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# Byron Place Apartments 433-435 Churchill Avenue and 468-472 Byron Place

**Transportation Impact Assessment** 

Byron Place Apartments 433-435 Churchill Avenue and 468-472 Byron Place

**Transportation Impact Assessment** 

Prepared By:

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April 2019

Novatech File: 118024 Ref: R-2018-028



April 5<sup>th</sup>, 2019

City of Ottawa Planning and Growth Management Department 110 Laurier Ave. W., 4<sup>th</sup> Floor, Ottawa, Ontario K1P 1J1

#### Attention: Mr. Wally Dubyk Project Manager, Infrastructure Approvals

Dear Mr. Dubyk:

#### Reference: 433-435 Churchill Avenue and 468-472 Byron Place Transportation Impact Assessment Novatech File No. 117198

We are pleased to submit the following Transportation Impact Assessment (TIA) in support of applications for rezoning and Site Plan Control for 433-435 Churchill Avenue and 468-472 Byron Place. The structure and format of this report is in accordance with the City of Ottawa Transportation Impact Assessment Guidelines (June 2017).

If you have any questions or comments regarding this report, please feel free to contact the undersigned.

Yours truly,

# NOVATECH

B. Byvelde

Brad Byvelds, P. Eng. Project Coordinator | Transportation/Traffic

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# **TIA Plan Reports**

On 14 June 2017, the Council of the City of Ottawa adopted new Transportation Impact Assessment (TIA) Guidelines. In adopting the guidelines, Council established a requirement for those preparing and delivering transportation impact assessments and reports to sign a letter of certification.

Individuals submitting TIA reports will be responsible for all aspects of development-related transportation assessment and reporting, and undertaking such work, in accordance and compliance with the City of Ottawa's Official Plan, the Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines.

By submitting the attached TIA report (and any associated documents) and signing this document, the individual acknowledges that s/he meets the four criteria listed below.

# CERTIFICATION

- 1. I have reviewed and have a sound understanding of the objectives, needs and requirements of the City of Ottawa's Official Plan, Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines;
- 2. I have a sound knowledge of industry standard practice with respect to the preparation of transportation impact assessment reports, including multi modal level of service review;
- 3. I have substantial experience (more than 5 years) in undertaking and delivering transportation impact studies (analysis, reporting and geometric design) with strong background knowledge in transportation planning, engineering or traffic operations; and
- I am either a licensed<sup>1</sup> or registered<sup>2</sup> professional in good standing, whose field of expertise [check √ appropriate field(s)] is either transportation engineering □ or transportation planning □.

<sup>1,2</sup> License of registration body that oversees the profession is required to have a code of conduct and ethics guidelines that will ensure appropriate conduct and representation for transportation planning and/or transportation engineering works.

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#### EXECUTIVE SUMMARY

This Transportation Impact Assessment (TIA) has been prepared in support of applications for rezoning and Site Plan Control for 433-435 Churchill Avenue and 468-472 Byron Place. The subject site has an area of approximately 0.2 hectares and is currently occupied by an automobile sales development and three low-rise residential developments. The subject site is surrounded by the following:

- Byron Place/Byron Avenue to the north;
- Low-rise residential development and office/commercial development to the south;
- Highcroft Avenue and low-rise residential development to the east; and
- Churchill Avenue, Churchill Alternative School and Westboro Masonic Hall to the west.

The proposed development will include 76 apartment units and two retail units with a combined gross floor area (GFA) of approximately 3,450ft<sup>2</sup>. The proposed development will include an underground parking garage containing 65 vehicle parking spaces and 45 bicycle parking spaces.

The City's 2017 TIA Guidelines identify three triggers for completing a TIA report, including trip generation, location, and safety. The criteria for each trigger are outlined in the City's TIA Screening Form. Based on the TIA Screening Form, the subject application satisfies the trip generation and safety triggers for completing a TIA.

The conclusions and recommendations of this TIA can be summarized as follows:

#### Development Design and Parking

- Byron Place will be partially closed to provide a public park area in the southeast corner of the Churchill Avenue/Byron Avenue intersection. The portion of Byron Place that remains open will function as the fire route for the proposed development. The width of Byron Place will be reduced to 6.0m in order to provide a wider landscape buffer between the proposed building and sidewalk. The turnaround hammerhead within Byron Place will also facilitate short-term parking for drop-offs/pick-ups.
- On-site pedestrian facilities will be provided between the retail entrances and the sidewalks along Churchill Avenue. A sidewalk will be provided along the northern frontage of the site along Byron Place, providing pedestrian connectivity between the residential entrances and the existing sidewalk along Churchill Avenue.
- A garbage room will be provided within the building. Garbage bins will be wheeled down the pathway south of the building, and will be picked up curbside along Churchill Avenue.
- All required TDM-supportive design and infrastructure measures in the TDM checklist are met.
- The proposed vehicular and bicycle parking meets the minimum requirement of the ZBL.

#### Boundary Streets

- Churchill Avenue currently meets the target BLOS B, TkLOS D and Auto LOS E, however it does not meet the target PLOS A. Byron Avenue meets the target Auto LOS E, however it does not meet the target PLOS A and BLOS B.
- To achieve the target PLOS A along both Churchill Avenue and Byron Avenue, either a reduction in the daily curb traffic to less than 3000 vehicles per day or a reduction in the operational speed to 30km/hr is required.

• The City of Ottawa's Ultimate Cycling Network identifies a future separated major pathway on the north side of Byron Avenue. The separated pathway will result in a BLOS A, achieving the target within 300m of a school.

#### Access Design

- Site access to Highcroft Avenue will provide access to the full movement intersection at Byron Avenue. The existing curb extension restricting the southbound through movement along Highcroft Avenue south of Byron Place will be removed and a new island will be provided to south of the proposed access to the parking garage for the proposed development.
- The width and length of the proposed island will eliminate any opportunity for vehicles leaving the site to turn right to travel southbound on Highcroft Avenue, as well as restrict westbound vehicles along Byron Avenue to cut through the residential community to avoid the traffic light at Churchill Avenue/Byron Avenue. The proposed island width (4.5m included curb offset) is an improvement on the existing curb extension which narrows the roadway by approximately 2.4m.
- The proposed driveway width and location adheres to the requirements of the City's ZBL and Private Approach By-law

# Transportation Demand Management and Transit

- To encourage travel by sustainable modes, the proponent agrees to implement the following TDM measures from the checklist:
  - Display local area maps with walking/cycling access routes and key destinations at major entrances;
  - Display relevant transit schedules and route maps at entrances;
  - o unbundle parking cost from monthly rent; and
  - o provide multimodal travel option information package to new residents.

#### Intersection Design

- The Churchill Avenue/Byron Avenue and Churchill Avenue/Richmond Road intersections do not meet the target PLOS A, BLOS B, TkLOS D, however they do meet the target Auto LOS E.
- A reduction in the pedestrian crossing distance would have the greatest improvement to the PLOS at these intersections.
- To achieve the target BLOS, consideration could be given by the City to reducing the operational speed to 50km/hr and providing a two-stage left turn bike box on all legs of these intersections.
- To achieve the target TkLOS, an effective turn radius greater than 15m is required on all four corners of these intersections. Increasing the turn radius at these intersections could be considered by the City, however it should be noted that the increased radius will have a negative impact on the PLOS.
- Critical movements at all study area intersections are anticipated to operate with a LOS D or better under background traffic conditions.
- The addition of site generated traffic volumes are not anticipated to have a significant impact to the intersection operations within the study area.
- Based on anticipated trip distribution, one vehicle during the AM peak hour and three vehicles during the PM peak hour are anticipated to arrive from the south. If these trips were to use Kenwood Avenue/Highcroft Avenue to access the site during the PM peak hour, this would result in one new vehicle approximately every 20 minutes. This increase in traffic is not

anticipated to be noticeable and will not have a significant impact on the existing operations along Highcroft Avenue.

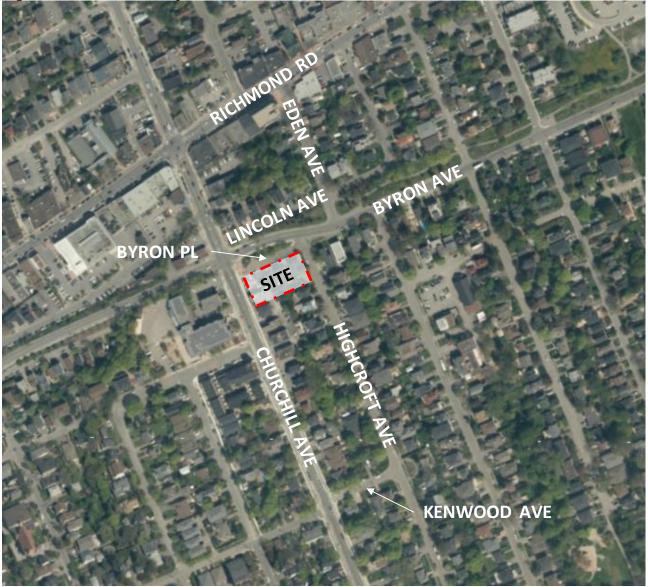
# 1.0 INTRODUCTION

This Transportation Impact Assessment (TIA) has been prepared in support of applications for rezoning and Site Plan Control for 433-435 Churchill Avenue and 468-472 Byron Place. The subject site has an area of approximately 0.2 hectares and is currently occupied by an automobile sales development and three low-rise residential developments. The subject site is surrounded by the following:

- Byron Place/Byron Avenue to the north;
- Low-rise residential development and office/commercial development to the south;
- Highcroft Avenue and low-rise residential development to the east; and
- Churchill Avenue, Churchill Alternative School and Westboro Masonic Hall to the west.

An aerial photo of the subject site is provided in Figure 1.

#### Figure 1: View of the Subject Lands



# 2.0 PROPOSED DEVELOPMENT

The proposed development will include 76 apartment units and two retail units with a combined gross floor area (GFA) of approximately 3,450ft<sup>2</sup>. The proposed development will include an underground parking garage containing 65 vehicle parking spaces and 45 bicycle parking spaces.

A preliminary review of various access options was conducted, including access to Churchill Avenue, Byron Avenue, Byron Place and Highcroft Avenue. Access to Highcroft Avenue is recommended based on the following factors.

- Access along Byron Avenue and Churchill Avenue requires a minimum corner clearance of 55m from the Churchill Avenue/Byron Avenue intersection, and is unachievable.
- OC Transpo bus stop and school bus loading zone are located along the west side of Churchill Avenue and create additional conflict opposite the site.
- The northbound left turn lane and taper at the Churchill Avenue/Byron Avenue extend past the site.
- The raised northbound cycle track transitions to on-road shared travel lane across the Churchill Avenue frontage.
- Westbound and northbound queues at the Churchill Avenue/Byron Avenue intersection periodically extend to Highcroft Avenue and Ravenhill Avenue during weekday peak hours.
- The non-standard intersection configuration/traffic calming feature at Byron Place/Highcroft Avenue/Byron Avenue currently operates with low-volume on Byron Place.
- The City of Ottawa's Private Approach By-law requires the access to be located on the lower class of roadway where possible.

Site access to Highcroft Avenue will provide access to the full movement intersection at Byron Avenue. A 16 unit residential development was recently constructed at 450 Churchill Avenue. All movement access to this development is provided along Churchill Avenue south of Ravenhill Avenue. This driveway serves a smaller development compared to the proposed development, is not located in close proximity to a signalized intersection with turn lanes and tapers, and is not located across from a transit stop/school bus loading zone. The access provided for 450 Churchill Avenue development is not comparable to the proposed development.

The existing curb extension restricting the southbound through movement along Highcroft Avenue south of Byron Place will be moved to south of the proposed access to the parking garage for the proposed development. This will eliminate any opportunity for vehicles leaving the site to turn right to travel southbound on Highcroft Avenue.

The proposed development will be constructed in one phase, with an estimated completion date of 2020. The proposed Site Plan is included in **Appendix A**.

# 3.0 SCREENING AND SCOPING

# 3.1 Screening Form

The City's 2017 TIA Guidelines identify three triggers for completing a TIA report, including trip generation, location, and safety. The criteria for each trigger are outlined in the City's TIA Screening Form. A copy of the TIA Screening Form is included in **Appendix B**.

Based on the TIA Screening Form, the subject application satisfies the trip generation and safety triggers for completing a TIA.

# 3.2 Existing Conditions

#### 3.2.1 Roadways

All roadways within the study area fall under the jurisdiction of the City of Ottawa.

Byron Avenue is a collector roadway that generally runs on an east-west alignment in the vicinity of the subject site. It has a two-lane undivided urban cross section with a regulatory speed limit of 50km/hr.

Richmond Road is an arterial roadway that generally runs on an east-west alignment in the vicinity of the subject site. It has a two-lane undivided urban cross section with a regulatory speed limit of 50km/h. Richmond Road is a full load truck route.

Churchill Avenue runs on a north-south alignment and is classified as a major collector roadway between Richmond Road and Carling Avenue, and an arterial roadway between Richmond Road and Scott Street. It has a two-lane undivided urban cross section with a posted speed limit of 50 km/hr. Churchill Avenue is a full load truck route.

Highroft Avenue is local roadway that runs on a north-south alignment. It has a two-lane undivided urban cross section and a regulatory speed limit of 50 km/hr.

Byron Place is a local roadway that runs on an east-west alignment parallel to Byron Avenue, commencing at Highcroft avenue and terminating approximately 60m to the west. It has a two-lane undivided urban cross section with a sidewalk on the south side.

#### 3.2.2 Intersections

A review of the existing lane configurations and traffic control at the study area intersections is provided below.

#### Churchill Avenue/Byron Avenue

- Signalized intersection
- Northbound/Southbound: one left turn lane and one shared through/right turn lane
- Eastbound/Westbound: one approach lane
- Ladder striped crosswalks are provided on all four legs
- A bike lane is provided on the west leg



# Richmond Road/Churchill Avenue

- Signalized intersection
- Northbound/Southbound: one approach lane
- Eastbound/Westbound: one left turn lane and one shared through/right turn lane
- Ladder striped crosswalks are provided on all four legs





#### Unsignalized intersection, stop control on Highcroft Avenue

Byron Avenue/Highcroft Avenue/Byron Place

- One approach lane on all legs
- Northbound right turn movement is channelized
- A concrete island and curb extension restrict the southbound through movement along Highcroft Avenue south of Byron Place

# 3.2.3 Driveways

In accordance with the City's 2017 TIA guidelines, a review of adjacent driveways along the boundary roads are provided as follows:

#### Highcroft Avenue, East Side:

 Residential driveways to 462 Byron Avenue, and 441 and 445 Highcroft Avenue

#### Highcroft Avenue, West Side:

• Residential driveways to 440 and 444 Highcroft Avenue

# Churchill Avenue, East Side:

• Driveways to commercial developments at 439 and 445 Churchill Avenue

#### **Churchill Avenue, West Side:**

- Driveway to Westboro Masonic Hall
- Driveway to Churchill Alternative School

The subject lands (435 Churchill Avenue) share a driveway with the development to the south (439 Churchill Avenue). There is no easement registered between the properties. The property at 439 Churchill Avenue also shares a driveway with 445 Churchill Avenue. Access to 439 Churchill Avenue can be maintained adjacent to the proposed building.

# 3.2.4 Pedestrian and Bicycle Facilities

Sidewalks are provided on both sides of Churchill Avenue, Richmond Road and Byron Avenue between Churchill Avenue and Eden Avenue. Byron Avenue, east of Eden Avenue and west of Churchill Avenue has a sidewalk on the south side of the roadway.

Churchill Avenue and Richmond Road are classified as spine cycling routes, and Byron Avenue is classified as a local cycling route in the City's Ultimate Cycling Network. A bike lane is provided on the south side of Byron Avenue west of Churchill Avenue. Cycle tracks are provided along Churchill Avenue south of Byron Avenue.

