	14-A					0.45	189	2.83	4.000	3.06						0.796	3.86	46.4	250	0.49	43.427	0.86	9%
COLCHESTER SQUARE	14-A	14						15147	259.51	2.774	170.21		29.505	25.612		201.540	81.300	478.66	14.7	900	0.11	626.373	0.95	76%
	Church	14										0.520	0.520	0.451			0.146	0.60	35.0	150	1.00	15.888	0.87	4%
																			20.0					7/0
COLCHESTER SQUARE	14	11	4			11	0.16	15158	259.67	2.774	170.31		30.025	26.063		201.540	81.491	479.41	72.6	900	0.11	626.373	0.95	77%
TERON	11	10						15158	259.67	2.774	170.31		30.025	26.063		201.540	81.491	479.41	29.6	900	0.11	626.373	0.95	77%
	10	EX.					0.25	15158	259.92	2.774	170.31		30.025	26.063		201.540	81.562	479.48	72.3	900	0.11	626.373	0.95	77%
TERON	0.P.P.	EX.													0.780	0.780		0.78	100	0 FORCEMAI	N			
TERON	EX.	EX. 2						15158	259.92	2.774	170.31		30.025	26.063		202.320	81.562	480.26	9.400	680.000	0.960	876.293	2.34	55%
																				+	+			
Notes:	1	L		1		1	1	1	1	1	1	1		1	1	1		I	1	1		1		

Notes:

1) As per Kanata Town Centre Sanitary Trunk Sewer Study revised March 27, 1996 by Robinson Consultants Inc.

2) Park or open space area.

3) Equivalent population base on 208 rooms and 20 staff members.

4) Allowance for an ultimate flow of 188 l/s to provide flexibility in future development as per Kanata Town Centre Sanitary Trunk Study.



LOCAT	ION							RESID	NTIAL						COMME	RCIAL/INSTI	TUTIONAL	PLUGG	ED FLOW	R	+ C			PROP	OSED SEWE	R	
					NUMBER C	OF UNIT:	S		INDI	/IDUAL	CUMUL	ATIVE								PEAK EXTR.	PEAK DESIGN						
STREET	FROM MH	то мн	ŀ	louses	Extende	d Care	H	lotel/Apt	POPUL	AREA	POPUL.	AREA	PEAK FACTOR (M)	POPUL. FLOW L/S	ACTUAL AREA ha	CUMM AREA ha	COMM FLOW I/s	FLOW I/s	COMM FLOW I/s	FLOW	FLOW	LENGTH (m)	PIPE SIZE (mm)	SLOPE %	CAPACITY (L/s)	FULL FLOW VELOCITY (m/s)	RATIO (Q/Qfull
			Singles	Stacks Town	s No. Units	Act Pop	No. Units	Act. Pop Equ. Po	People	ha	People	ha								l/s	L/S						
Additional flow associat	ed with hotel	l amenditie	s including	swimming pool	with bathroo	oms and	laudry as p	per design calcu	ations for B	ock 1 prov	ided by WSP ((October 2	016).														
Additional flow associat	ed with over	all ameniti	es including	beauty salon, s	staff, dining a	and laund	dry as per	design calculatio	ns for 1250	Maritime V	Vay (Timberwa	alk Retiren	nent Home) p	provided by N	lovatech (Jι	ıly 31, 2017).											
esign Parameters:																											
) Q(e) = 0.28 L/sec/ha							3	.4																			
Q(p) = (PxqxM/86,400)								2.7																			
) $Q(d) = Q(p) + Q(e)$							1	.4 1BDR; 2.1	2 BDR.																		
Definitions:							2	3																00 Maritir	ne Way DESIGN SHE	FT	
P = Population																							CANTAN	OLITEI	DEGIGINIONE		
= Average per capita flo	w = 350 L/pe	erson/day																									
/I = Residential Peaking Fa	actor (Harmo	on Formula	from section	on 4.4.1 of the (City Sewer D	esign Gu	uidelines):																				
		M = 1+[14	/(4+Pop/10	00)]^1/2*1 - (M	aximum of 4.	.0)																					
I = Commercial Peak Fac	tor 1.5																				Date		1	Janu	ary 27, 2021		1
(d) = Design Flow (L/sec)																					Design	GMAC					
(p) = Population Flow (L/s	,																				Job	No.	wg. Referen	-	Checked	d and Stamped	d:
(r) = Commercial Flow (L	/sec)																				120	144	120144-				
																					120		SAN	1		-	



JOB#	120144	

LOCATI	ION									RESIDEN	TIAL						COMMER	CIAL/INSTI	TUTIONAL	PLUGG	D FLOW	R	+ C			PROP	OSED SEWE	R	
					I	NUMBER		rs			INDIV	/IDUAL	СОМОГ	ATIVE	PEAK	POPUL.	ACTUAL	СЛММ	сомм	FLOW	сомм	PEAK EXTR. FLOW	PEAK DESIGN FLOW	LENGTH	PIPE SIZE	SLOPE	CAPACITY	FULL FLOW	RATIO
STREET	FROM MH	TO MH		Houses			led Care		Hotel/Ap	ot	POPUL. People	AREA ha	POPUL. People	AREA ha	FACTOR (M)	FLOW L/S	AREA ha	AREA ha	FLOW I/s	l/s	FLOW I/s			(m)	(mm)	%	(L/s)	VELOCITY (m/s)	(Q/Qfull)
			Singles	Stacks	Towns	No. Units	Act Pop	No. Units	Act. Pop	Equ. Pop												l/s	L/S						
Robinson - 1996	Upstream	7A									2588	28.38	2588	28.38	3.496	36.65	20.370	20.370	17.68	162.69	162.69	14.02	231.04						
1250 Maritime Way	Blk 122	7A									377	0.89	377	0.89	4.000	6.11	0.005	0.005	0.004	0.83	0.83	0.25	7.19						
1200 Maritime Way	Blk 126	7A						632		1062	1062	1.28	1062	1.28	3.226	11.10						0.422	11.53						
Maritime Way	7A	507											4027	30.55	3.331	54.33		20.375	17.687		163.520	14.26	249.80	81.9	825	0.14	534.563	1.00	47%
Maritime Way	507	506						125	225	174	174	1.02	4201	31.57	3.314	56.40	4.910	25.285	21.949		163.520	16.29	258.16	119.3	825	0.12	534.563	0.93	47%
Cordillera Street	534	533						125	207	207	207	0.58	207	0.58	4.000	3.35	0.550	0.550	0.477			0.32	4.16	66.6	200	1.65	43.952	1.36	9%
Can. Shield Avenue	533	532						125	207	207	207	0.30	207	0.58	4.000	3.35	0.550	0.550	0.477			0.32	4.16	69.9	200	1.00	37.482	1.16	9%
Can. Shield Avenue	532	531										0.33	207	0.91	4.000	3.35		0.550	0.477			0.41	4.24	69.9	200	1.20	37.482	1.16	11%
Great Lakes Avenue	536	531						100	180	139	139	0.78	139	0.78	4.000	2.25	0.040	0.040	0.035	0.300	0.300	0.23	2.82	60.0	200	2.40	53.008	1.63	5%
Ore at his hare. Assessme	504	500											0.40	1.00	4 000	5.04		0.500	0.540		0.000	0.044	7.00	00.0	000	0.75	00.000	0.04	
Great Lakes Avenue Great Lakes Avenue	531 530	530 506A											346 346	1.69 1.69	4.000 4.000	5.61 5.61		0.590	0.512		0.300	0.644	7.06	80.8 85.2	200 200	3.75 1.40	66.260 40.486	2.04 1.25	11% 17%
Great Lakes Avenue	506A	506										0.38	346	2.07	4.000	5.61		0.590	0.512		0.300	0.740	7.16	4.9	200	1.40	40.486	1.25	18%
Maritime Way	506	505						176	316.8	269	269	0.57	4816	34.21	3.260	63.60		25.875	22.461		163.820	17.184	267.07	111.0	825	0.12	518.749	0.94	51%
Maritime Way	505	504						146	262.8	230	230	0.56	5046	34.77	3.241	66.26	1.750	27.625	23.980		163.820	17.845	271.90	114.4	825	0.11	496.665	0.90	55%
Maritime Way	504	501										0.27	5046	35.04	3.241	66.26		27.625	23.980		163.820	17.922	271.98	29.9	825	0.11	496.665	0.90	55%
Can. Shield Avenue	542	541						176	316.8	269	269	0.74	269	0.74	4.000	4.36						0.212	4.57	71.3	200	2.20	50.751	1.56	9%
Can. Shield Avenue	541	540						154	272.2	232	232	0.51	501	1.25	3.974	8.06	1.360	1.360	1.181			0.731	9.98	77.7	200	0.90	32.461	1.00	31%
	Block 3	540				208	333			428	428	1.02	428	1.02	4.000	6.94						0.286	7.22	12.0	200	0.60	26.504	0.82	27%
Can. Shield Avenue	540	512										0.3	929	2.57	3.820	14.38		1.360	1.181			1.100	16.66	82.6	200	0.71	28.831	0.89	58%
Maritime Way	514	513								951	a= :													51.2	200	2.14	50.055	1.54	0%
Maritime Way (Blk 4)	513	512						144	271	271	271	1.12	271	1.12	4.000	4.39						0.314	4.70	51.9	200	2.28	51.666	1.59	9%
Maritime Way	512	511								58	58	0.73	1258	4.42	3.734	19.03		1.360	1.181			1.618	21.83	49.3	200	3.12	60.439	1.86	36%
	Block 5	511						154	301	301	301	0.92	301	0.92	4.000	4.88						0.258	5.13	12.2	200	2.00	48.390	1.49	11%
												-																	
Maritime Way	511	510											1559	5.34	3.667	23.16		1.360	1.181			1.876	26.22	38.4	200	1.70	44.613	1.38	59%
Maritime Way	510	501											1559	5.34	3.667	23.16		1.360	1.181			1.876	26.22	11.3	200	2.28	51.666	1.59	51%
Trunk Easement	501	500				1			1				6605	40.38	3.131	83.77		28.985	25.161		163.820	19.790	292.54	129.0	825	0.10	473.551	0.86	62%
Trunk Easement	500	94											6605	40.38	3.131	83.77		28.985	25.161		163.820	19.790	292.54						
Α	90	92			35	+					95	0.80	95	0.80	4.000	1.54						0.228	1.77	120.0	250	0.60	48.055	0.95	4%
	92	94			12						33	1.19	127	1.99	4.000	2.06						0.568	2.63	103.0	250	2.20	92.018	1.82	4% 3%
	94	95											6732	42.37	3.123	85.17		28.985	25.161		163.820	20.358	294.50	17.5	825	0.12	518.749	0.94	E70/
	94 95	95 89			10		+				27	0.52	6759	42.89	3.123	85.46		28.985	25.161		163.820	20.506	294.50	66.6	825	0.12	518.749	0.94	57% 57%



