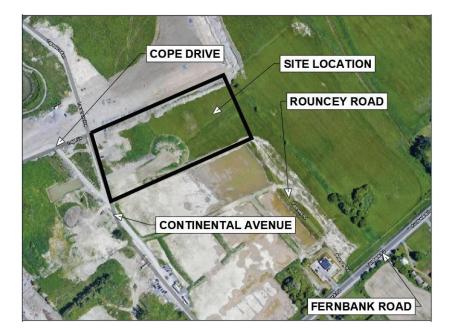


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Stormwater Management Report Fernbank Elementary School

480 Cope Drive, Ottawa, Ontario



Prepared for



City of Ottawa Infrastructure Services and Community Sustainability 110 Laurier Ave. West, 4th floor, Mail Code 01-14 Ottawa, Ontario, K1P 1J1

November 28, 2019

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*	SD-1	Stormwater Management Sub-Drainage Areas
*	Appendix A	Background Excerpts
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*	Appendix C	Watts Drainage Adjustable Flow Control for Roof Drains – Data Sheet

1 Introduction

Jp2g Consultants Inc. was retained by N45 Architecture Inc. to complete a Stormwater Management Report suitable for City of Ottawa Site Plan Control Application, for the Ottawa Carleton District School Board's proposed Elementary School and Daycare Addition at Cope Drive and Rouncey Road, in the City of Ottawa. The total site area is approximately 2.84 ha and the proposed site development area includes the construction of a 360 m² one-storey day-care building, associated parking areas, play area, and landscaped areas. A pre-Consultation meeting was held with City of Ottawa staff on August 23, 2019, to determine the project constraints and requirements. The following report details the stormwater management calculations used for water quantity and quality control in accordance with the City of Ottawa's requirements.

Reference Drawings: SD1 – Stormwater Management Sub-Drainage Areas, C02 – Servicing Tables, C03 - Site Servicing Plan (November 28, 2019), and C04 - Site Grading and Drainage, Erosion and Sediment Control Plan (November 28, 2019).

2 Objective

The objective of the stormwater management plan is to control post-development peak flows to predetermined levels, and detain onsite, stormwater up to and including the 100-year storm event without affecting adjacent lands. Stormwater quality control will be provided by the downstream pond 6, no onsite quality control is required.

3 Design Parameters

Stormwater management criteria for this site, in terms of quantity control, is based on the following approved reports:

Servicing and Stormwater Management Report – Blackstone Community Phase 4-8 by Stantec, April 11, 2019.

The maximum allowable release rate for this site shall not exceed the criteria set in the approved servicing report. Flows in excess of the allowable release rate up to and including the 100-year event will be detained onsite.

The Modified Rational Method (Q = 2.78CiA) was chosen to calculate the post-development release rates, and onsite storage requirements for this development. Detailed stormwater management calculations are included in **Appendix B**. All proposed storm sewers were assigned a Manning's coefficient of roughness of 0.013 corresponding to smooth wall pipes. In accordance with City of Ottawa Sewer Design Guidelines (Section 5.4.5.2.1), the coefficients used for calculating the post-development release rate were C = 0.25 for grassed areas and C = 0.90 for hard surfaced areas including rooftops. The rainfall intensities used in this analysis are based on the IDF curves and equations, as per City of Ottawa Sewer Design Guidelines (Section 5.4.2).

4 Water Quantity Controls

4.1 Pre-Development Conditions

The existing site is an undeveloped parcel with a generally flat site topography that is sloped toward the east side of the property with an approximate elevation difference of 0.5 meter over approximately a 220-meter length. The proposed day-care facility will be constructed at the same time as the main school building. Services have been installed along Rouncey Road, as part of the development of the Blackstone Community Phase 4-8.

4.2 Allowable Release Rate

The stormwater management design criteria for this site is based on the subdivision servicing report as noted above. According to the Stantec study, the school site has an allowable release rate up to the 5 year-event of **575.7 L/s**. Refer to **Appendix A** – Background excerpts – Storm Water Management. Minimum storage requirement was set to **50 m3 / ha** per the Subdivision study.

4.3 Post-Development Conditions

Proposed site grading and drainage of the overall site was designed such that stormwater runoff will be collected by the roof and a new storm water collection system which will be connected to the existing 1200mm diameter municipal storm sewer on Rouncey Road and discharged to Pond 6 of the subdivision.

The storm sewer system consists of manholes, catchbasins, storm sewers, perforated subdrain system in landscaped area of the sportsfield. Most of the drainage areas are piped to the storm system except for the grassed area fronting onto Cope Drive.

No surface ponding will occur during the 5-year event. Flows exceeding the allowable release rate up to the 100-year event are to be detained on site.

The overall site development area is approximately **2.84 ha** and has a post-development average weighted runoff coefficient of **C=0.60**, and **C=0.65** for the maximum and 100-year events, respectively. Overall onsite storage requirement was calculated to be **254 m³** for the 100-year event, which exceeds the minimum storage requirement of $50m^3$ /ha.

4.4 Onsite Stormwater Detention

Stormwater detention is proposed on the school roof, in the proposed parking area, in the paved school yard as well as along the sportsfield swale.

The maximum allowable ponding depth will be limited to 250mm for paved areas and 300mm in grassed areas during 100-year events. The maximum ponding elevation is 100.85 which is below the building finished floor elevation of 101.25.

Flow is to be restricted by installing an IPEX Tempest orifice plate at CBMH6 with a discharge rate of **530.0** L/s at an estimated head **2.11 m**.

Flow from the combined school and daycare roof will also be detained on the roof by installing parabolic weirs, (Watts Drainage Adjustable Flow Control for Roof Drains, or equivalent approved products), at the 29 proposed roof drains limiting the flow from the roof to **18.3 L/s**. The resulting required storage is **153 m**³ for the 100-year event. Based on a maximum ponding depth of 150mm on the roof, the total available storage is approximately **194 m**³, which is sufficient to accommodate the 100-year event. Each flow control roof drain, complete with a single slot parabolic weir, will restrict flow at 5 GPM (0.32 L/s) per inch (25.4mm) of head to a maximum of 10 GPM (0.63 L/s). The restricted flow will outlet to the school's 300mm diameter storm sewer service at 4.0% slope. The storm sewer service is connected to STMH1, upstream of the ICD; a backflow preventer valve will be installed in the mechanical room.

Total Area	Controlled /Uncontrolled	Run-off Coef	ficient	Outlet Location	Total Storage	100 Year e	vent
					Provided		
		5 year	100 year			Restricted	Required
						Flow	Storage
2.611	Controlled	0.62	0.67	STMH1	386	530	206
0.227	Uncontrolled	0.37	0.39	Cope Drive ROW		43.8	0
2.838					386	573.8	206

The following table summarizes on-site requirements during the 100-year event:

Note that storage within the subdrain clearstone trench was not included within the available storage volume. There is sufficient available ponding to accommodate the 100-year event.

The maximum ponding limits are indicated on **SD1** and grading plan. In the event of a rainfall exceeding the 100-year event, runoff will remain on-site until ponding reaches the overspill elevation. In such event, the school yard and sport field major overland flow route will overflow towards Rouncey Drive and the parking area will overflow to Continental Avenue. The grade elevation at the overflow point is greater than **0.30** m below the school's finished floor elevation.

4.5 Proposed Release Rates

The proposed release rate for this site during the 100-year event, including uncontrolled flows (43.8 l/s) and controlled flows (530 L/s) into the minor system of is 573.8 L/s. Therefore, proposed release rates are within the allowable release rate for this site, determined to be 575.7 L/s in Section 4.2.

5 **Erosion and Sediment Control**

In accordance with City of Ottawa requirements, best management practices are to be implemented by the Contractor to provide protection of the area drainage system and the receiving water course, during construction activities. This includes limiting the amount of exposed soil, using filter bag inserts under the

grates of catch basins and manholes, installing silt fences and other effective sediment traps, and installing and maintaining mud mats for outgoing construction traffic during construction activities.

6 Conclusion

The proposed site development includes a new school building with an attached daycare, a bus lay-by, asphalt parking, hard surface walkways and play areas, landscaped areas, a sports field and an area for portables. Roof drainage and surface runoff will be collected by a new storm sewer system which will be connected to the existing 1200mm diameter municipal storm sewer located on Rouncey Road. Post-development peak flows will be detained on the roof, in the parking area, in the school yard and in the bus lay-by in order to limit the post-development release rate to allowable levels. There is sufficient onsite storage to accommodate the 100-year event.