#### 3.2.5 Transit

The locations of all OC Transpo bus stops within a 400m walking distance, or approximately a 5 minute walk, of the subject site are described as follows:

- #7538 and #7539 are located along Churchill Avenue south of Byron Avenue;
- #4987 and #5616 are located along Churchill Avenue north of Richmond Road;
- #4864 and #4865 are located along Richmond Road between Eden Avenue and Edgewood Avenue; and
- #4876 is located along Richmond Avenue west of Churchill Avenue.

The location of the bus stops is shown in **Figure 2**.



Figure 2: OC Transpo Bus Stop Locations

The aforementioned bus stops serve OC Transpo Route 11, Route 50 and Route 151. Descriptions of the foregoing transit routes are provided in the following table. Route maps are included in **Appendix C**.

Route	Description	Schedule				
Roule	Description	Days	Service	Headways		
11	Travels between Lincoln Fields Transit Station and Parliament Transit Station	7 Days/ Week	All Day	<u>Weekday/Saturday</u> Morning/Afternoon: 15 min Night: 30 min		
50	Travels between Lincoln Fields Transit Station and Tunney's Pasture Transit Station	Monday to Saturday	All Day	<u>Weekday</u> AM/PM Peak: 15 min Mid-Day/Night: 30 min <u>Saturday</u> Morning/Afternoon: 30 min Night: 60 min		
151	Travels between Carlingwood Shopping Centre and Tunney's Pasture Transit Station	Monday to Friday	Selected Time Periods	Once in the morning and evening, twice in the afternoon		

# 3.2.6 Existing Area Traffic Management Measures

A concrete island and curb extension restrict the southbound through movement along Highcroft Avenue south of Byron Place. There are currently no other area traffic management measures in place along any of the study area roadways.

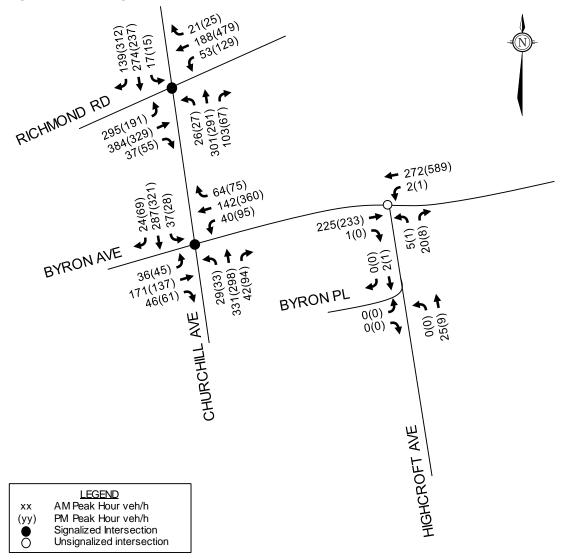
# 3.2.7 Existing Traffic Volumes

Weekday traffic counts were completed by the City of Ottawa and Novatech at the study area intersections on the following dates:

- Churchill Avenue/Richmond Road
   Novemb
- Churchill Avenue/Byron Avenue
- Byron Avenue/Highcroft Avenue/Byron Place

November 22<sup>nd</sup>, 2017 (City) August 25<sup>th</sup>, 2016 (City) October 3<sup>rd</sup>, 2018 (Novatech)

Due to seasonal variation, the traffic volumes between the Churchill Avenue/Byron Avenue and Byron Avenue/Highcroft Avnue/Byron Place have been balanced to within 10%. The existing traffic volumes at these intersections during the weekday AM and PM peak hours are shown in **Figure 3**. Peak hour summary sheets of the aforementioned traffic count are included in **Appendix D**.



# Figure 3: Existing Traffic Volumes

It is noteworthy that a total of nine illegal southbound through movements were recorded along Highcroft Avenue over the eight-hour period. As shown in Figure 3, two illegal southbound movements were recorded during the AM peak hour and one was recorded during the weekday PM peak hour.

#### 3.2.8 Collision Records

Historical collision data from the last five years was obtained from the City's Public Works and Service Department for the study area intersections. Copies of the collision summary reports are included in **Appendix E**. The following table summarizes the reported collisions at each intersection within the last five years.

	Number of Collision							
Intersection	Single Vehicle	Rear-End	Angle	Turning Movement	Sideswipe	Total		
Churchill Avenue/ Richmond Road	8	10	7	2	6	33		
Churchill Avenue/ Byron Avenue	1	3	2	2	0	8		
Byron Avenue/ Highcroft Avenue	1	0	0	0	0	1		

# **Table 2: Historical Collision Records**

# Churchill Avenue/Richmond Road

A total of 33 collisions were reported at the Churchill Avenue/Richmond Road intersection over the last five years. Personal injuries were incurred from six of the 33 collisions.

Ten of the total collisions were rear-end impacts, of which five involved eastbound vehicles, three involved southbound vehicles and two involved westbound vehicles. Forty percent of the rear-end impacts occurred under wet or icy surface conditions, suggesting environmental factors played a role in the rear-end collision history at this intersection.

Seven of the total collisions were angle impacts, of which five involved a southbound and an eastbound vehicle, one involved a northbound and an eastbound vehicle and one involved a northbound and a westbound vehicle. The setback of the existing building in the northwest quadrant of the Churchill Avenue/Richmond Road intersection (337 Richmond Road) from the right-of-way (ROW) is minimal. This building limits the sight distance between the southbound and eastbound approaches, and is anticipated to be a contributing factor for the number of angle impacts between these approaches.

Eight of the total collisions were single vehicle impacts, four of which involved a turning vehicle and a pedestrian and four involved an unattended vehicle. Ladder striped crosswalks are currently provided on all legs of this intersection to enhance visibility of the crosswalk and increase drivers' awareness of potential conflicts. Three of the impacts involving an unattended vehicle occurred on the north and south approaches (the fourth was unknown).

#### Churchill Avenue/Byron Avenue

A total of eight collisions were reported at the Churchill Avenue/Byron Avenue intersection over the last five years. Personal injuries were not incurred from any of the reported collisions at this intersection. There was no pattern associated with the collision history at this intersection.

#### Byron Avenue/Highcroft Avenue

The only collision reported at the Byron Avenue/Highcroft Avenue intersection over the last five years was a single vehicle impact with a building/wall on the northbound approach.

#### 3.3 Planned Conditions

The 2031 Rapid Transit and Transit Priority Network in the City of Ottawa's 2013 Transportation Master Plan (TMP) identifies the implementation of transit signal priority measures and queue jump lanes at select intersections along Richmond Road, Wellington Street West and Somerset Street

between Woodroffe Avenue and Bank Street. The City's 2013 TMP does not identify any roadway projects within the study area.

A Transportation Brief was submitted to the City of Ottawa dated May 2016, and updated in March and July 2017 in support of a Site Plan Control application for a 187 unit apartment building at 190 Richmond Road.

#### 3.4 Study Area and Time Periods

This report will review the design elements along Churchill Avenue and Byron Avenue boundary streets. The proposed study area for this report includes all accesses to the proposed development and the following intersections:

- Churchill Avenue/Richmond Road
- Churchill Avenue/Byron Avenue
- Byron Avenue/Highcroft Avenue

The selected time periods for the analysis are the weekday AM and PM peak hours, which represent the 'worst case' combination of site generated traffic and adjacent street traffic. Analysis will be completed for the 2020 build-out year and the 2025 horizon year.

#### 3.5 Exemptions Review

This module reviews possible exemptions from the final TIA, as outlined in the TIA Guidelines. The applicable exemptions for the subject lands are shown in **Table 3**.

Module	Element	ent Exemption Criteria					
Design Review Component							
4.1	<i>4.1.2</i> Circulation and Access	<ul> <li>Only required for site plans</li> </ul>	No				
Development Design	<i>4.1.3</i> New Street Networks	<ul> <li>Only required for plans of subdivision</li> </ul>	Yes				
4.2	<i>4.2.1</i> Parking Supply	Only required for site plans	No				
Parking	<i>4.2.2</i> Spillover Parking	<ul> <li>Only required for site plans where parking supply is 15% below unconstrained demand</li> </ul>	Yes				
Network Impact	Component						
<b>4.5</b> Transportation Demand Management	All elements	<ul> <li>Not required for non-residential site plans expected to have fewer than 60 employees and/or students on location at any given time</li> </ul>	No				
<b>4.6</b> Neighbourhood Traffic Management	<i>4.6.1</i> Adjacent Neighbourhoods	<ul> <li>Only required when the development relies on local or collector streets for access and total volumes exceed ATM capacity thresholds</li> </ul>	Yes				

#### **Table 3: TIA Exemptions**

Module	Element	Exemption Criteria	Exemption Applies
<b>4.8</b> Network Concept	All elements	<ul> <li>Only required when proposed development generates more than 200 person-trips during the peak hour in excess of the equivalent volume permitted by the established zoning</li> </ul>	Yes

Churchill Avenue is classified as a major collector roadway south of Richmond Road, and Byron Avenue is classified as a collector roadway. Based on the existing traffic volumes at the Churchill Avenue/Byron Avenue intersection, as presented in Figure 3, the following traffic volumes are currently using these roadways:

- Churchill Avenue south of Byron Avenue:
  - AM Peak: approximately 775 vehicles two-way (400 northbound, 375 southbound)
  - PM Peak: approximately 900 vehicles two-way (425 northbound, 475 southbound)
- Byron Avenue east of Churchill Avenue:
  - AM Peak: approximately 450 vehicles two-way (255 eastbound, 195 westbound)
  - PM Peak: approximately 705 vehicles two-way (245 eastbound, 460 westbound)

The lane capacity along Churchill Avenue and Byron Avenue are estimated at 600 vehicles per hour per lane (vphpl) based on the City's Long Range Transportation Model. Based on the foregoing the peak directional traffic along Churchill Avenue is operating with a volume to capacity ratio of 0.67 and 0.79 during the weekday AM and PM peak hours respectively. The peak directional traffic along Byron Avenue is operating with a volume to capacity ratio of 0.43 and 0.77 during the weekday AM and PM peak hours respectively. Total traffic, including the additional traffic generated by the proposed development, as presented in Figure 4 below, is not anticipated to increase above the peak directional capacity of 600vphpl. As such, the Neighbourhood Traffic Management module is exempt from the required analysis in the TIA.

#### 4.0 FORECASTING

#### 4.1 Development-Generated Traffic

#### 4.1.1 Trip Generation

Trips generated by the proposed residential development were estimated using the TRANS Trip Generation Manual prepared in 2009. Trips generated by the residential development were calculated using the recommended trip generation rates for mid-rise apartments (3-10 floors) in the urban area (inside the greenbelt), as presented in Table 3.18 of the TRANS report. The directional splits are based on the blended splits presented in Table 3.17 of the report.

Trips generated by the proposed retail development have been developed based on the Shopping Centre (Land Use 820) in the Institute of Transportation Engineers (ITE) Trip Generation Manual, 10<sup>th</sup> Edition.

The following table presents the trip generation for the proposed development.

Land Use	Units		AM Peak	۲.	PM Peak		
	/ GFA	IN	OUT	тот	IN	OUT	ТОТ
Mid-rise Apartment (3-10 Floors)	76	5	14	19	14	9	23
Shopping Centre	3,450 s.f.	2	1	3	7	6	13

# Table 4: TRANS/ITE Trip Generation

The residential trip generation was converted to person trips using the Apartment modal shares presented in Table 3.13 of the TRANS report. Trips generated by the Shopping Centre land use were converted to person trips using a 1.28 ITE trip to person trip adjustment factor. The person trips generated by the proposed development are summarized in the following table.

# Table 5: Person Trip Generation

Land Use		AM Peak		PM Peak			
Land Ose	IN	OUT	ТОТ	IN	OUT	ТОТ	
Mid-rise Apartment (3-10 Floors)	12	39	51	36	22	58	
Shopping Centre	2	2	4	8	9	17	

The modal shares for the proposed development are anticipated to be consistent with the modal shares outlined in the 2011 TRANS O-D Survey Report, specific to the Ottawa West Area. The modal shares applied to the residential development have been derived based on all observed trips within the Ottawa West Area, as well as trips departing during the AM peak and arriving during the PM peak. The modal shares applied to the commercial development have been derived based on all observed trips within the Ottawa West Area applied to the commercial development have been derived based on all observed trips within the Ottawa West Area during the weekday AM and PM peak hours. A full breakdown of the projected person trips by modal share are shown in the below table.

#### Table 6: Person Trips by Modal Share

Travel Mode	Modal		AM Peak			PM Peak			
Traver mode	Share	IN	OUT	тот	IN	OUT	ТОТ		
Residential Per	son Trips	12	39	51	36	22	58		
Auto Driver	45%	6	17	23	16	10	26		
Auto Passenger	15%	2	6	8	5	3	8		
Transit	20%	2	8	10	7	5	12		
Non-Auto	20%	2	8	10	8	4	12		
Commercial Per	son Trips	2	2	4	8	9	17		
Auto Driver	35%	1	1	2	3	3	6		
Auto Passenger	15%	0	0	0	1	2	3		
Transit	5%	0	0	0	0	0	0		
Non-Auto	45%	1	1	2	4	4	8		
Auto Driver (T	otal)	7	18	25	19	13	32		
Auto Passenger	(Total)	2	6	8	6	5	11		
Transit (Tota	al)	2	8	10	7	5	12		
Non-Auto (To	otal)	3	9	12	12	8	20		

# 4.1.2 Trip Distribution

The assumed distribution of trips generated by the proposed development has been derived from existing traffic patterns on the roadways within the study area. As the proposed development is predominantly residential, the majority of peak hour trips are anticipated to be to/from work. It is appropriate for the assumed trip distribution to be based on the distribution of existing traffic volumes exiting the study area during the AM peak hour and arriving to the study area during the PM peak hour. The projected distribution of trips is summarized as follows:

- 35% to/from the east via either Byron Avenue or Richmond Road
- 30% to/from the west via either Byron Avenue or Richmond Road
- 20% to/from the north via Churchill Avenue
- 15% to/from the south via Churchill Avenue

Site generated traffic volumes are shown in Figure 4.

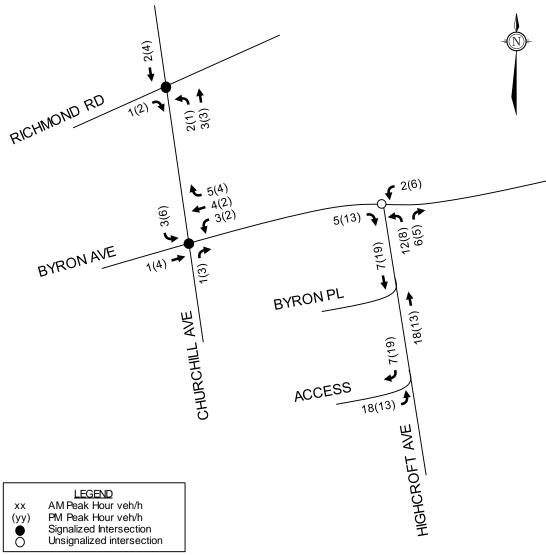


Figure 4: Site Generated Traffic

#### 4.2 Background Traffic

#### 4.2.1 General Background Growth Rate

A review of historic traffic counts, as well as snapshots from the City's Long Range Transportation Model were reviewed to determine an appropriate background growth rate along the study area roadways.

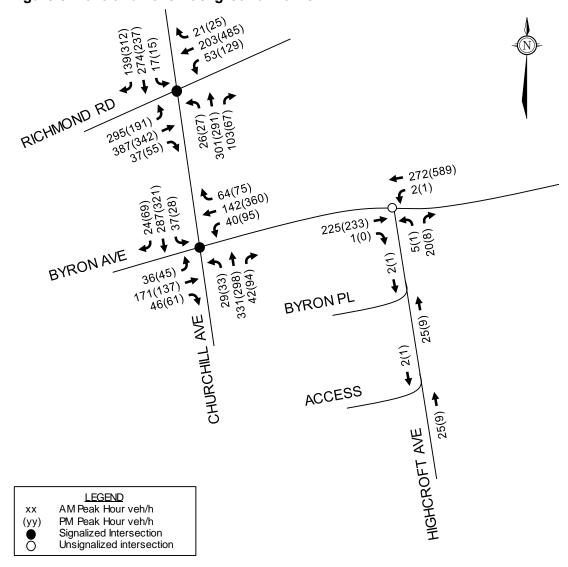
Based on the historic traffic counts (2012 and 2016 at Churchill Avenue/Byron Avenue, 2015 and 2017 at Churchill Avenue/Richmond Road), traffic volumes have generally decreased along the study area roadways. This is consistent with the 2031 and 2011 snapshots from the City's long range transportation model, which suggests no growth along the area roadways.

