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### 90 Champagne Avenue South

**Development Servicing and Stormwater** Management Report

#### 90 CHAMPAGNE AVENUE SOUTH

# DEVELOPMENT SERVICING AND STORMWATER MANAGEMENT REPORT

Prepared by:

#### **NOVATECH**

Suite 200, 240 Michael Cowpland Drive Kanata, Ontario K2M 1P6

March 22, 2019

Ref: R-2019-050 Novatech File No. 119013



March 22, 2019

City of Ottawa Planning and Growth Management Department Infrastructure Approvals Division 110 Laurier Avenue West, 4<sup>th</sup> Floor Ottawa, Ontario K1P 1J1

Attention: Mr. John Wu

Dear Sir:

Re: Development Servicing and Stormwater Management Report

90 Champagne Avenue South

Ottawa, Ontario Our File No.: 119013

Enclosed herein is the 'Development Servicing and Stormwater Management Report' for the proposed development located at 90 Champagne Avenue South, in the City of Ottawa. This report addresses the approach to site servicing and stormwater management for the subject property and is submitted in support of the site plan approval application.

Should you have any questions or require additional information, please contact the undersigned. Yours truly,

#### **NOVATECH**

Miroslav Savic, P. Eng.

Project Manager

WSairic

MS/sm

cc: Robert Verch (Roderick Lahey Architect Inc.)

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**Grading and Erosion & Sediment Control Plan (119013-GR)** 

**General Plan of Services (119013-GP)** 

**Stormwater Management Plan (119013-SWM)** 

#### 1.0 INTRODUCTION

Novatech has been retained to prepare the site servicing, grading and stormwater management design in support of a Site Plan Control application for the proposed development at 90 Champagne Avenue South in the City of Ottawa.

The subject site is currently occupied by a parking lot serving the existing residential building immediately to the west (285 Loretta Avenue). The site has recently been severed from the 285 Loretta Avenue property. An aerial photo of the subject site is shown in Figure 1.1 below.



#### 1.1 Consultation and Reference Material

A pre-consultation meeting was held with the City of Ottawa at which time the owner was advised of the general submission requirements. Further discussions were held with the City of Ottawa

regarding the approach to stormwater management for the site. Refer to **Appendix A** for a summary of the e-mail correspondence with the City of Ottawa.

The following reference material was reviewed. Relevant report excerpt and plans are provided in **Appendix E**.

- Servicing Brief and Stormwater Management Report, 285 Loretta Avenue South, prepared by D.B. Gray Engineering Inc., dated July 26, 2017.
- Site Servicing Plan, Property Severance 285 Loretta Avenue South, prepared by D.B. Gray Engineering Ltd., November 6, 2017
- Grading Plan & Notes, Property Severance 285 Loretta Avenue South, prepared by D.B. Gray Engineering Ltd., November 6, 2017

#### 2.0 PROPOSED DEVELOPMENT

The proposed development is a 14-storey residential building containing 236 dwellings and ground-floor amenity space for residents. A total of 100 parking spaces within a two-level underground parking garage have been proposed to serve the development.

The site will have two accesses off Champagne Avenue South, one near the north property line and one near the south property line. The northerly access will exclusively serve the parking garage for the proposed development. The southerly access will exclusively serve the parking garage of the existing residential building immediately west of the subject site (285 Loretta Avenue South). A copy of the site plan is included in **Appendix B**.

#### 3.0 SITE SERVICING

The objective of the site servicing design is to conform to the requirements of the City of Ottawa servicing design guidelines by providing a suitable domestic water supply, proper sewage outlets and ensuring that appropriate fire protection is provided.

The servicing criteria expected sewage flows and water demands for the site have been established using the City of Ottawa municipal design guidelines for sewer and water distribution. The City of Ottawa Servicing Study Guidelines for Development Applications requires a Development Servicing Study Checklist to confirm that each applicable item is deemed complete and ready for review by City of Ottawa Infrastructure Approvals. A completed checklist is enclosed in **Appendix F**.

As mentioned previously, the subject site has been recently severed from the 285 Loretta Avenue South property. The 285 Loretta Avenue South services (storm, sanitary, hydro, telephone and cable) crossing the existing parking lot have been disconnected and new services are constructed within the proposed southerly access driveway connecting the 285 Loreta Avenue South parking garage with Champagne Avenue South.

The design of the new 285 Loretta Avenue services as well as the grading design for the southerly access driveway has been completed by D.B. Gray Engineering Inc. An excerpt from 'Servicing Brief and Stormwater Management Report', 'Site Servicing Plan', and 'Grading Plan & Notes', prepared by D.B. Gray are enclosed in **Appendix E**.

#### 3.1 Water

The proposed development will be serviced by a 200mm dia. water service connected to the existing 200mm dia. watermain in Champagne Avenue South.

The proposed building will be sprinklered. The fire protection will be provided from the existing municipal fire hydrant in Champagne Avenue South near the southeast corner of the property. The hydrant is located within a 45m unobstructed path from the proposed fore department siamese connection.

The theoretical water demand for the proposed development, calculated as per the Ottawa Design Guidelines – Water Distribution is summarized in **Table 3.2**. Detailed calculations are shown in **Appendix C**.

Table 3.2.1: Water Demand

Average Day Demand	Maximum Day Demand	Peak Hour Demand
1.49 L/s	3.74 L/s	8.22 L/s

The Fire Underwriter's Survey (FUS) was used to estimate fire flow demands for the proposed building. The calculated fire flow demand is 100 L/s (6000 L/min). Refer to **Appendix C** for detailed calculations.

The water demand calculations, boundary conditions and watermain analysis calculations for the existing public infrastructure are provided in **Appendix C.** 

The results of the hydraulic analysis are summarized below in **Table 1**.

**Table1: Water Analysis Results Summary** 

Condition	Water Demand	Min/Max Allowable Operating Pressures	Limits of Design Operating Pressures
High Pressure	1.49 L/s	80 psi (Max)	70.7 psi
Max Day + Fire Flow	103.74 L/s	20 psi (Min)	49.1 psi
Peak Hour	8.22 L/s	40 psi (Min)	60.0 psi

The results of the water analysis show there is adequate flow and pressure in the existing 200mm watermain in Champagne Avenue South to meet the required domestic and fire flow demands.

#### 3.2 Sanitary Sewer

The proposed development will be serviced by connecting a 200 mm dia. sanitary service to the existing 1050mm diameter sanitary sewer in Champagne Avenue South. The proposed 200 mm dia. sanitary service will be a gravity pipe at a minimum slope of 1.0% with a full flow conveyance capacity of at least 34.2 L/s. The existing 1050mm diameter sanitary sewer in Champagne Avenue south at 0.20% slope has a full flow capacity of approximately 1274 L/s.

The calculated peak sanitary flow from the site, including infiltration, is 3.95 L/s. The flow has been calculated as per the City of Ottawa Sewer Design Guidelines. Refer to **Appendix C** for detailed calculations. The proposed 200mm diameter sanitary service has sufficient capacity to convey anticipated sanitary flows generated from the proposed development.

#### 3.3 Stormwater Management

The stormwater management design for the proposed development will include on-site water quantity control prior to releasing flows from the site. The proposed development will be serviced by connecting to the existing 1050mm diameter storm sewer in Champagne Avenue South.

Stormwater management will be provided by an underground storage tank. Further details on the sub-catchment drainage areas are explained in subsequent sections of the report. See the Stormwater Management Plan (119013-SWM) included in **Appendix G**, for catchment locations, areas, and runoff coefficients.

#### 3.3.1 Existing Conditions

The subject site is presently occupied by a paved parking lot serving the 285 Loretta Avenue South residential building. The existing parking lot drains towards the on-site catch basins that are connected to the 1050mm diameter storm sewer in Champagne Avenue South.

The storm service from 285 Loretta Avenue that crosses the existing parking lot has been recently disconnected and a new 300mm diameter storm sewer has been constructed within the proposed southerly access driveway to service 285 Loreta Avenue.

The design of the new 300mm diameter storm sewer as well as the grading and drainage design of the southerly access driveway connecting 285 Lorretta Avenue with Champagne Avenue South has been completed by D.B. Gray Engineering Inc. Refer excerpt for from 'Servicing Brief and Stormwater Management Report', 'Site Servicing Plan', and 'Grading Plan & Notes', prepared by D.B. Grey enclosed in **Appendix E**.

#### 3.3.2 Stormwater Management Objectives

The proposed stormwater management design is based on the latest City of Ottawa Sewer Design Guidelines and are as follows:

- Provide a dual drainage system (i.e. minor and major system flows).
- Control 1:100 year post-development flow from the site to the maximum 1:5 year allowable release rate as specified by the City of Ottawa. Post-development runoff in excess of the allowable release rate will be stored and controlled on site prior to being release into the municipal storm sewer system in Champagne Avenue South.
- Provide guidelines to ensure that site preparation and construction is in accordance with the current Best Management Practices for Erosion and Sediment Control.

#### 3.3.3 Storm Drainage Areas

The proposed site has been subdivided into two distinct storm drainage areas for the post-development condition. The size and location of the catchment areas are based on the proposed grading design for the site. The runoff coefficients for each catchment area were calculated for the proposed conditions and the catchment areas are shown on the Stormwater Management Plan (119013-SWM). A brief description of the subcatchment areas are as follows:

- Runoff from the landscaped area in front of the building will sheet drain towards catchbasins in Champagne Avenue South (Area A-1).
- Runoff from the building roof and landscaped areas at the back of the building (Area A-2) will be controlled and stored in the proposed underground stormwater management tank.

#### 3.3.4 Allowable Release Rate

The allowable release rate for the 0.235 ha site was calculated using the Rational Method to be 34.0 L/s. This release rate was based on an the runoff coefficient of C=0.5 and a 1:5 year rainfall intensity of 104.2 mm/hr, based on City of Ottawa IDF Curves using a time of concentration (t<sub>c</sub>) of 10 minutes. Refer to **Appendix A** for correspondence from the City of Ottawa.

#### 3.3.5 Post-Development Conditions

Under post-development conditions, the imperviousness of the site will increase. In order to mitigate the stormwater related impacts due to the proposed development, post-development flows will have to be controlled and stored on site via a storage tank prior to the runoff entering the existing municipal storm sewers in Champagne Avenue South. Refer to **Appendix D** for uncontrolled runoff calculations for the sub catchments areas for the site.

#### Area A-1 - Uncontrolled Landscaped Area

The post-development runoff from subcatchment area A-1 (uncontrolled runoff) was calculated using the Rational Method to be 1.8 L/s for the 1:5 year design event and 3.8 L/s for the 1:100 year design event (refer to **Appendix D** for detailed calculations).

#### Area A-2 - Controlled Building Roof and Landscaped Area

Stormwater runoff from this sub-catchment area will be captured by roof drains, deck drains and landscaped drains (within the rear yard amenity space) and by a trench drain at the bottom of the ramp to the underground parking level and directed to an internal stormwater storage tank.

Stormwater collected within the storage tank will be pumped up to the proposed storm service and released into the existing storm sewer in Champagne Avenue South. A pump (designed by the mechanical consultant) is required to control flow from the tank to a maximum rate of 30.0 L/s (475 USGPM), which corresponds to the maximum allowable flow for this catchment area. It is anticipated that a "stand-by" pump will be provided for emergency and/or maintenance purposes. An emergency power supply will also likely be provided. The storm service will be equipped with a backflow prevention device to protect the building from any potential sewer back-ups into the tank.

Table 2.3 summarizes the post-development design flows and storage volumes from area A-2 for both the 1:5-year and 1:100-year design events.

Table 2.3: Internal Stormwater Storage Tank and Pumped Flow

Design		Post-Development Condit	ions
Event	Design Flow (L/s)	Volume Required (m³)	Volume Provided (m³)
1:5 Year	30.0 L/s	11.8 m³	>42 m³
1:100 Year	30.0 L/s	41.4 m³	>42 m³

#### **Summary of Post-Development Flows**

**Table 3.4: Post-Development Stormwater Flow Table** 

	Post - Development Flows					
Area	Area Description Post-Developme Flow (L/s)				e Required (m³)	Provided
	•	5 year	100 year	5 year	100 year	(m³)
A-1	Uncontrolled Landscaped Area	1.8	3.8	N/A	N/A	N/A
A-2	Controlled Building Roof and Landscaped Area	30.0	30.0	11.8	41.4	>42
	Total Flow =	31.8	33.8			

As indicated in **Table 3.4** the total post-development flow from the sub-catchment areas will be released from the proposed development at a combined maximum rate of 33.8 L/s during the 1:100 year design event and 31.8 L/s during the 1:5 year design event; neither of which exceeds the allowable flow for the site of 34.0 L/s.

#### 4.0 SITE GRADING

The intent of the grading design was to propose the building finished floor elevation to best tie into the elevations along the existing adjacent roadway and surrounding property lines. The proposed grading design provides positive drainage away from the building and towards the onsite stormwater drainage structures. Refer to the enclosed Grading and Erosion & Sediment Control Plan (119013-GR) for details.

#### 4.1 Major System Overflow Route

In the case of a major rainfall event exceeding the design storms provided for, the stormwater located within the rear yard landscaped area and amenity space will overflow towards Champagne Avenue South at the south side of the building. The major system overflow route is shown on the enclosed Grading and Erosion & Sediment Control Plan (119013-GR).