ID	Description	Value/result
01	Allowable release rate	Q _{allowable} = 575.7 L/s
02	Proposed release rate	Q _{100-yr post} = 573.8 L/s
03	Post-development runoff coefficient	C _{5-yr post} = 0.60, C _{100-yr post} = 0.65
04	Post-development onsite storage requirement	206 m ³
05	Proposed onsite storage	Parking lot and school yard: 191 m ³ , Roof: 153m ³ , Sportsfield: 34 m ³ Lay-by: 8m ³
06	Discharge outlet location	1200mm
07	Emergency runoff overflow locations	West side – curb cut in parking area towards Continental Avenue East side – from school yard to layby towards Rouncey Road

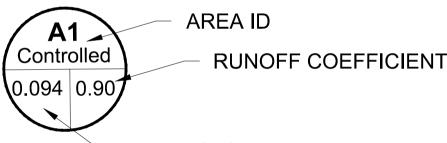
Summary of report

END OF REPORT



Barbra Kimmerle, P.Eng. Civil Engineer

PONDIN	G TABLE				
POND NO.	LOCATION	POND ELEV. (m)	TOP OF CB ELEV. (m)	POND DEPTH (m)	POND VOL. (m³)
P1	CB1	100.85	100.60	0.25	35.21
P2	CB2	100.90	100.67	0.23	31.99
P3	CBMH1	100.90	100.70	0.20	28.37
P4	CBMH2	100.92	100.72	0.20	41.03
P5	СВМНЗ	100.85	100.65	0.20	15.50
P6	RYCB1	100.90	100.68	0.22	3.89
P7	CBMH4	100.75	100.60	0.15	11.20
P8	CB3	100.70	100.55	0.15	16.95
P9	CBMH5	100.70	100.56	0.14	10.85
P10	CB4	100.59	100.47	0.12	2.39
P11	TCB1 - TCB11	100.96	100.90	0.06	10.36
P12	TCB12	100.90	100.75	0.15	2.96
P13	TCB13	100.90	100.75	0.15	3.61
P14	RYCB2	100.89	100.76	0.13	10.00
P15	TCB14	100.86	100.71	0.15	6.24
P16	TCB15	100.81	100.66	0.15	3.76
P17	TCB16	100.76	100.61	0.15	3.31
P18	СВМН6	100.62	100.50	0.12	5.41
P19	ROOF			0.12	153.01
				Total=	396.04



AREA (ha)

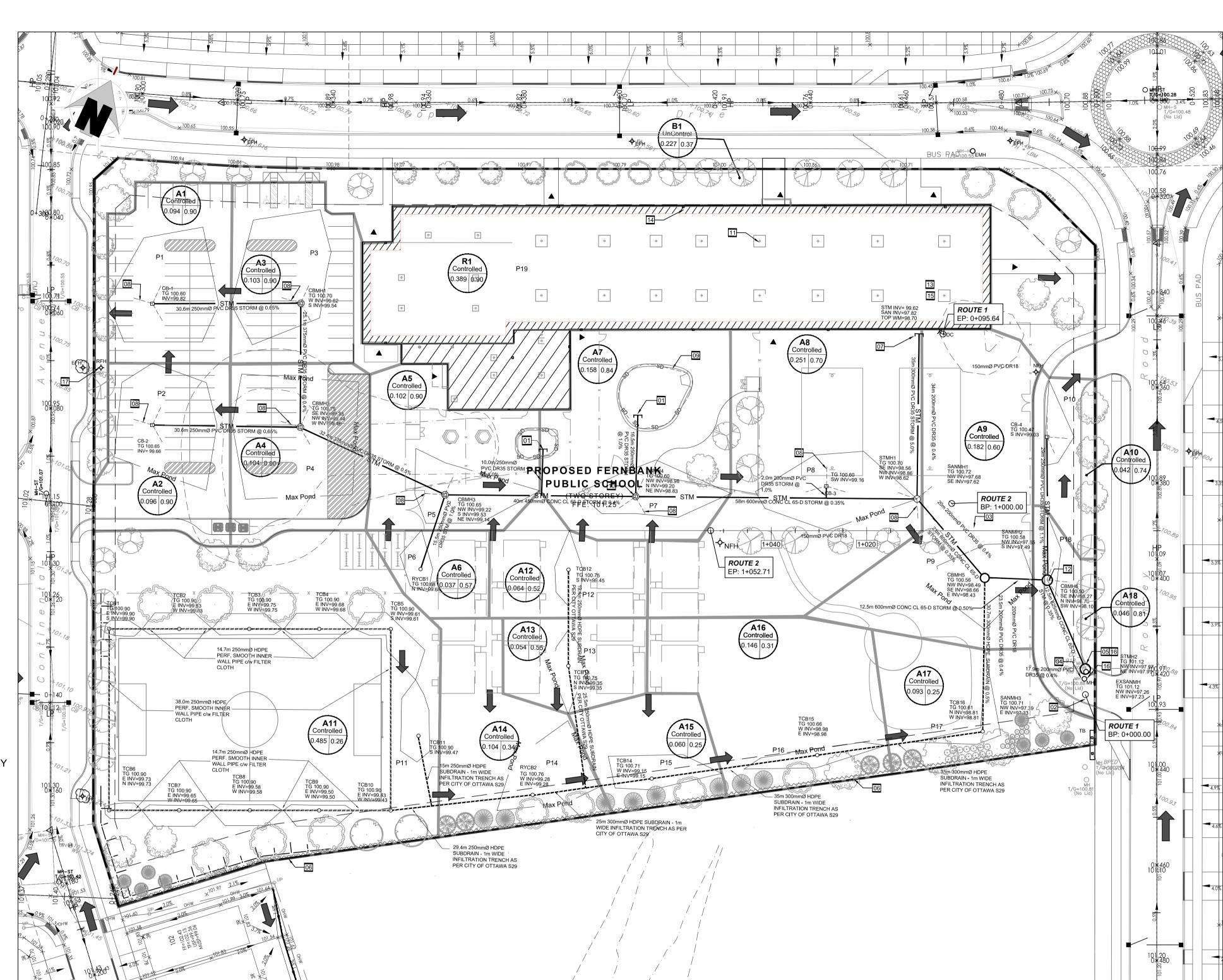
PROPOSED BUILDING

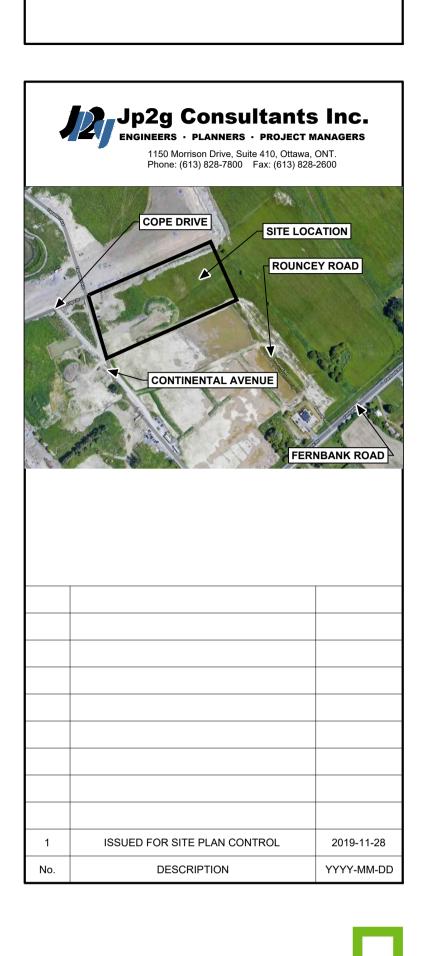
PROPOSED DRAINAGE AREA BOUNDARY

Folder: J:\5-Civil\2019\19-5070A - N45 New OCDSB Fernbank Elementary School\08 Drawings\04 ONGOING | Drawing: Fernbank Civil 11-26-2016.dwg | Layout: DRAINAGE | Print date: 4:25 PM November 28, 2019



MAJOR OVERLAND FLOW ROUTE







43 Eccles Street, 2nd Floor -	Ottawa, Ontario, K1R 6S3
tel. 613.224.0095	fax 613.224.9811

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drawing title STORM DRAINAGE PLAN drawn by _{M.S.} scale 1:500 date checked by NOV 2019 B.K. drawing number project number SD1 revision CONTRACTOR TO VERIFY ALL DIMENSIONS AND NOTIFY THE ARCHITECT OF ANY DISCREPANCIES BEFORE WORK COMMENCES.

DO NOT SCALE DRAWINGS.