1200 Maritime Way SANITARY SEWER DE JOB# 120144	SIGN SH	EET OF I	DOWNS	TREAM	SEWER	RS							1												
В	85	87	19					 65	1.19	65	1.19	4.000	1.05					0.340	1.39	116.9	250	0.40	39.237	0.77	404
5	87	89	13		24			 65	0.82	130	2.01	4.000	2.11					0.540	2.68	116.7	250	1.41	73.667	1.45	4% 4%
	-										-									-					170
A	89	84			12			32	0.35	6921	45.25	3.111	87.23	28.985	25.161		163.820	21.157	297.37	79.0	825	0.12	518.749	0.94	57%
С	80	82	19		05			65	1.08	65	1.08	4.000	1.05					0.308	1.36	120.0	250	0.40	39.237	0.77	3%
	82	84			25			 67	0.83	132	1.91	4.000	2.14					0.544	2.68	118.5	250	1.20	67.960	1.34	4%
A	84	79			14			38	0.54	7091	47.70	3.101	89.08	28.985	25.161		163.820	21.855	299.92	79.0	825	0.12	518.749	0.94	58%
	0.								0.01			0.101	00.00	20.000	20.101		1001020	21.000	200.02		020	0.12		0.01	5070
D	75	76			17			46	0.37	46	0.37	4.000	0.75					0.105	0.85	57.0	250	0.40	39.237	0.77	2%
	76	77			20			54	0.29	100	0.66	4.000	1.62					0.188	1.81	78.4	250	0.40	39.237	0.77	5%
	77	79			13			35	0.63	135	1.29	4.000	2.19					0.368	2.56	117.7	250	0.81	55.835	1.10	5%
Dark Feetwart	70	67					 		0.00	7000	40.07	2.000	00.55	00.005	05 404		462.000	00.400	201.00	FF 0	005	0.40	E40 740	0.01	
Park Easement	79 67	67 66			6	+	 	 16	0.98	7226 7242	49.97 50.30	3.093 3.092	90.55 90.72	28.985 28.985	25.161 25.161		163.820 163.820	22.463 22.557	301.99 302.26	55.0 70.0	825 825	0.12	518.749 518.749	0.94	58%
	07	00			U	+		 10	0.33	1242	30.30	3.032	30.12	20.903	20.101		103.020	22.001	502.20	10.0	020	0.12	510.749	0.94	58%
BELLROCK DRIVE	70	73		12	14			70	2.56	70	2.56	4.000	1.13					0.728	1.86	87.2	250	0.40	39.237	0.77	5%
	73	74			12			32	0.54	102	3.1	4.000	1.65					0.882	2.53	80.3	250	0.40	39.237	0.77	6%
EASEMENT	74	62							0.31	102	3.41	4.000	1.65					0.970	2.62	39.9	250	0.40	39.237	0.77	7%
CAMBRAY LANE	62	66			25			68	0.48	170	3.89	4.000	2.75					1.107	3.86	100.5	250	0.40	39.237	0.77	10%
								 	0.50	= 100					0.5.101							0.10			
BISHOPS MILLS WAY	66	65			9			 24	0.53	7436	54.72	3.081	92.81	28.985	25.161		163.820	23.814	305.61	62.0	825	0.12	518.749	0.94	59%
SOUTH OF HWY 7	EX.	65						 7792	191.6	7792	191.6	3.061	96.63			37.720	37.720	53.648	188.00	50.2	900	0.11	626.373	0.95	30%
	L/t.	00						1102	101.0	1102	101.0	0.001	00.00			01.120	01.120	00.010	100.00	00.2	000	0.11	020.070	0.00	3078
BISHOPS MILLS WAY	65	64			2			5		15233	246.32	2.771	171.02	28.985	25.161		201.540	77.443	475.17	17.0	900	0.11	626.373	0.95	76%
EDENVALE DRIVE	59	60			8			22	0.50	22	0.50	4.000	0.36					0.141	0.50	77.0	200	1.40	40.486	1.25	1%
KETTLEBY STREET	60	61			22			 59	0.62	81	1.12	4.000	1.31					0.315	1.63	103.6	250	0.40	39.237	0.77	4%
CAMBRAY LANE	58	61			5			 14	0.41	14	0.41	4.000	0.23					0.115	0.34	74.5	200	0.70	28.628	0.88	40/
CAMBRAT LANE	50	01			5			14	0.41	14	0.41	4.000	0.23					0.115	0.34	74.5	200	0.70	20.020	0.00	1%
KETTLEBY STREET	61	64			25			68	0.42	163	1.95	4.000	2.64					0.549	3.19	105.0	250	0.90	58.855	1.16	5%
BISHOPS MILLS WAY	64	63			3			8		15404	248.27	2.767	172.64	28.985	25.161		201.540	77.992	477.33	13.0	900	0.11	626.373	0.95	76%
	63	57			10			 27	0.68	15431	248.95	2.766	172.89	28.985	25.161		201.540	78.183	477.78	64.9	900	0.11	626.373	0.95	76%
	54	50		40				 100	0.04	400	0.04	4 000	0.44					0.004	0.07	400.0	000	0.70	00.000	0.00	
TER. BUNGALOW Ph. 2	51 53	53 54		48				 130	0.94	130 141	0.94	4.000 4.000	2.11 2.28					0.264	2.37 2.55	122.3 13.6	200 200	0.70	28.628 28.628	0.88	8%
	53	54 55				+	 		0.27	141	1.21	4.000	2.20					0.204	2.63	36.7	200	0.70	28.628	0.88	9% 9%
BISHOPS MILLS WAY	55	56	11					37	0.81	178	2.02	4.000	2.88					0.568	3.45	107.1	250	0.40	39.237	0.77	9%
	56	57	7		12			56	0.65	234	2.67	4.000	3.79					0.751	4.54	101.5	250	0.60	48.055	0.95	9%
PARK	57	34			1		 	 3	0.37	15668	251.99	2.759	175.12	28.985	25.161		201.540	79.038	480.86	53.5	900	0.11	626.373	0.95	77%
	34	33			3	<u> </u>	 	 8		15676	251.99	2.759	175.20	28.985	25.161		201.540	79.038	480.94	50.3	900	0.11	626.373	0.95	77%
HAWSTONE	43	44		22			 	 59	1.19	59	1.19	4.000	0.96					0.335	1.29	51.0	250	1.00	62.039	1.22	
HANDIONE	43	44		8		+	 	 22	0.09	39 81	1.19	4.000	1.31					0.335	1.29	29.0	250	0.50	43.868	0.87	2% 4%
EDENVALE	45	35		Ť					0.06	81	1.34	4.000	1.31					0.377	1.69	39.8	250	0.50	43.868	0.87	4%
BIRKENDALE DRIVE	35	36	7					24	1.18	105	2.52	4.000	1.70					0.709	2.41	93.2	250	0.37	37.737	0.74	6%
	36	37	13					44	0.79	149	3.31	4.000	2.41					0.931	3.35	77.1	250	0.37	37.737	0.74	9%
	37	33	2		3		 	 15		164	3.31	4.000	2.66					0.931	3.59	17.9	250	0.40	39.237	0.77	9%
BIRKENDALE DRIVE	33	32			10			27	0.56	15867	255.86	2.754	176.99	28.985	25.161		201.540	80.127	483.82	72.7	900	0.11	626.373	0.95	77%
II.																									

1200 Maritimo Wa

1200 Maritime Way SANITARY SEWER D JOB# 120144	ESIGN SH	EET OF I	DOWNSTRE	EAM S	SEWER	S																		1.1		
	31	32			19			51	0.41	94	1.07	4.000	1.52						0.301	1.82	77.9	250	0.40	39.237	0.77	5%
BIRKENDALE STREET	32	18			6			16	0.37	15977	257.30	2.751	178.03		28.985	25.161		201.540	80.532	485.26	44.4	900	0.11	626.373	0.95	77%
	18	16			4			11		15988	257.30	2.750	178.13		28.985	25.161		201.540	80.532	485.36	44.4	900	0.11	626.373	0.95	77%
COMMERCIAL PLAZA	19	17										4.000	0.00	0.520	0.520	0.451			0.146	0.60	26.5	150	0.90	15.073	0.83	4%
COLCHESTER SQUARE	17	16							0.10		0.10	4.000	0.00		0.520	0.451			0.174	0.63	33.2	250	0.40	39.237	0.77	2%
COLCHESTER SQUARE	16	15			10			27	0.56	16015	257.96	2.750	178.38		29.505	25.612		201.540	80.864	486.40	66.0	900	0.11	626.373	0.95	78%
	15	14A			2			5		16020	257.96	2.749	178.43		29.505	25.612		201.540	80.864	486.44	25.8	900	0.11	626.373	0.95	78%
ELSINORE LANE	39	28		32				86	0.53	86	0.53	4.000	1.39						0.149	1.54	56.7	250	1.00	62.039	1.22	2%
	28	24		18				49	1.47	135	2.00	4.000	2.19						0.563	2.75	43.0	250	0.40	39.237	0.77	7%
	24	23		12				32	0.14	167	2.14	4.000	2.71						0.602	3.31	34.0	250	0.40	39.237	0.77	8%
ELSINORE LANE	23	306		8				22	0.24	189	2.38	4.000	3.06						0.669	3.73	48.8	250	0.44	41.152	0.81	9%
ENDENVALE DRIVE	306	14-A							0.45	189	2.83	4.000	3.06						0.796	3.86	46.4	250	0.49	43.427	0.86	9%
COLCHESTER SQUARE	14-A	14								16209	260.79	2.744	180.20		29.505	25.612		201.540	81.660	489.01	14.7	900	0.11	626.373	0.95	78%
	Church	14												0.520	0.520	0.451			0.146	0.60	35.0	150	1.00	15.888	0.87	4%
COLCHESTER SQUARE	14	11		4				11	0.16	16220	260.95	2.744	180.30		30.025	26.063		201.540	81.851	489.75	72.6	900	0.11	626.373	0.95	78%
TERON	11	10								16220	260.95	2.744	180.30		30.025	26.063		201.540	81.851	489.75	29.6	900	0.11	626.373	0.95	78%
	10	EX.							0.25	16220	261.20	2.744	180.30		30.025	26.063		201.540	81.922	489.82	72.3	900	0.11	626.373	0.95	78%
TERON	0.P.P.	EX.															0.780	0.780		0.78	100) FORCEMA	IN			
TERON	EX.	EX. 2								16220	261.20	2.744	180.30		30.025	26.063		202.320	81.922	490.60	9.400	680.000	0.960	876.293	2.34	56%

Notes:

1) As per Kanata Town Centre Sanitary Trunk Sewer Study revised March 27, 1996 by Robinson Consultants Inc.