#### 4.2 Erosion and Sediment Control

Erosion and sediment control measures will be implemented during construction in accordance with the "Guidelines on Erosion and Sediment Control for Urban Construction Sites" (Government of Ontario, May 1987). Details are provided on the Grading and Erosion & Sediment Control Plan (113023-GR).

- All erosion and sediment control measures are to be installed to the satisfaction of the engineer, the municipality and the conservation authority prior to undertaking any site alterations (filling, grading, removal of vegetation, etc.) and remain present during all phases of site preparation and construction.
- A qualified inspector should conduct daily visits during construction to ensure that the contractor is working in accord with the design drawings and that mitigation measures are being implemented as specified.
  - A light duty silt fence is to be installed as per OPSS 577 and OPSD 219.110 along the surrounding construction limits.

- Filter cloth is to be placed under the grates of all proposed and existing catchbasins and catchbasin manhole drainage structures.
- Street sweeping and cleaning will be performed, as required, to suppress dust and to provide safe and clean roadways adjacent to the construction site.
- After complete build-out, all sewers are to be inspected and cleaned and all sediment and construction fencing is to be removed.
- The contractor shall immediately report to the engineer or inspector any accidental discharges of sediment material into any ditch or sewer system. Appropriate response measures shall be carried out by the contractor without delay.

The proposed temporary erosion and sediment control measures will be implemented prior to construction and will remain in place during all phases of construction.

#### 5.0 GEOTECHNICAL INVESTIGATIONS

A Geotechnical Investigation Report has been prepared for the proposed site. Refer to the Pinchin 'Geotechnical Investigation – Proposed Residential Development' (Report. No. 235750.001), dated March 20, 2019 for the existing subsurface conditions, construction recommendations and geotechnical inspection requirements for the proposed development.

#### 6.0 SUMMARY AND CONCLUSIONS

This report has been prepared in support of the site plan application for the proposed development located at 90 Champagne Avenue South, in the City of Ottawa.

The conclusions are as follows:

- The proposed development will be serviced by connecting to the existing municipal sanitary and storm sewer systems and the existing municipal watermain within the Champagne Avenue South Right-Of-Way.
- The proposed building will be sprinklered. The fire protection will be provided from the existing municipal fire hydrant in Champagne Avenue South located within 45m unobstructed path from the proposed fore department siamese connection.
- The site flows from sub-catchment areas A-1 uncontrolled. The flows from sub-catchment area A-2 will be stored in an internal storage tank and controlled by mechanical pumps.
- The total post-development flow from the will be controlled to a maximum of 33.8 L/s during the 1:100 year design event and to 31.8 L/s during the 1:5 year design event. Neither of which exceed the maximum allowable release rate of 34.0 L/s as calculated to meet the City of Ottawa stormwater quantity requirements.
- Temporary erosion and sediment controls are to be provided during construction.

Servicing assessments discussed in the preceding sections show that there are no major obstacles to servicing the proposed development. It is recommended that the proposed site servicing and stormwater management design be approved for implementation.

#### **NOVATECH**

Prepared by:



Miroslav Savic, P. Eng. Senior Project Manager | Land Development

Reviewed by:

Lee Sheets, C.E.T.

Director | Land Development & Public Sector Infrastructure

#### **APPENDIX A**

Correspondence

#### **Miro Savic**

**Subject:** FW: 1809 - 90 Champagne Avenue South - Submission List

**Attachments:** Submission list.pdf

From: Jaime Posen <posen@fotenn.com>

Sent: January-16-19 2:03 PM

To: Robert Verch < rverch@rlaarchitecture.ca>

Subject: FW: 90 Champagne Avenue South - Submission List

Hi Rob.

Per your email a moment ago, we received the attached submission list for this application. No specific comments were received.

Hope that's helpful,

#### Jaime Posen, MCIP RPP

Senior Planner

T 613.730.5709 ext. 236

From: Jaime Posen

Sent: January 2, 2019 3:27 PM

To: Roderick Lahey <rlahey@rlaarchitecture.ca>; kelly@districtrealty.com; Jason Shinder <Jason@districtrealty.com>

Cc: Ghada Zaki <Zaki@fotenn.com>

Subject: 90 Champagne Avenue South - Submission List

Hello team,

Happy new year to all. Please see the attached Submission List for 90 Champagne Avenue South, as provided by Steve Gauthier.

We will continue to prepare our Planning Rationale for the Site Plan application.

Cheers.

#### Jaime Posen, MCIP RPP

Senior Planner

#### **FOTENN**

223 McLeod St Ottawa, ON K2P 0Z8 T 613.730.5709 ext. 236 fotenn.com

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#### APPLICANT'S STUDY AND PLAN IDENTIFICATION LIST

Legend: S indicates that the study or plan is required with application submission.

A indicates that the study or plan may be required to satisfy a condition of approval/draft approval.

For information and guidance on preparing required studies and plans refer to:

http://ottawa.ca/en/development-application-review-process-0/guide-preparing-studies-and-plans

S/A	Number of copies	ENG	GINEERING	S/A	Number of copies
S	15	Site Servicing Plan	2. Site Servicing Study / Brief	S	3
S	15	3. Grade Control and Drainage Plan	4. Geotechnical Study / Slope Stability Study	S	3
		5. Composite Utility Plan	6. Groundwater Impact Study		
		7. Servicing Options Report	8. Wellhead Protection Study		
S	7	9. Transportation Impact Study / Brief	10.Erosion and Sediment Control Plan / Brief	s	3
S	3	11.Storm water Management Report / Brief	12.Hydro geological and Terrain Analysis		
		13.Hydraulic Water main Analysis	14.Noise / Vibration Study (If on-site stationary noise source)		
		15.Roadway Modification Design Plan	16.Confederation Line Proximity Study		

S/A	Number of copies	PLANNING / DESIGN / SURVEY		S/A	Number of copies
		17.Draft Plan of Subdivision	18.Plan Showing Layout of Parking Garage		
		19.Draft Plan of Condominium	20.Planning Rationale	S	3
S	15	21.Site Plan	22.Minimum Distance Separation (MDS)		
		23.Concept Plan Showing Proposed Land Uses and Landscaping	24.Agrology and Soil Capability Study		
		25.Concept Plan Showing Ultimate Use of Land	26.Cultural Heritage Impact Statement		
S	15	27.Landscape Plan (showing existing trees)	28.Archaeological Resource Assessment Requirements: <b>S</b> (site plan) <b>A</b> (subdivision, condo)		
S	2	29.Survey Plan	30.Shadow Analysis	S	3
s	3	31.Architectural Building Elevation Drawings (dimensioned)	32.Design Brief (includes the Design Review Panel Submission Requirements)	S	3
S	3	33.Wind Analysis			

S/A	Number of copies	ENVIRONMENTAL		S/A	Number of copies
S	3	34.Phase 1 Environmental Site Assessment	35.Impact Assessment of Adjacent Waste Disposal/Former Landfill Site		
		36.Phase 2 Environmental Site Assessment (depends on the outcome of Phase 1)	37.Assessment of Landform Features		
		38.Record of Site Condition	39.Mineral Resource Impact Assessment		
		40.Tree Conservation Report	41.Environmental Impact Statement / Impact Assessment of Endangered Species		
		42.Mine Hazard Study / Abandoned Pit or Quarry Study			

S/A	Number of copies			S/A	Number of copies
		43.	44.		

Meeting Date: December 17, 2018	Application Type: Site Plan Control
File Lead (Assigned Planner): Steve Gauthier	Infrastructure Approvals Project Manager: John Wu
Site Address (Municipal Address): 90 Champagne Avenue	*Preliminary Assessment: 1 2 3 4 5 5

\*One (1) indicates that considerable major revisions are required before a planning application is submitted, while five (5) suggests that proposal appears to meet the City's key land use policies and guidelines. This assessment is purely advisory and does not consider technical aspects of the proposal or in any way guarantee application approval.

It is important to note that the need for additional studies and plans may result during application review. If following the submission of your application, it is determined that material that is not identified in this checklist is required to achieve complete application status, in accordance with the Planning Act and Official Plan requirements, the Planning and Growth Management Department will notify you of outstanding material required within the required 30 day period. Mandatory pre-application consultation will not shorten the City's standard processing timelines, or guarantee that an application will be approved. It is intended to help educate and inform the applicant about submission requirements as well as municipal processes, policies, and key issues in advance of submitting a formal development application. This list is valid for one year following the meeting date. If the application is not submitted within this timeframe the applicant must again pre-consult with the Planning and Growth Management Department.

110 Laurier Avenue West, Ottawa ON K1P 1J1 Mail code: 01-14 Visit us: Ottawa.ca/planning
110, av. Laurier Ouest, Ottawa (Ontario) K1P 1J1 Courrier interne : 01-14 Visitez-nous : Ottawa.ca/urbanisme

#### **Miro Savic**

From: Wu, John <John.Wu@ottawa.ca>
Sent: Thursday, March 07, 2019 2:27 PM

**To:** Miro Savic

**Subject:** RE: 90 Champagne Ave S - SWM Criteria

#### C 0.5, 5 year's storm

From: Miro Savic <m.savic@novatech-eng.com>

Sent: March 7, 2019 2:16 PM

To: Wu, John < John. Wu@ottawa.ca>

**Cc:** Cara Ruddle <c.ruddle@novatech-eng.com> **Subject:** 90 Champagne Ave S - SWM Criteria

Hi John,

Could you please provide the stormwater management criteria for the 90 Champagne Avenue South project.

Thank you,

Miroslav Savic, P.Eng., Senior Project Manager | Land Development Engineering

**NOVATECH** Engineers, Planners & Landscape Architects

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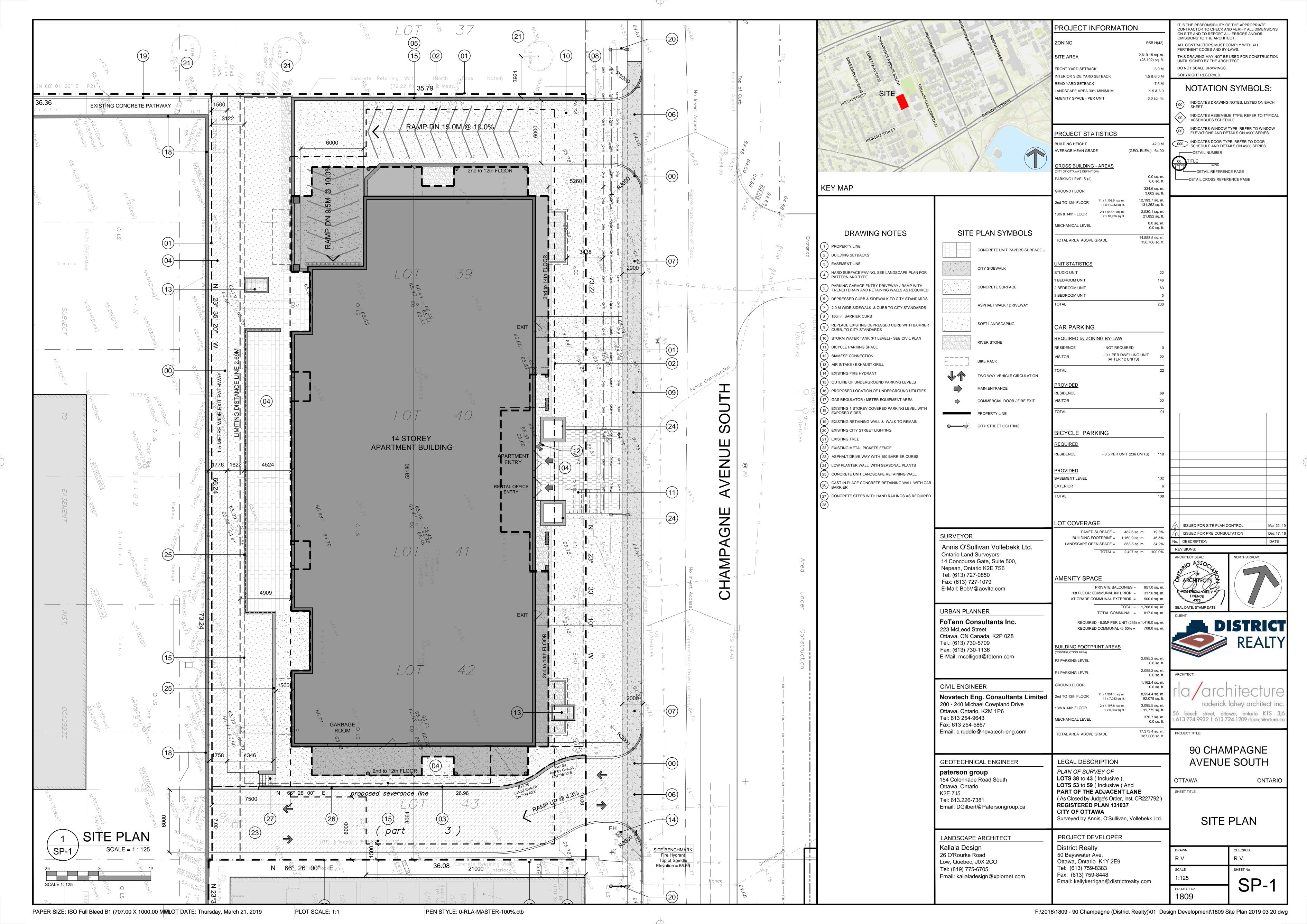
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#### **APPENDIX B**

Site Plan



|--|

90 Champagne Avenue South

#### **APPENDIX C**

Sanitary Sewer, Watermain and Fire Flow Calculations

# 90 CHAMPAGNE AVENUE SOUTH SANITARY FLOW

Number of 1 bdr Units	182
Persons per 1bdr Unit	1.4
Number of 2 bdr Apartment Units	54
Persons per 2 bdr Apprtment Unit	2.1
Total Population	369
Average Daily Flow	280 L/c/day
Peak Factor (Harmon Formula)	3.23
Peak Sanitary Flow	3.86 L/s
Site Area	0.26 ha
Infiltration Allowance	0.33 L/s/ha
Peak Extraneous Flows	0.09 L/s

#### **Miro Savic**

**From:** Wu, John <John.Wu@ottawa.ca> **Sent:** Friday, March 15, 2019 9:28 AM

**To:** Miro Savic

**Subject:** RE: 90 Champagne - Water Boundary Conditions

**Attachments:** 90 Champagne March 2019.pdf

#### Here it is:

The following are boundary conditions, HGL, for hydraulic analysis at 90 Champagne (zone 1W) assumed to be connected to the 203mm on Champagne (see attached PDF for location).