Appendix A - Background Excerpts Stormwater Management

		The of FLOW (min)	0.50	0.15	1.54 1.41 1.13 1.64	0.44	0.16	1.91 1.91 0.92 0.92	1.96 1.05 1.05 0.89	2.53 2.10 1.65	0.84	1.27 0.22 1.27 0.55 0.77	1.16	1.72	1.07	1.84	1.11	0.93	0.00 0.00 2.14	1.35	1.99	1.99	0.14 0.79	1.69 0.22 1.97 1.73 0.29	1.81	0.18 1.18	1.34
		VEL T (ACT) I (m/s)	2.60	1.42	0.70 0.76 0.73 0.72	0.86	1.32	1.05 1.04 1.02	0.62 0.71 0.71 0.84	0.85 0.83 0.91	0.76	1,19 1,18 1,26 1,25 0,91	1.13	1.04	0.75	0.96	1.26	0.79 (0.00 0.00 0.08	1.01	0.69	0.93	1.18 (0.76 (0.74 0.72 0.72 0.75 0.75 0.75 0.73 0.73 0.73	0.75 1	1.01 0	1.10 1
		VEL. (mis)	2.64	1.42	0.81 0.98 0.99 0.91	0.91	1.36	1.10 1.19 1.19	0.81 0.78 0.78 0.92	0.91 0.90 0.98	1.10	150 150 105	1.10	1.10	0.78	0.99	1.10	0.86	0.86 0.86 0.91 0.91	1.11	0.81 0.86	1.10	1.26	0.97 0.97 0.98 0.98 0.98 0.98 0.98 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92	0.86	1.10	1.19
		e entr	79.35% 81.52%	84.57%	51.75% 36.41% 30.85% 37.71%	70.37%	76.89%	71.09% 53.78% 50.54% 51.39%	35.13% 63.34% 60.52% 60.40%	66.69% 1 66.25% 1 63.83% 1	24.33%	39.29% 37.57% 47.30% 45.37% 52.26%	64.78% 58.51%	69.08%	74.90% (.80%	70.85%	61.93% (0.00% 0.00% 77.30%	60.47%	48.10% (21.06% (48.07% 1	67.23% 1 66.71% 0	34.62% 0 31.95% 0 34.71% 0 39.88% 0 37.30% 0	53.76% 0	63.34% 1 49.02% 1	04% 1
		Gev ⁵ (FUL)	8131.0 79	317.2 84	133.0 5 ⁻ 286.5 38 449.8 30 597.2 31	597.2 70	748.8 76	1286.2 71 1760.8 53 1760.8 50 1760.8 50	133.0 34 173.8 63 173.8 63 339.6 60	148.7 66 200.6 66 286.5 63	77.5 24	554.6 39 554.6 37 554.6 37 554.6 47 554.6 45 554.6 45 550.0 52	77.5 64	77.5 69	173.8 74	04.3 74	77.5 70 257.6 41	473.6 61	60.8 0. 60.8 0. 148.7 77	731.4 60	133.0 48 473.6 21	286.2 48	132.9 67 133.0 66	68.0 34 68.0 31 58.5 34 339.6 33 339.6 33	473.6 53	1760.8 49	1760.8 64.04%
		SPE &	0.30 8	0.50 3	0.20 1 0.20 2 0.15 4 0.10 5	0.10 5	0.25 7	0.0 0.0 0.0 0.0 0.0 0.0	0.20 1 0.15 1 0.15 3 0.15 3	0.25 1 0.20 2 0.20 2	0.65	0.40 5 0.40 5 0.40 5 0.40 5 0.40 5 0.40 5 0.40 5 0.40 5 5 0.40 5 5 5 0.40 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	0.65 7	0.65 7	0.15 1	0.40 1	0.65 7 0.75 23	0.10 4	0.40 6 0.40 6 0.25 1	0.15 73	0.20 10	0.10 12	0.65 10	0.50 6 0.50 6 0.20 26 0.15 33	0.10 47	0.10 12 0.10 17	0.10 17
		PIPE SELECTION CLASS SLO (-)																•				•			0	00	.0
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				AR COM	20 23 23			CO CO CO			~			~		8		R CONCRETE	8	R CONCRETE		R CONC		R PVC R PVC R CONCRETE R CONCRETE R CONCRETE	R CONCRETI	R CONCRETE R CONCRETE	R CONGRETE
		PIPE SHAPE (-)	CIRCULAR	CIRCUL	CIRCULAR CIRCULAR CIRCULAR CIRCULAR	CIRCULAR	CIRCULAR	CIRCULAR CIRCULAR CIRCULAR CIRCULAR	CIRCULAR CIRCULAR CIRCULAR CIRCULAR	CIRCULAR CIRCULAR CIRCULAR	CIRCULA	CIRCULAR CIRCULAR CIRCULAR CIRCULAR CIRCULAR	CIRCULAR	CIRCULAF	CIRCULAR	CIRCUL	CIRCULAR	CIRCULAR	CIRCULAR	CIRCULAR	CIRCULAR	CIRCULAR	CIRCULAR	CIRCULAR CIRCULAR CIRCULAR CIRCULAR CIRCULAR	CIRCULAR	CIRCULAR	CIRCULAR
		H PIPE E HEIGHT (mm)	1950 2100	525	450 600 750 900	006	825	1200 1350 1350 1350	450 525 525 675	450 525 600	300	675 675 675 675 825	300	300	525	375	300 450	825	300 300 450	006	450 825	1200	375 450	300 300 675 675	825	1200	1350
		PIPE WIDTH OR DIAMETE (mm)	1950 2100	525	450 600 750 900	006	825	1200 1350 1350 1350	450 525 525 675	450 525 600	300	675 675 675 675 825	300 375	300	525	375	300 450	825	300 300 450	800	450 825	1200	375 450	300 300 675 675	825	1200	1350
		LENGTH (m)	78.3	12.9	64.5 64.5 49.4 70.3	22.9	13.0	120.0 119.0 55.9 56.7	73.3 44.9 44.5	128.6 105.3 89.5	38.3	91.3 15.8 95.7 41.4 42.1	70.8	107.2	48.3	105.7	69.9 83.4	44.0	85.0 11.7 113.4	82.0	82.0 102.4	111.7	9.7 35.9	75.3 9.6 89.0 76.1 12.2	81.3	11.0 71.4	88.2
		GACT (CIM360) (U/s)	6451.8 6594.9	268.3	68.8 104.3 138.8 225.2	420.3	575.7	914.4 947.0 889.9 904.8	48.7 110.1 105.2 205.1	99.2 132.9 182.9	18.9	217.9 208.4 262.3 251.6 303.1	50.2 77.8	53.6	130.2	78.0	54.9 105.6	293.3	0.0 0.0 115.0	442.3	64.0 99.7	618.2	89.3 88.7	23.5 21.7 99.4 135.5 126.7	254.6	814.7 863.2	1127.6
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		Gcorreo. (Us)	6451.8	0.0	0.0	0.0	0.0	0.0.0	0.0.0	0.0	0.0	88888	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 0.0 0.0	0.0	0.0	0.0
		isovere (mmfh)	178.56	178.56	178.56 165.63 155.47 148.27	139.05	178.56	136.77 127.80 120.04 116.68	178.56 162.42 155.02 148.32	178.56 158.34 144.99	178.56	136.15 130.08 129.08 123.70 121.51	178.56	178.56	159.30	178.56	178.56	152.09	178.56 178.56 178.56	146.37	178.56	138.87	178.56	178.56 164.46 162.80 149.48 139.58	138.08	129.24 128.43	118.58
		isoraer (mmh)	122.14	122.14	122.14 113.34 106.42 101.52	95.23	122.14	93.68 87.56 82.27 79.98	122.14 111.15 106.12 101.55	122.14 108.38 99.28	122.14	93.26 89.12 88.44 84.77 83.27	122.14 115.36	122.14	109.03	122.14	122.14	104.12	122.14 122.14 122.14	100.22	122.14	95.11	122.14	122.14 112.54 111.41 102.34 95.59	94.57	88.54 88.00	81.27
	80	is-rave (mm/h)	104.19	104.19	104.19 96.72 90.84 86.67	81,33	104.19	80.01 74.80 70.30 68.35	104.19 94.85 90.58 86.70	104.19 92.50 84.77	104.19	79.65 76.13 75.55 72.43 71.15	104.19 98.43	104.19	93.06	104.19	104.19 98.66	88.89	104.19 104.19 104.19	85,57	104.19 94.74	81.23	104.19 103.48	104.19 96.04 95.08 87.37 81.63	80.77	75.64	69.45
	EDDING CLASS =	Anarch (mmh)	76.81	76.81	76.81 71.37 67.08 64.04	60.14	76.81	59.17 55.37 52.07 50.64	76.81 70.01 66.89 64.06	76.81 68.30 62.66	76.81	58.91 56.34 55.91 53.63 52.69	76.81 72.61	76.81	68.70	76.81	76.81 72.78	65.66	76.81 76.81 76.81	63.24	76.81 69.92	60.06	76.81 76.29	76.81 70.87 70.17 64.55 60.36	59,73	55.98 55.64	51.45
	BEDDING	T of C R) (min)	10.00 10.50 11.08	10.00	10.00 11.54 12.95 14.07 15.71	15.71 16.15	10.00	16.15 18.07 19.98 20.89 21.81	10.00 11.96 13.01 14.07 14.95	10.00 12.53 14.63 16.28	10.00	16.28 17.55 17.77 19.04 19.59 20.37	10.00 11.16 12.39	10.00	12.39	10.00	10.00 11.11 12.21	13.46 14.39	10.00 10.00 12.14	14.39 15.74	10.00 11.99 15.01	15.74 17.74	10.00 10.14 10.92	10.00 11.69 11.91 13.88 15.61 15.61	15.90	17.74 17.92 19.10	20.37