Based on the foregoing, no growth rate has been applied to the existing traffic volumes within the study area.

#### 4.2.2 Other Area Development Traffic

As identified above, a Transportation Brief was submitted to the City of Ottawa dated May 2016, and updated in March and July 2017 in support of a Site Plan Control application for a 187 unit apartment building at 190 Richmond Road. Traffic generated by this development has been added to the through traffic volumes along Richmond Road in the future background traffic projections. Relevant excerpts from the 190 Richmond Road report are included in **Appendix F**.

As no annual growth rate has been applied to the existing traffic volumes, the 2020 and 2025 background traffic volumes are anticipated to be the same. The background traffic volumes along the study area roadways are shown in **Figure 5**. Total traffic volumes for the 2020 and 2025 build-out years are shown in **Figure 6**.



# Figure 5: 2020 and 2025 Background Traffic

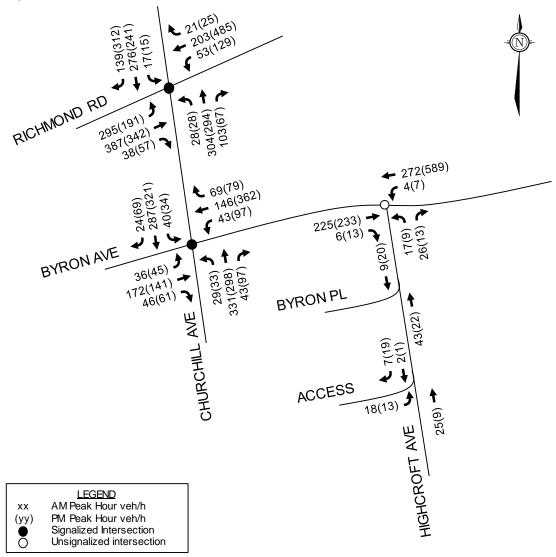


Figure 6: 2020 and 2025 Total Traffic

# 5.0 ANALYSIS

#### 5.1 Development Design

Byron Place will be partially closed to provide a public park area in the southeast corner of the Churchill Avenue/Byron Avenue intersection. The portion of Byron Place that remains open will function as the fire route for the proposed development. The width of Byron Place will be reduced to 6.0m in order to provide a wider landscape buffer between the proposed building and sidewalk. The turnaround hammerhead within Byron Place will also facilitate short-term parking for drop-offs/pick-ups.

A Road Modification Approval (RMA) application is required to alter the existing curbs along Highcroft Avenue and Byron Place. A functional design of the proposed Highcroft Avenue and Byron Place roadway modifications is provided in **Appendix G**. A RMA application will be submitted to the City of Ottawa under a separate cover.

On-site pedestrian facilities will be provided between the retail entrances and the sidewalks along Churchill Avenue. A sidewalk will be provided along the northern frontage of the site along Byron Place, providing pedestrian connectivity between the residential entrances and the existing sidewalk along Churchill Avenue.

Bicycle parking for the proposed development will be in accordance with the minimum requirements of the City's Zoning By-law (ZBL), as described in Section 5.2. Bicycle parking will be provided underground.

A garbage room will be provided within the building. Garbage bins will be wheeled down the pathway south of the building, and will be picked up curbside along Churchill Avenue.

A review of the Transportation Demand Management (TDM) – *Supportive Development Design and Infrastructure Checklist* has been conducted. A copy of the TDM checklist is included in **Appendix H**. All required TDM-supportive design and infrastructure measures in the TDM checklist are met.

#### 5.2 Parking

The subject site is located in Area B on Schedule 1 and Area X on Schedule 1A of the City's Zoning By-law (ZBL). Minimum vehicular and bicycle parking rates for the proposed development are identified in the ZBL and are summarized in the following table.

Land	Use	Rate	Units/GFA	Required	Provided
Vehicle Par	king				
Apartmont	Tenant	0.5 per dwelling unit in excess of 12 units	76 Units	32	57
Apartment	Visitor 0.1 per dwelling unit in excess of 12 units		70 Onits	6	7
Commercia	al Unit One	None for commercial units	158m <sup>2</sup>	0	0
Commercia	al Unit Two	with 200m <sup>2</sup> GFA or less	197m <sup>2</sup>	0	0
			Total 38		65
Bicycle Parl	king				
Apart	ment	0.5 per Dwelling Unit	76 Units	38	46
Commercia	al Unit One	$1 \text{ max} 250 \text{ m}^2 \text{ of } 050$	161m <sup>2</sup>	0	0
Commercia	al Unit Two	1 per 250m <sup>2</sup> of GFA	160m <sup>2</sup>	0	0
			Total	38	46

#### Table 7: Parking Requirement

Based on the foregoing table, the proposed vehicular and bicycle parking meets the minimum requirement of the ZBL.

# 5.3 Boundary Streets

This section provides a review of the boundary streets using complete streets principles. The Multi-Modal Level of Service (MMLOS) guidelines produced by IBI Group in 2015 were used to evaluate the Level of Service (LOS) of the boundary roadways for each mode of transportation. Schedule B of the City of Ottawa's Official Plan indicates Churchill Avenue and Byron Avenue are within the General Urban Area. The boundary streets are also located within 600m of the Dominion Transit Station and within 300m of the Churchill Alternative School. Photos of the boundary Streets (provided by Google Streetview) are provided below.

#### Figure 7: Churchill Avenue



# Figure 8: Byron Avenue



Target Pedestrian LOS (PLOS), Bicycle LOS (BLOS), Transit LOS (TLOS), Truck LOS (TkLOS) and Auto LOS for the study area roadways are based on the targets within 300m of a school, as identified in Exhibit 22 of the MMLOS guidelines. The following table summarizes the findings of the MMLOS segment analysis. Detailed segment MMLOS calculations are included in **Appendix I**.

Segment	PLOS	BLOS	TLOS	TkLOS	Auto LOS
Churchill Avenue	С	А	F	В	С
Target	Α	В	-	D	E
Byron Avenue	E	F	D	В	В
Target	A	В	-	-	E

#### Table 8: Segment MMLOS Summary

Churchill Avenue currently meets the target BLOS B, TkLOS D and Auto LOS E, however it does not meet the target PLOS A. Byron Avenue meets the target Auto LOS E, however it does not meet the target PLOS A and BLOS B.

Based on an operating speed of 60km/hr and average daily curb lane traffic greater than 3000 vehicles per day, the target PLOS A is unachievable. To achieve the target PLOS A along both Churchill Avenue and Byron Avenue, either a reduction in the daily curb traffic to less than 3000 vehicles per day or a reduction in the operational speed to 30km/hr is required.

A review of the Ontario Traffic Manual (OTM) Book 18 Desirable Cycling Facility Pre-Selection Nomograph suggests either bike lanes or a separated facility are appropriate along Byron Avenue based on the AADT and operating speed. A copy of the OTM Desirable Cycling Facility Pre-Selection Nomograph is included in **Appendix I**. The City of Ottawa's Ultimate Cycling Network identifies a future separated major pathway on the north side of Byron Avenue. The separated pathway will result in a BLOS A, achieving the target within 300m of a school.

# 5.4 Access Intersections Design

As described Section 2.0, a preliminary review of various access options was conducted, including access to Churchill Avenue, Byron Avenue, Byron Place and Highcroft Avenue. Access to Highcroft Avenue is recommended based on the following factors.

- Access along Byron Avenue and Churchill Avenue requires a minimum corner clearance of 55m from the Churchill Avenue/Byron Avenue intersection, and is unachievable.
- OC Transpo bus stop and school bus loading zone are located along the west side of Churchill Avenue and create additional conflict opposite the site.
- The northbound left turn lane and taper at the Churchill Avenue/Byron Avenue extend past the site.
- The raised northbound cycle track transitions to on-road shared travel lane across the Churchill Avenue frontage.
- Westbound and northbound queues at the Churchill Avenue/Byron Avenue intersection periodically extend to Highcroft Avenue and Ravenhill Avenue during weekday peak hours.
- The non-standard intersection configuration/traffic calming feature at Byron Place/Highcroft Avenue/Byron Avenue currently operates with low-volume on Byron Place.
- The City of Ottawa's Private Approach By-law requires the access to be located on the lower class of roadway where possible.

Site access to Highcroft Avenue will provide access to the full movement intersection at Byron Avenue. The existing curb extension restricting the southbound through movement along Highcroft Avenue south of Byron Place will be removed and a new island will be provided to south of the proposed access to the parking garage for the proposed development.

The proposed island will be 3m in width and 8.7m in length. The island will be offset 1.5m from the existing curb-line to permit southbound cyclists along Highbury Park Drive. The width and length of the proposed island will eliminate any opportunity for vehicles leaving the site to turn right to travel southbound on Highcroft Avenue, as well as restrict westbound vehicles along Byron Avenue to cut through the residential community to avoid the traffic light at Churchill Avenue/Byron Avenue. The proposed island width (4.5m included curb offset) is an improvement on the existing curb extension which narrows the roadway by approximately 2.4m.

A RMA application is required to alter the existing curbs along Highcroft Avenue and Byron Place. A functional design of the proposed Highcroft Avenue and Byron Place roadway modifications is provided in **Appendix G**. A RMA application will be submitted to the City of Ottawa under a separate cover.

The proposed access will be 6.2m in width and will be located approximately 6.7m from the southern property line, and approximately 19m from the Byron Place right-of-way limit. The City's ZBL identifies a minimum width of 6.0m for a driveway leading to a parking garage containing more than 20 parking spaces. The City's Private Approach By-law requires the access to be 3.0m from any adjacent property line. The Private Approach By-law also identifies where a property abuts or is within 46m of an arterial or major collector roadway, in the case of an apartment building containing 20 to 99 parking spaces, a minimum distance of 18m is required between the proposed access and the nearest intersecting street line. The proposed driveway width and location adheres to the requirements of the City's ZBL and Private Approach By-law.

A review of the intersection operations at the proposed access was conducted for the 2020 and 2025 total traffic conditions was conducted. Based on the analysis, the proposed access will operate with a LOS A under the 2020 and 2025 total traffic conditions. Detailed summary sheets of the Synchro analysis are provided in **Appendix J**.

# 5.5 Transportation Demand Management

A review of the Transportation Demand Management (TDM) Measures checklist was conducted and can be found in **Appendix H**. To encourage travel by sustainable modes, the proponent agrees to implement the following TDM measures from the checklist:

- Display local area maps with walking/cycling access routes and key destinations at major entrances;
- Display relevant transit schedules and route maps at entrances;
- unbundle parking cost from monthly rent; and
- provide multimodal travel option information package to new residents.

#### 5.6 Transit

Relevant boarding/alighting information and bus occupancy information from the winter 2018 service period for OC Transpo bus stops #4876, #4987, #5616, #7538 and #7539 were received from OC Transpo. OC Transpo has also advised that both Route 11 and Route 50 operate with 40-foot buses during the weekday AM and PM peak periods. Information received from OC Transpo is included in **Appendix C**. The following table summarizes the transit information received from OC Transpo.

OC	OC		AM Period			PM Period	
Transpo Stop	Transpo Route	Total Boarding	Total Alighting	Average Load	Total Boarding	Total Alighting	Average Load
#4876	11	1	15	8	13	19	17
#4987	11	5	19	17	18	7	14
#4907	50	1	4	15	3	1	10
#5616	50	0	0	9	5	3	12
#7538	50	6	5	15	1	1	10
#7539	50	0	2	9	2	5	11

#### Table 9: Existing OC Transpo Utilization

Based on the trip generation presented in Section 4.1, the proposed development is anticipated to generate 10 transit trips (2 in, 8 out) during the AM peak hour and 11 transit trips (7 in, 4 out) during the PM peak hour. Based on the transit utilization data received from OC Transpo, the existing bus stops/routes in the vicinity of the subject site have capacity to accommodate the transit trips generated by the proposed development.

#### 5.7 Intersection Design

#### 5.7.1 Existing Intersection MMLOS Analysis

This section provides a review of the signalized study area intersections using complete streets principles. The MMLOS guidelines produced by IBI Group in October 2015 were used to evaluate the LOS of the signalized study area intersections for each mode of transportation. Schedule B of

the City of Ottawa's Official Plan indicates the study area intersections are located in the General Urban Area. The study area intersections are also located within 600m of the Dominion Transit Station and within 300m of the Churchill Alternative School. Aerial photos of the study area intersections are provided in Section 3.2.2.

Target PLOS, BLOS, TLOS, TkLOS and Auto LOS for the study area intersections are based on the targets within 300m of a school, as identified in Exhibit 22 of the MMLOS guidelines. The following table summarizes the findings of the MMLOS intersection analysis.

A site visit was conducted to review the intersection operations. Although the north and south approaches to the Churchill Avenue/Richmond Road intersection are painted as one approach lane, the wide lane widths permit drivers to travel around a queued vehicle. For the purposes of the intersection capacity analysis, the north and south approaches to this intersection have been modeled as a through/left turn lane and a right turn lane.

Detailed intersection MMLOS calculations are included in Appendix K.

Intersection	PLOS	BLOS	TLOS	TkLOS	Auto LOS
Churchill Avenue/ Byron Avenue	D	F	С	F	E
Target	А	В	-	D	E
Churchill Avenue/ Richmond Road	D	F	F	Е	E
Target	Α	В	-	D	E

#### Table 10: Intersection MMLOS Summary

The Churchill Avenue/Byron Avenue and Churchill Avenue/Richmond Road intersections do not meet the target PLOS A, BLOS B, TkLOS D, however they do meet the target Auto LOS E.

A reduction in the pedestrian crossing distance would have the greatest improvement to the PLOS at these intersections. To achieve the target BLOS, consideration could be given by the City to reducing the operational speed to 50km/hr and providing a two-stage left turn bike box on all legs of these intersections. To achieve the target TkLOS, an effective turn radius greater than 15m is required on all four corners of these intersections. Increasing the turn radius at these intersections could be considered by the City, however it should be noted that the increased radius will have a negative impact on the PLOS.

# 5.7.2 2020 and 2025 Background Intersection Operations

Intersection capacity analysis has been completed for the 2020 and 2025 background traffic conditions. The intersection parameters used in the analysis are consistent with the TIA guidelines (saturation flow rate: 1800 vphpl, PHF: 1.0).

The results of the intersection capacity analysis are summarized in the following table. Detailed summary sheets are provided in **Appendix J**.

		AM Peak			PM Peak			
Intersection	Max V/C or Delay	LOS	Mvmt	Max V/C or Delay	LOS	Mvmt		
Churchill Avenue/ Byron Avenue	0.73	С	WBT	0.88	D	WBT		
Churchill Avenue/ Richmond Road	0.86	D	EBL	0.83	D	WBT/R		
Byron Avenue/ Highcroft Avenue	10 sec	В	NB	10 sec	В	NB		

#### Table 11: 2020 and 2025 Background Intersection Operations

Based on the foregoing, critical movements at all study area intersections are anticipated to operate with a LOS D or better under background traffic conditions. Based on the previous table, the background traffic conditions appear to improve when compared to the existing traffic conditions. This can be attributed to differences in the Peak Hour Factor (set to 0.90 in existing conditions and 1.0 in future conditions, as per the 2017 TIA Guidelines).

# 5.7.3 2020 and 2025 Total Intersection Operations

Intersection capacity analysis has been completed for the 2020 and 2025 total traffic conditions. The intersection parameters used in the analysis are consistent with the TIA guidelines (saturation flow rate: 1800 vphpl, PHF: 1.0).

The results of the intersection capacity analysis are summarized in the following table. Detailed summary sheets are provided in **Appendix J**.