Minimum HGL = 107.2m

Maximum HGL = 114.7m

Max Day + Fire Flow (100 L/s) = 99.5m

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation.

#### John

From: Miro Savic <m.savic@novatech-eng.com>

Sent: March 12, 2019 11:04 AM
To: Wu, John < John.Wu@ottawa.ca>

**Cc:** Cara Ruddle <c.ruddle@novatech-eng.com> **Subject:** 90 Champagne - Water Boundary Conditions

John,

Please provide water boundary conditions for the 90 Champagne Avenue South project.

The water service will be connected to the existing 203mm diameter watermain in Champagne Avenue South at approximate location shown on the attached sketch. Fire protection for the building will be provide from the existing municipal fire hydrant located near the southeast corner of the site.

The water demands are calculated as follows: Average Day Demand = 1.49 L/s Maximum Day Demand = 3.74 L/s Peak Hour Demand = 8.22 L/s

The fire flow demand estimated using the Fire Underwrites Survey (FUS) = 100 L/s (6,000 L/min).

The FUS and the water demand calculations are attached for reference.

Regards,

Miroslav Savic, P.Eng., Senior Project Manager | Land Development Engineering

NOVATECH Engineers, Planners & Landscape Architects

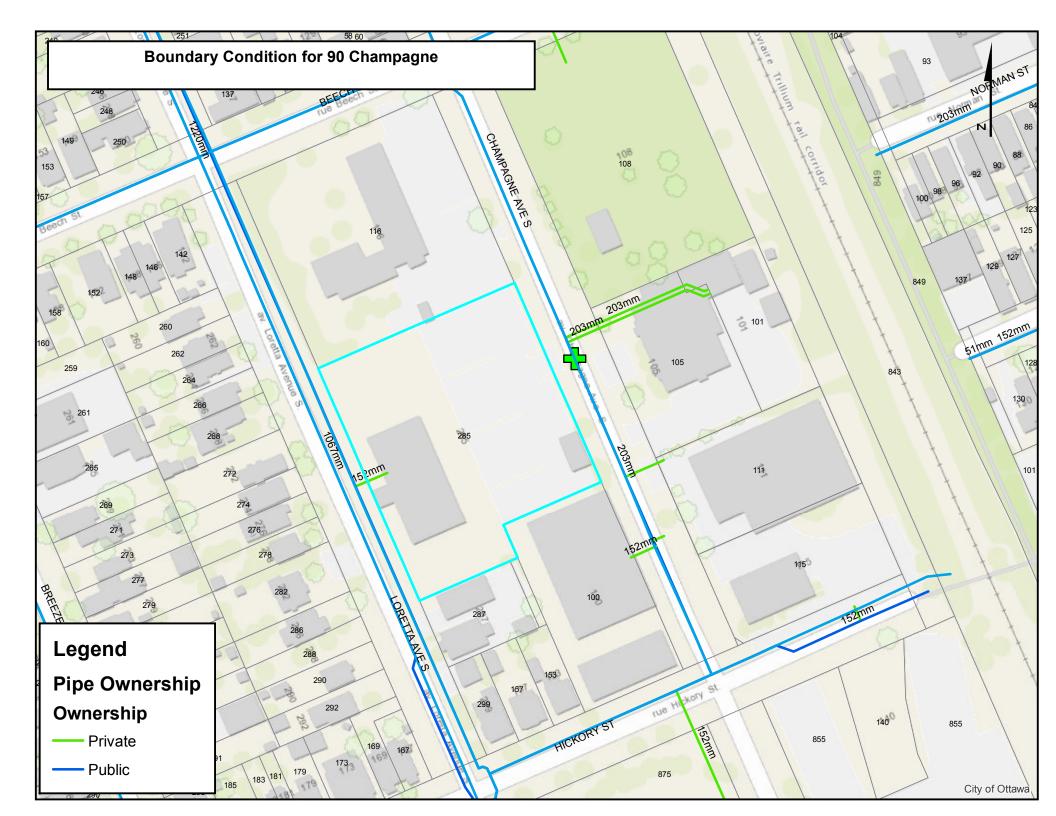
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# 90 CHAMPAGNE AVENUE SOUTH WATER ANALYSIS

#### **WATER DEMND**

Number of 1 bdr Units	182
Persons per 1bdr Unit	1.4
Number of 2 bdr Units	54
Persons per 2 bdr Unit	2.1
Total Population	369

Average Day Demand 350 L/c/day

Average Day Demand	1.49 L/s
Maximum Day Demand (2.5 x avg. day)	3.74 L/s
Peak Hour Demand (2.2 x avg. day)	8.22 L/s

#### **BOUNDAY CONDITIONS**

Maximum HGL =	114.7 m
Minimum HGL =	107.2 m
Max Day + Fire Flow =	99.5 m

#### **PRESSURE TESTS**

AVERAGE GROUND ELEVATION

65.0 m

HIGH PRESSURE TEST = MAX HGL - AVG GROUND ELEV x 1.42197 PSI/m < 80 PSI HIGH PRESSURE = **70.7** PSI

LOW PRESSURE TEST = MIN HGL - AVG GROUND ELEV x 1.42197 PSI/m > 40 PSI

LOW PRESSURE = 60.0 PSI

MAX DAY + FIRE FLOW TEST = MAX DAY + FIRE - AVG GROUND ELEV x 1.42197 PSI/m > 20 PSI

MAX DAY + FIRE PRESSURE = 49.1 PSI

#### **FUS - Fire Flow Calculations**

As per 1999 Fire Underwriter's Survey Guidelines

Novatech Project #: 119013

Project Name: 90 Champagne Avenue South

Date: 3/11/2019
Input By: Miroslav Savic

Reviewed By:

NOVATECH
Engineers, Planners & Landscape Architects

Legend Input by User

No Information or Input Required

Building Description: 14-Storey Residential Tower

**Fire Resistive Construction** 

Step		Choose		Value Used	Total Fire Flow (L/min)	
		Base Fire Flo	W			
	Construction Ma	nterial		Multi	plier	
	Coefficient	Wood frame		1.5		
1	related to type	Ordinary construction		1		
	of construction	Non-combustible construction		0.8	0.6	
	С	Modified Fire resistive construction (2 hrs)	Yes	0.6		
	 	Fire resistive construction (> 3 hrs)		0.6		
	Floor Area		1001			
		Building Footprint (m²)	1301			
	Α	Number of Floors/Storeys	14			
2		Protected Openings (1 hr)	Yes			
		Area of structure considered (m <sup>2</sup> )			1,952	
	F	Base fire flow without reductions				6,000
	$F = 220 \text{ C } (A)^{0.5}$					.,
		Reductions or Surc	harges			
	Occupancy haza	rd reduction or surcharge		Reduction/	Surcharge	
3		Non-combustible		-25%		
	(1)	Limited combustible	Yes	-15%		
3		Combustible		0%	-15%	5,100
		Free burning		15%		
		Rapid burning		25%		
	Sprinkler Reduction Reduct				ction	
		Adequately Designed System (NFPA 13)	Yes	-30%	-30%	
4	(0)	Standard Water Supply	Yes	-10%	-10%	0.040
	(2)	Fully Supervised System	No	-10%		-2,040
			Cun	nulative Total	-40%	
	Exposure Surch	arge (cumulative %)			Surcharge	
	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	North Side	3.1 - 10 m		20%	
_		East Side	30.1- 45 m		5%	
5	(3)	South Side	3.1 - 10 m		20%	3,060
	, ,	West Side	10.1 - 20 m		15%	
			Cum	nulative Total	60%	
		Results				
		Total Required Fire Flow, rounded to nea	rest 1000L/mir	ı	L/min	6,000
6	(1) + (2) + (3)	) lor		or	L/s	100
		(2,000 L/min < Fire Flow < 45,000 L/min)		or	USGPM	1,585
		Required Duration of Fire Flow (hours)			Hours	2
7	Storage Volume	Required Volume of Fire Flow (nodis)			m <sup>3</sup>	720

#### **APPENDIX D**

**Stormwater Management Calculations** 

#### Proposed Development 90 Champagne Avenue South

Allowable Flow					
Description	A (ha)	C <sub>5</sub>	C <sub>100</sub>	5 year (L/s)	100 year (L/s)
Site Area	0.235	0.50	0.50	34.0	34.0

Post - Development : Total Uncontrolled Site Flows								
Area	Description	A (ha)	A imp (ha)	A perv (ha)	C <sub>5</sub>	C <sub>100</sub>	Uncontrolle	ed Flow (L/s)
Area	Description	A (IIa)	C=0.9	C=0.2	0,5	0100	5 year	100 year
A-1	Uncontroled Landscaped Area	0.021	0.003	0.018	0.30	0.36	1.8	3.8
A-2	Controlled Building Roof and Landscaped Area	0.214	0.184	0.029	0.80	0.89	49.7	95.0

Summed Area Check: 0.235 t<sub>c</sub>=10mins

Post - Development : Total Flows for Controlled Site							
Aroo	Area Description		Flow (L/s)		Storage Required (m <sup>3</sup> )		Provided
Alea			100 year	5 year	100 year	(m³)	
A-1	Uncontroled Landscaped Area	1.8	3.8	N/A	N/A	N/A	
A-2	2 Controlled Building Roof and Landscaped Area		30.0	11.8	41.4	> 39.3	
<u>-</u>	Totals =	31.8	33.8	11.8	41.4	> 39.4	

Over-Controlled by: 2.2 0.2

90 Champagne Avenie South					
Project No.	119013				
REQUIRED	STORAGE - 1:5 YEAR EVENT				
AREA A-2	Controlled Flow Storage Tank				

OTTAWA IDF (	CURVE				
Area =	0.214	ha	Qallow =	30.00	L/s
C =	0.80		Vol(max) =	11.8	m3
Time	Intensity	Q	Qnet	Vol	
(min)	(mm/hr)	(L/s)	(L/s)	(m3)	
5	141.18	67.27	37.27	11.18	
10	104.19	49.65	19.65	11.79	
15	83.56	39.81	9.81	8.83	
20	70.25	33.47	3.47	4.17	
25	60.90	29.02	-0.98	-1.48	
30	53.93	25.70	-4.30	-7.75	
35	48.52	23.12	-6.88	-14.45	
40	44.18	21.05	-8.95	-21.47	
45	40.63	19.36	-10.64	-28.73	
50	37.65	17.94	-12.06	-36.18	
55	35.12	16.74	-13.26	-43.77	
60	32.94	15.70	-14.30	-51.49	
65	31.04	14.79	-15.21	-59.31	
70	29.37	14.00	-16.00	-67.22	
75	27.89	13.29	-16.71	-75.20	
90	24.29	11.57	-18.43	-99.50	
105	21.58	10.28	-19.72	-124.21	
120	19.47	9.28	-20.72	-149.21	

90 Champagne Avenie South Project No. 119013					
REQUIRED S	REQUIRED STORAGE - 1:100 YEAR EVENT				
AREA A-2 Controlled Flow Storage Tank					
OTTAWA IDE	OTTAWA IDE CLIRVE				

OTTAWA IDF	CURVE				
Area =	0.214	ha	Qallow =	30.00	L/s
C =	0.89		Vol(max) =	41.4	m3
Time	Intensity	Q	Qnet	Vol	
(min)	(mm/hr)	(L/s)	(L/s)	(m3)	
5	242.70	129.04	99.04	29.71	
10	178.56	94.94	64.94	38.96	
15	142.89	75.97	45.97	41.38	
20	119.95	63.77	33.77	40.53	
25	103.85	55.21	25.21	37.82	
30	91.87	48.84	18.84	33.92	
35	82.58	43.90	13.90	29.20	
40	75.15	39.95	9.95	23.89	
45	69.05	36.71	6.71	18.12	
50	63.95	34.00	4.00	12.01	
55	59.62	31.70	1.70	5.61	
60	55.89	29.72	-0.28	-1.02	
65	52.65	27.99	-2.01	-7.84	
70	49.79	26.47	-3.53	-14.82	
75	47.26	25.12	-4.88	-21.94	
90	41.11	21.86	-8.14	-43.97	
105	36.50	19.40	-10.60	-66.75	
120	32.89	17.49	-12.51	-90.08	

Development Servicing and SWM Report	90 Champagne Avenue Sou
APPENDIX E	
Excerpt from D. B. Grey Engineering Inc. Servicing Brief and Report	d Stormwater Management

# SERVICING BRIEF & STORMWATER MANAGEMENT REPORT

# 285 LORETTA AVENUE SOUTH Ottawa, Ontario

Report No. 16088

June 27, 2017 Revised July 26, 2017



#### D.B. GRAY ENGINEERING INC.

Stormwater Management - Grading & Drainage - Storm & Sanitary Sewers - Watermains

700 Long Point Circle Ottawa, Ontario K1T 4E9 613-425-8044 dbgray@rogers.com

### SERVICING BRIEF & STORMWATER MANAGEMENT REPORT

## 285 LORETTA AVENUE SOUTH Ottawa, Ontario

This Servicing Brief & Stormwater Management Report is a description of the proposed required changes to the services for an existing 15-storey apartment building as result of conditions set out in the Decision of the Committee of Adjustment for a proposed severance. The retained lands, 3,104 sq.m. in area, are fronting on Loretta Avenue South and it includes the 15-storey apartment building. This report also addresses the stormwater management requirements of the retained lands. The severed lands, (2,619 sq.m.) are fronting on Champagne Avenue South and is currently a parking lot. The severed lands are expected to be developed in the future and will be required to submit to the site plan control approval process. At that time the servicing and the stormwater management of the severed lands will be addressed.