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	0.013 2.00 10	AxC (100-YEAF (ha)	0.000	0.541	0.000 0.0000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000	0000	0.000	0.000 0.000 0.000	0.000 0.000 0.0000	0.000 0.000 0.000	0.000	0.000 0.000 0.000 0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000 0.000.0	0000	0.000	0.000	0.000	0.000 0.000 0.000 0.000 0.000	0.000	0.000	0.000
	awa Guidelines, 2012) MANNING'S n = MINIMUM COVER: TIME OF ENTRY	ACCUM.) AKC (10YR) (ha)	000.0	0.000	0.000 0.0000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000	0.000	0.000	0.000 0.0000 0.0000	0.000	0.000 0.000 0.000	0.000	0.000 0.0000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000 0.000.0	0.000	0.000	0.000	0.000	0.000 0.0000	0.000	0.000	0.000
		REA AxC (10-YEAR) (ha)	0.000	0.000	0.000 0.0000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000	0.000	0.000	0,000 0,000 0,000	0.000 0.000 0.000	0.000 0.000 0.000	0.000	0.000 0.0000 0.0000	0.000	000'0	0000	0.000	0.000	0.000	0.000 0.000	0.000	0.000	0.000	0.000	0.000 0.000 0.000 0.000 0.000	0.000	0.000	0.000
	1:100 yr Uni 1:100 yr 1735.684 6.014 0.820		0.000	0.000	0.238 0.388 0.388 0.774	0.774	1.989	3.028 3.471 3.471 3.680	0.000 0.000 0.000	0.000 0.000.0	0.000	0.000 0.0000 0.0000 0.0000	0.000	0.000	0.000	0.000	0.000	0.000	0.000 0.000 0.000	0.000	0.000	0.000	0.000	0.000 0.000 0.000 0.000 0.000	0.000	0.000	0.000
	(As per Ull 1:10 yr 6.014 0.816	DF AxC (5-YEAR) (ha)	0.000	0.000	0.238 0.150 0.000 0.385	0.000	1.989	0.265 0.444 0.000 0.209	000000000000000000000000000000000000000	0.000.0	0.000	0.000 0.000 0.000 0.000 0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000 0.000 0.000	0.000	0.000	0.000	0.000	0.000 0.000 0.000 0.000 0.000	0.000	0.000	0.000
TERS	1:5 yr 998.071 6.053 0.814		0.000	0.000	0.000 0.000 0.219 0.219	0.219	0.000	0.219 0.219 0.219 0.219	0.219 0.566 0.566 1.153	0.465 0.701 1.051	0.088	1.332 1.332 1.689 1.689 2.071	0.235	0.251	0.682	0.366	0.257	1.608	0.000 0.000 0.539	2.518	0.300	3.705	0.419 0.419	0.110 0.110 0.510 0.755 0.755	1.534	5.240	7,890
DESIGN PARAMETERS	1:2 yr 732.951 6.199 0.810	AxC (ha)	0.000	0.000	0.000 0.219 0.200 0.000	0.000	0.000	0.000 0.000 0.000	0.219 0.347 0.000 0.587	0.465 0.236 0.350	0.088	0.192 0.000 0.357 0.367 0.000	0.235 0.150	0.251	0.045	0.366	0.257	0.038	0.000 0.539	0.371	0.300 0.214	0.674	0.419	0.110 0.000 0.400 0.245 0.000	0.360	0.000 0.346	0.00 0.00 0.234 7.890
DESIGN	a= b= c= c=	с с (10-YEAR) (100-YEAR) (-) (-)	0.00	0.40	0.00	0.00	0.00	0.00 0.00 0.00	000000000000000000000000000000000000000	0.00 0.00 0.00	00.0	0.00 0.00 0.00 0.00	0.00	00.0	00'0	00.0	0.00	00.0	0.00	0.00	0.00	0.00	0.00	0.00 0.00 0.00 0.00 0.00	0.00	0.00	0.00
			0.00	0.00	0.00 0.00 0.00	0.00	0.00	0.00	00.0	0.00	00'0	0.00 0.00 0.00 0.00	0.00	00.0	00'0	0.00	0.00	0.00	0.00	000	0.00	0.00	00.0	0.00 0.00 0.00 0.00	0.00	0.00	0.00
ж н		C (-) (-)	0.00	0.00	0.70 0.70 0.70	0.00	0.70	0.70 0.70 0.00 0.70	0.00	0.00	00.0	0.00 0.00 0.00 0.00 0.00	00.0	00.0	00'0	00.0	0.00	0.00	0.00 0.00 0.00	0.00	0.00	0.00	0.00	0.0 0.0 0.0 0.0 0.0	0.00	0.00	0.00
1 SEWE	f Ottawa 30	c (2-YEAR) (-)	0.00 0.45	00.0	0.00 0.39 0.30	0.00	00.0	0.00	0.71 0.71 0.00 0.49	0.59 0.70 0.70	0.50	02.0 00.0 07.0 07.0	0.74 0.74	0.74	0.57	0.74	0.74 0.74	0.57	0.00 0.00 0.74	0.51	0.70 0.61	0.52	0.52	0.70 0.00 0.70 0.70 0.00	0.56	0.00	0.49
STORM SEWER	(City of (160401130	AREA (ROOF) (ha)	0.00	0.00	0000	0.00	0.00	0.00 0.00 0.00	0.00 0.00 0.00	00.0	00'0	0.00 0.00 0.00 0.00 0.00	0.00	00'0	00'0	0.00	00.0	0.00	0.00	00'0	0.00	0.00	0.00	0.0 0.0 0.00 0.00	0.00	00.0	
		AREA (100-YEAR) (ha)	0.00	1.35	0.00 0.00 0.00	0.00	0.00	00.0	0.00	0.00 0.00	00'0	0.00 0.	0.00	0.00	00'0	00.0	0.00	0.00	0.00	00.00	00.00	00.00	0.00	0.00 0.00 0.00 0.00 0.00	00.0	0.00	0.00 0.00 0.00
	FILE NUMBER:	AREA (10-YEAR) (ha)	0.00	0.00	0000	0.00	0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00	00.0	0.00 0.00 0.00 0.00 0.00	0.00	00.0	00'0	00.0	0.00	0.00	0.00	00'0	0.00	0.00	0.00	0.0 0.00 0.00 0.00	0.00	0.00	0.00
4-8	2019-04-15 4 DT SG	AREA) (5-YEAR) (ha)	0.00	0.00	0.34 0.21 0.00 0.55	0.00	2.84	0.38 0.63 0.00 0.30	0.00 0.00 0.00	0.00 0.00	0.00	0.00 0.00 0.00 0.00 0.00	0.00	00.0	00.00	00'0	00.00	0.00	0.00	0.00	00.0	0.00	0.00	0.00 0.00 0.00 0.00	0.00	0.00	
Blackstone Phases 4-8	201	AREA (2-YEAR) (ha)	0.00	0.00	0.00 0.56 0.00	0.00	0.00	0.00	0.31 0.49 0.00 1.20	0.79 0.34 0.50	0.18	0.27 0.00 0.51 0.55 0.55	0.32	0.34	0.08	0.49	0.35	0.07	0.00 0.00 0.73	0.73	0.43	1.30	0.81	0.16 0.00 0.57 0.35 0.00	0.64	0.00	0.48
Blackston	ON: IED BY: ED BY:	TO M.H.	M97 M98	2005A	2008 2007 2006 2005A	2005	2005	2004 2003 2002 2001	2046 2045 2044 2001	2026 2017 2015	2015	2014 2013 2012 2011 2011	2041 2040	2040	2036	2036	2037 2036	2032	2034 2033 2032	2029	2030 2029	2028	2020 2019	2025 2024 2023 2023 2019	2028	2018 2010	2010 2001 0.48 0.00
	DATE: REVISION: DESIGNED BY: CHECKED BY:	FROM M.H.	301 M97	2048	2009 2008 2007 2006	2005A	2049	2005 2004 2003 2003	2047 2046 2045 2045	2027 2026 2017	2016	2015 2014 2013 2013 2011 2011	2042 2041	2043	2040	2039	2038 2037	2036	2035 2034 2033	2032	2031 2030	2029	2021 2020	2026 2025 2024 2023 2023 2023	2019	2028 2018	2010
	U stantec	LOCATION AREA ID NUMBER	Fernbank Crossing P1-4 LM97A, CM97A	F2006A	C2009A, C2009B C2008A L2007A C2006A, C2006B		C2049A	C2005A C2004A, C2004B C2002A	L2047A L2046A L2046C, L2044A	L2027B, L2027A L2026A L2017B, L2017A	L2016A, L2016B	L2015A L2013A L2011A	L2042A L2041A	L2043B, L2043A	L2040A	L2039B, L2039A	L2038A L2037A	L2036A	L2033A, L2033B	L2032A, L2032B	L2031A, L2031B L2030B, L2030A	L2029A, L2029B	L2021A, L2021B	L2026B L2024A, L2024B L2023A	L2019A, L2019B	L2018A, L2018B	L2010A, L2010B
6)																										