	AM Peak			PM Peak			
Intersection	Max V/C or Delay	LOS	Mvmt	Max V/C or Delay	LOS	Mvmt	
Churchill Avenue/ Byron Avenue	0.76	С	WBT	0.89	D	WBT	
Churchill Avenue/ Richmond Road	0.86	D	EBL	0.83	D	WBT/R	
Byron Avenue/ Highcroft Avenue	11 sec	В	NB	13 sec	В	NB	

#### Table 12: 2020 and 2025 Total Intersection Operations

Based on the foregoing, the addition of site generated traffic volumes are not anticipated to have a significant impact to the intersection operations within the study area.

For the purposes of this analysis, it has been assumed that all trips from the south will travel north on Churchill Avenue to Byron Avenue, and turn right from Byron Avenue onto Highcroft Avenue. However it is acknowledged that some of these drivers may choose to turn right from Churchill Avenue onto Kenwood Avenue, and travel northbound on Highcroft Avenue to access the site.

Based on anticipated trip distribution, one vehicle during the AM peak hour and three vehicles during the PM peak hour are anticipated to arrive from the south. If these trips were to use Kenwood Avenue/Highcroft Avenue to access the site during the PM peak hour, this would result in one new

vehicle approximately every 20 minutes. This increase in traffic is not anticipated to be noticeable and will not have a significant impact on the existing operations along Highcroft Avenue.

# 6.0 CONCLUSIONS AND RECOMMENDATIONS

Based on the foregoing, the conclusions and recommendations of this TIA can be summarized as follows:

#### Development Design and Parking

- Byron Place will be partially closed to provide a public park area in the southeast corner of the Churchill Avenue/Byron Avenue intersection. The portion of Byron Place that remains open will function as the fire route for the proposed development. The width of Byron Place will be reduced to 6.0m in order to provide a wider landscape buffer between the proposed building and sidewalk. The turnaround hammerhead within Byron Place will also facilitate short-term parking for drop-offs/pick-ups.
- On-site pedestrian facilities will be provided between the retail entrances and the sidewalks along Churchill Avenue. A sidewalk will be provided along the northern frontage of the site along Byron Place, providing pedestrian connectivity between the residential entrances and the existing sidewalk along Churchill Avenue.
- A garbage room will be provided within the building. Garbage bins will be wheeled down the pathway south of the building, and will be picked up curbside along Churchill Avenue.
- All required TDM-supportive design and infrastructure measures in the TDM checklist are met.
- The proposed vehicular and bicycle parking meets the minimum requirement of the ZBL.

#### Boundary Streets

- Churchill Avenue currently meets the target BLOS B, TkLOS D and Auto LOS E, however it does not meet the target PLOS A. Byron Avenue meets the target Auto LOS E, however it does not meet the target PLOS A and BLOS B.
- To achieve the target PLOS A along both Churchill Avenue and Byron Avenue, either a reduction in the daily curb traffic to less than 3000 vehicles per day or a reduction in the operational speed to 30km/hr is required.
- The City of Ottawa's Ultimate Cycling Network identifies a future separated major pathway on the north side of Byron Avenue. The separated pathway will result in a BLOS A, achieving the target within 300m of a school.

#### Access Design

- Site access to Highcroft Avenue will provide access to the full movement intersection at Byron Avenue. The existing curb extension restricting the southbound through movement along Highcroft Avenue south of Byron Place will be removed and a new island will be provided to south of the proposed access to the parking garage for the proposed development.
- The width and length of the proposed island will eliminate any opportunity for vehicles leaving the site to turn right to travel southbound on Highcroft Avenue, as well as restrict westbound vehicles along Byron Avenue to cut through the residential community to avoid the traffic light at Churchill Avenue/Byron Avenue. The proposed island width (4.5m included curb offset) is an improvement on the existing curb extension which narrows the roadway by approximately 2.4m.
- The proposed driveway width and location adheres to the requirements of the City's ZBL and Private Approach By-law

# Transportation Demand Management and Transit

- To encourage travel by sustainable modes, the proponent agrees to implement the following TDM measures from the checklist:
  - Display local area maps with walking/cycling access routes and key destinations at major entrances;
  - o Display relevant transit schedules and route maps at entrances;
  - o unbundle parking cost from monthly rent; and
  - o provide multimodal travel option information package to new residents.

#### Intersection Design

- The Churchill Avenue/Byron Avenue and Churchill Avenue/Richmond Road intersections do not meet the target PLOS A, BLOS B, TkLOS D, however they do meet the target Auto LOS E.
- A reduction in the pedestrian crossing distance would have the greatest improvement to the PLOS at these intersections.
- To achieve the target BLOS, consideration could be given by the City to reducing the operational speed to 50km/hr and providing a two-stage left turn bike box on all legs of these intersections.
- To achieve the target TkLOS, an effective turn radius greater than 15m is required on all four corners of these intersections. Increasing the turn radius at these intersections could be considered by the City, however it should be noted that the increased radius will have a negative impact on the PLOS.
- Critical movements at all study area intersections are anticipated to operate with a LOS D or better under background traffic conditions.
- The addition of site generated traffic volumes are not anticipated to have a significant impact to the intersection operations within the study area.
- Based on anticipated trip distribution, one vehicle during the AM peak hour and three vehicles during the PM peak hour are anticipated to arrive from the south. If these trips were to use Kenwood Avenue/Highcroft Avenue to access the site during the PM peak hour, this would result in one new vehicle approximately every 20 minutes. This increase in traffic is not anticipated to be noticeable and will not have a significant impact on the existing operations along Highcroft Avenue.

#### NOVATECH

#### Prepared by:



Brad Byvelds, P. Eng. Project Coordinator | Transportation/Traffic

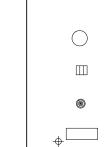
# **APPENDIX A**

Proposed Site Plan

#### LEGEND

PROPERTY LINE \_\_\_\_\_

ZONING SETBACKS ENTRANCE EXIT



(MH) MANHOLE, REFER TO CIVIL DRAWINGS (CB) CATCH BASIN, REFER TO CIVIL DRAWINGS (LS) LIGHT STANDARD

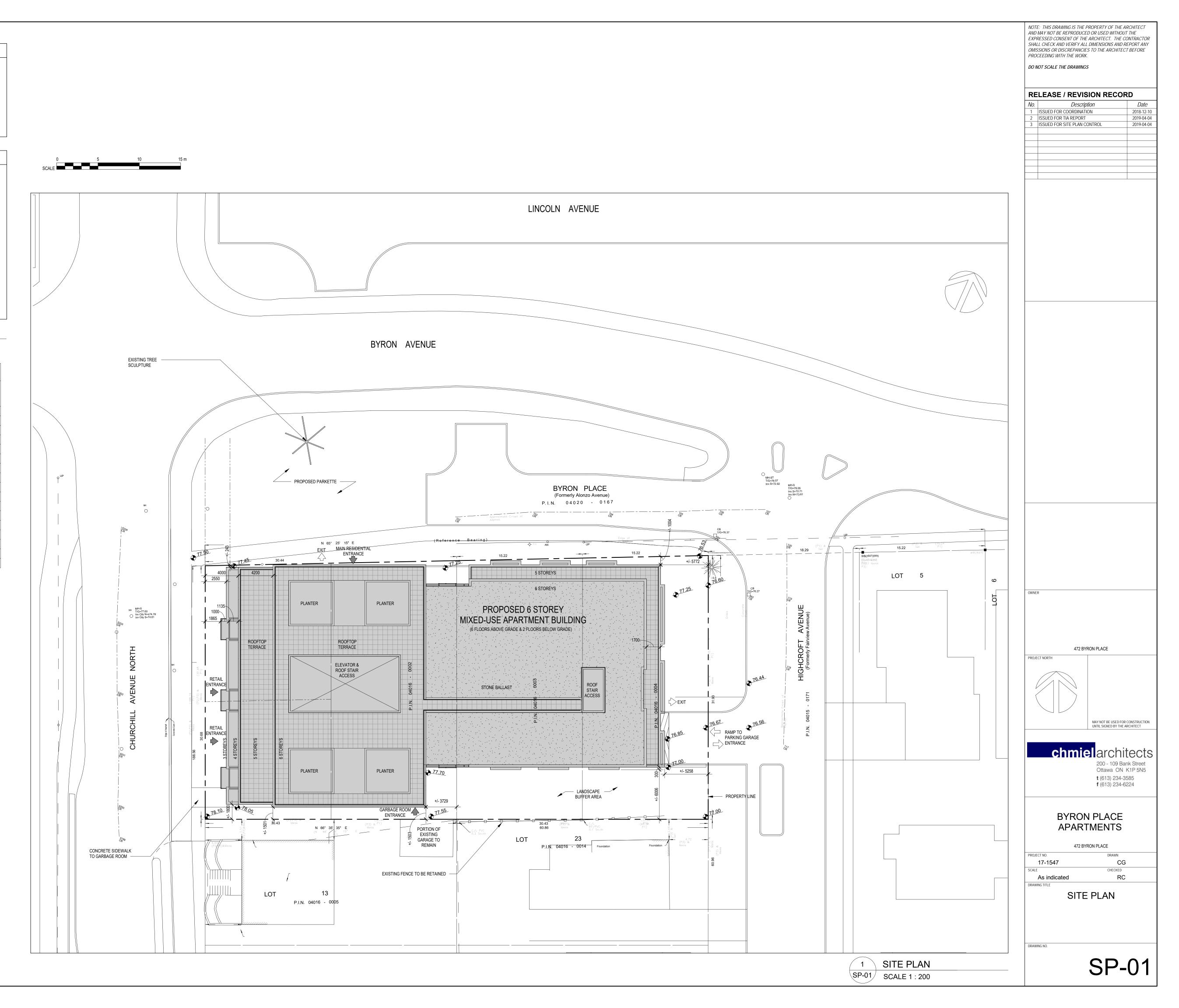
ELEVATION MARKER

PROJECT INFORMATION	PROJECT TEAN	1
LEGAL DESCRIPTION: LOTS 1,2,3 AND 4 REGISTERED PLAN 269 CITY OF OTTAWA	PROJECT NAME OWNER	BYRON & CHURCHILL APARTMENTS SIMON SI
PIN #: 04016-0001 04016-0002 04016-0003 04016-0004	ARCHITECT	CHMIEL ARCHITECTS 109 BANK STREET, SUITE 200, OTTAWA, ONTARIO, K1P 5N5
ZONING: R5B H(19.5) RESIDENTIAL UNITS: 81	PLANNER & CIVIL	NOVATECH 240 MICHAEL COWPLAND DR., SUITE 200, OTTAWA, ONTARIO, K2M 1P6
RETAIL UNITS: 2	MECHANICAL & ELECTRICAL	S+A ENGINEERS 1600 CARLING AVE, SUITE 530 OTTAWA, ONTARIO, K1Z 1G3
	SURVEYOR	FARLEY, SMITH & DENIS SURVEYING LTD 190 COLONNADE ROAD, SUITE 10 OTTAWA, ONTARIO, K2E 7J5
	LANDSCAPE	JAMES B. LENNOX & ASSOCIATES INC. 3332 CARLING AVE OTTAWA, ONTARIO K2H 5A8

PROJECT INFO SCALE1 : 125

City Area Calculations						
Level City G		Amenity City GLA Area		Communal Amenity Area		
	425.002	0.00 m2	0.00 m²	0.00 mm2		
LEVEL 1	435.00 m <sup>2</sup>	0.00 m²	0.00 m <sup>2</sup>	0.00 m <sup>2</sup>		
LEVEL 1	0.00 m <sup>2</sup>	320.92 m <sup>2</sup>	0.00 m <sup>2</sup>	0.00 m <sup>2</sup>		
LEVEL 1	0.00 m <sup>2</sup>	0.00 m <sup>2</sup>	49.13 m <sup>2</sup>	0.00 m <sup>2</sup>		
LEVEL 1	0.00 m <sup>2</sup>	0.00 m <sup>2</sup>	0.00 m <sup>2</sup>	52.39 m <sup>2</sup>		
LEVEL 2	1133.74 m <sup>2</sup>	0.00 m <sup>2</sup>	0.00 m <sup>2</sup>	0.00 m <sup>2</sup>		
LEVEL 2	0.00 m <sup>2</sup>	0.00 m <sup>2</sup>	51.77 m²	0.00 m <sup>2</sup>		
Level 3	1133.45 m <sup>2</sup>	0.00 m <sup>2</sup>	0.00 m <sup>2</sup>	0.00 m <sup>2</sup>		
Level 3	0.00 m <sup>2</sup>	0.00 m <sup>2</sup>	51.74 m²	0.00 m <sup>2</sup>		
LEVEL 4	1140.52 m <sup>2</sup>	0.00 m²	0.00 m <sup>2</sup>	0.00 m <sup>2</sup>		
LEVEL 4	0.00 m <sup>2</sup>	0.00 m <sup>2</sup>	49.17 m <sup>2</sup>	0.00 m <sup>2</sup>		
LEVEL 5	1029.69 m <sup>2</sup>	0.00 m²	0.00 m <sup>2</sup>	0.00 m <sup>2</sup>		
LEVEL 5	0.00 m <sup>2</sup>	0.00 m²	100.98 m <sup>2</sup>	0.00 m <sup>2</sup>		
LEVEL 6	918.89 m <sup>2</sup>	0.00 m <sup>2</sup>	0.00 m <sup>2</sup>	0.00 m <sup>2</sup>		
LEVEL 6	0.00 m <sup>2</sup>	0.00 m <sup>2</sup>	132.92 m <sup>2</sup>	0.00 m <sup>2</sup>		
ROOF	0.00 m <sup>2</sup>	0.00 m <sup>2</sup>	0.00 m <sup>2</sup>	441.04 m <sup>2</sup>		
Grand total: 59	5791.29 m <sup>2</sup>	320.92 m <sup>2</sup>	435.71 m <sup>2</sup>	493.43 m <sup>2</sup>		

Zoning Information:		
City of Ottawa Consolidate		
Proposed Mixed-Use		
Performance Standards	Required	Proposed
Residential Fifth Density (Sections 163-164)		
Minimum Lot Area (m²)	675	1900
Minimum Lot Width (m)	22.5	30.6
Minimum Front Yard (m)	3	2.5
Corner Side Yard Setback (m)	3	0
Minimum Interior Side Yard Setback (m) Abutting a Residential Zone If located within 21m of front lot line If located beyond 21m of front lot line	7.5 1.5 6.0	6.0 1.5 1.5 and 6.0
Minimum Rear Yard Setback (m)	3	5.7
Maximum Building Height (m)	Varies	19.5
Parking Requirements (Area X) (Section 100-114)		
Minimum Parking Space Rates		
Dwelling Units in a Mixed-use Building (in excess of 12)	0.5 per dwelling unit = 32	59
Non-Residential Uses (where gfa is less than 200 m <sup>2</sup> )	0	0
Minimum Visitor Parking Space Rate		
Dwelling Units in a Mixed-use Building (in excess of 12)	0.1 per dwelling unit = 6	6
Aisle and Driveway Provisions		
Minimum Width of a Double Traffic Lane (m)	6	6
Driveway providing access to Parking Garage (m)	6	6.2
Minimum Bicycle Parking Space Rates		
Dwelling Unit in a Mixed-use Building	0.5 per dwelling unit = 38	46
Amenity Area (Section 137)	1	1
Minimum Total Amenity Area (m <sup>2</sup> ) Mixed Use Building, with 9 or more Dwelling Units	6m <sup>2</sup> per dwelling unit = 456	929.1
Minimum Communal Amenity Area (m <sup>2</sup> ) Mixed Use Building, with 9 or more Dwelling Units	50% of the required total amenity area = 228	493.4



# **APPENDIX B**

TIA Screening Form



Transportation Impact Assessment Screening Form

### City of Ottawa 2017 TIA Guidelines Screening Form

1. Description of Proposed Development				
Municipal Address	433 and 435 Churchill Avenue North, 468 and 472 Byron Place			
Description of Location	The 0.19 ha parcel is four existing properties bound by Churchill Avenue North to west, Highcroft Avenue to the east and Byron Place to the north			
Land Use Classification	General Mixed Use			
Development Size (units)	84 residential units			
Development Size (m <sup>2</sup> )	325m <sup>2</sup> retail			
Number of Accesses and Locations	One proposed access on Highcroft Avenue			
Phase of Development	N/A			
Buildout Year	2019			

If available, please attach a sketch of the development or site plan to this form.