Condition 1 of the Decision of the Committee of Adjustment states that proof is required that "each parcel can have its own independent storm (if applicable), sanitary & water services [that] are connected directly to City Infrastructure. These services should not cross the proposed severed property. If they do cross or they are not independent then the owner will be required to relocate or construct new services from the city sewers / watermain at his costs." The Decision of the Committee of Adjustment also states a "Right of Way over [the severed lands] for the benefit of the Owner of [the retained lands] for access to the [apartment building] parking garage and grant an Easement over [the severed lands] for Hydro Ottawa for facilities to service the existing apartment building ...." Servicing of the existing apartment building has been discussed with City of Ottawa Infrastructure Approvals staff and they have agreed that the storm and sanitary services can also be located in the right of way and easement (the water service does not need to be relocated).

Refer to drawing C-1 to C-3 also prepared by D. B. Gray Engineering Inc.

#### WATER SUPPLY FOR FIRE FIGHTING:

There is an existing fire hydrant in the Loretta Avenue South municipal right-of-way directly in front of the apartment building approximately 15m unobstructed distance from the main entrance. The proposed severance and changes will not affect the water supply for firefighting.

We understand that Loretta Avenue South will be re-constructed in the near future. Preliminary drawings indicate that the existing fire hydrant will be removed and a new hydrant located near the south-west corner of the subject property approximately 47m unobstructed distance from the main entrance. Also the existing 150mm watermain will

be abandoned and new 200mm watermain will be constructed; as result the available water supply is expected to increase.

#### WATER SERVICE:

The existing 150mm water service connects to an existing 150mm municipal watermain in Loretta Avenue South. The existing water service does not cross the severed lands; the proposed changes do not affect the water service; and there have been no issues with the water pressure in the building; as such the existing water service will remain.

As previously mentioned Loretta Avenue South will be re-constructed, with the existing 150mm watermain abandoned and new 200mm watermain constructed. As result the water pressure is expected to increase.

#### SANITARY SERVICE:

The existing 150mm sanitary service for the apartment building crosses the proposed severed lands and connects to 1050mm sanitary trunk sewer in Champagne Avenue South. The existing sanitary service will be abandoned to the property line. The existing sanitary service connection in the Champagne Avenue South R.O.W. from the property line to the point of connection to the trunk sewer will remain to serve the future development. A new 150mm / 200mm sanitary service is proposed to be located in a 7.0m wide right of way and easement located at the south end of the severed property.

The existing 15-storey apartment building consists of 88 one-bedroom and bachelor units and 56 two-bedroom units. Based on the City of Ottawa Sewer Design Guidelines for a residential property (1.4 persons per one-bedroom unit & 2.1 persons per two-bedroom unit -350 l/person/day - 4.0 peaking factor and a 0.28 l/s/ha infiltration flow) the post development flow is calculated to be 4.0 l/s. This flow will be adequately handled by the proposed sanitary sewer service having a minimum capacity of 16.3 l/s.

There are no proposed changes to the number or type of units to the existing apartment therefore there will be no increase to sanitary flows contributing to the existing 1050mm sanitary trunk sewer and as such the severance will have no impact on the trunk sewer until the severed lands are developed. The nature of the future development on the severed lands is not known at this time. When the severed lands are being proposed to be developed a servicing study will be submitted as part of site plan control that will determine the impact on the trunk sewer.

#### STORM SEWER SERVICE:

The existing 150mm / 200mm storm service for the apartment building crosses the proposed severed lands and connects to 1050mm storm trunk sewer in Champagne

Avenue South. The existing storm service will be abandoned to the property line. The existing storm service connection in the Champagne Avenue South R.O.W. from the property line to the point of connection to the trunk sewer will remain to serve the future development. There is also an existing storm sewer system for the parking lot on the severed lands which will remain in place until the severed lands are developed at which time they will be abandoned. A new 300mm storm sewer service is proposed to be located in a 7.0m wide right of way and easement located at the south end of the severed property.

The flowrate resulting from one in five year storm event will produce a peak flow of 78.9 l/s which will be adequately handled by a proposed storm sewer service (300mm @ 1.00% - 100.9 l/s capacity).

There are no proposed changes to the retained or severed lands that will increase to storm flows contributing to the existing 1050mm storm trunk sewer and as such the severance will have no impact on the trunk sewer.

#### STORMWATER MANAGEMENT:

#### Water Quality:

There are no existing quality control measures on the subject lands and no permanent on-site quality control measures are proposed.

An erosion and sediment control plan has been developed to be implemented during construction, (see notes 2.1 to 2.5 on drawing C-2). In summary: to filter out construction sediment capture filter sock inserts will be installed in all existing catch basins adjacent to the site and all new catch basins as they are installed; and geotextile fabric mud mats will be install at all points of egress to public roads.

#### Water Quantity:

There are no existing quantity control measures on the subject lands and none are proposed on the retained lands. The nature of the future development on the severed lands is not known at this time. When the severed lands are being proposed to be developed a stormwater management design will be submitted as part of the site plan control that I expect will demonstrate that the future development will have no negative impact on the trunk sewer, especially since the severed lands are currently all hard surfaces and the stormwater currently flows uncontrolled to the trunk sewer.

#### CONCLUSIONS:

- 1. The proposed severance and changes will not affect the water supply for firefighting.
- 2. The existing water service does not cross the severed lands; the proposed changes do not affect the water service; and there have been no issues with the water pressure in the building; as such the existing water service will remain
- 3. The existing sanitary service for the apartment building crosses the proposed severed lands and will be abandoned.
- 4. A new sanitary sewer service is proposed to be located in a 7.0m wide right of way and easement located at the south end of the severed property.
- 5. There are no proposed changes to the number or type of units to the existing apartment therefore there will be no proposed increase to sanitary flows contributing to the existing 1050mm sanitary trunk sewer and as such the severance will have no impact until the severed lands are developed. When the severed lands are being proposed to be developed a servicing study will be submitted as part of the site plan control that will determine the impact on the trunk sewer.
- 6. The existing storm sewer service for the apartment building crosses the proposed severed lands and will be abandoned.
- 7. A new storm sewer service is proposed to be located in a 7.0m wide right of way and easement located at the south end of the severed property.
- 8. There are no proposed changes to the retained lands that will increase the storm flows contributing to the existing 1050mm storm trunk sewer and future development on the severed lands will require a stormwater management plan and as such the severance will have no negative impact on the trunk sewer.

# D.B. GRAY ENGINEERING INC.

Somwater Management - Grading & Drainage - Storm & Samitary Servers - Watermains

700 Long Point Circle Ottawa, Ontario K1T 4E9

613-425-8044 dbgray@rogers.com

# SANITARY SEWER DESIGN FORM

 Average Daily Flows:
 Peaking Factor:

 Residential:
 350 1/ capital / day
 Residential (Harmon Equation):
 P.F.= 1 + 14

 Commercial:
 350 00 1 / ha / day
 P = Population / 1000
 4 + p 0.5

 Light inclusions:
 350 00 1 / ha / day
 Commercial & institutional:
 1.5

 Light inclusions:
 350 00 1 / ha / day
 Inclusived As per Ottawa objected & Appendix 4.B

 Heavy inclusions:
 350 01 / ha / day
 Inclusived Appendix 4.B

Infiltration Allowance: 0.28 1/s/ha

PROJECT: 285 Loretta Avenue South := 1 +  $\frac{14}{4 + p^{05}}$  Designed By. DBG

Date: ######

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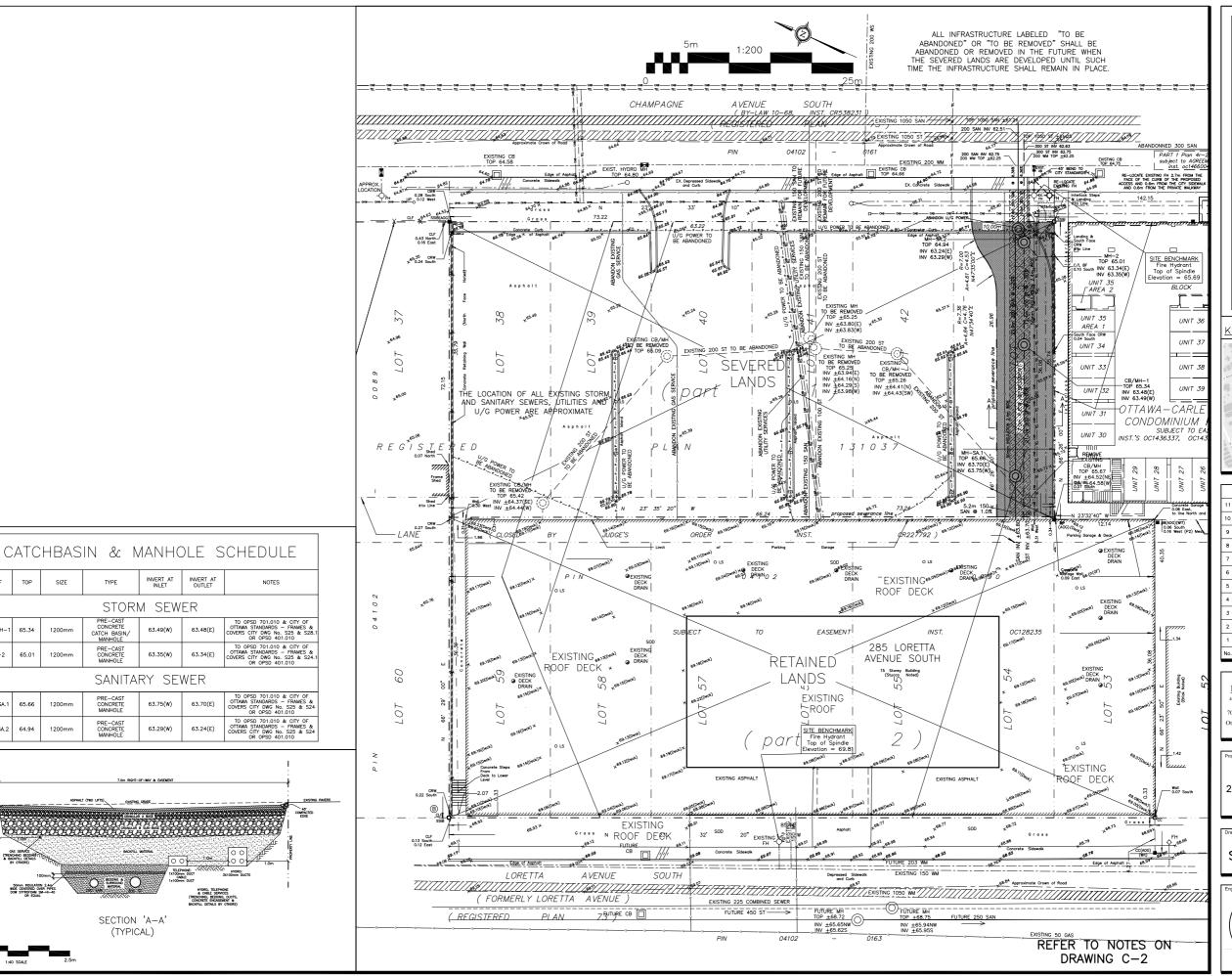
RATIONAL METHOD Q = 2.78 A I R FIVE YEAR EVENT STORM SEWER COMPUTATION FORM

n = 0.013

PROJECT: 285 Loretta Avenue South

Date: 27-Jun-17 Designed By: DBG

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of 1	cite	Q/Qfull		0.78	0.77	0.30																									Ħ		
Date: 27-Jun-17	Time of	Flow (min)		0.25	0.15	80.0																									П		
	Velocity	(m/s) F		1.38	1.38	3.49																									П		
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	Slope	(%)		1 00	1.00	6.36																									Ī		
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	Time of Conc. (min)			10.00	10.25	10.40																											
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		STREET																															



REF

CB/MH-1

MH-2

MH-SA.1 65.66

MH-SA.2

65.34

65.01

SIZE

1200mm

1200mm

1200mm

STORM SEWER

SANITARY SEWER

SECTION 'A-A' (TYPICAL)

63.49(W)

63.35(W)

63.75(W)