		TUME OF FLOW (min)	1	0.72	5	10	0.87	x	Q	040	1.27	0.15	w 0 9	0.21	5		a	4000	0.23	8 8	- 0 0	0.05
		VEL TIME (ACT) FLC (m/s) (mi		1.29 0.1	17 0.21	84 0.27 32 2.01		6 0.84	07 1.09	13 0.62 11 1.14 11 0.60			5 0.66 5 0.10 4 0.89		2 0.55	5 0.78 4 1.58 2 0.13 5 0.70	2 1.09	0 0.74 7 0.65 5 0.68 6 0.69		9 1.22 8 0.08	3 0.51 3 0.16 2 0.08	
		~	ALCONDUCT N		5 2.17	0 0.84 9 0.92	IS 0.82	6 1.16	1 1.07	6 1.43 0 1.61 0 1.21	0 0.69 6 0.94	8 1.62	1 1.75 1 1.45 2 1.44	0 0.99	8 1.42	0 0.95 5 1.14 5 1.12 7 1.20 6 0.95	6 1.32	9 1.40 8 1.07 6 1.05 5 1.06	3 1.55	5 1.29 5 1.28	4 1.43 4 1.43	3 3.12
				0% 1.44 2% 1.44	07% 2.05	3% 0.90 0% 0.99	6% 0.85	60% 1.26	6% 1.11	8% 1.56 6% 1.80 4% 1.30	1% 1.10 4% 1.26	0% 1.68	3% 1.91 0% 1.61 3% 1.72	7% 1.00	79% 1.58	3% 1.10 1% 1.35 9% 1.35 3% 1.47 1% 1.06	3% 1.56	% 1,69 % 1.28 % 1.36	% 1.56	% 1.36 % 1.36	% 1.44 % 1.44 % 1.44	% 5.06
		4 SFULL	Constant of the	2.1 57.30% 2.1 58.02%	0.8 103.07%	.6 67.13% .8 68.20%	.1 73.26%	9 64.	.4 73.16%	.8 63.38% .3 57.56% .0 65.94%	5 17.91% .9 32.24%	.1 75.60%	.0 63.43% .5 59.40% .1 47.53%	.3 81.77%	.4 59.	5 52.13% 0 49.21% 0 45.79% 7 42.73% 2 58.41%	.0 48.43%	(2 44.73% 0 46.53% 9 35.27% 9 36.06%	.0 82.41%	.9 69.92% .9 67.28%	.1 82.25% .1 80.98% .1 80.59%	18088.7 16,90%
		PE Ucur (FULL) (US)		0 3792.	5 8420.8	0 200.6 5 449.8	5 248.1	5 132.	5 731.4	0 164.8 0 401.3 5 379.0	5 77.5 5 132.9	0 620.1	5 707.0 734.5 947.1	5 224.3	1034	77.5 301.0 301.0 429.7 392.2	1819.0	2490.2 2332.0 3006.9 3006.9	1819.0	3006.9	3792.	
		PIPE SELECTION CLASS SLOPE (-) %		0.10 0.10	0.15	0.20	0.15	0.65	0,15	1.00 0.80 0.35	0.65	0:50	0.65 0.40 0.40	0.25	0.30	0.65 0.45 0.45 0.45 0.20	0.20	0.20 0.10 0.10	0.20	0.10	0.10 0.10 0.10	1.00
			COLUMN STATE	· · 315	- 31E		ete -		ete .			те -					- 2	16 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	TE .	 	 222	re .
		MATERIAL (-)		CONCRETE	CONCRETE	CONCRETE	CONCRETE	PVC	CONCRETE	PVC CONCRETE CONCRETE	PVC	CONCRETE	CONCRETE CONCRETE CONCRETE	CONCRETE	CONCRETE	PVC CONCRETE CONCRETE CONCRETE CONCRETE	CONCRETE	CONCRETE CONCRETE CONCRETE CONCRETE CONCRETE	CONCRETE	CONCRETE	CONCRETE CONCRETE CONCRETE	CONCRETE
		PIPE SHAPE (-)		CIRCULAR	CIRCULAR	CIRCULAR	CIRCULAR	CIRCULAR	CIRCULAR	CRCULAR CIRCULAR CIRCULAR	CIRCULAR	CIRCULAR	CIRCULAR CIRCULAR CIRCULAR	CIRCULAR	CIRCULAR	CIRCULAR CIRCULAR CIRCULAR CIRCULAR CIRCULAR	CIRCULAR	GIRGULAR CIRCULAR CIRCULAR CIRCULAR	CIRCULAR	CIRCULAR	CIRCULAR CIRCULAR CIRCULAR	CIRCULAR
		H PIPE E HEIGHT (mm)	Non-Colored	1800	2400 2400	525 750	600	375	006	375 525 600	300 375	675	675 750 825	525	006	300 525 525 600 675	1200	1350 1500 1650 1650	1200	1650 1650	1800 1800 1800	2100 2100
		PIPE WIDTH OR DIAMETE (mm)		1800	2400 2400	525 750	009	375	006 006	375 525 600	300 375	675	675 750 825	525	006	300 525 525 600 675	1200	1350 1500 1650 1650	1200	1650 1650	1800 1800 1800	2100 2100
		(m)		55.7 79.7	26.9	13.5 111.4	42.6	58.5	70,0	53.6 110.1 43.1	52.8 97.2	15.0	69.3 8.9 75.8	12.5	47.2	44.2 108.0 8.8 104.3 39.7	86.6	61.7 42.5 42.5 43.7	21.8	94.7 6.3	43.9 14.0 6.5	8.4
		UACT (CIA/360) (U/6)		2172.9 2200.2	8679.2	134.7 306.8	181.8	85.8	535,1	104.5 231.0 249.9	13.9 42.8	468.8	448.4 436.3 450.2	183.4	618.5	40.4 148.1 137.8 183.6 229.1	881.0	1113.9 1085.1 1060.6 1084.3	1499.0	2102.5 2022.9	3119.1 3071.0 3056.0	3057.8
		ACCUM. Reormon (L/b)		0.0	6451.8	0.0	0.0	0.0	0'0	0.0 0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 0.0	0.0
		Uccorrator. (US)		0.0	0.0	0.0	0.0	0.0	0.0	0.0 0.0	0.0	0.0	0.0	0:0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
		Provena (nhmh)		113.51 111.16	107.98	178.56	178.56	178.56	160.10	178.56 173.05 163.87	178,56 167,69	178.56	155.18 150.90 150.26	178.56	145.00	178,56 171.76 159,57 159,57 158,64 149,11	141.89	136.15 132.57 129.54 126.58	178.56	123.73	118.69 116.83 116.26	115,99
		Ineran (membr)		76.21	74.04	122.14 120.50	122.14	122.14	109.58	122.14 118.39 112.14	122.14	122.14	106.22 103.31 102.87	122.14	99.29	122.14 117.52 109.21 108.58 102.09	97.17	93.26 90.82 88.75 86.73	122.14	84.79 81.56	81.35 80.09 79.69	79.51
8		lisneau (mimh)		66.51 65.15	63,30	104.19 102.80	104.19	104.19	93.52	104.19 101.01 95.70	104.19 97.91	104.19	90.67 88.20 87.82	104.19	84.78	104.19 100.27 93.21 92.67 87.16	82.97	79.65 77.58 75.81 74.10	104.19	72.44 69.70	69.52 68.44 68.11	67.95
SEDDING CLASS =		lovese (mmh)		49.29 48.29	46.93	76.81 75.79	76.81	76.81	69,04	76.81 74.49 70.63	76.81 72.24	76.81	66.96 65.15 64.88	76.81	62.66	76.81 73.95 68.81 68.42 64.40	61.34	58.91 57.39 56.11 54.85	76.81	53.64 51.62	51.50 50.70 50.46	50,34