#### 2. Trip Generation Trigger

Considering the Development's Land Use type and Size (as filled out in the previous section), please refer to the Trip Generation Trigger checks below.

Land Use Type	Minimum Development Size		
Single-family homes	40 units		
Townhomes or apartments	90 units		
Office	3,500 m²		
Industrial	5,000 m <sup>2</sup>		
Fast-food restaurant or coffee shop	100 m²		
Destination retail	1,000 m²		
Gas station or convenience market	75 m <sup>2</sup>		

\* If the development has a land use type other than what is presented in the table above, estimates of person-trip generation may be made based on average trip generation characteristics represented in the current edition of the Institute of Transportation Engineers (ITE) Trip Generation Manual.

If the proposed development size is greater than the sizes identified above, <u>the Trip Generation Trigger is</u> <u>satisfied.</u>



#### Transportation Impact Assessment Screening Form

#### **3. Location Triggers**

	Yes	No
Does the development propose a new driveway to a boundary street that is designated as part of the City's Transit Priority, Rapid Transit or Spine Bicycle Networks?		Х
Is the development in a Design Priority Area (DPA) or Transit-oriented Development (TOD) zone?*		Х

\*DPA and TOD are identified in the City of Ottawa Official Plan (DPA in Section 2.5.1 and Schedules A and B; TOD in Annex 6). See Chapter 4 for a list of City of Ottawa Planning and Engineering documents that support the completion of TIA).

#### If any of the above questions were answered with 'Yes,' the Location Trigger is satisfied.

#### 4. Safety Triggers

	Yes	No
Are posted speed limits on a boundary street are 80 km/hr or greater?		Х
Are there any horizontal/vertical curvatures on a boundary street limits sight lines at a proposed driveway?		Х
Is the proposed driveway within the area of influence of an adjacent traffic signal or roundabout (i.e. within 300 m of intersection in rural conditions, or within 150 m of intersection in urban/ suburban conditions)?	х	
Is the proposed driveway within auxiliary lanes of an intersection?		Х
Does the proposed driveway make use of an existing median break that serves an existing site?		Х
Is there is a documented history of traffic operations or safety concerns on the boundary streets within 500 m of the development?		Х
Does the development include a drive-thru facility?		Х

If any of the above questions were answered with 'Yes,' the Safety Trigger is satisfied.

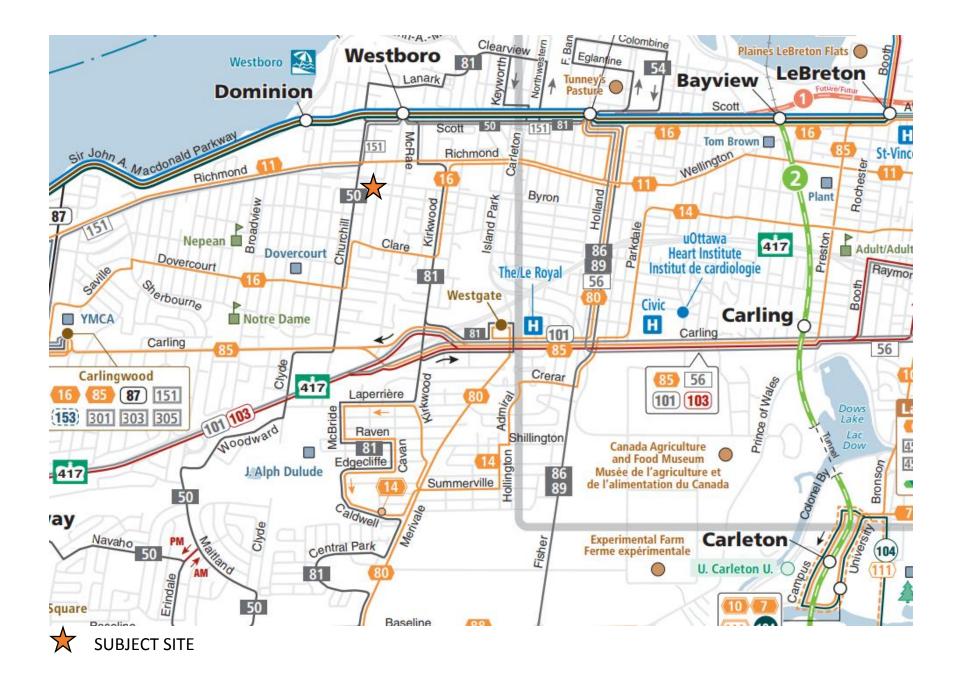
#### 5. Summary

	Yes	No
Does the development satisfy the Trip Generation Trigger?	x	
Does the development satisfy the Location Trigger?		х
Does the development satisfy the Safety Trigger?	x	

If none of the triggers are satisfied, <u>the TIA Study is complete</u>. If one or more of the triggers is satisfied, <u>the TIA Study must continue into the next stage</u> (Screening and Scoping).

# **APPENDIX C**

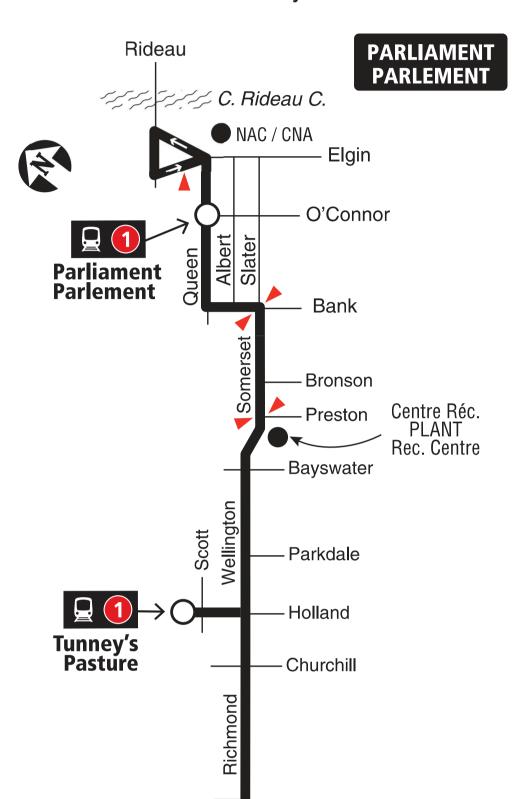
OC Transpo System Information

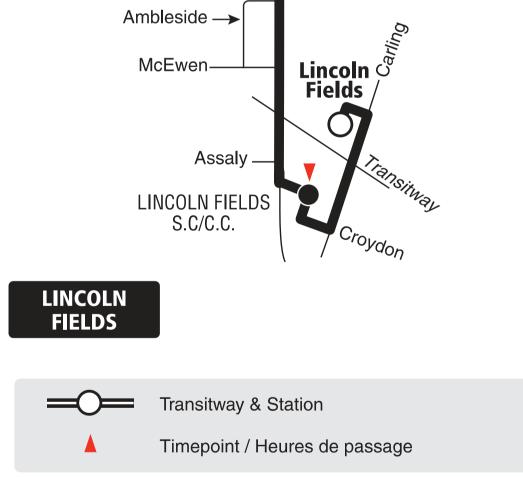




# 7 days a week / 7 jours par semaine

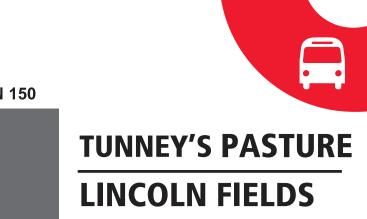
All day service Service toute la journée





## 2018.09

Schedule / Horaire613-560-1000 Text / Texto
Customer Relations Service à la clientèle
Effective Septembre 2, 2018 En vigueur 2 septembre 2018
CC Transpo INFO 613-741-4390 octranspo.com

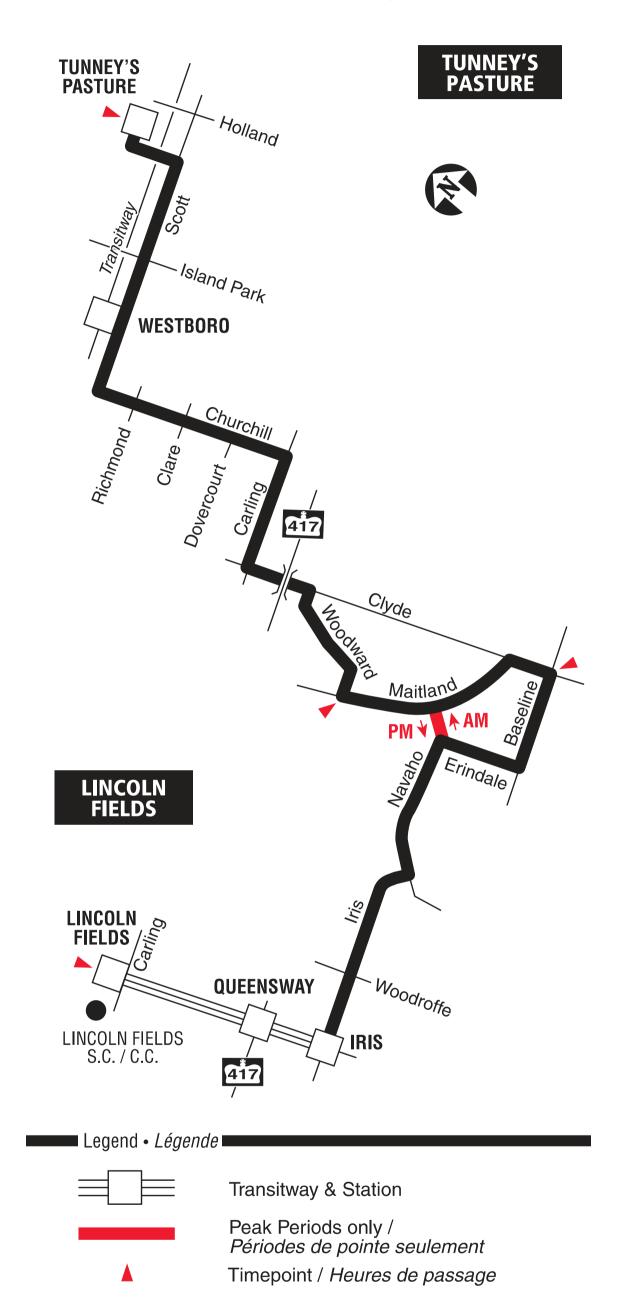


FORMER / ANCIEN 150



# Monday to Saturday / Lundi au samedi

No service Sat. eve. or all day Sunday / Aucun service le soir le sam. ou toute la journée dimanche



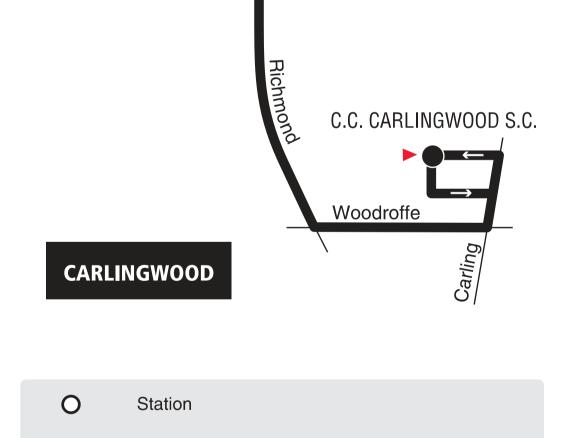
#### 2017.04

Schedule / Horaire613-560-1000 Text / Texto
<i>plus</i> your four digit bus stop number / <i>plus</i> votre numéro d'arrêt à quatre chiffres
Customer Relations Service à la clientèle <b>613-842-3600</b>
Lost and Found / Objets perdus 613-563-4011
Security / Sécurité
Effective April 24, 2017 En vigueur 24 avril 2017
<b>CETranspo</b> INFO 613-741-4390 octranspo.com



Monday to Friday / Lundi au vendredi Selected time periods only Périodes sélectionnées seulement





Timepoint / Heures de passage 

## 2018.06

Text / Texto	aire613-560-1000 560560 plus votre numéro d'arrêt à quatre chiffres			
pius your four digit bus stop flumber 7	plus votre numero d'arret a quatre chimes			
Customer Relations Service à la clientèle				
Lost and Found / Ohiets n	erdus 613-563-4011			
Lost and Found / Objets p				
Security / Sécurité				
Effective June 24, 2018 En vigueur 24 juin 2018				
<b>CC</b> Transpo	INFO 613-741-4390 octranspo.com			

Hi Brad,

Please see the requested data below. All data is for an average weekday of the Winter 2018 booking period, during the AM and PM peak periods (6-9am and 3-6pm, respectively). Passenger counts for stops 4864 and 4865 are not available as Route 11 did not serve them during the Winter 2018 booking.

As shown below, some stops are served by Route 11 and others by Route 50. Both routes are planned to operate with 40-foot buses during the AM and PM peak periods.

			AM Peak Period			PM Peak Period		
Stop Route	Direction	Total	Total	Average Load	Total	Total	Average Load	
	Direction	Boardings	Alightings	at Departure	Boardings	Alightings	at Departure	
4876	11	Westbound	1	15	8	13	19	17
4987	11	Eastbound	5	19	17	18	7	14
4907	50	Eastbound	1	4	15	3	1	10
5616	50	Westbound	0	0	9	5	3	12
7538	50	Eastbound	6	5	15	1	1	10
7539	50	Westbound	0	2	9	2	5	11

If you have any questions, please don't hesitate to contact me.