2) Park or open space area.

3) Equivalent population base on 208 rooms and 20 staff members.

4) Allowance for an ultimate flow of 188 l/s to provide flexibility in future development as per Kanata Town Centre Sanitary Trunk Study.

5) Additional flow associated with hotel amendities including swimming pool with bathrooms and laudry as per design calculations for Block 1 provided by WSP (October 2016).

6) Additional flow associated with overall amenities including beauty salon, staff, dining and laundry as per design calculations for 1250 Maritime Way (Timberwalk Retirement Home) provided by Novatech (July 31, 2017).

7) JLR Spreadsheet up-dated to include development flows from 1200 Maritime Way. Reference Appendix A of Serviceability Report for 1250 Maritime Way attached in Appendix of 1200 Maritime Way Serviceability Report (Novatech January 28, 2021)...

Design Parameters:	
1) Q(e) = 0.28 L/sec/ha	3.4
2) Q(p) = (PxqxM/86,400)	2.7
3) $Q(d) = Q(p) + Q(e)$	1.4 1BDR; 2.1 2 BDR.
Definitions:	2.3
P = Population	
q = Average per capita flow = 350 L/person/day	
M = Residential Peaking Factor (Harmon Formula from section 4.4.1 of the City Sewer	Design Guidelines):
M = 1+[14/(4+Pop/1000)]^1/2*1 - (Maximum of	4.0)
N = Commercial Peak Factor 1.5	
Q(d) = Design Flow (L/sec)	
Q(p) = Population Flow (L/sec)	
Q(r) = Commercial Flow (L/sec)	
Q(e) = Extraneous Flow (L/sec)	

Date Design Job No 120144

1200 Maritime Way SANITARY SEWER DESIGN SHEET

		Janu	ary 27, 2021		
GMAC					
lo.	wg. Referenc		Checked	and Stampe	d:
44	120144- SAN				

APPENDIX C

Stormwater Management Calculations



Runoff Coefficients

Drainage Area	Total Area (m ²)	Hard Surf	ace Area	Grass	Area	5-Year Runoff	100-Year Runoff
	(m)	Area (m ²)	С	Area (m ²)	С	Coefficient	Coefficient
A-01	253.7	27.3	0.95	226.4	0.20	0.28	0.33
A-02	462.2	199.9	0.95	262.3	0.20	0.52	0.57
A-03	1578.1	32.6	0.95	1545.5	0.20	0.22	0.27
A-04	585.8	267.4	0.95	318.4	0.20	0.54	0.59
A-05	685.0	418.1	0.95	266.9	0.20	0.66	0.71
A-06	296.4	236.0	0.95	60.4	0.20	0.80	0.85
Total	3861.16	1181.3	0.95	2679.8	0.20	0.43	0.48



Controlled Flow

5 YR

Area No.	Area (ha)	C _{5yr}	Time (min)	intensity mm/hr	Uncontrolled runoff L/s	Control System	Zurn Model Number	Release Rate (L/s/m of head)	Notches	Depth (m)	Controlled Flow (L/s)	Storage available (m ³)	Storage used (m ³)
A-01	0.0254	0.28	20.00	70.25	1.39	no control	-	-	-	-	-	-	-
A-02	0.0462	0.52	20.00	70.25	4.73	no control	-	-	-	-	-	-	-
A-03	0.1578	0.22	20.00	70.25	6.64	no control	-	-	-	-	-	-	-
A-04	0.0586	0.54	20.00	70.25	6.20	no control	-	-	-	-	-	-	-
A-05	0.0685	0.66	20.00	70.25	8.80	no control	-	-	-	-	-	-	-
A-06	0.0296	0.80	20.00	70.25	4.61	no control	-	-	-	-	-	-	-
CB Storage	-	-	-	-	-	-	-	-	-	-	-	-	- 1
Total:	0.3861				32.38								

100 YR

Area ID	Area (ha)	C _{100yr}	Time (min)	intensity mm/hr	Uncontrolled runoff L/s	Control System	Zurn Model Number	Release Rate (L/s/m of head)	Notches	Depth (m)	Controlled Flow (L/s)	Storage available (m ³)	Storage used (m ³)
A-01	0.0254	0.33	10.00	178.56	4.16	no control	-	-	-	-	-	-	-
A-02	0.0462	0.57	10.00	178.56	13.18	no control	-	-	-	-	-	-	-
A-03	0.1578	0.27	10.00	178.56	20.80	no control	-	-	-	-	-	-	-
A-04	0.0586	0.59	10.00	178.56	17.22	no control	-	-	-	-	-	-	-
A-05	0.0685	0.71	10.00	178.56	24.07	no control	-	-	-	-	-	-	-
A-06	0.0296	0.85	20.00	119.95	8.37	no control	-	-	-	-	-	-	-
CB Storage	-	-	-	-	-	-	-	-	-	-	-	-	-
Total:	0.3861				87.81								

Note: In all cases, there is only one notch in the Zurn roof drain and and flows through each drain is further reduced with and adjustable weir. See Zurn roof drains

sheet and adjustable weir specification for more details on the reduction of flow.

55.36

Allowable release rate

Area	1.28 h	a	
С	0.8		
tc	20 m	in	
i ₅	70.25		
Q allowable = 2.	78 x C x i x A 199.99 L/	's	
tank all =	112.18	tank A =	70.44
		tank B =	41.74

1200	Maritime	Way

	Ν	C		Λ	Т		C	Ή	1
_		~	N.	-	-	-			~
E	Ν	G	N	E	E	к	I.	Ν	G

REQUIRED S AREA	STORAGE - 5- East Tower (YEAR EVENT incl. CB1/2)		: TANK	
ottawa idf					
Area =	0.5400	ha		Qallow =	70.44
C =	0.95	na		Vol(max) =	46.89
0-	0.95			VOI(IIIax) –	40.09
Time	Intensity	Q Uncontrolled	Q Controlled	Qnet	Vol
(min)	(mm/hr)	(L/s)	(L/s)	(L/s)	(m ³)
5	141.18	201.34	0.00	130.90	39.27
10	104.19	148.59	0.00	78.15	46.89
15	83.56	119.16	0.00	48.72	43.85
20	70.25	100.19	0.00	29.75	35.70
25	60.90	86.85	0.00	16.41	24.61
30	53.93	76.91	0.00	6.47	11.64
35	48.52	69.19	0.00	-1.25	-2.62
40	44.18	63.01	0.00	-7.43	-17.82
45	40.63	57.94	0.00	-12.50	-33.74
50	37.65	53.70	0.00	-16.74	-50.22
55	35.12	50.09	0.00	-20.35	-67.15
60	32.94	46.98	0.00	-23.46	-84.45
65	31.04	44.27	0.00	-26.17	-102.05
70	29.37	41.89	0.00	-28.55	-119.92
75	27.89	39.77	0.00	-30.67	-138.00
80	26.56	37.88	0.00	-32.56	-156.28
85	25.37	36.18	0.00	-34.26	-174.73
90	24.29	34.64	0.00	-35.80	-193.33
95	23.31	33.24	0.00	-37.20	-212.06
100	22.41	31.96	0.00	-38.48	-230.91
105	21.58	30.78	0.00	-39.66	-249.86
110	20.82	29.70	0.00	-40.74	-268.91
115	20.12	28.69	0.00	-41.75	-288.05
120	19.47	27.76	0.00	-42.68	-307.27
125	18.86	26.90	0.00	-43.54	-326.56
130	18.29	26.09	0.00	-44.35	-345.92
135	17.76	25.34	0.00	-45.10	-365.35
140	17.27	24.63	0.00	-45.81	-384.83
145	16.80	23.96	0.00	-46.48	-404.36
150	16.36	23.33	0.00	-47.11	-423.95
155	15.95	22.74	0.00	-47.70	-443.58
160	15.56	22.18	0.00	-48.26	-463.26
165	15.18	21.65	0.00	-48.79	-482.97
170	14.83	21.15	0.00	-49.29	-502.73

		00-YEAR EVE	NT		
AREA	East Tower (Inci. CB1/2)		: TANK	
OTTAWA IDF	CURVE				
Area =	0.5400	ha		Qallow =	70.44
C =	1.00			Vol(max) =	131.56
Time	Intensity	Q Uncontrolled	Q Controlled	Qnet	Vol
(min)	(mm/hr)	(L/s)	(L/s)	(L/s)	(m ³)
5	242.70	364.35	0.00	293.91	88.17
10	178.56	268.05	0.00	197.61	118.57
15	142.89	214.51	0.00	144.07	129.67
20	119.95	180.07	0.00	109.63	131.56
25	103.85	155.90	0.00	85.46	128.18
30	91.87	137.91	0.00	67.47	121.45
35	82.58	123.97	0.00	53.53	112.41
40	75.15	112.81	0.00	42.37	101.68
45	69.05	103.66	0.00	33.22	89.69
50	63.95	96.01	0.00	25.57	76.70
55	59.62	89.51	0.00	19.07	62.92
60	55.89	83.91	0.00	13.47	48.49
65	52.65	79.03	0.00	8.59	33.51
70	49.79	74.74	0.00	4.30	18.08
75	47.26	70.94	0.00	0.50	2.25
80	44.99	67.54	0.00	-2.90	-13.92
85	42.95	64.48	0.00	-5.96	-30.38
90	41.11	61.72	0.00	-8.72	-47.11
95	39.43	59.20	0.00	-11.24	-64.07
100	37.90	56.90	0.00	-13.54	-81.24
105	36.50	54.79	0.00	-15.65	-98.60
110	35.20	52.85	0.00	-17.59	-116.12
115	34.01	51.05	0.00	-19.39	-133.80
120	32.89	49.38	0.00	-21.06	-151.62
125	31.86	47.83	0.00	-22.61	-169.57
130	30.90	46.38	0.00	-24.06	-187.63
135	30.00	45.03	0.00	-25.41	-205.81
140	29.15	43.76	0.00	-26.68	-224.09
145	28.36	42.57	0.00	-27.87	-242.46
150	27.61	41.45	0.00	-28.99	-260.92
155	26.91	40.39	0.00	-30.05	-279.46
160	26.24	39.39	0.00	-31.05	-298.08
165	25.61	38.44	0.00	-32.00	-316.76
170	25.01	37.55	0.00	-32.89	-335.52