BEDDING		R) ToliC R) (min)	21.71	21.81 22.53 23.56	23.56	10.00 10.27 12.28	10.00 10.87	10.00	12.28 13.37	10.00 10.62 11.77 12.36	10.00 11.27 12.99	10.00	12.99 13.65 13.75 14.63	10.00 10.21	14.63 15.18	10.00 10.78 12.35 12.48 13.94 14.63	15.18 16.28	16.28 17.01 17.67 18.35 19.03	10.00	19.03 20.26 20.34	20.34 20.85 21.01 21.09	21.09 21.13
E	- I	A X U AUUM. (100-YEAR) AAC (100YR) (ha) (ha)		0.541	0.541	0.000	0000	0.000	0.000	0.000 0.000 0.000	0.000	0.000	0.000	0.370	0.370	0,000 0,000 0,000 0,000	0.370	0.370 0.370 0.370 0.370	0.000	0.370 0.370	0.370 0.370 0.370	0.370
2012) = 0.013 ER: 2.00	10	L AXU R) (100-YEA (ha)		0.000	0.000	0.000	0000	0.000	0.000	0.000 0.000	0.000	0000	0.000	0.370	000'0	0.000 0.000 0.000 0.000	0.000	0.000	00000	00000	0.000	0,000
Se LS	TIME OF ENTRY	ALCUM. R) ALC (10YR) (ha)		0.000	0000	0.000	0.000	0.000	0000	0.000	0,000	0000	0.000	0000	0.000	0,000 0,000 0,000 0,000 0,000	0.000	0.000 0.000 0.000	0,000	0.000	0.000 0.000 0.000	0.000
0 . 0	TIMEO	AKEA . AxC (10-YEAR) (ha)		0.000	0.000	0.000	0000	0.000	0,000	0.000 0.000	0,000	0000	0.000 0.000	0000	0.000	0.000 0.000 0.000 0.000 0.000	0.000	0.000 0.000 0.0000	0,000	0.000	0.000	0.000
11/10 of 11/10 11/35 6.0	0.820	DIMANNAGE AMEA AUCUM. R) AXC (SYR) (1 (ha)		3.974	4.654	0.000	0000	0.127	0,736	0.361 0.823 0.823	0,000	0000	0.000	0:000	0,000	0.000 0.000 0.000 0.000 0.000	0.000	1.094 1.094 1.094 1.325	5.179	6.505	7.240 7.240 7.240	7.287
(As per C 1:10 yr 1 1174.184 6.014	0.816	AXC (5-YEAU (ha)		0.294 0.396	0.000	0.000	0000	0.127	0000	0.361 0.462 0.000	0,000	000'0	0.000 0.000	000'0	0.000	0.000 0.000 0.000 0.000	0,000	0.271 0.000 0.000 0.231	5.179	000.0	0.000	0.000
ETERS 1:5 yr 6.053	_	ACCUM ACC2YR) (ha)		9.262	9.565	0.631	0,852	0.230	1.794	0.000 0.000 0.158	0.065	2.197	2.411 2.411 2.498	0000	2.697	0.189 0.721 0.721 0.966 1.281	4.315	4.473 4.473 4.473 4.473	0000	4.473	11.179 11.179 11.179	0.000 11.179
DESIGN PARAMETERS I = a / (t+b)° 1:2 yr 1:5 a = 732.951 998.1 b = 6.01	0.810	АхС .R) (2-YEAR) (ha)		0.000	0.000	0.631	0.852	0.230	0.080	0.000 0.000 0.158	0.065	2.197	0.000 0.000 0.087	0000	0.200	0.532 0.532 0.000 0.245 0.315	0.337	0.000 0.000 0.0000	0.000	0.000	4.912 0.000 0.000	0000
$\frac{\text{DESIGN PA}}{1 = a / (t+b)^{4}}$ $a = \frac{7}{1}$	c =	c (100-YEAR) (-)		00.0	00.0	0.00	00'0	00'0	00.0	0.00 0.00 0.00	0,00	00.0	0.00	0.40	00'0	0000	0.00	0.00	00'0	00.0	00.0	00.0 0.00
		c (10-YEAR) (-)		00.0	0.00	00.0	000	0.00	00.0	0.00	00.0	00.0	0.00	00.0	00.0	0.00 0.00 0.00 0.00	00.0	0.0 0.0 0.0	0.00	0.00	0.00	00'0
ER a) ET		с (5-YEAR) (-)		0.71	0.00	0.00	0.00	0.71	00.0	0.72 0.72 0.00	0.00	0.00	0.00	00'0	00'0	0.00 0.00 0.00	00'0	0.70 0.00 0.00	0.70	0.00	0.00	00'0
STORM SEWER DESIGN SHEET (City of Ottawa) 160401130		A C F) (2-YEAR) F)		00.0	00.00	0.00	0.58	0.47	0.47	0.00 0.00 0.47	0.57	0.70	0.00 0.00 0.57	00'0	0.51	0.70 0.48 0.00 0.70 0.70	0.44	0.00	000	0.00	0.59 0.00	000 000
STORM S DESIGN (City of C		AREA (R) (ROOF) (ha)		0.00	0.00	00.0	0.00	00.0	00'0	0.00	0.00	0.00	0.00	00'0	00'0	0.00 00.0 00.0	0.0	0.00 0.00 0.00	0.00	0.00	0.00	
FILE NUMBER:		А АКЕА VR) (100-YEAR) (ha)		0.00	00.00	0.00	00'0	0.00	00.00	0.00	0.00	00.00	0.00	0.92	00'0	0.00 0.00 0.00 0.00	0.00	0.00 0.00 0.00	0.00	0.00	0.00	0.00
FILE N		ч АНЕА R) (10-YEAR) (ha)		00.00	00.00	0.00	00.00	00.00	00.0	0.00	0.00	0.00	0.00	00.0	00.0	0.00 00.0 00.0	0.00	0000	0.00	0.00	0.00	0.00
ses 4-8 2019-04-15 4 DT	SG	R AREA R) (S-YEAR) (ha)		0.41	00.0	0.00	00'0	0.18	00.0	0.50	0.00	0.00	0.00	00'0	00.0	0.00 00.0 00.0 0.00	00.00	0.39 0.00 0.36 0.36	7.40	0.00	0.00	00'0
ne Pha	-	AREA (2-YEAR) (ha)		0.00	0.00	2 1.07	0 1.48	0 0.49	9 0.17	5 0.00 5 0.00 5 0.34	0.11	1 3.14	0.00 0.00 0.15	00'0 1	0.39	0.27 1.11 0.00 0.35 0.35	0.77	0.00	0.00	0.00	8.33 0.00	00'00
	KED BY:	м то с м.н.		1 2000 0 M98	66W 8	4 1052 2 1050	3 1050	1 1050	0 1049	9 1008 8 1007 7 1006	5 1020 0 1014	2 1014	4 1013 3 1012 2 1011	1 1011	1 1010	0 1019 9 1018 8 1017 7 1016 6 1010	0 1006	6 1005 5 1004 3 1002 3 1002	3 1002	2 1001	222	MH
	CHEC	FROM M.H.	Con Brown	2001	W98	1054	1053	1051	1050	1009	1015	1022	1014 1013 1012	1021	1011	1020 1019 1018 1017 1016	1010	1006 1005 1004 1003	1023	1001	2 2 <u>1</u> 00	£
Stantec	HULLOU I	LOCATION ANEA IU NUMBER		C2001A C2000A		L1054A C1052A, C1052B	L1053B, L1053A	L1051B, C1051A	L1050B	C1009A C1008A L1007A	L1020C, L1020B	L1022A	L1012A	F1021A	L1011B, L1011A	L1020A L1019B, L1019C, L1019A L1017A L1016A	L1010A, L1010B	C1006A C1003A	C1023A		L1000A	