Best regards,

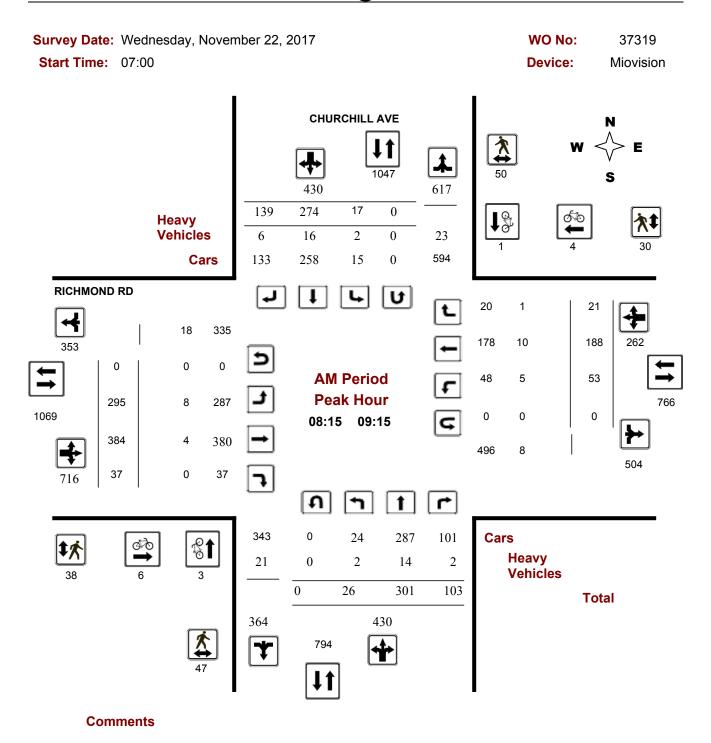
Genya

# APPENDIX D

Traffic Count Data

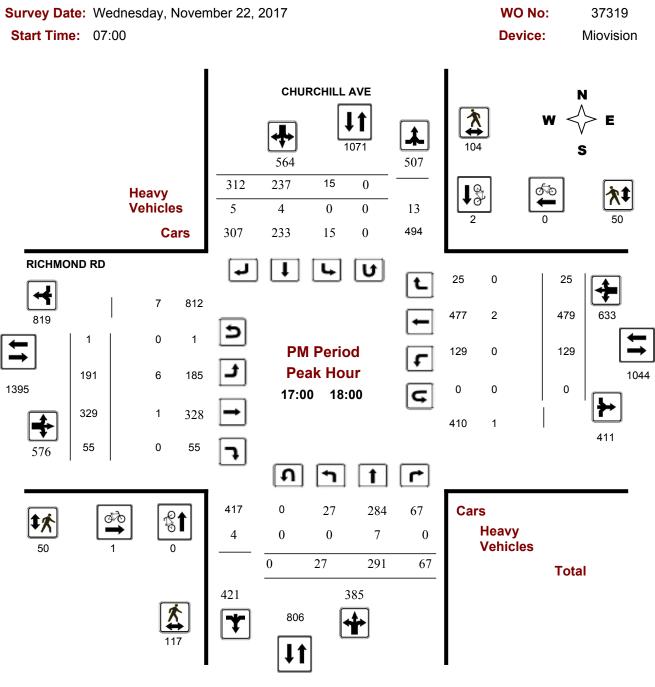


# Turning Movement Count - Peak Hour Diagram CHURCHILL AVE @ RICHMOND RD





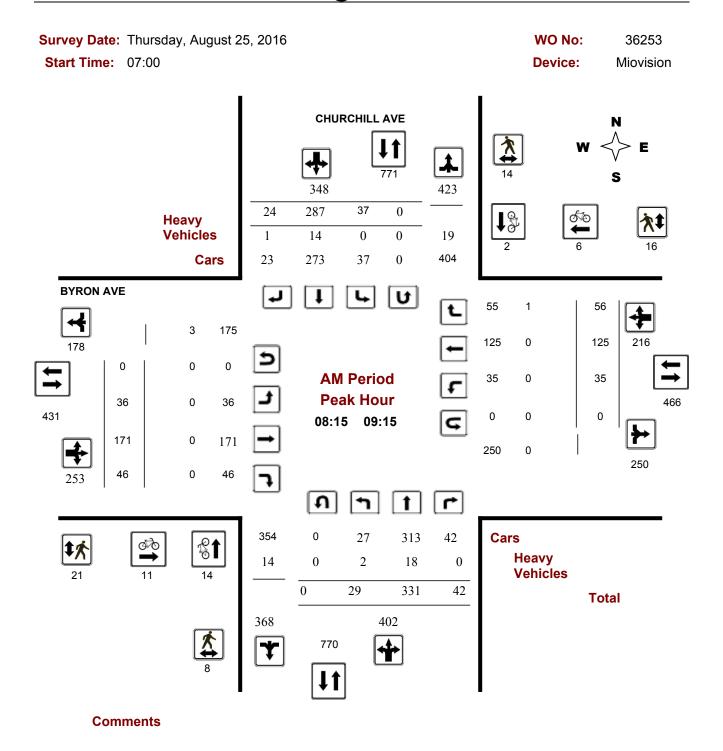
# Turning Movement Count - Peak Hour Diagram CHURCHILL AVE @ RICHMOND RD



Comments

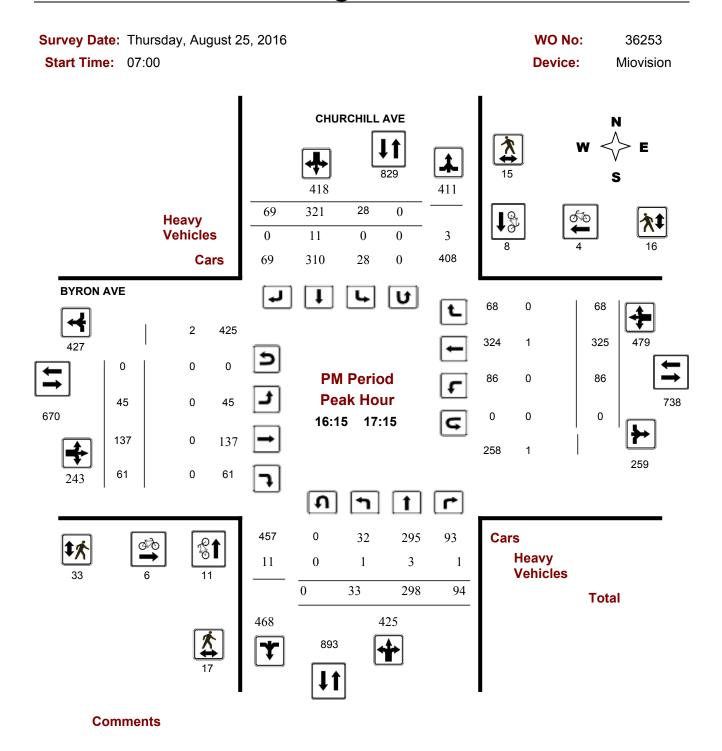


# Turning Movement Count - Peak Hour Diagram BYRON AVE @ CHURCHILL AVE

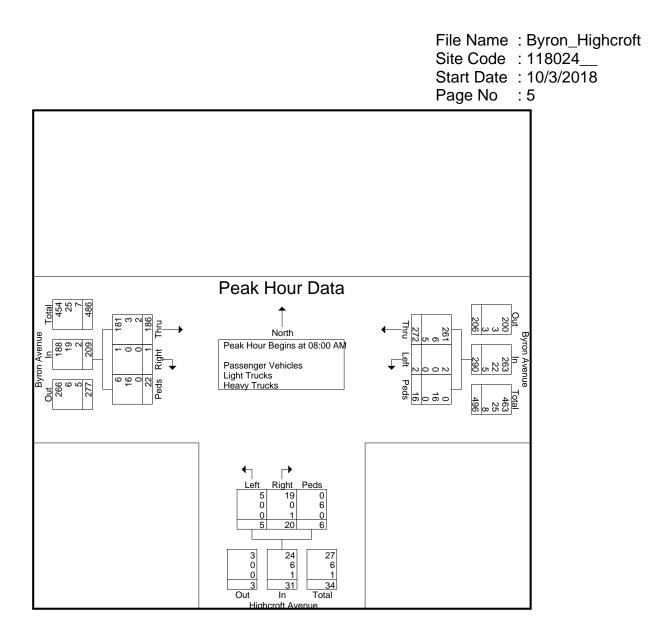




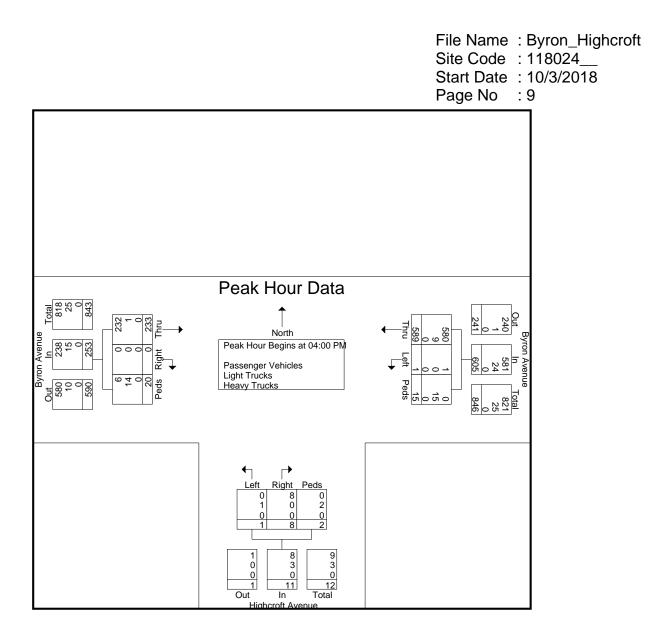
# Turning Movement Count - Peak Hour Diagram BYRON AVE @ CHURCHILL AVE



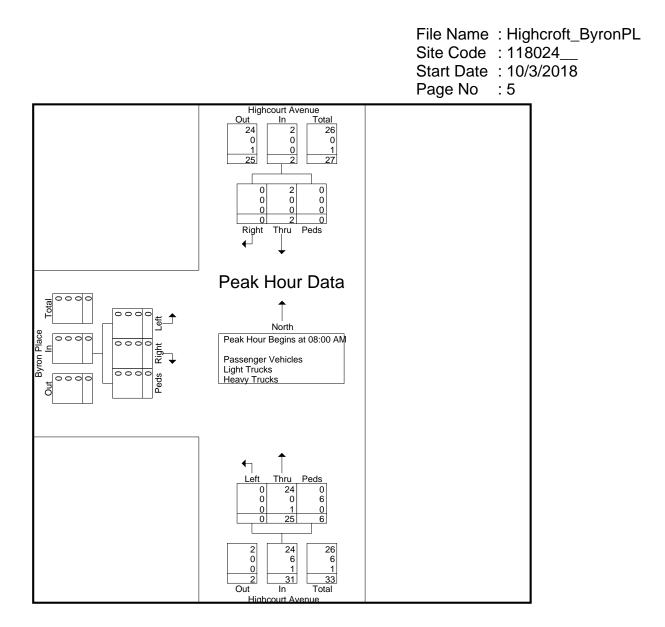




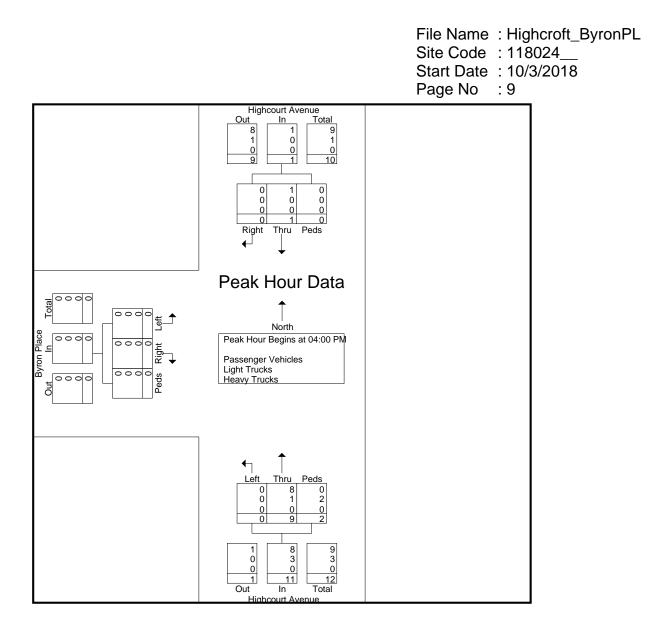












#### **Traffic Signal Timing**

	City of Ott	awa, Transportation S	Services Departme	nt
		Traffic Signal Operat	ions Unit	
Intersection:	Main:	Richmond	Side:	Churchill N
Controller:	MS-32	00	TSD:	5229
Author:	Yassine	e Bennani	Date:	24-Sep-2018

#### **Existing Timing Plans<sup>†</sup>**

	Plan		Ped Minimum Time								
	AM Peak	Off Peak	PM Peak	Night	Weekend	Walk	DW	A+R			
	1	2	3	4	5						
Cycle	80	75	90	65	75						
Offset	43	16	0	29	16						
EB Thru	45	43	60	33	43	16	9	3.3+2.8			
WB Thru	31	31	45	33	31	16	9	3.3+2.8			
NB Thru	35	32	30	32	32	7	9	3.6+2.6			
SB Thru	35	32	30	32	32	7	9	3.6+2.6			
EB Left	14	12	15	-	12	-	-	3.3+2.8			

#### Phasing Sequence<sup>‡</sup>

5s ADV Walk	•	<b>•</b>		*	*
	<b>↓</b>	5s ADV Walk			
◄►		↓ ↓	↓ • · ↓ • •	5s ADV Walk ◀·····►	<>

Plan:

5s ADV Walk	5s ADV Walk	× ▲	$\downarrow \uparrow \uparrow$
-------------	-------------	-----	--------------------------------

Notes: 1) For the east-west direction, there is a straight thru green arrow displayed during the 5 second advanced walk interval. After this 5 seconds, the green arrow changes to a green ball.

#### Schedule

Weekday			Saturda	y		Sunday	
Time	Plan		Time	Plan		Time	Plan
0:15	4		0:15	4	-	0:15	4
6:30	1		6:30	2	_	6:30	2
9:30	2		9:00	5	_	9:00	5
15:00	3		18:30	2	_	18:00	2
18:30	2		22:30	4	_	22:30	4
22:30	4	-			-		

#### Notes

†: Time for each direction includes amber and all red intervals

‡: Start of first phase should be used as reference point for offset

Asterisk (\*) Indicates actuated phase

(fp): Fully Protected Left Turn

Pedestrian signal

Cost is \$56.50 (\$50 + HST)

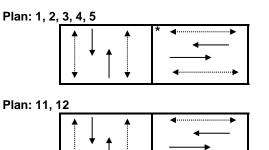
#### **Traffic Signal Timing**

	City of C	Ottawa, Transportatior Traffic Signal Oper	1	ent
Intersection:	Main:	Churchill	Side:	Byron
Controller:	ATC-3		TSD:	5634
Author:	Yassine	e Bennani	Date:	24-Sep-2018

#### **Existing Timing Plans<sup>†</sup>**

	Plan							Ped Min	nimum Tin	ne
	AM Peak	Off Peak	PM Peak	Night	Weekend	AM School	PM School	Walk	DW	A+R
	1	2	3	4	5	11	12			
Cycle	80	75	90	60	75	80	75			
Offset	74	45	40	х	45	74	45			
NB Thru	42	40	45	32	40	42	40	10	9	3.3+2.1
SB Thru	42	40	45	32	40	42	40	10	9	3.3+2.1
EB Thru	38	35	45	28	35	38	35	10	15	3.3+2.3
WB Thru	38	35	45	28	35	38	35	10	15	3.3+2.3

#### Phasing Sequence<sup>‡</sup>



#### Schedule

Weekday									
Time	Plan								
0:15	4								
6:30	1								
7:45	11								
8:15	1								
9:30	2								
14:15	12								
15:00	3								
18:30	2								
22:30	4								

Saturda	у
Time	Plan
0:15	4
6:30	2
9:00	5
18:30	2
22:30	4

Sunday									
Time	Plan								
0:15	4								
6:30	2								
9:00	5								
18:00	2								
22:30	4								

#### Notes

 $\ensuremath{\ensuremath{\mathsf{T}}}$  Time for each direction includes amber and all red intervals

‡: Start of first phase should be used as reference point for offset

Asterisk (\*) Indicates actuated phase

(fp): Fully Protected Left Turn

Pedestrian signal

Cost is \$56.50 (\$50 + HST)

# **APPENDIX E**

**Collision Records** 



# City Operations - Transportation Services Collision Details Report - Public Version

From: January 1, 2013 To: December 31, 2017

Traffic Control: Tra	affic signal					Total Collisions: 8			
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver	Vehicle type	First Event	No. Ped
2014-Jul-25, Fri,15:29	Clear	Other	P.D. only	Loose sand or gravel	North	Reversing	Construction equipment	Other motor vehicle	
					North	Stopped	Pick-up truck	Other motor vehicle	
2014-Oct-02, Thu,10:57	Clear	Rear end	P.D. only	Dry	South	Going ahead	Automobile, station wagon	Other motor vehicle	
					South	Slowing or stopping	Automobile, station wagon	Other motor vehicle	
2015-Jan-29, Thu,16:00	Clear	Angle	P.D. only	Ice	North	Turning right	Pick-up truck	Skidding/sliding	
					West	Stopped	Pick-up truck	Other motor vehicle	
2015-Jun-09, Tue,10:04	Rain	Rear end	P.D. only	Wet	North	Slowing or stopping	Automobile, station wagon	Other motor vehicle	
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Mar-26, Sat,12:30	Clear	Rear end	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Jun-11, Sat,09:49	Rain	Turning movement	P.D. only	Wet	East	Turning left	Automobile, station wagon	Other motor vehicle	