East Tower

1200	Maritime	Way



REQUIRED S	REQUIRED STORAGE - 5-YEAR EVENT							
AREA	West Tower	(incl. CB3/4 &	TD)	: TANK				
OTTAWA IDF	CURVE							
Area =	0.3200	ha		Qallow =	41.74			
C =	0.95			Vol(max) =	27.79			
Time	Intensity	Q Uncontrolled	Q Controlled	Qnet	Vol			
(min)	(mm/hr)	(L/s)	(L/s)	(L/s)	(m ³)			
5	141.18	119.31	0.00	77.57	23.27			
10	104.19	88.06	0.00	46.32	27.79			
15	83.56	70.62	0.00	28.88	25.99			
20	70.25	59.37	0.00	17.63	21.16			
25	60.90	51.46	0.00	9.72	14.59			
30	53.93	45.58	0.00	3.84	6.90			
35	48.52	41.00	0.00	-0.74	-1.55			
40	44.18	37.34	0.00	-4.40	-10.56			
45	40.63	34.34	0.00	-7.40	-19.99			
50	37.65	31.82	0.00	-9.92	-29.76			
55	35.12	29.68	0.00	-12.06	-39.79			
60	32.94	27.84	0.00	-13.90	-50.04			
65	31.04	26.24	0.00	-15.50	-60.47			
70	29.37	24.82	0.00	-16.92	-71.05			
75	27.89	23.57	0.00	-18.17	-81.77			
80	26.56	22.45	0.00	-19.29	-92.60			
85	25.37	21.44	0.00	-20.30	-103.53			
90	24.29	20.53	0.00	-21.21	-114.55			
95	23.31	19.70	0.00	-22.04	-125.65			
100	22.41	18.94	0.00	-22.80	-136.82			
105	21.58	18.24	0.00	-23.50	-148.05			
110	20.82	17.60	0.00	-24.14	-159.34			
115	20.12	17.00	0.00	-24.74	-170.68			
120	19.47	16.45	0.00	-25.29	-182.07			
125	18.86	15.94	0.00	-25.80	-193.50			
130	18.29	15.46	0.00	-26.28	-204.98			
135	17.76	15.01	0.00	-26.73	-216.48			
140	17.27	14.59	0.00	-27.15	-228.03			
145	16.80	14.20	0.00	-27.54	-239.60			
150	16.36	13.83	0.00	-27.91	-251.21			
155	15.95	13.48	0.00	-28.26	-262.84			
160	15.56	13.15	0.00	-28.59	-274.50			
165	15.18	12.83	0.00	-28.91	-286.18			
170	14.83	12.54	0.00	-29.20	-297.89			

REQUIRED STORAGE - 100-YEAR EVENT						
AREA	West Tower	(incl. CB3/4 &	TD)	: TANK		
OTTAWA IDF	CURVE					
Area =	0.3200	ha		Qallow =	41.74	
C =	1.00			Vol(max) =	77.96	
Time	Intensity	Q Uncontrolled	Q Controlled	Qnet	Vol	
(min)	(mm/hr)	(L/s)	(L/s)	(L/s)	(m ³)	
5	242.70	215.91	0.00	174.17	52.25	
10	178.56	158.85	0.00	117.11	70.26	
15	142.89	127.12	0.00	85.38	76.84	
20	119.95	106.71	0.00	64.97	77.96	
25	103.85	92.38	0.00	50.64	75.96	
30	91.87	81.73	0.00	39.99	71.97	
35	82.58	73.46	0.00	31.72	66.62	
40	75.15	66.85	0.00	25.11	60.26	
45	69.05	61.43	0.00	19.69	53.16	
50	63.95	56.89	0.00	15.15	45.46	
55	59.62	53.04	0.00	11.30	37.29	
60	55.89	49.72	0.00	7.98	28.74	
65	52.65	46.83	0.00	5.09	19.87	
70	49.79	44.29	0.00	2.55	10.72	
75	47.26	42.04	0.00	0.30	1.34	
80	44.99	40.02	0.00	-1.72	-8.24	
85	42.95	38.21	0.00	-3.53	-17.99	
90	41.11	36.57	0.00	-5.17	-27.91	
95	39.43	35.08	0.00	-6.66	-37.96	
100	37.90	33.72	0.00	-8.02	-48.13	
105	36.50	32.47	0.00	-9.27	-58.41	
110	35.20	31.32	0.00	-10.42	-68.80	
115	34.01	30.25	0.00	-11.49	-79.27	
120	32.89	29.26	0.00	-12.48	-89.83	
125	31.86	28.34	0.00	-13.40	-100.47	
130	30.90	27.49	0.00	-14.25	-111.17	
135	30.00	26.69	0.00	-15.05	-121.94	
140	29.15	25.93	0.00	-15.81	-132.77	
145	28.36	25.23	0.00	-16.51	-143.66	
150	27.61	24.56	0.00	-17.18	-154.60	
155	26.91	23.94	0.00	-17.80	-165.58	
160	26.24	23.34	0.00	-18.40	-176.62	
165	25.61	22.78	0.00	-18.96	-187.69	
170	25.01	22.25	0.00	-19.49	-198.80	

West Tower

PREPARED BY: NOVATECH DATE: January 25, 2021

APPENDIX D

Fire Demand Calculations

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines

Novatech Project #: 120144 Project Name: 1200 Maritime Way - East Tower Date: 1/22/2021 Input By: Jazmine Gauthier Reviewed By: Greg MacDonald



Engineers, Planners & Landscape Architects

Legend

Input by User No Information or Input Required

Building Description: 28 Storey Building with 7 Storey Podium Fire Resistive Construction

Step			Choose		Value Used	Total Fire Flow (L/min)
		Base Fire Flo	w			<i>i</i>
	Construction Ma	iterial		Multi	iplier	
1	Coefficient related to type of construction C	Wood frame Ordinary construction Non-combustible construction Modified Fire resistive construction (2 hrs) Fire resistive construction (> 3 hrs)	Yes	1.5 1 0.8 0.6 0.6	0.6	
	Floor Area			0.0		
2	A	Podium Level Footprint (m ²) Total Floors/Storeys (Podium) Tower Footprint (m ²) Total Floors/Storeys (Tower) Protected Openings (1 hr) Area of structure considered (m ²)	2204 7 742 28 Yes		3,306	
	F	Base fire flow without reductions F = 220 C (A) ^{0.5}	-			8,000
		Reductions or Surc	harges			
	Occupancy haza	rd reduction or surcharge		Reduction/	Surcharge	
3	(1)	Non-combustible Limited combustible Combustible Free burning Rapid burning	Yes	-25% -15% 0% 15% 25%	-15%	6,800
	Sprinkler Reduct			Redu	ction	
4	(2)	Adequately Designed System (NFPA 13) Standard Water Supply Fully Supervised System	Yes Yes Yes Cum	-30% -10% -10% nulative Total	-30% -10% -10% - 50%	-3,400
	Exposure Surch	arge (cumulative %)			Surcharge	
5	(3)	North Side East Side South Side West Side	30.1- 45 m 0 - 3 m > 45.1m 20.1 - 30 m Cum	nulative Total	5% 25% 0% 10% 40%	2,720
	-	Results				
		Total Required Fire Flow, rounded to nea	rest 1000L/mi	n	L/min	6,000
6	(1) + (2) + (3)	(2,000 L/min < Fire Flow < 45,000 L/min)		or or	L/s USGPM	100 1,585
7	Storage Volume	Required Duration of Fire Flow (hours) Required Volume of Fire Flow (m ³)			Hours m ³	2 720

	FUS - Fire Flow Calculations - User Guide - Fire Resistive						
	Project Name: 1200 Maritime Way - East Date: 1/22/2021 Input By: Jazmine Gauthier Reviewed By: Greg MacDonald	ow Calculat When in do chitect/own	ubt, confirm construction mate				
	Note: This form only applies for Fire Resistive						
	Enter a description of the building or unit being conside	ered, i.e. use	e/most stringent condition/addr Summary	ess			
			Construction Type	Fire Resistive Const	truction		
			Floor Area Considered Occupancy Reduction	3,306 -15%	m ²		
	Base Fire Flow		Sprinkler Reduction	-50%			
	Construction Material		Exposure Surcharge	40%			
	Does not apply for this form		Total Fire Flow	6,000	L/min		
1	Does not apply for this form Does not apply for this form Only Use if can be confirmed with client/architect (ISO Only Use if can be confirmed with client/architect (ISO						
2	If considered gross floor area, then enter 1 floor/storey. Un-Protected 8 = number of floors above firs Protected 2 = number of additional immer Do vertical openings have minimum 1 hour rating betwee For unprotected openings scenario only, can be m	t 2, up to ma diately adjoir een floors? (ax of 10 floors total ning floors to be considered, u Confirm this with the architect.	p to 2			
	Reductions or Surcharges						
3	Occupancy hazard reduction or surcharge Residential - with no garage Residential - with garage General Commercial - Generally, no reduction Check usage with FUS Check usage with FUS						
	Sprinkler Reduction						
4	Only Use if can be confirmed with client/architect Only Use if can be confirmed with client/architect Only Use if can be confirmed with client/architect						
	Exposure Surcharge (cumulative %)						
5	For Fire walls: FUS considers a Fire wall to have a mini	imum 2 hou	r rating per NBC.				
	Results						
6	NOTE: Refer to City Technical Bulletin ISDTB-2014-02	2 for additio	nal considerations to cap this v	alue at 10,000L/min			
υ	If IGPM is needed, divide USGPM by 1.20095						
7	For Rural areas, or where required						

FUS - Fire Flow Calculations

As per 1999 Fire Underwriter's Survey Guidelines

Novatech Project #: 120144 Project Name: 1200 Maritime Way - West Tower Date: 1/22/2021 Input By: Jazmine Gauthier Reviewed By: Greg MacDonald