Appendix B - Stormwater Management Calculations

Fernbank Elementary School - 480 Cope Drive

B.1.1 - Allowable release rate



Using the criteria for the site from the subdivision Final Serviceability Report Development by Stantec (April 2019), the maximum allowable release rate is up to the 5 year discharge rate.

 $\begin{aligned} & \mathsf{Q}_{\mathsf{allowable}} = \mathsf{Q} = 2.78 \ \mathsf{C} \ \mathsf{I} \ \mathsf{A} \\ & \mathsf{Q}_{\mathsf{allowable}} = 575.7 \qquad |/s \end{aligned} \tag{1}$

B.1.2 - Post-development release rate

			Are	as (m²)			
ID	Description	Туре	C _{0.90}	C _{0.25}	Total (m ²)	C _{post-5-year}	C _{post-100-yr} *
B1	Front of building	uncontrolled	419	1854	2273	0.37	0.39
A1	Western parking area 1	controlled	945		945	0.90	1.00
A2	Western parking area 2	controlled	960		960	0.90	1.00
A3	Western parking area 3	controlled	1033		1033	0.90	1.00
A4	Western parking area 4	controlled	1041		1041	0.90	1.00
A5	Playground at Daycare area 1	controlled	1022		1022	0.90	1.00
A6	Future portables area 1	controlled	182	187	369	0.57	0.62
A7	Playground at Daycare area 2	controlled	1433	145	1578	0.84	0.93
A8	Courts area 1	controlled	1720	786	2506	0.70	0.76
A9	Courts area 2	controlled	970	845	1815	0.60	0.65
A10	Bus loop area 1	controlled	317	106	423	0.74	0.81
A11	Sports Field	controlled	63	4791	4854	0.26	0.26
A12	Future portables area 2	controlled	265	374	639	0.52	0.56
A13	Future portables area 3	controlled	246	296	542	0.55	0.59
A14	Back lot area 1	controlled	142	904	1046	0.34	0.35
A15	Back lot area 2	controlled		598	598	0.25	0.25
A16	Back lot area 3	controlled	131	1327	1458	0.31	0.32
A17	Back lot area 4	controlled		933	933	0.25	0.25
A18	Bus loop area 2	controlled	391	65	456	0.81	0.89
R1	Roof	controlled	3890		3890	0.90	1.00
			15170	13211	28381	0.60	0.65

*including 25% increase as per City of Ottawa Sewer Design Guidelines

Calculations for post-development runoff coefficient

 $\begin{array}{ll} C_{\text{post-5-year (col. D)}} &=\!\!(\text{column A }^* \ 0.9 + \text{column B }^* \ 0.2) \ / \ \text{column C} \\ C_{\text{post-100-yr (col. E)}} &=\!\!(\text{column A }^* \ 1.0 + \text{column B }^* \ 0.2^* 1.25) \ / \ \text{column C} \\ & note: \ 0.90 \ x \ 1.25 = 1.125, \ use \ max. \ 1.0 \end{array}$

Calculations for average weighted runoff coefficient		C _{post-5-year} C _{post-100-yr}	=((15170*0.9)+(13211*0.25))/28381 =((15170*1.0)+(13211*0.25*1.25))/28381	0.60 0.65
Estimated time of concentration, $t_{\rm c}$ =	10.0	minutes	***As per City of Ottawa Sewer Design Guidelin	es (Section 5.4.5.2)
Based on Ottawa IDF curve, i _{5-year} =	998.071/ (t _c +			
	104.2	mm/hr		
Based on Ottawa IDF curve, i _{100-years} =	1735.688/ (t	_c +6.014) ^{0.820}		
	178.6	mm/hr		
B.1.2.1 - uncontrolled flow				
Total uncontrolled area =	0.227	ha		
5-year Runoff coefficient, C =	0.37			
100-year Runoff coefficient, C =	0.39			
Estimated time of concentration, t_c =	10.0	minutes		
Quncontrolled 5-year	= 24.3	l/s	0	
Q _{net-allowable} 5-year	= 551.4	l/s	3 = 1-2	
Quncontrolled 100-year		l/s	4	
Q _{net-allowable} 100-year		l/s	\$ = 1-4	

B.1.3 - Post-development onsite storage

A.1.3.1 - Overall onsite storage requirements		
Total controlled area, A1 to A18 & R1	2.611	ha
5-year Runoff coefficient, C	0.62	
100-year Runoff coefficient, C	0.67	
net-allowable release rate	530.0	l/s

	Time (minutes)	i _{5-year} (mm/hr)	Q _{actual} (I/s)	Q _{allowable} (I/s)	Q _{stored} (I/s)	V _{stored} (m ³)
	10	104.2	466.8	530.0	-63.2	-37.9
eak Vstored>						
	15	83.6	374.3	530.0	-155.7	-140.1
	20	70.3	314.7	530.0	-215.3	-258.3
	25	60.9	272.8	530.0	-257.2	-385.8
	30	53.9	241.6	530.0	-288.4	-519.1
	35	48.5	217.4	530.0	-312.6	-656.5
	40	44.2	197.9	530.0	-332.1	-796.9
	45	40.6	182.0	530.0	-348.0	-939.6
	50	37.7	168.7	530.0	-361.3	-1083.9
	55	35.1	157.4	530.0	-372.6	-1229.7
	60	32.9	147.6	530.0	-382.4	-1376.7
1	herefore	-38	m ³ of onsite sto	orage required dur	ing 5-vear event	

(5)

Table 1.3.1b - 100-year onsite storage requirements

	Time (min)	i _{100-year} (mm/hr)	Q _{actual} (I/s)	Q _{allowable} (I/s)	Q _{stored} (I/s)	V _{stored} (m³)
	10	178.6	873.2	530.0	343.2	205.9
	15	142.9	698.8	530.0	168.8	151.9
	20	120.0	586.6	530.0	56.6	67.9
oeak V stored>	25	103.8	507.8	530.0	-22.2	-33.3
	30	91.9	449.2	530.0	-80.8	-145.4
	35	82.6	403.8	530.0	-126.2	-265.0
	40	75.1	367.5	530.0	-162.5	-390.1
	45	69.1	337.7	530.0	-192.3	-519.3
	50	64.0	312.7	530.0	-217.3	-651.8
	55	59.6	291.6	530.0	-238.4	-786.8
	60	55.9	273.3	530.0	-256.7	-924.0
_ <u></u>	nerefore	206	m ³ of onsite sto	orage required du	ing 100-year eve	en

Watts Drainage Adjustable Flow Control for Roof Drains, or approved equivalent

B.1.3.2 - Estimated detention created by installing roof weirs

Total roof area, A3 0.389

ha

l/s

5-year Runoff coefficient, C 0.90

100-year Runoff coefficient, C 1.00

Install 0.6309 l/s weirs at each of the 29 roof drains 18.3

	Time	İ _{5-year}	Q _{actual}	Q _{allowable}	Q _{stored}	V _{stored}
	(minutes)	(mm/hr)	(l/s)	(l/s)	(l/s)	(m³)
-	10	104.2	101.4	18.3	83.1	49.9
	15	83.6	81.3	18.3	63.0	56.7
eak V stored>	20	70.3	68.4	18.3	50.1	60.1
	25	60.9	59.3	18.3	41.0	61.4
	30	53.9	52.5	18.3	34.2	61.5
	35	48.5	47.2	18.3	28.9	60.7
	40	44.2	43.0	18.3	24.7	59.3
	45	40.6	39.5	18.3	21.2	57.3
	50	37.7	36.6	18.3	18.3	55.0
	55	35.1	34.2	18.3	15.9	52.4
	60	32.9	32.1	18.3	13.8	49.5

Therefore 62 m³ estimated roof detention

Table 1.3.2b - 100-year estimated detention on new roof

	Time (min)	i _{100-year} (mm/hr)	Q _{actual} (I/s)	Q _{allowable} (I/s)	Q _{stored} (I/s)	V _{stored} (m³)
	10	178.6	193.0	18.3	174.8	104.9
	15	142.9	154.5	18.3	136.2	122.6
	20	120.0	129.7	18.3	111.4	133.7
	25	103.8	112.3	18.3	94.0	141.0
	30	91.9	99.3	18.3	81.0	145.8
peak V stored>	35	82.6	89.3	18.3	71.0	149.1
	40	75.1	81.2	18.3	62.9	151.1
	45	69.1	74.7	18.3	56.4	152.2
	50	64.0	69.1	18.3	50.8	152.5
	55	59.6	64.5	18.3	46.2	152.3
	60	55.9	60.4	18.3	42.1	151.7
	Therefore	153	m ³ estimated ro	oof detention		

B.1.4 - Site storage

Total release rate (100-yr)

Total release rate (100-yr) < Allowable release rate (5-year)