63.29(W)

PRE-CAST CONCRETE CATCH BASIN/ MANHOLE

DRAWING LEGEND

CB CATCH BASIN

MH () MANHOLE CB/MH (C) CATCH BASIN/MANHOLE

ws / wm WATER SERVICE / WATERMAIN

st \_\_ storm sewer

- VALVE & VALVE BOX

FH - FIRE HYDRANT UP O UTILITY POLE

EXISTING GRADE ELEVATION

0.5% SLOPE OF GRADE

. T.O.S. - TOP OF SLOPE

\_B.O.S. — BOTTOM OF SLOPE EMERGENCY OVERLAND

\_\_\_ P \_\_\_ U/G POWER

\_\_ CAB \_\_\_ ROGERS CABLE

\_\_\_ G\_\_\_\_ GAS

KEY PLAN



11	NOV 6-17	ISSUED FOR TENDER
10	AUG 8-17	RE-LOCATED FH LEAD REVISED RE-ISSUED FOR APPROVAL
9	AUG 3-17	RE-LOCATED FH REVISED RE-ISSUED FOR APPROVAL
8	JUL 27-17	RE-ISSUED FOR APPROVAL
7	JUL 26-17	RE-ISSUED FOR APPROVAL
6	JUN 27-17	STORM SEWERS RE-SIZED
5	MAY 15-17	UTILITIES ADDED IN ROAD R.O.W. EXISTING U/G POWER ADDED
4	APR 12-17	RE-ISSUED FOR APPROVAL
3	APR 10-17	ISSUED FOR JOB SHOWING
2	APR 3-17	ISSUED FOR APPROVAL
1	MAR 31-17	ISSUED TO CLIENT FOR REVIEW
No.	Date	REVISION

D. B. GRAY ENGINEERING INC.

00 Long Point Circle

tawa, Ontario K1T 4E9 dbgray@rogers.c

Tel: 613-425-804

PROPERTY SEVERANCE 285 LORETTA AVENUE SOUTH OTTAWA, ONTARIO

SITE SERVICING PLAN





of 3

### 1. GENERAL

1.1 USE BAR SCALE TO CONFIRM ACTUAL PLOT SCALE. EXISTING AND NEW ELEVATIONS AND INVERTS SHOWN ARE GEODETIC AND ARE IN METERS. SERVICES AND UTILITIES ARE ONLY INVESTIGATED AND THOSE SHOWN AND THOSE SHOWN AND THOSE SHOWN AND THOSE SHOWN ARE DERIVED FROM AVAILABLE INFORMATION AND MUST BE CONFIRMED ON SITE BEFORE COMMENSION. CONSTRUCTION, REPORT ANY DIFFERENCES TO

ENGINEER.

1.3 SITE BOUNDARIES AND EXISTING GRADES AND OTHER FEATURES DERIVED FROM TOPOGRAPHIC SURVEY PREPARED BY ANNIS O'SULLIVAN, VOLLEBEKK LTD JOB No. 16822—16.

1.4 REINSTATE ADJACENT PROPERTIES TO PRE-CONSTRUCTION CONDITIONS.

1.5 REINSTATE CITY PROPERTIES TO CITY STANDARDS AND TO CITY OF

1.5 REINSTATE CITY PROPERTIES IN CHIEF STAMMANDS AND TO CHI. TO CONTAINS SARISACTION.

1.6 ALL RELEVANT WORK SHALL BE DONE IN ACCORDANCE WITH CURRENT CITY STANDARDS AND SECREFICATION.

1.7 ONTARO PROVINCES ARE ANALAGEL.

1.7 ONTARO PROVINCES ARE ANALAGEL.

2. ELOSON CHILD ASSISTED LOURISM.

2. THE CONTRACTOR SHALL IMPLEMENT BEST MANAGEMENT PRACTICES TO PROVIDE FOR PROTECTION OF THE AREA DRAINAGE SYSTEM AND THE RECEIVED WATER COURSE QUINNEY CONSTRUCTION ACTIVITIES. THIS INCLUCIOS LIMITING WATER COURSE QUINNEY CONTRACT. THIS INCLUCIO SUMMINGERS IN CATCH BASINS AND MANHOLES AND INSTALLING SILT FENCES AND OTHER EFFECTIVE SEMENT THAN THE CONTRACTOR ACKNOWLEGOES THAT FAILURE TO IMPLEMENT APPROPRIATE EROSION AND SEDMENT CONTROL. REPOLICIES THAT FAILURE TO IMPLEMENT APPROPRIATE EROSION AND SEDMENT CONTROL.

FALLURE TO IMPLEMENT APPROPRIATE EROSION AND SEDIMENT CONTROL.

MEASURES MY EL SUBJECT TO PENALTIES IMPOSED BY ANY APPLICABLE
REQUIATORY AGENCY.

REGISTRATION OF CONSTRUCTION AT ALL MUNICIPAL CATCH
BASINS ADMINISTRATION OF CONSTRUCTION AT ALL MUNICIPAL CATCH
BASINS ADMINISTRATION OF CONSTRUCTION AT ALL MUNICIPAL CATCH
BASINS AS THEY ARE INSTALLED. INSTALL SEMBLEN CAPTURE FILTER SOCK
INSPECT AT THE SEN OF EACH DAY AND ATTER EACH SANIFALL REMOVE
SIDURENT AS RECOMMENDED BY THE MANUFACTURE. MIMEDIATELY REPAIR OR

REPLACE ANY DAMAGED FILTER SOCK INSERTS. DO NOT REMOVE UNTIL

2.3. CONTROL AND LIMIT ENELES ACCESSION ON ADMINISTRATION FROM TO

COMMENCEMENT OF CONSTRUCTION INSTALL AT ALL POINTS OF GRESS TO

PUBLIC ROADS GEOTETHIE FARRIC MUD MATS (TERRAPIX GEOSYNTHETICS INC.

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PUBLIC ROADS GEOTETHIE FARRIC MUD MATS (TERRAPIX DEPOSITED ON PUBLIC ROADS

SHALL BE REMOVED AT THE END OF EACH DAY BY SWEEPING AND SHOVEING

MATS SHALL BE WASHED AS REQUIRED TO PREVENT MATERIAL FROM BEING

TRACKED DOTN THE PUBLIC ROAD. DO NOT REMOVE MUD MATS (TERRA THE

PUBLIC ROADS GEOTETHIE FARRIC MUD MATS (TERRAPIX DAYS BERNOLL)

AND THE PUBLIC ROAD, MOST PUBLIC ROAD

AND THE PUBLIC ROAD, MATS PUBLIC PUBLIC ROAD ON THE PUBLIC ROAD.

ALL HARD SURFACES HAVE BEEN LONGSTRUCTION

2.4 CONSTRUCTION IS CONSIDERED COMPLETE WHEN THE FOLLOWING

CONDITIONS HAVE BEEN MED THE RESEN

#### 3. GRADING & DRAINAGE

3.1 NEW GRADES TO MATCH EXISTING AT PROPERTY LINE. NO EXCESS DRAINAGE WILL BE DIRECTED TOWARDS THE ADJACENT PROPERTIES DURING AN AFTER CONSTRUCTION. THERE WILL BE NO ALTERIATION TO DISTRING GRADE CONTROL OF A STATE OF THE ADJACENT OF THE

#### 4. SITE SERVICES

4.1 ALL FIRE HYDRANT LEAD MATERIALS AND CONSTRUCTION METHODS TO CITY OF OTTAWA STANDARDS AND SPECIFICATIONS.
4.2 CONNECTION TO EXISTING FIRE HYDRANT LEAD BY CITY OF OTTAWA, CONTRACTOR SHALL PROVIDE EXCAVATION, BACKFILL AND REINSTATEMENT.

CONTRACTOR SHALL PROVIDE EXCAVATION, BACKFILL AND REINSTATEMENT.

43. AT THE PROPERTY LINE, INSTALL A WARETRIFF CAP AT ENDS OF EXISTING
STORM AND SANITARY SAMENS CONNECTIONS. INSTALL A WATERTRIFF CAP
ENDS OF EXISTING SEMENS TO BE ARMONOMED. WATER CAPACITY
FROM THE CONTRACT OF THE CAPACITY OF THE CAPAC

. BE PANTED TO CITY STANDARDS.
CONNECT PROPOSED SANITARY SEWER SERVICE CONNECTION TO EXISTING SIPAL SANITARY SEWER AS PER CITY OF OTTAWA DWG No. S11 (RIGID MAIR R) AND S11.2 (BELL DIO INSERT METHOD).
CONNECT PROPOSED STORM SEWER SERVICE CONNECTION TO EXISTING SIPAL STORM SEWER AS PER CITY OF OTTAWA DWG No. S11 (RIGID MAIN R)AND S11.2 (BELL END INSERT METHOD).

SEWER/AND S11.2 (BELLE NO INSERT WETHOD).

4.7 SEWER MATERIA SHALL BE PVC SDR-35 (SDR-25 FOR DIAMETERS 150mm OR LESS) AND SHALL CONFORM TO LOAD 185.2 AND SHALL HAVE 150mm OR LESS) AND SHALL HAVE 185.3 EVEN SHALL HAVE 185.4 EVEN SHALL HAVE 185

WAIERSID/PJUNIN SELEVIN MAIERWIL.
F. SANITARY SEWERS: BENCH TO PROVIDE A SMOOTH U-SHAPED CHANNEL PER OPSD 701.021. SLOPE INVERT TO ESTABLISH SEWER GRADE.
G. STORM SEWERS: MANHOLES SHALL HAVE A 300mm SUMP AND CATCH BASINS AND DITCH INLETS SHALL HAVE A 600mm SUMP.

I, GRANULAR BEDDING AND BACKFILL: OPSS GRANULAR A. RE-CYLCLED GRANULAR MATERIALS ARE NOT PERMITTED.

OBTAIN AND PAY FOR ALL NECESSARY PERMITS AND APPROVALS FROM

UNHORITIES.

3. SIZE, DEPTH AND LOCATION OF EXISTING UTILITIES AND STRUCTURES AS ITED ARE FOR GUIDANCE ONLY. EXISTING UTILITIES AND STRUCTURES ARE STRUCTURES AND EXPOSED ONLY. EXISTING UTILITIES AND ACQUIRACY ARE NOT DEPENDENCY. AND ACQUIRACY ARE NOT DEPENDENCY. AND ACQUIRACY ARE NOT DEPENDENCY. AND ACQUIRACY AND ACQUIRACY DEPENDENCY. AND ACQUIRACY DEPENDENCY. UTILITIES AND STRUCTURES ON AND ENT TO STRUCTURES.

C. COORDINATE AND SCHEDULE WORK WITH THE AUTHORITIES AND OTHER TRADES. D. SCHEDULE WORK TO PROVIDE THE MINIMUM DISRUPTION TO SERVICES.

5.5 REMOVE OBSTRUCTIONS, ICE AND SNOW, FROM SURFACES TO BE

EACHARD.

5.6 CUT PAVEMENT AND / OR SIDEWALK NEATLY ALONG LIMITS OF PROPE EXCAVATION IN ORDER THAT SURFACE MAY BREAK EVENLY AND CLEANLY. 5.7 COORDINATE AND PAY FOR GEOTECHNICAL INSPECTIONS OF FILL, SUB-GRADE, PIPE BEDDING AND SURROUND MATERIAL, BACKFILL, SUB-BASE, AND ASPHALT TO THE SATISFACTION OF THE CONSULTANT. SUBMIT COMPACTION REPORTS TO ENGINEER.

COMPACTION REPORTS TO ENGINEER.

5.8 CUT AND FILL AS NECESSARY TO ACHIEVE THE REQUIRED SUB-GRADE ELEVATION. DISPOSE OF SURPLUS AND UNSUITABLE EXCAVATED MATERIAL OF SECRECATION. AND PROTECT FROM CONTAMINATION.

5.9 EXCAVATION, TRENCHING & BACKFILL

A. SHOBE AND BRACE EXCAVATIONS, PROTECT SLOPES AND BANKS AND PERFORM ALL WORK IN ACCORDANCE WITH OCCUPATIONAL HEALTH AND SAFETY ACT AND OTHER AUTHORITIES HANNIS JURISDICTION.

B. KEEP EXCAVATIONS FREE OF WATER WHILE WORK IS IN PROGRESS.
PROTECT OPEN EXCAVATIONS AGAINST FLOODING AND DAMAGE DUE TO SURFAC RUN-OFF.

C. EXCAVATION MUST NOT INTERFERE WITH BEARING CAPACITY OF ADJACENT FOUNDATIONS. D. DO NOT OBSTRUCT FLOW OF SURFACE DRAINAGE OR NATURAL WATERCOURSES.

E. EXCAVATE TO LINES, GRADES, ELEVATIONS AND DIMENSIONS AS INDICATED. INDICATED.

F. EARTH BOTTOMS OF EXCAVATIONS TO BE UNDISTURBED SOIL, LEVEL, FREE FROM LOOSE, SOFT OR ORGANIC MATTER.

C. ALL STRUCTURES WITHIN PAVED AREAS SHALL HAVE 4:1 FROST TAPERS FROM FROST LINE TO SUB-GRADE.

TAPERS FROM FROST LINE TO SUB-GRADE.

H. CORRECT OVER-EXCAVATION WITH GRANULAR A COMPACTED TO NOT LESS THAN 950 OF CORRECTED MAXIMUM DRY DENSITY.

SUB-GRADE AND AREAS TO BE BACKFILLED TO BE FREE FROM DEBRIS, SIOW, ICE, WARTER AND FROZEN GROUND.