Engineers, Planners & Landscape Architects

Legend

Input by User No Information or Input Required

Building Description: 30 Storey Building with 7 Storey Podium Fire Resistive Construction

Step			Choose		Value Used	Total Fire Flow
		Base Fire Flo	w			(L/min)
	Construction Ma			Multi	iplier	
		Wood frame		1.5		
	Coefficient	Ordinary construction		1.0		
1	related to type	Non-combustible construction		0.8	0.6	
	of construction	Modified Fire resistive construction (2 hrs)	Yes	0.6		
	С	Fire resistive construction (> 3 hrs)		0.6		
	Floor Area					
		Podium Level Footprint (m ²)	1947			
		Total Floors/Storeys (Podium)	7			
		Tower Footprint (m ²)	906			
2	Α	Total Floors/Storeys (Tower)	30			
-		Protected Openings (1 hr)	Yes			
		Area of structure considered (m ²)			2,921	
		Base fire flow without reductions			,	
	F	$F = 220 C (A)^{0.5}$	-			7,000
	<u></u>	Reductions or Surg	harges			
	Occupancy haza	rd reduction or surcharge	J	Reduction/	Surcharge	
	(1)	Non-combustible		-25%	-	
3		Limited combustible	Yes	-15%		
3		Combustible		0%	-15%	5,950
		Free burning		15%		
		Rapid burning		25%		
	Sprinkler Reduct	tion		Redu	ction	
	-	Adequately Designed System (NFPA 13)	Yes	-30%	-30%	
4		Standard Water Supply	Yes	-10%	-10%	
	(2)	Fully Supervised System	Yes	-10%	-10%	-2,975
			Cun	ulative Total	-50%	
	Exposure Surch	arge (cumulative %)			Surcharge	
		North Side	> 45.1m		0%	
-		East Side	20.1 - 30 m		10%	
5	(3)	South Side	30.1- 45 m		5%	2,380
		West Side	0 - 3 m		25%	
			Cun	nulative Total	40%	
		Results				
		Total Required Fire Flow, rounded to nea	rest 1000L/mi	n	L/min	5,000
6	(1) + (2) + (3)	1) + (2) + (3)		or	L/s	83
		(2,000 L/min < Fire Flow < 45,000 L/min)		or	USGPM	1,321
7	Storage Volume	Required Duration of Fire Flow (hours)			Hours	1.75
1	Storage Volume	Required Volume of Fire Flow (m ³)		-	m ³	525

	FUS - Fire Flow Calculations - User Guide - Fire Resistive							
	Novatech Project #: 120144 • Please use the notes below as a guide when completing the FUS Fire Project Name: 1200 Maritime Way - Wes Flow Calculations Date: 1/22/2021 • When in doubt, confirm construction material, firewalls, etc. with architect/owner Reviewed By: Greg MacDonald • When in doubt, err on conservative side							
	Note: This form only applies for Fire Resistive							
	Enter a description of the building or unit being considered, i.e. use/most stringent condition/address							
			Summary Construction Type	Fire Resistive Const	ruction			
				File Resistive Const				
			Floor Area Considered	2,921	m ²			
			Occupancy Reduction	-15%				
	Base Fire Flow		Sprinkler Reduction	-50%				
	Construction Material		Exposure Surcharge	40%				
	Does not apply for this form		Total Fire Flow	5,000	L/min			
1	Does not apply for this form Does not apply for this form Only Use if can be confirmed with client/architect (ISO	CI 5)		:				
	Only Use if can be confirmed with client/architect (ISO	CI 6)						
2	Floor Area Signature: If considered gross floor area, then enter 1 floor/storey. If Fire wall, then reduce footprint accordingly. Un-Protected 8 = number of floors above first 2, up to max of 10 floors total Protected 2 = number of additional immediately adjoining floors to be considered, up to 2 Do vertical openings have minimum 1 hour rating between floors? Confirm this with the architect. For unprotected openings scenario only, can be mix of podium and tower							
	Reductions or Surcharges							
3	Occupancy hazard reduction or surcharge Residential - with no garage Residential - with garage General Commercial - Generally, no reduction Check usage with FUS Check usage with FUS							
	Sprinkler Reduction							
4	Only Use if can be confirmed with client/architect Only Use if can be confirmed with client/architect Only Use if can be confirmed with client/architect							
	Exposure Surcharge (cumulative %)							
5	For Fire walls: FUS considers a Fire wall to have a mini	imum 2 hou	r rating per NBC.					
	Results							
6	NOTE: Refer to City Technical Bulletin ISDTB-2014-02	2 for additio	nal considerations to cap this v	alue at 10,000L/min				
	If IGPM is needed, divide USGPM by 1.20095							
7	For Rural areas, or where required							

APPENDIX E

Servicing Study Guidelines Checklist



4.1 General Content	Addressed (Y/N/NA)	Section	Comments
Executive Summary (for larger reports only).	NA		
Date and revision number of the report.	Y	p.1	
Location map and plan showing municipal address, boundary, and layout of proposed development.	Y	Dwgs	GP, GR, STM
Plan showing the site and location of all existing services.	Y	Dwg	GP
Development statistics, land use, density, adherence to zoning and official plan, and reference to applicable subwatershed and watershed plans that provide context to which individual developments must adhere.	Y	Intro	
Summary of Pre-consultation Meetings with City and other approval agencies.	N		
Reference and confirm conformance to higher level studies and reports (Master Servicing Studies, Environmental Assessments, Community Design Plans), or in the case where it is not in conformance, the proponent must provide justification and develop a defendable design criteria.	Y	Report	All sections
Statement of objectives and servicing criteria.	Y	Report	
Identification of existing and proposed infrastructure available in the immediate area.	Y	Dwg	GP
Identification of Environmentally Significant Areas, watercourses and Municipal Drains potentially impacted by the proposed development (Reference can be made to the Natural Heritage Studies, if available).	NA		
Concept level master grading plan to confirm existing and proposed grades in the development. This is required to confirm the feasibility of proposed stormwater management and drainage, soil removal and fill constraints, and potential impacts to neighboring properties. This is also required to confirm that the proposed grading will not impede existing major system flow paths.	Y	Report	



4.1 General Content	Addressed (Y/N/NA)	Section	Comments
Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.	NA		
Proposed phasing of the development, if applicable.	Y		
Reference to geotechnical studies and recommendations concerning servicing.	Y	Report	
All preliminary and formal site plan submissions should have the following information:			
Metric scale	Y		All Drawings
North arrow (including construction North)	Y		All Drawings
Key plan	Y		All Drawings
Name and contact information of applicant and property owner	Y		Drawings/Report
Property limits including bearings and dimensions	Y		Report
Existing and proposed structures and parking areas	Y		All Drawings
Easements, road widening and rights-of-way	Y		All Drawings
Adjacent street names	Y		All Drawings



4.2 Water	Addressed (Y/N/NA)	Section	Comments
Confirm consistency with Master Servicing Study, if available.	NA		
Availability of public infrastructure to service proposed development.	Y		
Identification of system constraints.	NA		
Identify boundary conditions.	NA		
Confirmation of adequate domestic supply and pressure.	NA		
Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development.	Y		Appendix
Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.	NA		
Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design.	NA		
Address reliability requirements such as appropriate location of shut-off valves.	Y		Drawings
Check on the necessity of a pressure zone boundary modification.	NA		
Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range.	NA		
Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions.	Y	Report	
Description of off-site required feedermains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.	NA		
Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.	Y	Report	
Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.	NA		



4.3 Wastewater	Addressed (Y/N/NA)	Section	Comments
Summary of proposed design criteria (Note: Wet-weather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed infrastructure).	Y	Report	
Confirm consistency with Master Servicing Study and/or justifications for deviations.	NA		
Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.	NA		
Description of existing sanitary sewer available for discharge of wastewater from proposed development. Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the	Y	Report	Drawings
proposed development. (Reference can be made to previously completed Master Servicing Study if applicable)	Y	Report	Appendix
Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.	NA		
Description of proposed sewer network including sewers, pumping stations, and forcemains.	Y		
Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).	NA		
Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.	NA		
Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.	NA		
Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.	NA		
Special considerations such as contamination, corrosive environment etc.	NA		



4.4 Stormwater	Addressed (Y/N/NA)	Section	Comments
Description of drainage outlets and downstream constraints including legality of outlet (i.e. municipal drain, right-of-way, watercourse, or private property).	Y	Report	
Analysis of the available capacity in existing public infrastructure.	NA		
A drawing showing the subject lands, its surroundings, the receiving watercourse, existing drainage patterns and proposed drainage patterns.	Y		GR, STM
Water quantity control objective (e.g. controlling post- development peak flows to pre-development level for storm events ranging from the 2 or 5 year event (dependent on the receiving sewer design) to 100 year return period); if other objectives are being applied, a rationale must be included with reference to hydrologic analyses of the potentially affected subwatersheds, taking into account long-term cumulative effects.	Y	Report	
Water Quality control objective (basic, normal or enhanced level of protection based on the sensitivities of the receiving watercourse) and storage requirements.	Y	Report	
Description of stormwater management concept with facility locations and descriptions with references and supporting information.	γ	Report	
Set-back from private sewage disposal systems.	NA		
Watercourse and hazard lands setbacks.	Y		
Record of pre-consultation with the Ontario Ministry of Environment and the Conservation Authority that has jurisdiction on the affected watershed.	Ν		
Confirm consistency with sub-watershed and Master Servicing Study, if applicable study exists.	N		
Storage requirements (complete with calcs) and conveyance capacity for 5 yr and 100 yr events.	Y		Appendix
Identification of watercourse within the proposed development and how watercourses will be protected, or, if necessary, altered by the proposed development with applicable approvals.	NA		
Calculate pre and post development peak flow rates including a description of existing site conditions and proposed impervious areas and drainage catchments in comparison to existing conditions.	Y		Appendix
Any proposed diversion of drainage catchment areas from one outlet to another.	NA		
Proposed minor and major systems including locations and sizes of stormwater trunk sewers, and SWM facilities.	Υ	Report	And Appendix
If quantity control is not proposed, demonstration that downstream system has adequate capacity for the post- development flows up to and including the 100-year return period storm event.	Y	Report	And Appendix