D.1.4	- Site storage		100-year event		
	overall storage requirements		206	m ³	Table B.1.3.1
	estimated roof ponding volume		153	m ³	Table B.1.3.2
	roof ponding depth		0.118	m	maximum allowable: 0.15m
	estimated parking area and school yard volume		191	m ³	
	maximum parking area and school yard ponding depth		0.25	m	maximum allowable: 0.25m
	estimated bus lay-by		8	m ³	Table B.1.3.3
	maximum bus lay-by		0.13	m	maximum allowable: 0.25m
	Sportsfield ponding volume		34	m ³	Table B.1.3.4
	Maximum sportsfield ponding depth		0.15	m	maximum allowable: 0.30m
	Total available roof storage		153	m ³	at maximum ponding depth of 0.15m
	Total available parking area and school yard storage		233	m ³	
	Total available onsite storage > overall storage requirements Total available onsite storage > estimated detention		<u>OK</u> OK		
D 4 5	-				
B.1.3	- Release rate for site				
	Release rate				
	Allowable release rate (5-year)	575.7			Section B.1.1
	Uncontrolled release rate for (100-yr)	43.8			Section B.1.2.1
	Controlled release rate at roof drain (100-yr)	18.3			Section B.1.3.2
	Controlled release rate at CBMH-1 (100-yr)	530.0			Section B.1.3.3
	Tatal release rate (100 yr)	E72 0			

573.8

<u>0K</u>

Fernbank Elementary School - 480 Cope Drive



Storm Sewer Pipe Design Without ICD Flow Control

Definitions	Manning's Coefficient =	Return Frequency (yrs) =	1 acre = 0.4047 hectares
Definitions	Manning's Co	Return Frequ	1 acre = 0.40

Rational Method Q = 2.78 CIA (I/s), where C = Runoff Coefficient i = Rainfall Intensity (mm/hr) A = Areas in Hectares (ha) 0.013 5

<u>Notes</u> 1) Used City of Ottawa IDF Curve 2) Min. velocity = 0.8 m/sec 3) Max. velocity = 6.0 m/sec

Designed BK Checked NC Dwg. Reference C03 Jp2g project No 19-5070A

LOCATION	N	Area no.	AREA (ha)	(ha)		FLOW								SEW	SEWER DATA				
			=)	C=	Individual	Cum.	tc	i 5-year	i 100 years	Flow 5-year	Flow 100 years	Dia.	Slope	Length	Capacity	Velocity	Sect.Time	Tot. Time	Utilization
From	To		06.0	0.25	2.78CA	2.78CA	(min.)	(mm/hr)	(mm/hr)	(I/s)	(I/s)	(mm)	(%)	(L)	(full) (I/s)	(full) (m/s)	(minutes)	(minutes)	(%)
CB-1	CBMH1	A1	0.094		0.24	0.24	10.0	104.2	178.6	24.5	42.0	250	0.65	30.6	47.9	1.0	0.5	10.5	51.1
CBMH1	CBMH2	A3	0.094		0.24	0.47	10.5	101.5	173.9	47.7	81.8	375	0.40	25.1	115.7	1.0	0.4	10.9	41.3
CB-2	CBMH2	A2	0.103		0.26	0.26	10.0	104.2	178.6	26.9	46.0	250	0.65	30.6	47.9	1.0	0.5	10.5	56.0
CBMH2	CBMH3	A4	0.103		0.26	0.99	10.9	99.5	170.4	98.1	168.0	375	0.50	32.4	129.3	1.1	0.5	11.4	75.8
RYCB1	CBMH3	A6	0.017	0.020	0.06	0.06	10.0	104.2	178.6	5.9	10.1	250	1.00	15.5	59.5	1.2	0.2	10.2	9.9
CBMH3	CBMH4	A5	0 102		0.26	130	11.4	6 76	166.5	126.2	216.1	450	0.40	40.0	187.9	1	0.6	12.0	67.2
CRMH4	STMH1	47	0320		0.82	2 12	12.0	2 70	162.1	200.7	2437	800	0.35	58.0	379.6	1.5	0.8	12.8	50 G
			0.010		40:0	1	Ì					2	0000	200	222	2	2	i	2
Roof *	STMH1	R1	0.389	0.000	0.97	0.97	10.0	104.2	178.6	101.4	173.7	300	4.00	19.0	193.4	2.7	0.1	10.1	52
STMH1	CBMH5				00.0	3.09	12.8	91.6	156.7	283.3	484.9	600	0.35	21.0	379.6	1.3	0.3	13.0	74.6
TCB1	TCB10	A11		0.485	0.34	0.34	15.0	83.6	142.9	28.2	48.2	250	0.50	123.0	42.0	0.9	2.4	17.4	67.0
RYCB2	TCB14	A14+A12+A13	0.065	0.157	0.27	0.61	17.4	76.5	130.8	46.7	79.8	300	0.50	54.0	68.4	1.0	0.9	18.3	68.3
TCB14	TCB15	A15		0.060	0.04	0.65	18.3	74.2	126.7	48.3	82.5	300	0.50	35.0	68.4	1.0	0.6	18.9	70.7
TCB15	TCB16	A16	0.137	0.013	0.35	1.00	18.9	72.7	124.2	72.9	124.6	300	0.50	35.0	68.4	1.0	0.6	19.5	106.7
TCB16***	CBMH5	A17		0.093	0.06	1.07	19.5	71.3	121.8	76.1	130.0	300	0.50	35.0	68.4	1.0	0.6	20.1	111.4
CBMH5	CBMH6	A9	0.158		0.40	4.56	13.0	90.5	154.9	412.4	705.8	600	0.50	32.4	453.7	1.5	0.4	13.4	6.06
CB-4	CBMH6	A10	0.149		0.37	0.37	10.0	104.2	178.6	38.8	66.6	250	0.65	32.4	47.9	1.0	0.6	10.6	81.0
	CTM114	410	0000		010	5 0.4	101	000	150.0	140.0	L 03L	200	0.05	010	0.01.0		0	961	0
		AIO	0.040	Ţ	0.12	9.0°C	10.4	03.2	0.201	443.0	1.03.1	670	0.33	21.0	049.2	0.	0.2	0.01	0.00

Flow control to be installed at outlet

* * *

Notes:

Flow from controlled roof drains is limited to 18.3 L/s How restricted to 450 L/s How restricted to account of the stone infiltration trench

Appendix C - Stormwater Management Calculations Watts Drainage Adjustable Flow Control for Roof Drains – Data Sheet

WATTS DRAINAGE	RD-100	Large Capacity Roof Drain
Components:	B2 B2-DM	B2-FLG FC-2
SPECIFICATION: Watts Drainage Provide served device with integral gravel stop and served dome strainer.	elf-locking polyethylene (standard)	
6-1/4"(158) - PIPE SI 2"(51), 3"(76), 4" 5"(127), 8"(203)	(102), 6"(152)	-GSS Stainless Steel Ballast Guard -H Adj. to 6" IRMA Ballast Guard -K Ductile Iron Dome -K80 Aluminum Dome -L Vandal Proof Dome -R 2" High External Water Dam -SO Side Outlet** -V Fixed Extension (1-1/2",2",3",4") -W Adj. Water Level Regulator -W-1 Waterproofing Flange
Free Area Sq. In. 137	Deck opening 10" (254) with sump receiver 13-1/4" (337)	-Z Extended Integral Wide Flange -5 Sediment Bucket -12 Galvanized Dome -13 All Galvanized -83 Mesh Covered Dome -113M Special Epoxy from 3M Range Optional Body Material (NH Only) Suffix Description
	lable in 2"(51), 3"(76), 4"(102) pipe sizes. options) are not available when -SO is select	-60 PVC Body w/Socket Outlet
	Contractor Contractor's P.O. No	
Engineer).
previously or subsequently sold. See your WATTS Drainage re	ct design or construction without prior notice and without incurring presentative for any clarification. Dimensions are subject to manu	any obligation to make similar changes and modifications to products

ES-WD-RD-100 CANADA 0403



Tag: _

ACCUTROL WEIR FLOW CONTROL

SPECIFICATION: Watts Drainage Products epoxy coated cast iron Accutrol Weir is designed with parabolic openings which limit the flow of rain water off a roof. Each weir slot controls flow to 5 gpm per inch of head to a maximum of 30 gpm at 6" head(for large sump), 25 gpm at 5" head(for small sump). The Accutrol Weir is secured to the flashing clamp of the roof drain. The Accutrol Weir is available with 1 to 4 slots for the large sump drain and up to 3 slots for the small sump drain.

For Large Sump Roof Drains Specify the "-A" option and number of slots required. (ie. "RD-100-A2" for two slot weir) For Small Sump Roof Drains Specify the "-A" option and number of slots required. (ie. "RD-200-A1" for one slot weir)