J. DO NOT USE BACKFILL MATERIAL WHICH IS FROZEN OR CONTAINS ICE, SNOW OR DEBRIS.

K. BEDDING AND SURROUND MATERIAL FOR SEWERS SHALL BE OPSS
GRANULAR A. RE—CYLCLED GRANULAR MATERIALS ARE NOT PERMITTED.

GRANULAR A. RE-CYLCLED GRANULAR MATERIALS AFEND FEMITIED.

ON NOT USE BEDDING, SURROUND OR BACKFILL MATERIAL WHICH IS FROM THE STATE OF Q. DO NOT BACKFILL AROUND OR OVER CAST—IN-PLACE CONCRETE WITHIN 24 HOURS AFTER PLACING OF CONCRETE.

WITHIN 24 HOURS AFTER PLACING OF CONCRETE.

R BACKFILL MATERIALS WITHIN 1.8m OF PROPOSED GRADE SHALL MATCH
THE MATERIALS EXPOSED ON THE TRENCH WALLS. BACKFILL BELLOW 1.8m OF
THE PROPOSED CON CONSIST OF ETHER ACCEPTABLE NATIVE MATERIAL; ROCK,
OR MAPORTED GRANULAR MATERIAL CONCRAMING TO OPES GRANULAR B TYPE:
OF THE PROPOSED OF THE PRO 5.10 PIPES:

A. HANDLE PIPE USING METHODS APPROVED BY MANUFACTURER B. LAY, CUT AND JOIN PIPES IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS

RECOMMENDATIONS.

C. USE ONLY FITTINGS AS RECOMMENDED BY PIPE MANUFACTURERS.

C. USE ONLY FITTINGS AS RECOMMENDED BY PIPE MANUFACTURER.

D. LAY PIPES ON PREPARED BED, TRUE TO LINE AND GRADE AND ENSURE BARREL OF EACH PIPE IS IN CONTACT WITH SHAPED BED THROUGHOUT ITS FULL LEIGHT, FREE OF SAGS OR HIGH POINTS.

E. DO NOT EXCEED MAXIMUM JOINT DEPLECTION RECOMMENDED BY PIPE MANUFACTURED.

F. AT RIGID STRUCTURES, INSTALL PIPE JOINTS NOT MORE THAN 1.2m FROM SIDE OF STRUCTURE. . NOW SULE OF STRUCTURE.

G. WHENEVER WORK IS SUSPENDED, INSTALL REMOVABLE WATERTIGHT BULKHEAD AT OPEN END OF LAST PIPE LAID TO PREVENT ENTRY OF FOREIGN MATERIALS.

H. WHEN STOPPAGE OF WORK OCCURS, BLOCK PIPES TO PREVENT CREEP DURING DOWN TIME. MAKE WATERTIGHT CONNECTIONS TO MANHOLES.

H. WHEN STOPPAGE OF WORK OCCURS, BLOCK PIPES TO PREVENT GREEP DURING DOWN TIME. MAKE WATERIGHT CONNECTIONS TO MANUFACT.

I. USE NON-SHRING GROUT WHEN SUITABLE CASETS ARE NOT AVAILABLE.

J. JOINTS SHALL BE STRUCTURALLY SOUND AND WATERIGHT.

K. MANTAIN ENSITING SEWAGE FLOWS DURING CONSTRUCTION.

REPAIR OR REPUGLE PIPE, PIPE JOINT OR BEDIONS FOUND DEFECTIVE.

SEMES THE PROPOSE THE LOWER CONTROL OF ALL SANITARY SEMES.

SEMES WHIT OPES 140.

M. REPAIR NOR RETEST SEWER LINE AS REQUIRED.

N. REPAIR VISIBLE LEANS REGARDLESS OF TEST RESULTS.

O. CONDUCT TWO COTV INSPECTIONS OF SEWERS. FIRST INSPECTION FOR THE COMPLETION OF CONSTRUCTION. SECOND INSPECTION IMMEDIATELY PRIOR TO END OF WARRANTY PERIOD.

A PAN AND THE CONTROL THE SEMEST LINES AND CONDINANCE WITH PERIOD.

FOR PAN CONDUCT TO TEST OF SANITARY SEWERS AND CONDINANCE WITH ENDIFFER. CONDUCT THE TEST OF SANITARY SEWERS AND CONDINANCE WITH ENDIFFER.

A. JOINTS: SHALL BE WITHESSED BY ENGINEER.

B. SET PRECAST CONCRETE BASE ON 150mm MINIMUM OF GRANULAR

E. PLACE FRAME AND COVER ON TOP SECTION TO ELEVATION AS INDICATED. IF ADJUSTMENT REQUIRED USE CONCRETE RINGS TO A MAXIMUM OF 300mm.

F. CLEAN UNITS OF DEBRIS, FOREIGN AND SURPLUS MATERIALS. REMOVE FINS AND SHARP PROJECTIONS. PREVENT DEBRIS FROM ENTERING SYSTEM.

SERBEN, SPECIMENT HE PARAGE TESTING SHALL BE COMMETED IN SERBEN, SPECIMENT HE PARAGE TESTING SHALL BE COMMETED AND SECOND FROM THE ORIGINAL CONTRACT DOCUMENTS CAUSED BY SITE CONDITIONS AND CHANGES MADE OF CHANGES MADE OF ADDITIONAL INSTRUCTION. UPDATE DAILY AND MAKE AVAILABLE ON SITE FOR REVIEW INHOUGHOUT THE DID OF CONSTRUCTION. MARK CHANGES IN RED INK. RECORD DRAWING SHALL INCLUDE BUT NOT NECESSARILY UMITED TO CHANGES OF DIMENSION AND DETAIL, CHANGES TO GRADE ELEVATIONS, AND HORSOUTH AND VERTICAL PROPERTION. SERVICE AND THE CHANGES OF THE SERVICE OF THE

6. PAKEMENT

6.1 HEAVY DUTY PAVEMENT STRUCTURE:
40mm HL—3 OR SUPERPAVE 12.5 ASPHALTIC CONCRETE
50mm HL—6 OR SUPERPAVE 19.0 ASPHALTIC CONCRETE
150mm OPSS GRANULAR A BASE
450mm OPSS GRANULAR A BASE
450mm OPSS GRANULAR B TYPE II SUB—BASE
EE-CYCLED GRANULAR WATERLAS ARE NOT PERMITTED.
ASPHALTO CONCRETE SHALL BE PERFORMANCE GRADE POSB—34
PAVEMENT SUB—GRADE PREPAVATION AND CONSTRUCTION OF THE PAVEMENT
STRUCTURE OF THE SUB—GRADE LEPENT OF THE PAVEMENT
SANISFACTION OF THE GEOTECHNICAL ENGINEER. 6.2. WERMOVE ALL EXISTING
ASPHALT AND HAUL TO A FACIOUTLY APPROVED FOR ACCEPTING SUCH MATERIAS.
REMOVE ALL MATERIAS TO THE SUB—GRADE LEVEL. REMOVE ORGANIC OR
SANISFACTION OF THE GEOTECHNICAL ENGINEER. SUB—GRADE TO 95 ME. ASPHALT AND FOR THE GEOTECHNICAL ENGINEER. SUB—GRADE TO 95 ME. ASPHALT AND OF THE GEOTECHNICAL ENGINEER. SUB—GRADE TO 95 ME. ASPHALT AND SANISFACTION OF THE GEOTECHNICAL ENGINEER. SUB—GRADE TO 95 ME. ASPHALT AND SANISFACTION OF THE GEOTECHNICAL ENGINEER. SUB—GRADE TO 95 ME. ASPHALT AND SANISFACTION OF THE GEOTECHNICAL ENGINEER. SUB—GRADE TO 95 ME. ASPHALT AND SANISFACTION OF THE GEOTECHNICAL ENGINEER.
8.3 CONSTRUCT GRANUTARY THE PAVEMENT STRUCTURES AND BEFORE IN
8.3 CONSTRUCT GRANUTARY THE PAVEMENT STRUCTURES AND BETWEEN PAVEMENT AND CUBBS AND SIDEWALKS.
8.4 TRANSITION BETWEEN DIFFERING PAVEMENT STRUCTURES AND BETWEEN PAV

CLEAN UNFROZEN SUPFACE, FREE FROM SNOW OR ICE.

6.5 PLACE MATERIAL TO FULL WORTH IN UNFORM LAYERS NOT EXCEEDING 150mm COMPACTED THICKNESS. SHAPE EACH LAYER TO SMOOTH CONTOUR AND COMPACT TO SPECIFIED DENTST DEFORES LOCEEDING LAYER IS PLACED.

6.6 COMPACT SUB-BASE MATERIAL TO DENSITY OF NOT LESS THAN 98% OF CORRECTED MANUM DRY DENSITY. FILL OYER-ECKNATED SUB-BASE MATERIAL TO DENSITY AND COMPACT DESS MATERIAL TO DENSITY CAN SUB-COMPACT DESS MATERIAL TO DESS MATERIAL TO DENSITY CAN SUB-COMPACT DESS MATERIAL TO DESS MAT (A) N AREA NOT ACCESSIBLE TO ROLLING CUMPMENT, COMPACT TO SPECIFIED DENSITY WITH MECHANICAL TAMPERS.

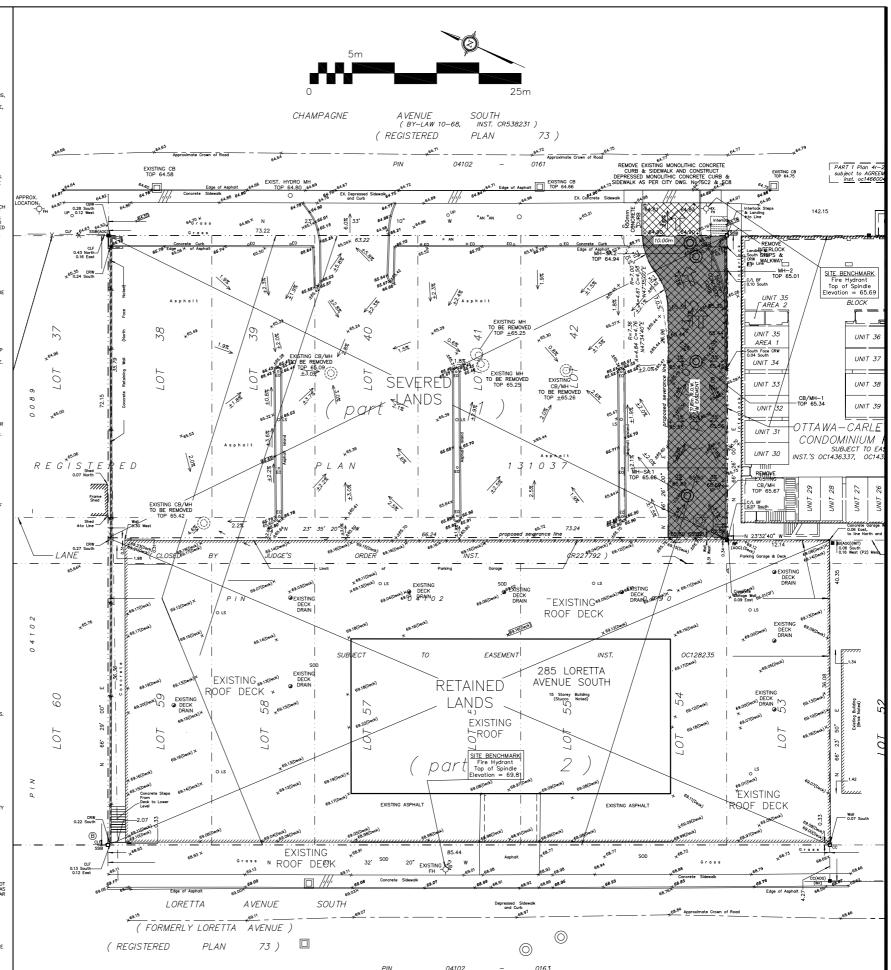
6.8 REPLACE PARWENT DISTURBED BY CONSTRUCTION AND REPLACE WITH PAVEMENT STRUCTURE ABOVE.

6.9 WHERE NEW ASPHALT COMES IN CONTACT WITH EXISTING PAVEMENT CUT EXISTING ASPHALT LAYER CLEANLY. 6.10 SHAPE BASE TO SMOOTH CONTOUR AND COMPACT TO NOT LESS THAN 100% CORRECTED MAXIMUM DRY DENSITY BEFORE BEGINNING PAVING OPERATIONS.

6.11 APPLY ASPHALTIC CONCRETE ONLY WHEN BASE OR PREVIOUS COURSE IS DRY AND AIR TEMPERATURE IS ABOVE 5 DEG.C

6.15 DIVERT UNUSED AND WASTE ASPHALT TO A FACILITY APPROVED FOR ACCEPTING SUCH MATERIALS.

ACCEPTING SUCH MATERIALS.
6.16 APPLY TRAFFIC PAINT AS IDENTIFIED ON PLAN. TRAFFIC PAINT:
NON-DARKENING, HOMOGENEOUS, UNIFORM AND SMOOTH, FREE FROM SKIN,
DIRT AND OTHER FOREIGN PARTICLES, APPLY TO DRY PAYEMENT SURFACE FREE
FROM FROST, ICE, DUST, OIL, GREASE AND OTHER FOREIGN MATERIALS.
PROTECT PAREMENT MARKINGS UNTIL DRY.