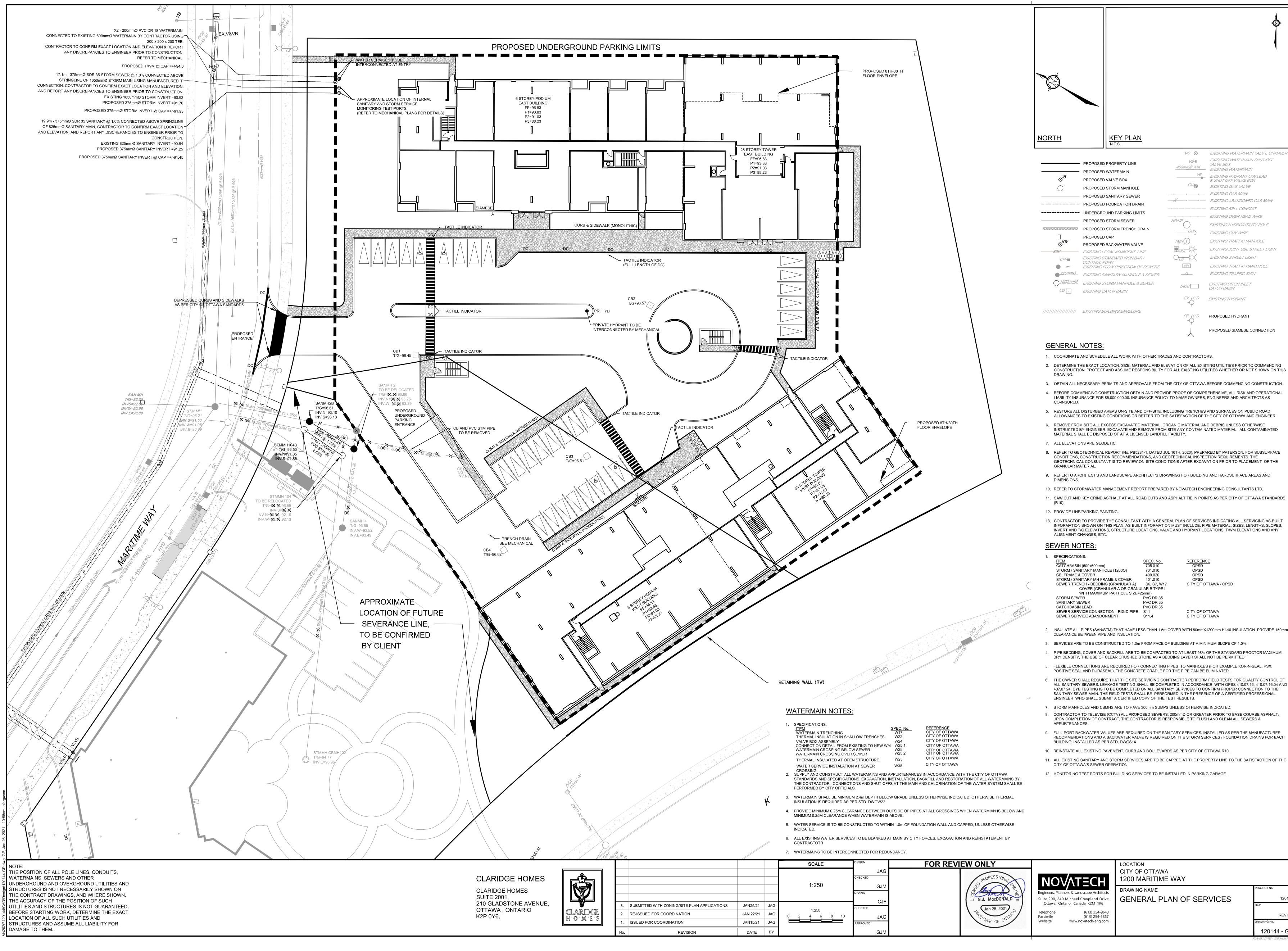
4.4 Stormwater	Addressed (Y/N/NA)	Section	Comments
Identification of municipal drains and related approval requirements.	Y	Report	
Description of how the conveyance and storage capacity will be achieved for the development.	Y	Report	
100 year flood levels and major flow routing to protect proposed development from flooding for establishing minimum building elevations (MBE) and overall grading.	Y		Appendix
Inclusion of hydraulic analysis including HGL elevations.	Y		Appendix
Description of approach to erosion and sediment control during construction for the protection of receiving watercourse or drainage corridors.	Y	Report	Drawings
Identification of floodplains – proponent to obtain relevant floodplain information from the appropriate Conservation Authority. The proponent may be required to delineate floodplain elevations to the satisfaction of the Conservation Authority if such information is not available or if information does not match current conditions.	NA		
Identification of fill constrains related to floodplain and geotechnical investigation.	NA		



4.5 Approval and Permit Requirements	Addressed (Y/N/NA)	Section	Comments
Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act.	NA		
Application for Certificate of Approval (CofA) under the Ontario Water Resources Act.	NA		
Changes to Municipal Drains.	NA		
Other permits (National Capital Commission, Parks			
Canada, Public Works and Government Services Canada,	NA		
Ministry of Transportation etc.)			

4.6 Conclusion	Addressed (Y/N/NA)	Section	Comments
Clearly stated conclusions and recommendations.	Y	Report	
Comments received from review agencies including the City of Ottawa and information on how the comments were addressed. Final sign-off from the responsible reviewing agency.	NA		
All draft and final reports shall be signed and stamped by a professional Engineer registered in Ontario.	Y	Report	

DRAWINGS

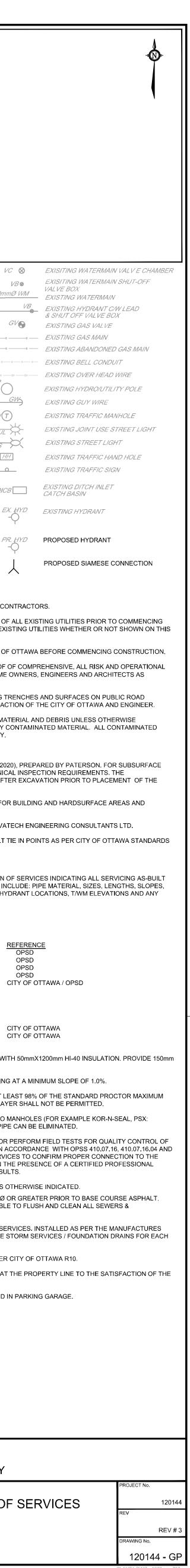


- 4. BEFORE COMMENCING CONSTRUCTION OBTAIN AND PROVIDE PROOF OF COMPREHENSIVE, ALL RISK AND OPERATIONAL LIABILITY INSURANCE FOR \$5,000,000.00. INSURANCE POLICY TO NAME OWNERS, ENGINEERS AND ARCHITECTS AS
- 5. RESTORE ALL DISTURBED AREAS ON-SITE AND OFF-SITE, INCLUDING TRENCHES AND SURFACES ON PUBLIC ROAD ALLOWANCES TO EXISTING CONDITIONS OR BETTER TO THE SATISFACTION OF THE CITY OF OTTAWA AND ENGINEER
- 6. REMOVE FROM SITE ALL EXCESS EXCAVATED MATERIAL, ORGANIC MATERIAL AND DEBRIS UNLESS OTHERWISE INSTRUCTED BY ENGINEER. EXCAVATE AND REMOVE FROM SITE ANY CONTAMINATED MATERIAL. ALL CONTAMINATED
- 8. REFER TO GEOTECHNICAL REPORT (No. PB5281-1, DATED JUL 16TH, 2020), PREPARED BY PATERSON. FOR SUBSURFACE CONDITIONS, CONSTRUCTION RECOMMENDATIONS, AND GEOTECHNICAL INSPECTION REQUIREMENTS. THE GEOTECHNICAL CONSULTANT IS TO REVIEW ON-SITE CONDITIONS AFTER EXCAVATION PRIOR TO PLACEMENT OF THE
- 9. REFER TO ARCHITECT'S AND LANDSCAPE ARCHITECT'S DRAWINGS FOR BUILDING AND HARDSURFACE AREAS AND
- 10. REFER TO STORMWATER MANAGEMENT REPORT PREPARED BY NOVATECH ENGINEERING CONSULTANTS LTD.
- 13. CONTRACTOR TO PROVIDE THE CONSULTANT WITH A GENERAL PLAN OF SERVICES INDICATING ALL SERVICING AS-BUILT INFORMATION SHOWN ON THIS PLAN. AS-BUILT INFORMATION MUST INCLUDE: PIPE MATERIAL, SIZES, LENGTHS, SLOPES, INVERT AND T/G ELEVATIONS, STRUCTURE LOCATIONS, VALVE AND HYDRANT LOCATIONS, T/WM ELEVATIONS AND ANY

-	SPECIFICATIONS:		
	ITEM	SPEC. No.	REFERENCE
	CATCHBASIN (600x600mm)	705.010	OPSD
	STORM / SANITARY MANHOLE (1200Ø)	701.010	OPSD
	CB, FRAME & COVER	400.020	OPSD
	STORM / SANITARY MH FRAME & COVER	401.010	OPSD
	SEWER TRENCH - BEDDING (GRANULAR A)	S6, S7, W17	CITY OF OTTAWA / OPSD
	COVER (GRANULAR A OR GRANU	ILAR B TYPE I,	
	WITH MAXIMUM PARTICLE SIZE=2	25mm)	
	STORM SEWER	PVC DR 35	
	SANITARY SEWER	PVC DR 35	
	CATCHBASIN LEAD	PVC DR 35	
	SEWER SERVICE CONNECTION - RIGID PIPE	S11	CITY OF OTTAWA
	SEWER SERVICE ABANDONMENT	S11,4	CITY OF OTTAWA