					West	Going ahead	Automobile,	Other motor vehicle	
					South	Stopped	station wagon Pick-up truck	Other motor vehicle	
2017-Sep-12, Tue,14:43	Clear	Turning movement	P.D. only	Dry	West	Turning right	Unknown	Cyclist	
					West	Going ahead	Bicycle	Other motor vehicle	
2013-Jun-11, Tue,13:50	Rain	Angle	P.D. only	Wet	South	Going ahead	Automobile, station wagon	Other motor vehicle	
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
Location: BYRON	N AVE @ HIGH	ICROFT AVE							
Traffic Control: Stop	p sign						Total Co	ollisions: 1	
ate/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver	r Vehicle type	First Event	No. Ped
2013-Jun-27, Thu,04:16	Clear	SMV other	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Building or wall	
Location: CHURC	CHILL AVE @	RICHMOND RD							
Traffic Control: Trat	ffic signal						Total Co	ollisions: 33	
Date/Day/Time	Environment	Impact Type	Classification	Surface					
				Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2014-Feb-26, Wed,14:52	Clear	Angle	P.D. only		East	Going ahead	r Vehicle type Automobile, station wagon	First Event Other motor vehicle	No. Ped
2014-Feb-26, Wed,14:52	Clear			Cond'n			Automobile,	Other motor	No. Ped
	Clear			Cond'n	East	Going ahead	Automobile, station wagon Automobile,	Other motor vehicle Other motor	No. Ped
2014-Feb-26, Wed,14:52 2014-May-01, Thu,19:54 2014-Jun-13, Fri,06:47		Angle	P.D. only	Cond'n Dry	East South	Going ahead Going ahead	Automobile, station wagon Automobile, station wagon	Other motor vehicle Other motor vehicle	

2014-Oct-06, Mon,00:46	Clear	Angle	P.D. only	Dry	South	Going ahead	Unknown	Other motor vehicle
					East	Going ahead	Automobile, station wagon	Other motor vehicle
2014-Oct-20, Mon,09:40	Clear	Sideswipe	P.D. only	Dry	North	Going ahead	Construction equipment	Other motor vehicle
					North	Going ahead	Automobile, station wagon	Other motor vehicle
2015-Jan-16, Fri,10:39	Clear	Angle	P.D. only	Slush	West	Pulling away from shoulder or curb	Automobile, station wagon	Other motor vehicle
					North	Turning left	Automobile, station wagon	Other motor vehicle
2015-Jun-18, Thu,09:14	Clear	Sideswipe	P.D. only	Dry	South	Turning right	Truck - tank	Other motor vehicle
					South	Turning right	Automobile, station wagon	Other motor vehicle
2015-Feb-26, Thu, 17:03	Clear	Angle	P.D. only	Wet	East	Going ahead	Automobile, station wagon	Other motor vehicle
					South	Going ahead	Pick-up truck	Other motor vehicle
2015-Jan-31, Sat,20:21	Snow	Turning movement	P.D. only	Loose snow	East	Turning left	Automobile, station wagon	Other motor vehicle
					West	Going ahead	Automobile, station wagon	Other motor vehicle
2015-Apr-11, Sat,12:34	Clear	Sideswipe	P.D. only	Dry	West	Changing lanes	Automobile, station wagon	Other motor vehicle
					West	Turning left	Pick-up truck	Other motor vehicle

2015-Jul-24, Fri,14:25	Clear	Rear end	P.D. only	Dry	East		Automobile, station wagon	Other motor vehicle
					East	Stopped	Truck and trailer	Other motor vehicle
2015-Jan-25, Sun,14:02	Clear	Rear end	Non-fatal injury	Dry	East	Turning left	Pick-up truck	Other motor vehicle
					East		Automobile, station wagon	Other motor vehicle
					East		Automobile, station wagon	Other motor vehicle
2016-Jun-17, Fri,05:29	Clear	Angle	P.D. only	Dry	East	Going ahead	Pick-up truck	Other motor vehicle
					South	Going ahead	Pick-up truck	Other motor vehicle
2015-Nov-19, Thu,10:56	Clear	Rear end	P.D. only	Wet	West	Unknown	Unknown	Other motor vehicle
					West	Stopped	Pick-up truck	Other motor vehicle
2015-Oct-03, Sat,11:00	Clear	Rear end	P.D. only	Dry	West	Slowing or stopping	Pick-up truck	Other motor vehicle
					West	Stopped	Pick-up truck	Other motor vehicle
2016-Jan-04, Mon,12:08	Clear	Rear end	P.D. only	Wet	South	Changing lanes	Pick-up truck	Other motor vehicle
					South		Automobile, station wagon	Other motor vehicle
2016-Aug-07, Sun,12:12	Clear	Rear end	P.D. only	Dry	South	Slowing or stopping	Automobile, station wagon	Other motor vehicle
					South	Stopped	Pick-up truck	Other motor vehicle

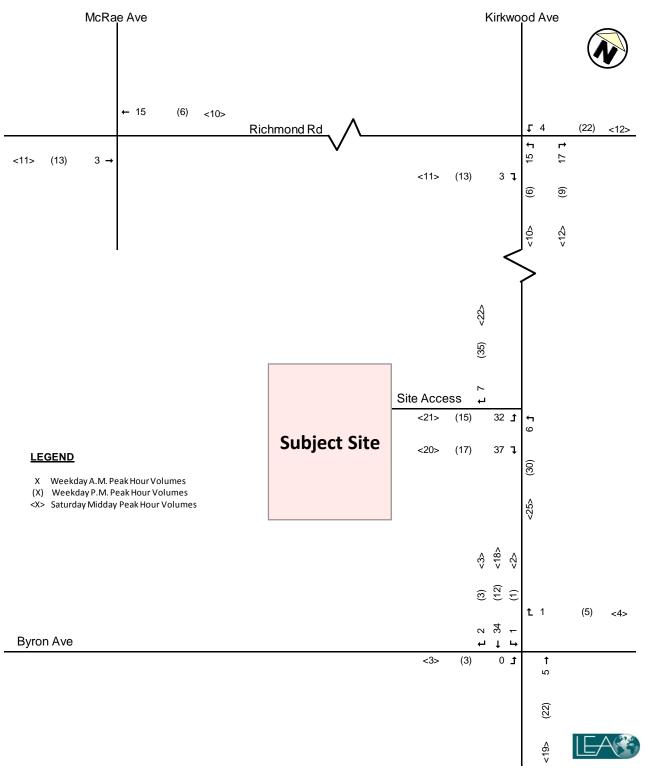
2016-Jun-02, Thu,14:22	Clear	SMV other	Non-fatal injury	Dry	South	Turning right	Automobile, station wagon	Pedestrian	1
2017-Sep-06, Wed,00:00	Clear	SMV unattended vehicle	P.D. only	Dry	Unknown	Unknown	Unknown	Unattended vehicle	
2017-Jan-28, Sat,15:02	Snow	Rear end	Non-fatal injury	Wet	East		Automobile, station wagon	Other motor vehicle	1
					East	Stopped	Automobile, station wagon	Other motor vehicle	
					West	Stopped	Automobile, station wagon	Other motor vehicle	
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Dec-31, Sat,12:01	Snow	SMV other	Non-fatal injury	Loose snow	East	Turning left	Automobile, station wagon	Pedestrian	1
2016-Aug-13, Sat,00:00	Clear	SMV unattended vehicle	P.D. only	Dry	North	Unknown	Unknown	Unattended vehicle	
2016-Dec-09, Fri,08:40	Clear	Rear end	P.D. only	Ice	East S	Slowing or stopping	Automobile, station wagon	Other motor vehicle	
					East	Stopped	Automobile, station wagon	Other motor vehicle	
2017-Sep-26, Tue,19:08	Clear	Turning movement	P.D. only	Dry	North	Going ahead	Pick-up truck	Other motor vehicle	
					South	Turning left	Automobile, station wagon	Other motor vehicle	
2013-Jan-17, Thu,16:33	Clear	SMV unattended vehicle	P.D. only	Dry	South		Municipal transit bus	Unattended vehicle	
2013-Feb-16, Sat,10:33	Clear	SMV other	Non-fatal injury	Dry	North		Municipal transit bus	Pedestrian	1

2013-Apr-06, Sat,11:44	Clear	Rear end	P.D. only	Dry	East	Going ahead		Other motor vehicle
					East	Going ahead		Other motor vehicle
2013-Jun-13, Thu,13:00	Clear	Angle	P.D. only	Dry	South	Turning right		Other motor vehicle
					East	Turning left	Automobile,	Other motor vehicle
2013-Jun-17, Mon,18:52	Clear	Sideswipe	P.D. only	Dry	East	Merging		Other motor vehicle
					East	Turning left	Municipal transit bus	Other motor vehicle
2013-Jun-13, Thu,14:30	Clear	Rear end	P.D. only	Dry	South SI	owing or stopping		Other motor vehicle
					South	Stopped		Other motor vehicle
2013-Jul-28, Sun,12:50	Clear	SMV unattended vehicle	P.D. only	Dry	South	Turning right	Municipal transit bus	Unattended vehicle
2013-Aug-28, Wed,15:34	Clear	Sideswipe	P.D. only	Dry	South	Merging		Other motor vehicle
					South	Going ahead		Other motor vehicle
2013-Sep-24, Tue,11:51	Clear	Sideswipe	P.D. only	Dry		Pulling away from shoulder or curb		Other motor vehicle
					East	Going ahead		Other motor vehicle

# **APPENDIX F**

Relevant Excerpts from Other Reports

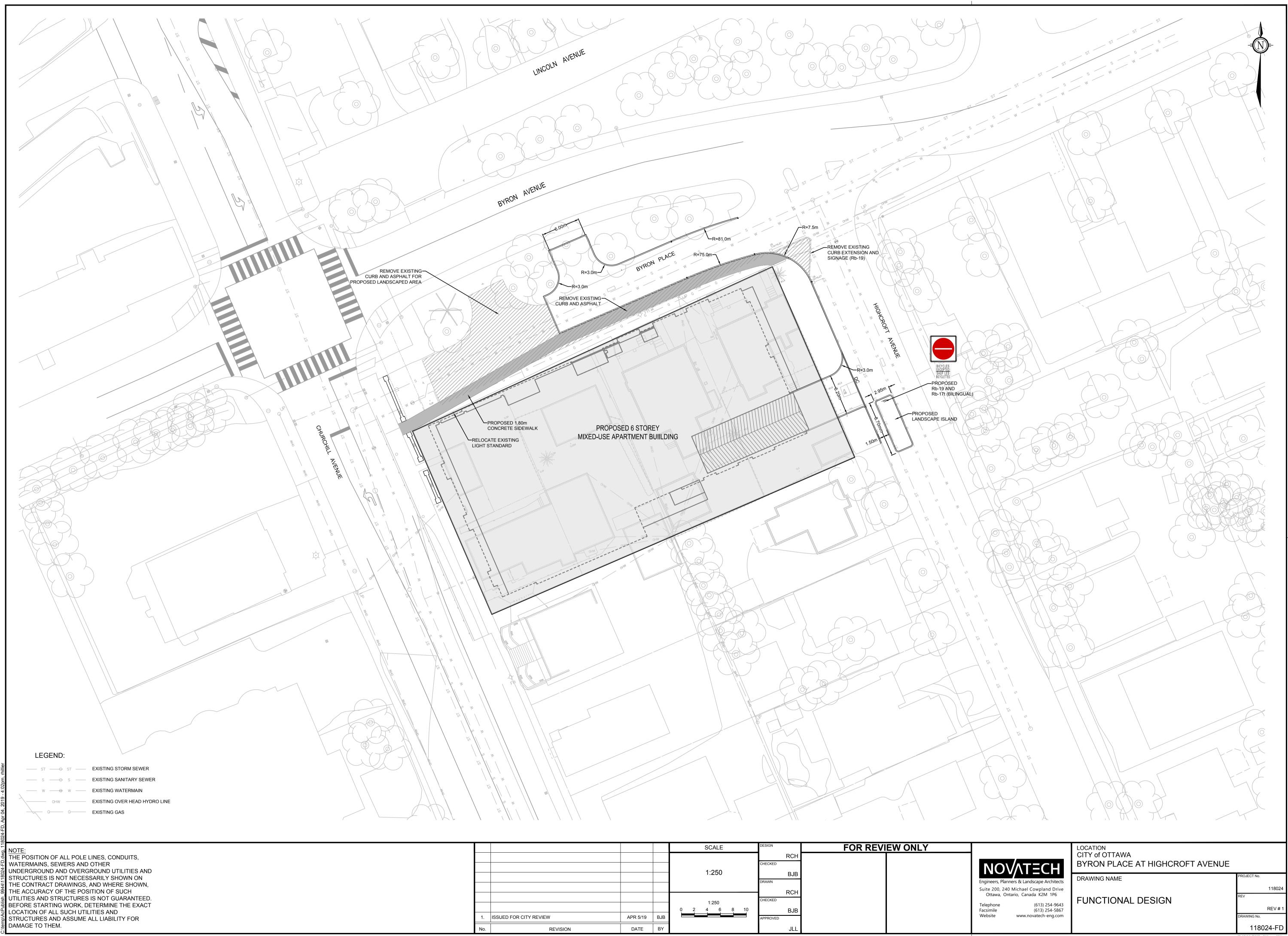




#### Figure 3: Site Trips

# **APPENDIX G**

Functional Design of Roadway Modifications



					SCALE	DESIGN	FOR REVIEW ONLY
						RCH	
						CHECKED	
					1:250	BJB	
						DRAWN	
						RCH	
					1:250 0 2 4 6 8 10		
	1.	ISSUED FOR CITY REVIEW	APR 5/19	BJB		BJB	
Ν	No.	REVISION	DATE	BY		JLL	

# APPENDIX H

Transportation Demand Management Checklists

# **TDM-Supportive Development Design and Infrastructure Checklist:**

Residential Developments (multi-family or condominium)

Legend					
REQUIRED	The Official Plan or Zoning By-law provides related guidance that must be followed				
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users				
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance				

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references		
	1.	WALKING & CYCLING: ROUTES			
	1.1	Building location & access points			
BASIC	1.1.1	Locate building close to the street, and do not locate parking areas between the street and building entrances			
BASIC	1.1.2	Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations			
BASIC	1.1.3	Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort			
]	1.2	Facilities for walking & cycling			
REQUIRED	1.2.1	Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see Official Plan policy 4.3.3)			
REQUIRED	1.2.2	Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible <i>(see Official</i> <i>Plan policy 4.3.12)</i>			

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3	Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see Official Plan policy 4.3.10)	
REQUIRED	1.2.4	Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see Official Plan policy 4.3.10)	
REQUIRED	1.2.5	Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and on- road cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see Official Plan policy 4.3.11)	
BASIC	1.2.6	Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	
BASIC	1.2.7	Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	
BASIC	1.2.8	Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	
	1.3	Amenities for walking & cycling	
BASIC	1.3.1	Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	
BASIC	1.3.2	Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	2.	WALKING & CYCLING: END-OF-TRIP FACILI	TIES
	2.1	Bicycle parking	
REQUIRED	2.1.1	Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see Official Plan policy 4.3.6)	
REQUIRED	2.1.2	Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well- used areas ( <i>see Zoning By-law Section 111</i> )	
REQUIRED	2.1.3	Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored (see Zoning By-law Section 111)	
BASIC	2.1.4	Provide bicycle parking spaces equivalent to the expected number of resident-owned bicycles, plus the expected peak number of visitor cyclists	
	2.2	Secure bicycle parking	
REQUIRED	2.2.1	Where more than 50 bicycle parking spaces are provided for a single residential building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see Zoning By-law Section 111)	X Not Applicable (Less than 50 bicycle parking spaces required)
BETTER	2.2.2	Provide secure bicycle parking spaces equivalent to at least the number of units at condominiums or multi- family residential developments	
	2.3	Bicycle repair station	
BETTER	2.3.1	Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	
	3.	TRANSIT	
	3.1	Customer amenities	
BASIC	3.1.1	Provide shelters, lighting and benches at any on-site transit stops	
BASIC	3.1.2	Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	
BETTER	3.1.3	Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	