DRAWING LEGEND

CB CATCH BASIN

MH (O) MANHOLE CB/MH ( CATCH BASIN/MANHOLE

SAN SANITARY SEWER \_\_ st \_\_ storm sewer

HPO UTILITY POLE

EXISTING GRADE ELEVATION + 93.79 PROPOSED GRADE ELEVATION

HEAVY DUTY PAVEMENT 0.5% SLOPE OF GRADE

T.O.S. - TOP OF SLOPE

\_\_B.O.S.\_ — BOTTOM OF SLOPE EMERGENCY OVERLAND

----- PROPERTY LINE

- TEL - BELL TELEPHONE - CAB ROGERS CABLE

KEY PLAN



7	NOV 6-17	ISSUED FOR TENDER
6	AUG 8-17	FH NOTES (4.1 & 4.2) REVISED RE-ISSUED FOR APPROVAL
5	JUL 26-17	RE-ISSUED FOR APPROVAL
4	APR 12-17	RE-ISSUED FOR APPROVAL
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PROPERTY SEVERANCE 285 LORETTA AVENUE SOUTH OTTAWA, ONTARIO

GRADING PLAN & NOTES





of 3

## **APPENDIX F**

**Development Servicing Study Checklist** 

4.1 General Content	Addressed (Y/N/NA)	Comments
Executive Summary (for larger reports only).	N/A	
Date and revision number of the report.	Υ	
Location map and plan showing municipal address,	Υ	
boundary, and layout of proposed development.		
Plan showing the site and location of all existing services.	Υ	
Development statistics, land use, density, adherence to		
zoning and official plan, and reference to applicable subwatershed and watershed plans that provide	N	Refer to Site Plan
context to which individual developments must adhere.		
Summary of Pre-consultation Meetings with City and other approval agencies.	N	
Reference and confirm conformance to higher level		
studies and reports (Master Servicing Studies,		
Environmental Assessments, Community Design Plans),	N/A	
or in the case where it is not in conformance, the	N/A	
proponent must provide justification and develop a		
defendable design criteria.		
Statement of objectives and servicing criteria.	Υ	
Identification of existing and proposed infrastructure	Υ	
available in the immediate area.	'	
Identification of Environmentally Significant Areas,		
watercourses and Municipal Drains potentially		
impacted by the proposed development (Reference can	N/A	
be made to the Natural Heritage Studies, if available).		
Concept level master grading plan to confirm existing		
and proposed grades in the development. This is		
required to confirm the feasibility of proposed		
stormwater management and drainage, soil removal	N/A	
and fill constraints, and potential impacts to		
neighboring properties. This is also required to confirm		
that the proposed grading will not impede existing		
major system flow paths.		

4.1 General Content	Addressed (Y/N/NA)	Comments
Identification of potential impacts of proposed piped services on private services (such as wells and septic fields on adjacent lands) and mitigation required to address potential impacts.	N/A	
Proposed phasing of the development, if applicable.	N/A	
Reference to geotechnical studies and recommendations concerning servicing.	Υ	
All preliminary and formal site plan submissions should have the following information:		
Metric scale	Υ	
North arrow (including construction North)	Υ	
Key plan	Υ	
Name and contact information of applicant and property owner	Υ	
Property limits including bearings and dimensions	Υ	
Existing and proposed structures and parking areas	Υ	
Easements, road widening and rights-of-way	Υ	
Adjacent street names	Υ	

4.2 Water	Addressed (Y/N/NA)	Comments
Confirm consistency with Master Servicing Study, if available.	N/A	
Availability of public infrastructure to service proposed development.	Υ	
Identification of system constraints.	N/A	
Identify boundary conditions.	Y	Provided by City of Ottawa
Confirmation of adequate domestic supply and pressure.	Υ	
Confirmation of adequate fire flow protection and confirmation that fire flow is calculated as per the Fire Underwriter's Survey. Output should show available fire flow at locations throughout the development.	Υ	
Provide a check of high pressures. If pressure is found to be high, an assessment is required to confirm the application of pressure reducing valves.	Υ	
Definition of phasing constraints. Hydraulic modeling is required to confirm servicing for all defined phases of the project including the ultimate design.	N/A	
Address reliability requirements such as appropriate location of shut-off valves.	Υ	
Check on the necessity of a pressure zone boundary modification.	N/A	
Reference to water supply analysis to show that major infrastructure is capable of delivering sufficient water for the proposed land use. This includes data that shows that the expected demands under average day, peak hour and fire flow conditions provide water within the required pressure range.	Y	
Description of the proposed water distribution network, including locations of proposed connections to the existing system, provisions for necessary looping, and appurtenances (valves, pressure reducing valves, valve chambers, and fire hydrants) including special metering provisions.	Υ	
Description of off-site required feedermains, booster pumping stations, and other water infrastructure that will be ultimately required to service proposed development, including financing, interim facilities, and timing of implementation.	N/A	
Confirmation that water demands are calculated based on the City of Ottawa Design Guidelines.	Υ	
Provision of a model schematic showing the boundary conditions locations, streets, parcels, and building locations for reference.	N	

4.3 Wastewater	Addressed (Y/N/NA)	Comments
Summary of proposed design criteria (Note: Wetweather flow criteria should not deviate from the City of Ottawa Sewer Design Guidelines. Monitored flow data from relatively new infrastructure cannot be used to justify capacity requirements for proposed	Υ	
Confirm consistency with Master Servicing Study and/or justifications for deviations.	N/A	
Consideration of local conditions that may contribute to extraneous flows that are higher than the recommended flows in the guidelines. This includes groundwater and soil conditions, and age and condition of sewers.	N/A	
Description of existing sanitary sewer available for discharge of wastewater from proposed development.	Υ	
Verify available capacity in downstream sanitary sewer and/or identification of upgrades necessary to service the proposed development. (Reference can be made to previously completed Master Servicing Study if applicable)	N	
Calculations related to dry-weather and wet-weather flow rates from the development in standard MOE sanitary sewer design table (Appendix 'C') format.	Υ	
Description of proposed sewer network including sewers, pumping stations, and forcemains.	Υ	
Discussion of previously identified environmental constraints and impact on servicing (environmental constraints are related to limitations imposed on the development in order to preserve the physical condition of watercourses, vegetation, soil cover, as well as protecting against water quantity and quality).	N/A	
Pumping stations: impacts of proposed development on existing pumping stations or requirements for new pumping station to service development.	N/A	
Forcemain capacity in terms of operational redundancy, surge pressure and maximum flow velocity.	N/A	
Identification and implementation of the emergency overflow from sanitary pumping stations in relation to the hydraulic grade line to protect against basement flooding.	N/A	
Special considerations such as contamination, corrosive environment etc.	N/A	

4.4 Stormwater	Addressed (Y/N/NA)	Comments
Description of drainage outlets and downstream		
constraints including legality of outlet (i.e. municipal	Υ	
drain, right-of-way, watercourse, or private property).		
Analysis of the available capacity in existing public	21/2	All 11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
infrastructure.	N/A	Allowable release rate provide by City of Ottawa
A drawing showing the subject lands, its surroundings,		
the receiving watercourse, existing drainage patterns	Υ	
and proposed drainage patterns.		
Water quantity control objective (e.g. controlling post-		
development peak flows to pre-development level for		
storm events ranging from the 2 or 5 year event		
(dependent on the receiving sewer design) to 100 year		
return period); if other objectives are being applied, a	Υ	
rationale must be included with reference to hydrologic		
analyses of the potentially affected subwatersheds,		
taking into account long-term cumulative effects.		
Water Quality control objective (basic, normal or		
enhanced level of protection based on the sensitivities	N	
of the receiving watercourse) and storage	.,	
Description of stormwater management concept with		
facility locations and descriptions with references and	Υ	
supporting information.	'	
Set-back from private sewage disposal systems.	N/A	
Watercourse and hazard lands setbacks.	N/A	
Record of pre-consultation with the Ontario Ministry of	11,71	
Environment and the Conservation Authority that has	N/A	
jurisdiction on the affected watershed.	,	
Confirm consistency with sub-watershed and Master		
Servicing Study, if applicable study exists.	N/A	
Storage requirements (complete with calcs) and		
conveyance capacity for 5 yr and 100 yr events.	Υ	
Identification of watercourse within the proposed		
development and how watercourses will be protected,		
or, if necessary, altered by the proposed development	N/A	
with applicable approvals.		
Calculate pre and post development peak flow rates		
including a description of existing site conditions and		
proposed impervious areas and drainage catchments in	Υ	
comparison to existing conditions.	'	
companion to existing conditions.		
Any proposed diversion of drainage catchment areas	NI/A	
from one outlet to another.	N/A	
Proposed minor and major systems including locations	v	
and sizes of stormwater trunk sewers, and SWM	Υ	
If quantity control is not proposed, demonstration that		
downstream system has adequate capacity for the post-	N1 / A	
development flows up to and including the 100-year	N/A	
return period storm event.		

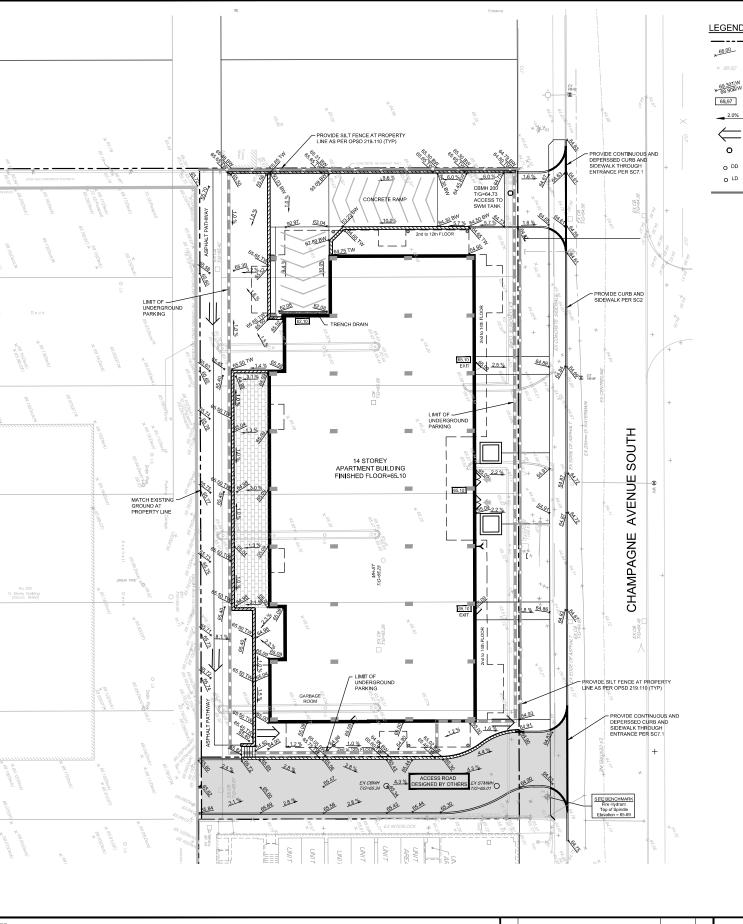
4.4 Stormwater	Addressed (Y/N/NA)	Comments
Identification of municipal drains and related approval	N1/A	
requirements.	N/A	
Description of how the conveyance and storage	Υ	
capacity will be achieved for the development.	r	
100 year flood levels and major flow routing to protect		
proposed development from flooding for establishing	Υ	
minimum building elevations (MBE) and overall grading.		
Inclusion of hydraulic analysis including HGL elevations.	N/A	
Description of approach to erosion and sediment		
control during construction for the protection of	Υ	
receiving watercourse or drainage corridors.		
Identification of floodplains – proponent to obtain		
relevant floodplain information from the appropriate	N/A	
Conservation Authority. The proponent may be		
required to delineate floodplain elevations to the		
satisfaction of the Conservation Authority if such		
information is not available or if information does not		
match current conditions.		
Identification of fill constrains related to floodplain and	N1 / A	
geotechnical investigation.	N/A	

4.5 Approval and Permit Requirements	Addressed (Y/N/NA)	Comments
Conservation Authority as the designated approval agency for modification of floodplain, potential impact on fish habitat, proposed works in or adjacent to a watercourse, cut/fill permits and Approval under Lakes and Rivers Improvement Act. The Conservation Authority is not the approval authority for the Lakes and Rivers Improvement Act. Where there are Conservation Authority regulations in place, approval under the Lakes and Rivers Improvement Act is not required, except in cases of dams as defined in the Act.	N/A	
Application for Certificate of Approval (CofA) under the Ontario Water Resources Act.	N/A	
Changes to Municipal Drains.	N/A	
Other permits (National Capital Commission, Parks Canada, Public Works and Government Services Canada, Ministry of Transportation etc.)	N/A	

4.6 Conclusion	Addressed (Y/N/NA)	Comments
Clearly stated conclusions and recommendations.	Υ	
Comments received from review agencies including the		
City of Ottawa and information on how the comments	v	T.B.D.
were addressed. Final sign-off from the responsible	Ť	1.6.0.
reviewing agency.		
All draft and final reports shall be signed and stamped		
by a professional Engineer registered in Ontario.	Υ	