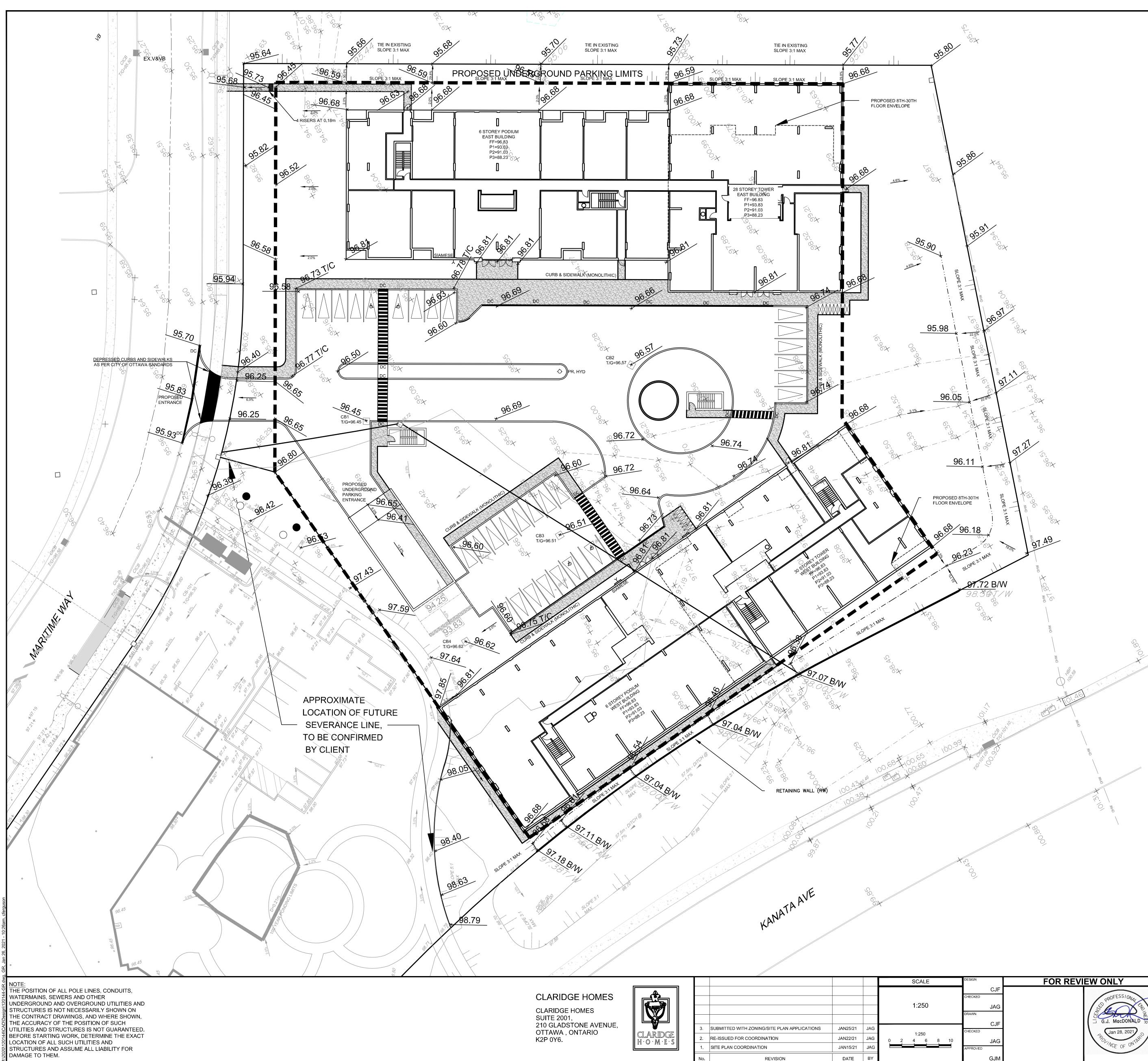
- INSULATE ALL PIPES (SAN/STM) THAT HAVE LESS THAN 1.5m COVER WITH 50mmX1200mm HI-40 INSULATION. PROVIDE 150mm
- 3. SERVICES ARE TO BE CONSTRUCTED TO 1.0m FROM FACE OF BUILDING AT A MINIMUM SLOPE OF 1.0%.
- DRY DENSITY. THE USE OF CLEAR CRUSHED STONE AS A BEDDING LAYER SHALL NOT BE PERMITTED.
- POSITIVE SEAL AND DURASEAL). THE CONCRETE CRADLE FOR THE PIPE CAN BE ELIMINATED.
- 6. THE OWNER SHALL REQUIRE THAT THE SITE SERVICING CONTRACTOR PERFORM FIELD TESTS FOR QUALITY CONTROL OF ALL SANITARY SEWERS. LEAKAGE TESTING SHALL BE COMPLETED IN ACCORDANCE WITH OPSS 410.07.16, 410.07.16.04 AND 407.07.24. DYE TESTING IS TO BE COMPLETED ON ALL SANITARY SERVICES TO CONFIRM PROPER CONNECTION TO THE SANITARY SEWER MAIN. THE FIELD TESTS SHALL BE PERFORMED IN THE PRESENCE OF A CERTIFIED PROFESSIONAL
- 7. STORM MANHOLES AND CBMHS ARE TO HAVE 300mm SUMPS UNLESS OTHERWISE INDICATED. 8. CONTRACTOR TO TELEVISE (CCTV) ALL PROPOSED SEWERS, 200mmØ OR GREATER PRIOR TO BASE COURSE ASPHALT. UPON COMPLETION OF CONTRACT, THE CONTRACTOR IS RESPONSIBLE TO FLUSH AND CLEAN ALL SEWERS &
- 9. FULL PORT BACKWATER VALUES ARE REQUIRED ON THE SANITARY SERVICES. INSTALLED AS PER THE MANUFACTURES RECOMMENDATIONS AND A BACKWATER VALVE IS REQUIRED ON THE STORM SERVICES / FOUNDATION DRAINS FOR EACH
- 10. REINSTATE ALL EXISTING PAVEMENT, CURB AND BOULEVARDS AS PER CITY OF OTTAWA R10.
- 12. MONITORING TEST PORTS FOR BUILDING SERVICES TO BE INSTALLED IN PARKING GARAGE.

GENERAL PLAN OF SERVICES

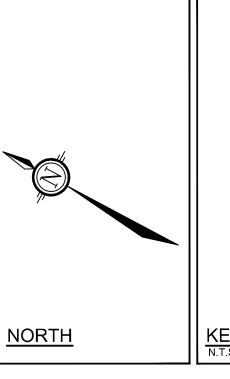


VAL VE BOX

CATCH BASIN



DAMAGE TO THEM.



KEY PLAN

LEGEND		GV	EXISTINO
	PROPOSED PROPERTY LINE		EXISTINC
	EXISTING LEGAL ADJACENT LINE	S S S S	- EXISTING
1 23			- EXISTING
×	EXISTING ELEVATION		EXISTING
33 ²²	PROPOSED GRADES	OH _O	- EXISTING
* 119		TTTTTT	EXISTING
9271151 9271151 1714	PROPOSED SWALE GRADES		EXISTING
	PROPOSED TOP OF RETAINING WALL	$\longrightarrow GW$	EXISTING
3 ³ 1. BW 3 ³ 1. BW	PROPOSED BOTTOM OF RETAINING WALL	TMHT	EXISTING
* \$		OJUL	EXISTING
331118	PROPOSED UNDERGROUND PARKING RAMP GRADES	O_{LS}	EXISTING
2.4 _%	PROPOSED SLOPE AND FLOW DIRECTION	HH	EXISTING
			EXISTING
FF=93.80	PROPOSED FINISHED FLOOR ELEVATION	SIB	EXISTING .
DC	PROPOSED DEPRESSED CURB	CB .	EXISITING
TD	TRENCH DRAIN		EXISITING
		DICB	EXISITING
փիկ	PROPOSED TERRACING SLOPE 3:1 MAX	///////////////////////////////////////	EXISTING B
	PROPOSED PONDING AREA	EX.CB	PROPOSED EXISTING C
$ \longrightarrow $	DIRECTION OF MAJOR OVERLAND FLOW		PROPOSED

GENERAL NOTES:

----- PROPOSED SWALE

- 1. COORDINATE AND SCHEDULE ALL WORK WITH OTHER TRADES AND CONTRACTORS.
- 2. DETERMINE THE EXACT LOCATION, SIZE, MATERIAL AND ELEVATION OF ALL EXISTING UTILITIES PRIOR TO COMMENCING CONSTRUCTION. PROTECT AND ASSUME RESPONSIBILITY FOR ALL EXISTING UTILITIES WHETHER OR NOT SHOWN ON THIS DRAWING.
- 3. OBTAIN ALL NECESSARY PERMITS AND APPROVALS FROM THE CITY OF OTTAWA BEFORE COMMENCING CONSTRUCTION. 4. BEFORE COMMENCING CONSTRUCTION OBTAIN AND PROVIDE PROOF OF COMPREHENSIVE, ALL RISK AND
- OPERATIONAL LIABILITY INSURANCE FOR \$5,000,000.00. INSURANCE POLICY TO NAME OWNERS, ENGINEERS AND ARCHITECTS AS CO-INSURED.
- 5. RESTORE ALL DISTURBED AREAS ON-SITE AND OFF-SITE, INCLUDING TRENCHES AND SURFACES ON PUBLIC ROAD ALLOWANCES TO EXISTING CONDITIONS OR BETTER TO THE SATISFACTION OF THE CITY OF OTTAWA AND ENGINEER.
- 6. REMOVE FROM SITE ALL EXCESS EXCAVATED MATERIAL, ORGANIC MATERIAL AND DEBRIS UNLESS OTHERWISE INSTRUCTED BY ENGINEER. EXCAVATE AND REMOVE FROM SITE ANY CONTAMINATED MATERIAL. ALL CONTAMINATED MATERIAL SHALL BE DISPOSED OF AT A LICENSED LANDFILL FACILITY. 7. ALL ELEVATIONS ARE GEODETIC.
- 8. REFER TO GEOTECHNICAL REPORT (No. PB5281-1, DATED JUL 16TH, 2020), PREPARED BY PATERSON) FOR SUBSURFACE CONDITIONS, CONSTRUCTION RECOMMENDATIONS, AND GEOTECHNICAL INSPECTION REQUIREMENTS. THE GEOTECHNICAL CONSULTANT IS TO REVIEW ON-SITE CONDITIONS AFTER EXCAVATION PRIOR
- TO PLACEMENT OF THE GRANULAR MATERIAL. 9. REFER TO ARCHITECT'S AND LANDSCAPE ARCHITECT'S DRAWINGS FOR BUILDING AND HARDSURFACE AREAS AND
- DIMENSIONS. 10. REFER TO SERVICEABILITY AND STORMWATER MANAGEMENT REPORT PREPARED BY NOVATECH ENGINEERING CONSULTANTS LTD.
- 11. SAW CUT AND KEY GRIND ASPHALT AT ALL ROAD CUTS AND ASPHALT TIE IN POINTS AS PER CITY OF OTTAWA STANDARDS (R10).
- 12. PROVIDE LINE/PARKING PAINTING.
- 13. ALL MATERIALS AND CONSTRUCTION METHODS SHALL BE IN ACCORDANCE WITH THE CITY OF OTTAWA STANDARDS AND SPECIFICATIONS AND ONTARIO PROVINCIAL STANDARDS AND SPECIFICATIONS. ONTARIO PRO STANDARDS AND SPECIFICATIONS WILL APPLY WHERE NO CITY STANDARDS ARE AVAILABLE.
- 14. ALL PRIVATE APPROACHES MUST BE CONSTRUCTED AS PER CITY SPECIFICATION SC13.
- 15. ALL EXISTING INFRASTRUCTURE (STORM AND SANITARY NETWORK & MANHOLES) ON-SITE TO BE REI

GRADING NOTES

- 1. ALL TOPSOIL, ORGANIC OR DELETERIOUS MATERIAL MUST BE ENTIRELY REMOVED FROM BENEATH T PROPOSED PAVED AREAS AS DIRECTED BY THE SITE ENGINEER OR GEOTECHNICAL ENGINEER.
- 2. EXPOSED SUBGRADES IN PROPOSED PAVED AREAS SHOULD BE PROOF ROLLED WITH A LARGE STEE ROLLER AND INSPECTED BY THE GEOTECHNICAL ENGINEER PRIOR TO THE PLACEMENT OF GRANULA
- 3. ANY SOFT AREAS EVIDENT FROM THE PROOF ROLLING SHOULD BE SUB-EXCAVATED AND REPLACED SUITABLE MATERIAL THAT IS FROST COMPATIBLE WITH THE EXISTING SOILS AS RECOMMENDED BY T GEOTECHNICAL ENGINEER.
- 4. THE GRANULAR BASE SHOULD BE COMPACTED TO AT LEAST 100% OF THE STANDARD PROCTOR MAX DENSITY VALUE. ANY ADDITIONAL GRANULAR FILL USED BELOW THE PROPOSED PAVEMENT SHOULD COMPACTED TO AT LEAST 95% OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY VALUE.
- 5. MINIMUM OF 2% GRADE FOR ALL GRASS AREAS UNLESS OTHERWISE NOTED.
- 6. MAXIMUM TERRACING GRADE TO BE 3:1 UNLESS OTHERWISE NOTED.
- 7. ALL GRADES BY CURBS ARE EDGE OF PAVEMENT GRADES UNLESS OTHERWISE INDICATED.
- 8. ALL CURBS SHALL BE BARRIER CURB (150mm) UNLESS OTHERWISE NOTED AND CONSTRUCTED AS P OTTAWA STANDARDS (SC1.1).
- 9. REFER TO LANDSCAPE PLAN FOR PLANTING AND OTHER LANDSCAPE FEATURE DETAILS.
- 10. CONTRACTOR TO PROVIDE THE CONSULTANT WITH A GRADING PLAN INDICATING AS-BUILT ELEVATI DESIGN GRADES SHOWN ON THIS PLAN.

PAVEMENT STRUCTURE:

- LIGHT DUTY 50mm HL3 150mm GRAN "A" 300mm GRAN "B" TYPE II
- HEAVY DUTY 40mm HL3
- 50mm HL8 150mm GRAN "A" 450mm GRAN "B" TYPE II

LE	DESIGN	FOR REVI	EW ONLY
	CJF		\bigcap
	CHECKED		PROFESSIONAL
50	JAG		PROFESSIONAL G.J. MacDONALD
	DRAWN		SCHAR AN
	CJF		⊐ G.J. MacDONALD 🛒
0	CHECKED		Jan 28, 2021
6 8 10	JAG		PROLINCE OF ONTARIO
	APPROVED		ICE OF O
	GJM		

REVISION



LOCATION CITY OF OTTAWA 1200 MARITIME WAY

DRAWING NAME GRADING AND EROSION SEDIMEN CONTROL PLAN

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IT	PROJECT №. 120144				
-	REV REV # 3				
	DRAWING No.				
ŀ	120144-GR PLANB1.DWG - 1000mmx707mn				

SED FILTER CLOTH IN G CATCHBASIN BED SILT FENCE

NG CATCHBASIN IG DITCH INLET CATCHBASI

VG TRAFFIC SIGN G STANDARD IRON BAR

NG STREET LIGHT IG TRAFFIC HAND HOLE

IG JOINT USE STREET LIGH

IG TRAFFIC MANHOLE

IG GUY WIRE

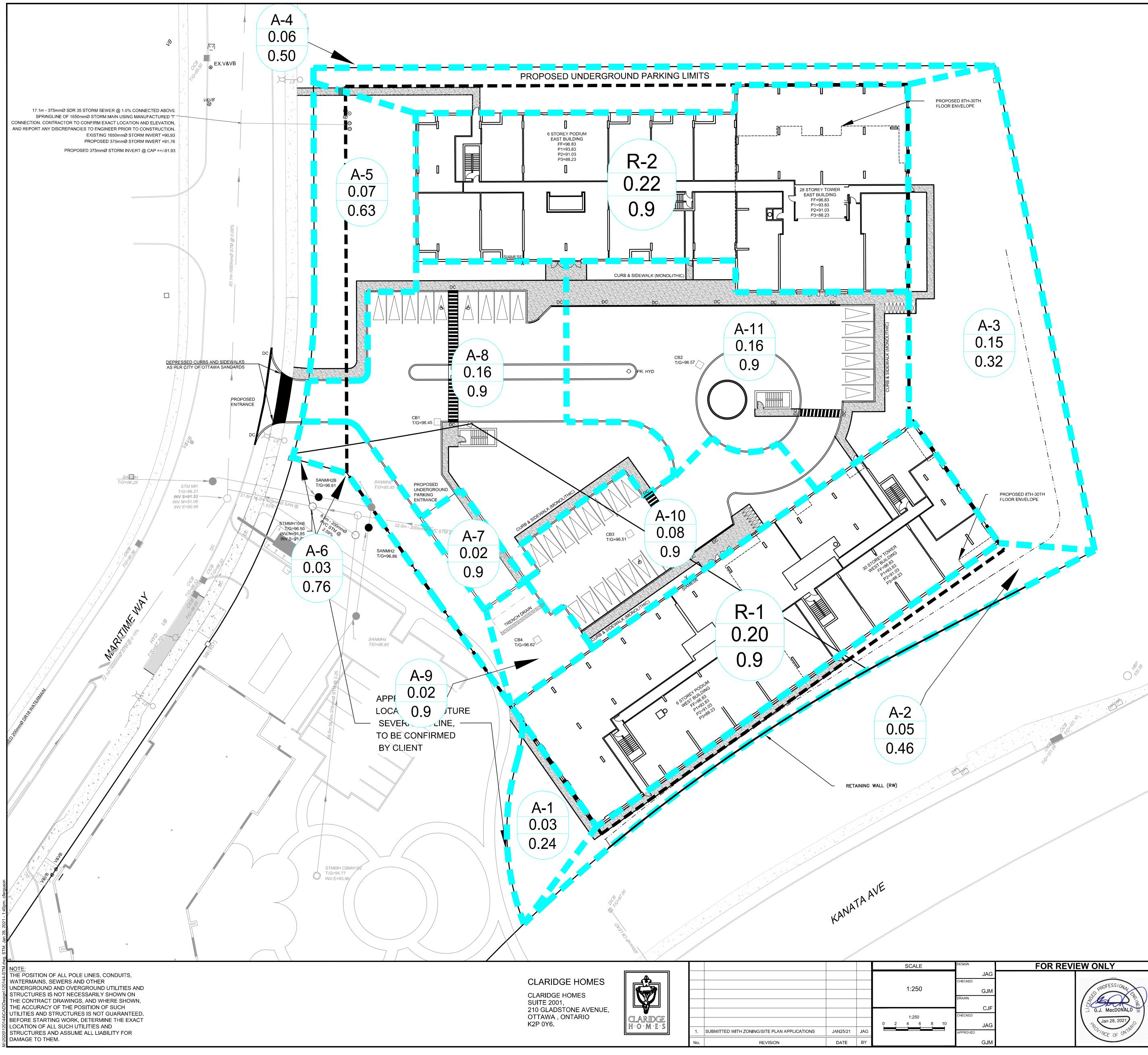
NG HYDRO/UTILITY POLE

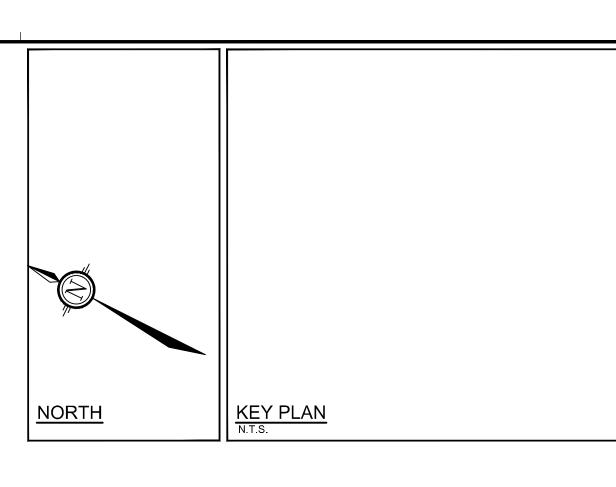
NG TRAFFIC CONDUIT

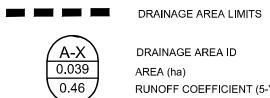
ING OVER HEAD WIRE

ING ABANDONED GAS MAII ING BELL CONDUIT

TING GAS VALVE ING GAS MAIN







DRAINAGE AREA ID AREA (ha) RUNOFF COEFFICIENT (5-YEAR)

STORMWATER MANAGEMENT NOTES:

- REFER TO 'SERVICEABILITY AND STORMWATER MANAGEMENT REPORT' PREPARED BY NOVATECH ENGINEERING CONSULTANTS LTD.
- 2. NO ROOF TOP CONTROL ALLOWED.
- 3. TOTAL AREA: 1.28 ha

CALE	DESIGN	FOR REVIEW ONLY	
	JAG		
1:250	CHECKED		PROFESSIONAL
			G.J. MacDONALD
1:250 4 6 8 10	CHECKED		
	JAG APPROVED		BOLINCE OF ONTARIO
	GJM		



LOCATION CITY OF OTTAWA MARITIME WAY - KANATA RENTAL

DRAWING NAME STORM WATER MANAGEMENT PLAN