## **APPENDIX G**

**Engineering Drawings** 

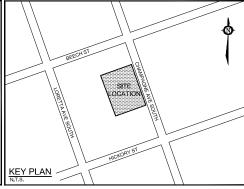




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PROPOSED ELEVATION EXISTING ELEVATION PROPOSED TOP OF WALL ELEVATION PROPOSED BOTTOM OF WALL ELEVATION PROPOSED FLOOR ELEVATION ANMH GRADE AND DIRECTION DIRECTION OF MAJOR OVERLAND FLOW PROPOSED 1200mm@ CATCHBASIN MANHOLE (ACCESS TO SWM STORAGE TANK) PROPOSED DECK DRAIN EX UP PROPOSED LANDSCAPE DRAIN ⊗—≒ EXISTING LIGHT STANDARD





## **GENERAL NOTES:**

PROPOSED BARRIER CURB

- 1. COORDINATE AND SCHEDULE ALL WORK WITH OTHER TRADES AND CONTRACTORS.
- 2. DETERMINE THE EXACT LOCATION, SIZE, MATERIAL AND ELEVATION OF ALL EXISTING UTILITIES PRIOR TO COMMENCING CONSTRUCTION, PROTECT AND ASSUME RESPONSIBILITY FOR ALL EXISTING UTILITIES WHETHER OR NOT SHOWN ON THIS DRAWING.
- OBTAIN ALL NECESSARY PERMITS AND APPROVALS FROM THE CITY OF OTTAWA BEFORE COMMENCING CONSTRUCTION.
- BEFORE COMMENCING CONSTRUCTION OBTAIN AND PROVIDE PROOF OF COMPREHENSIVE, ALL RISK AND OPERATIONAL LUBLITY INSURANCE FOR \$5.000,000.001. INSURANCE POLICY TO NAME OWNERS, ENGINEERS AND ARCHITECTS AS CO-INSURED.
- RESTORE ALL DISTURBED AREAS ON-SITE AND OFF-SITE, INCLUDING TRENCHES AND SURFACES ON PUBLIC ROAD ALLOWANCES TO EXISTING CONDITIONS OR BETTER TO THE SATISFACTION OF THE CITY OF OTTAWA AND ENGINEER.
- 6. REMOVE FROM SITE ALL EXCESS EXCAVATED MATERIAL, ORGANIC MATERIAL AND DEBRIS UNLESS OTHERWISE INSTRUCTED BY ENGINEER. EXCAVATE AND REMOVE FROM SITE ANY CONTAMINATED MATERIAL. ALL CONTAMINATED MATERIAL SALL BE DISPOSED OF AT A LICENSED LANDRILL FACILITY.
- 7. ALL ELEVATIONS ARE GEODETIC.
- REFER TO GEOTECHNICAL REPORT (235750.001, DATED MAR 20, 2019), PREPARED BY PINCHIN LTD, FOR SUBSURFACE CONDITIONS, CONSTRUCTION RECOMMENDATIONS, AND GEOTECHNICAL INSPECTION REQUIREMENTS. THE GEOTECHNICAL CONSULTANT IS TO REVIEW ON-SITE CONDITIONS AFTER EXCAVATION PRIOR TO PLACEMENT OF THE GRANULAR MATERIAL.
- 9. REFER TO ARCHITECT'S AND LANDSCAPE ARCHITECT'S DRAWINGS FOR BUILDING AND HARD SURFACE AREAS
- REFER TO STORMWATER MANAGEMENT REPORT (R-XXXX-XXX) PREPARED BY NOVATECH ENGINEERING CONSULTANTS LTD.
- SAW CUT AND KEY GRIND ASPHALT AT ALL ROAD CUTS AND ASPHALT TIE IN POINTS AS PER CITY OF OTTAWA STANDARDS (R10).
- 12. PROVIDE LINE/PARKING PAINTING

## **GRADING NOTES:**

- ALL TOPSOIL, ORGANIC OR DELETERIOUS MATERIAL MUST BE ENTIRELY REMOVED FROM BENEATH THE PROPOSED PAVED AREAS AS DIRECTED BY THE SITE ENGINEER OR GEOTECHNICAL ENGINEER.
- 2. EXPOSED SUBGRADES IN PROPOSED PAVED AREAS SHOULD BE PROOF ROLLED WITH A LARGE STEEL DRUM ROLLER AND INSPECTED BY THE GEOTECHNICAL ENGINEER PRIOR TO THE PLACEMENT OF GRANULARS.
- ANY SOFT AREAS EVIDENT FROM THE PROOF ROLLING SHOULD BE SUB-EXCAVATED AND REPLACED WITH SUITABLE MATERIAL THAT IS FROST COMPATIBLE WITH THE EXISTING SOILS AS RECOMMENDED BY THE GEOTECHNICAL ENGINEER
- 4. THE GRANULAR BASE SHOULD BE COMPACTED TO AT LEAST 100% OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY VALUE. ANY ADDITIONAL GRANULAR FILL USED BELOW THE PROPOSED PAVEMENT SHOULD BE COMPACTED TO AT LEAST 98% OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY VALUE.
- 5. MINIMUM OF 2% GRADE FOR ALL GRASS AREAS UNLESS OTHERWISE NOTED.
- 6. MAXIMUM TERRACING GRADE TO BE 3:1 UNLESS OTHERWISE NOTED.
- 7. ALL GRADES BY CURBS ARE EDGE OF PAVEMENT GRADES UNLESS OTHERWISE INDICATED.
- 8. ALL CURBS SHALL BE BARRIER CURB (150mm) UNLESS OTHERWISE NOTED AND CONSTRUCTED AS PER CITY OF OTTAWA STANDARDS (SC1.1).
- 9. REFER TO LANDSCAPE PLAN FOR PLANTING AND OTHER LANDSCAPE FEATURE DETAILS, CONTRACTOR TO PROVIDE THE CONSULTANT WITH A GRADING PLAN INDICATING AS-BUILT ELEVATIONS OF ALL DESIGN GRADES SHOWN ON THIS PLAN.

### **EROSION AND SEDIMENT CONTROL NOTES:**

- 1. THE CONTRACTOR SHALL IMPLEMENT BEST MANAGEMENT PRACTICES, TO PROVIDE FOR PROTECTION OF THE AREA DRAINAGE SYSTEM AND THE RECEIVING WATERCOURSE, DURING CONSTRUCTION ACTIVITIES. THE CONTRACTOR ACKNOWLEDGES THAT FAILURE TO IMPLEMENT APPROPRIATE EROSION AND SEDIMENT CONTROL MEASURES MAY BE SUBJECT TO PENALTIES IMPOSED BY ANY APPLICABLE REGULATORY AGENCY.

  ALL EROSION AND SEDIMENT CONTROLS ARE TO BE INSTALLED TO THE SATIFFACTION OF THE ENGINEER AND THE CITY OF OTTAWA. THEY ARE TO BE APPROPRIATE TO THE SITE CONDITIONS, PRONT O INDEPTAKING ANY SITE ALTERATIONS (FILLING, BRADING, REMOVAL OF VEGETATION, ETC.) AND DURING ALL PHASES OF SITE PREPARATION AND CONSTRUCTION. THESE PRACTICES ARE TO BE IMPLEMENTED IN ACCORDANCE WITH THE CURRENT BEST MANAGEMENT PRACTICES FOR EROSION AND SEDIMENT CONTROL AND SHOULD INCLUDE AS A MINIMUM THOSE MEASURES INDICATED ON THE PLAN.
- INTUINION THOSE MEASONES INCOMES IN THE LEVEL SET THE MEMBERS DULL BE IMPLEMENTED DURING CONSTRUCTION IN ACCORDANCE WITH THE "GUIDELINES ON EROSION AND SEDIMENT CONTROL FOR URBAN CONSTRUCTION SITES" (GOVERNMENT OF ONTARIO, MAY 1987), THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR MEETING ALL REGULATORY AGENCY REQUIREMENTS.
- A COPIE AND SURFACE EROSION FROM ENTERING ANY STORM SEWER SYSTEM DURING CONSTRUCTION, FILTER CLOTH WILL BE PLACED UNDER GRATES OF NEARBY CATCHBASING AND STRUCTURES, A LIGHT DUTY SILT FENCE BARRIER WILL ALSO BE INSTALLED AROUND THE CONSTRUCTION AREA (WHERE APPLICABLE). THESE CONTROL MEASURES WILL REMAIN IN PLACE UNTIL CONSTRUCTION IS COMPLETE.
- 5. TO LIMIT EROSION: MINIMIZE THE AMOUNT OF EXPOSED SOILS AT ANY GIVEN TIME, RE-VEGETATE EXPOSED AREAS AND SLOPES AS SOON AS POSSIBLE AND PROTECT EXPOSED SLOPES WITH NATURAL OR SYNTHETIC MULCHES.
- FOR MATERIAL STOCKPILINE, MINIMIZE THE AMOUNT OF EXPOSED SUCPENITION TO WITH THE INTERPLY TEMPORARY SEGUING, TAPES, COMPACTION AND/OR SURFACE ROUGHENING AS REQUIRED TO STABILIZE STOCKPILED MATERIALS AT ANY GIVEN TIME: APPLY TEMPORARY SEGUINED TO STABILIZE STOCKPILED MATERIALS TAT WILL NOT BE USED WITHIN 14 MEDIC BROWNING AS REQUIRED TO STABILIZE STOCKPILED.
- THE SEDIMENT CONTROL MEASURES SHALL ONLY BE REMOVED WHEN, IN THE OPINION OF THE ENGINEER, THE MEASURES ARE NO LONGER REQUIRED, NO CONTROL MEASURES MAY BE PERMANENTLY REMOVED WITHOUT PRIOR LETTER OF THE ENGINEER.
- PHOUR AUTHORIZATION PROM THE ENGINEERS.

  8. THE CONTRACTOR SHALL IMMEDITELY REPORT TO THE ENGINEER ANY ACCIDENTAL DISCHARGES OF SEDIMENT MATERIAL INTO ANY STORM SEWER SYSTEM. APPROPRIATE RESPONSE MEASURES, INCLUDING ANY REPAIRS TO EXISTING CONTROL MEASURES OR THE IMPLEMENTATION OF ADDITIONAL CONTROL MEASURES, SHALL BE CARRIED OUT BY THE CONTRACTOR WITHOUT DELAY.
- THE CONTRACTOR ACKNOWLEDGES THAT FAILURE TO IMPLEMENT EROSION AND SEDIMENT CONTROL MEASURES
   MAY BE SUBJECT TO PENALTIES IMPOSED BY ANY APPLICABLE REQULATORY AGENCY.
- 10. ROADWAYS ARE TO BE SWEPT AS REQUIRED OR AS DIRECTED BY THE ENGINEER AND/OR THE MUNICIPALITY.
- 11. THE CONTRACTOR SHALL ENSURE PROPER DUST CONTROL IS PROVIDED WITH THE APPLICATION OF WATER (AND IF RECUIRED, CALCIUM CHLORDE) DURING BYR PERIODS. MONTRO BUST LEVEL SECOME VISUALLY APPARENT SPRAY WATER TO MINIMIZE THE RELEASE OF DUST FROM GRAVEL, PAVED AREAS AND EXPOSED SOILS. USE CHEMICAL DUST SUPPRESSANTS ONLY WHERE NECESSARY OF MORGAVEL, PAVED AREAS AND EXPOSED SOILS. USES CHEMICAL DUST SUPPRESSANTS ONLY WHERE NECESSARY OF MORGLEMAREAS.

## Erosion and Sediment Control Responsibilities:

				During Construction			After Construction Price	After Final Acceptance	
	ESC Measure	Symbol	Specification	Installation Responsibility	Inspection/Maintenance Responsibility	Inspection Frequency	Approval to Remove	Removal Responsibility	Inspection/Maintenance Responsibility
	Silt Fence		OPSD 219.110	Developer's Contractor	Developer's Contractor	Weekly (as a minimum)	Consultant	Developer's Contractor	N/A
	Filter Fabric	Location as Indicated On Plans	Erosion and Sediment Control Notes	Developer's Contractor	Developer's Contractor	Weekly (as a minimum)	Consultant	Developer's Contractor	N/A
	Mud Mat	мм	Drawing Details	Developer's Contractor	Developer's Contractor	Weekly (as a minimum)	Developer's Contractor	Developer's Contractor	N/A
Temporary Measures	Dust Control	Location as Required Around Site	Erosion and Sediment Control Notes	Developer's Contractor	Developer's Contractor	Weekly (as a minimum)	Consultant	Developer's Contractor	N/A
	Stabilized Material Stockpiling	Location as Required by Contractor	Erosion and Sediment Control Notes	Developer's Contractor	Developer's Contractor	Weekly (as a minimum)	Developer's Contractor	Developer's Contractor	N/A
	Sediment Basin (for flows being pumped out of excavations)	Location as Required by Contractor		Developer's Contractor	Developer's Contractor	After Every Rainstorm	Developer's Contractor	Developer's Contractor	N/A

<u>NOTE:</u> THE POSITION OF ALL POLE LINES, CONDUITS, WATERMAINS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWNINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR AMAGE TO THEM.

SCALE 1:200 ISSUED FOR SITE PLAN APPLICATION MAR 22/19 MS ISSUED FOR COORDINATION MAR 19/19 MS

FOR REVIEW ONLY

NOVATECH

LOCATION CITY OF OTTAWA CHAMPAGNE AVENUE SOUTH

DRAWING NAME

GRADING AND EROSION & SEDIMENT CONTROL PLAN

1190 119013-GR