	TDM-s	upportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	4.	RIDESHARING	
	4.1	Pick-up & drop-off facilities	
BASIC	4.1.1	Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones	
	5.	CARSHARING & BIKESHARING	
	5.1	Carshare parking spaces	
BETTER	5.1.1	Provide up to three carshare parking spaces in an R3, R4 or R5 Zone for specified residential uses <i>(see Zoning By-law Section 94)</i>	
	5.2	Bikeshare station location	
BETTER	5.2.1	Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection	
	6.	PARKING	
	6.1	Number of parking spaces	
REQUIRED	6.1.1	Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	
BASIC	6.1.2	Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	
BASIC	6.1.3	Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (see Zoning By-law Section 104)	
BETTER	6.1.4	Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking <i>(see Zoning By-law Section 111)</i>	
	6.2	Separate long-term & short-term parking areas	
BETTER	6.2.1	Provide separate areas for short-term and long-term parking (using signage or physical barriers) to permit access controls and simplify enforcement (i.e. to discourage residents from parking in visitor spaces, and vice versa)	

### **TDM Measures Checklist:**

Residential Developments (multi-family, condominium or subdivision)

### Legend

BASIC The measure is generally feasible and effective, and in most cases would benefit the development and its users

**BETTER** The measure could maximize support for users of sustainable modes, and optimize development performance

The measure is one of the most dependably effective tools to encourage the use of sustainable modes

	TDM	measures: Residential developments	Check if proposed & add descriptions
	1.	TDM PROGRAM MANAGEMENT	
	1.1	Program coordinator	
BASIC ★	1.1.1	Designate an internal coordinator, or contract with an external coordinator	
	1.2	Travel surveys	
BETTER	1.2.1	Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress	
	2.	WALKING AND CYCLING	
	2.1	Information on walking/cycling routes & des	tinations
BASIC	2.1.1	Display local area maps with walking/cycling access routes and key destinations at major entrances (multi-family, condominium)	
	2.2	Bicycle skills training	
BETTER	2.2.1	Offer on-site cycling courses for residents, or subsidize off-site courses	

	TDM	measures: Residential developments	Check if proposed & add descriptions
	3.	TRANSIT	
	3.1	Transit information	
BASIC	3.1.1	Display relevant transit schedules and route maps at entrances (multi-family, condominium)	
BETTER	3.1.2	Provide real-time arrival information display at entrances (multi-family, condominium)	
	3.2	Transit fare incentives	
BASIC ★	3.2.1	Offer PRESTO cards preloaded with one monthly transit pass on residence purchase/move-in, to encourage residents to use transit	
BETTER	3.2.2	Offer at least one year of free monthly transit passes on residence purchase/move-in	
	3.3	Enhanced public transit service	
BETTER ★	3.3.1	Contract with OC Transpo to provide early transit services until regular services are warranted by occupancy levels ( <i>subdivision</i> )	
	3.4	Private transit service	
BETTER	3.4.1	Provide shuttle service for seniors homes or lifestyle communities (e.g. scheduled mall or supermarket runs)	
	4.	CARSHARING & BIKESHARING	
	4.1	Bikeshare stations & memberships	
BETTER	4.1.1	Contract with provider to install on-site bikeshare station ( <i>multi-family</i> )	
BETTER	4.1.2	Provide residents with bikeshare memberships, either free or subsidized (multi-family)	
	4.2	Carshare vehicles & memberships	
BETTER	4.2.1	Contract with provider to install on-site carshare vehicles and promote their use by residents	
BETTER	4.2.2	Provide residents with carshare memberships, either free or subsidized	
	5.	PARKING	
	5.1	Priced parking	
BASIC ★	5.1.1	Unbundle parking cost from purchase price (condominium)	
BASIC ★	5.1.2	Unbundle parking cost from monthly rent (multi-family)	

	TDM	measures: Residential developments	Check if proposed & add descriptions
	6.	TDM MARKETING & COMMUNICATION	S
	6.1	Multimodal travel information	
BASIC 🖈	6.1.1	Provide a multimodal travel option information package to new residents	
	6.2	Personalized trip planning	
BETTER	6.2.1	Offer personalized trip planning to new residents	

### **APPENDIX I**

Segment MMLOS Analysis

### Pedestrian Level of Service (PLOS)

Sidewalk Width	Boulevard Width	Avg. Daily CurbPresence of On-StreetOperatingLane TrafficOn-StreetSpeedVolumeParking		Operating Speed	Segment PLOS						
Churchill /	Churchill Avenue (East Side)										
2.0m	>2.0m	>3,000 vpd	No	60 km/hr	С						
Churchill	Churchill Avenue (West Side)										
2.0m	>2.0m	>3,000 vpd	>3,000 vpd Yes		В						
Byron Ave	enue (North S	Side)									
2.0m	None	>3,000 vpd	No	60 km/hr	E						
Byron Ave	Byron Avenue (South Side)										
1.8m	None	<3,000 vpd	No	60 km/hr	С						

### **Bicycle Level of Service (BLOS)**

Road Class	Bike Route	Type of Bikeway	Travel Lanes	Centerline Markings	Operating Speed	Segment BLOS					
Churchill Avenue											
Major Collector	Spine	Separated Facility	2	Yes	60 km/hr	А					
Byron Aven	Byron Avenue										
Collector	Local	Mixed Traffic	2	Yes	60 km/hr	F					

### Transit Level of Service (TLOS)

	Level/Exposure	Segment TLOS								
Facility Type	Congestion	Congestion Friction Inc		Segment TLOS						
Churchill Avenue	Churchill Avenue									
Mixed Traffic	Yes	High	High	F						
Byron Avenue	Byron Avenue									
Mixed Traffic	Yes	Low	Medium	D						

### Truck Level of Service (TkLOS)

Curb Lane Width	Number of Travel Lanes (Per Direction)	Segment TkLOS						
Churchill Avenue								
>3.7m	1	В						
Byron Avenue								
>3.7m	1	В						

### Auto LOS

Direction	Directional	Traffic Volumes			Auto					
	Capacity <sup>1</sup>	AM	AM PM		AM Peak		PM Peak			
	Capacity	Peak	Peak	v/c	LOS	v/c	LOS	LOS		
Churchill Avenue										
NB	600vph	402	425	0.67	В	0.71	С	с		
SB	600vph	373	477	0.62	В	0.80	С	C		
Byron Ave	enue									
EB	600vph	250	259	0.41	А	0.43	А	D		
WB	600vph	246	530	0.41	А	0.88	D	U		

1. Typical lane capacity based on the City's guidelines for the TRANS long-range transportation model

### Segment MMLOS Summary

	Segment	Churchil East	l Avenue West	Byron / North	Avenue South	
	Sidewalk Width	2.0m	2.0m	2.0m	2.0m	
	Boulevard Width	>2.0m	>2.0m	None	None	
Pedestrian	Average Daily Curb Lane Traffic Volume	>3000vpd	>3000vpd	>3000vpd	<3000vpd	
dest	Sidewalk Width Boulevard Width Average Daily Curb Lane Traffic	No	Yes	No	No	
Ъе	Operating Speed	60 km/h	60 km/h	60 km/hr	60 km/hr	
	Level of Service	С	В	E	С	
	Target		(	C		
	Road Classification	Major C	ollector	Colle	ector	
	Bike Route Classification	Sp	ine	Lo	cal	
	Type of Bikeway	Separate	d Facility	Mixed	Traffic	
Cyclist	Travel Lanes	2	2	2		
Cyc	Centerline Markings	Ye	es	Yes		
	Operating Speed	60 k	m/h	60 km/hr		
	Level of Service	ŀ	A	F		
	Target	С		В		
	Facility Type	Mixod	Traffic	Mixed	Traffic	
		IVIIXeu	Traine	IVIIXEU	Traffic	
nsit			uent		ited	
Transit	Friction/Congestion/Incident Potential		uent	Lim		
Transit	Friction/Congestion/Incident Potential Level of Service	Freq	uent	Lim	ited	
Transit	Friction/Congestion/Incident Potential Level of Service Target	Freq	uent	Lim	ited	
	Friction/Congestion/Incident Potential Level of Service Target Lane Width	Freq	uent - 7m	Lim [ >3.	ited D	
Truck Transit	Friction/Congestion/Incident Potential Level of Service Target Lane Width Travel Lanes (per direction)	Freq F >3.	uent - 7m	Lim [ 	ited D 7m	
	Friction/Congestion/Incident Potential Level of Service Target Lane Width Travel Lanes (per direction) Level of Service	Freq F >3.	uent - 7m 3	Lim [ 	ited D 7m 1	
	Friction/Congestion/Incident Potential Level of Service Target Lane Width Travel Lanes (per direction) Level of Service Target	Freq F >3.	uent - 7m 3	Lim [ >3. -	ited D 7m 1	
Truck	Friction/Congestion/Incident Potential Level of Service Target Lane Width Travel Lanes (per direction) Level of Service Target Volume	Freq F >3. E E 477	uent - 7m - 3	Lim [ >3. - - - - - - - - - - - - - - - - - - -	ited D 7m 1 3	
Truck	Friction/Congestion/Incident Potential Level of Service Target Lane Width Travel Lanes (per direction) Level of Service Target Volume	Freq F >3. 600	uent - 7m - 3 - vph	Lim [ >3. 2 530 600	ited D 7m 1 3 - vph	
	Friction/Congestion/Incident Potential Level of Service Target Lane Width Travel Lanes (per direction) Level of Service Target Volume Capacity	Freq F >3. 2 5 600 0.3	uent - 7m - 3 - - - - - - - - - - - - - - - - -	Lim [ >3. - - - - - - - - - - - - - - - - - - -	ited D 7m 1 3 Vph vph	

Figure 3.3

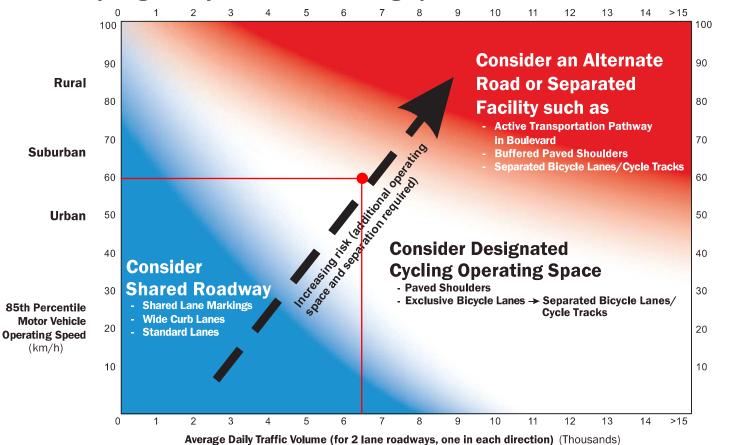
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**Desirable Bicycle Facility Pre-Selection Nomograph** 

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Source: MMM, 2013

### STEP 1 of 3 **Desirable Cycling Facility Pre-selection Nomograph**



Footnotes: - This nomograph is the first of a three step bicycle facility selection process., and should not be used by itself as the justification for facility selection (see Steps 2 and 3). The nomograph simply helps practitioners pre-select a desirable cycling facility type, however the context of the situation governs the final decision.

> - The nomograph has been adapted for the North American context and is based on international examples and research for two lane roadways. It is, however, still applicable for multi-lane roadways. For these situations, designers should consider the operating speed, total combined traffic volume and traffic mix of the vehicles traveling in the lanes immediately adjacent to the cycling facilities.

- Consider a Separated Facility or an Alternate Road for roadways with an AADT greater than 15,000 vehicles and an operating speed of greater then 50 km/h.

- For rural and suburban locations this nomograph assumes good sightlines are provided for all road users. In urban areas, there are typically more frequent conflict points at driveways, midblock crossings and intersections (especially on multi-lane roads), as well as on road segments with on-street parking. This needs to be considered when assessing risk exposure in urban environments since it will influence the selection of a suitable facility type.

### **APPENDIX J**

Synchro Analysis Reports

# 3: Churchill Avenue & Byron Avenue AM Peak

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		Υ.	î,		۲.	ĥ	
Traffic Volume (vph)	36	171	46	40	<b>4</b> 142	64	29	331	42	37	287	24
Future Volume (vph)	36	171	46	40	142	64	29	331	42	37	287	24
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	0.0		0.0	15.0		0.0	15.0		0.0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (m)	7.6			7.6			25.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.99			0.99		0.98	0.99		0.98	0.99	
Frt		0.975			0.965			0.983			0.988	
Flt Protected		0.993			0.992		0.950			0.950		
Satd. Flow (prot)	0	1748	0	0	1712	0	1616	1701	0	1729	1705	0
Flt Permitted		0.881			0.850		0.535			0.484		
Satd. Flow (perm)	0	1548	0	0	1465	0	888	1701	0	868	1705	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		17			27			11			7	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		207.2			85.0			153.8			123.1	
Travel Time (s)		14.9			6.1			11.1			8.9	
Confl. Peds. (#/hr)	14		8	8		14	21		16	16		21
Confl. Bikes (#/hr)			11			6			14			6
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	0%	0%	0%	0%	2%	7%	5%	0%	0%	5%	4%
Adj. Flow (vph)	40	190	51	44	158	71	32	368	47	41	319	27
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	281	0	0	273	0	32	415	0	41	346	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0	J		0.0	<b>J</b> -		3.7	J		3.7	<b>J</b> .
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.9			4.9			4.9			4.9	
Two way Left Turn Lane												
Headway Factor	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5		6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8		6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex		CI+Ex	Cl+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel		OI! EX			OI! EX			OI! EX			OI! EX	
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases	T OIIII	4		T OIIII	8		r onn	2		T OIIII	6	
Permitted Phases	4	т		8	0		2	2		6	0	
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase	4	4		0	0		2	2		0	0	
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	30.6	30.6		30.6	30.6		25.5	25.5		25.5	25.5	
	30.6 38.0	30.6		30.6	30.6		25.5 42.0	25.5 42.0		25.5 42.0	25.5 42.0	
	30.0											
Total Split (s) Total Split (%)		17 50/		17 50/	17 50/		50 50/	L') L0/		50 60/	50 EV/	
Total Split (s) Total Split (%) Maximum Green (s)	47.5% 32.4	47.5% 32.4		47.5% 32.4	47.5% 32.4		52.5% 36.6	52.5% 36.6		52.5% 36.6	52.5% 36.6	

Brad Byvelds, Novatech

# 3: Churchill Avenue & Byron Avenue AM Peak

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ane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SE
All-Red Time (s)	2.3	2.3		2.3	2.3		2.1	2.1		2.1	2.1	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6		5.4	5.4		5.4	5.4	
_ead/Lag												
_ead-Lag Optimize?												
/ehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		C-Max	C-Max		C-Max	C-Max	
Valk Time (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
lash Dont Walk (s)	15.0	15.0		15.0	15.0		9.0	9.0		9.0	9.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0		0	0	
Act Effct Green (s)		18.6			18.6		50.4	50.4		50.4	50.4	
Actuated g/C Ratio		0.23			0.23		0.63	0.63		0.63	0.63	
/c Ratio		0.75			0.75		0.06	0.39		0.08	0.32	
Control Delay		39.0			38.3		7.9	9.5		8.0	8.9	
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.0	
Total Delay		39.0			38.3		7.9	9.5		8.0	8.9	
.0S		D			D		А	А		А	А	
Approach Delay		39.0			38.3			9.4			8.8	
Approach LOS		D			D			A			A	
Queue Length 50th (m)		37.4			34.9		1.7	26.6		2.2	21.2	
Queue Length 95th (m)		55.7			53.6		6.2	55.5		7.5	45.0	
nternal Link Dist (m)		183.2			61.0		45.0	129.8		45.0	99.1	
Furn Bay Length (m)		007			000		15.0	1074		15.0	1075	
Base Capacity (vph)		637 0			609 0		559 0	1074		546 0	1075	
Starvation Cap Reductn		0			0		-			-	0	
Spillback Cap Reductn		0			0		0	0		0	-	
Storage Cap Reductn Reduced v/c Ratio		0.44			0.45		0 0.06	0 0.39		0.08	0 0.32	
		0.44			0.45		0.00	0.39		0.00	0.32	
ntersection Summary Area Type:	Other											
Cycle Length: 80	Other											
Actuated Cycle Length: 80												
Offset: 74 (93%), Referenced to p	hase 2.NRTL a	nd 6.SBTI	Start of Gre	en								
Natural Cycle: 60	1400 2.NDTE 4	na 0.001L,										
Control Type: Actuated-Coordinat	ted											
Aaximum v/c Ratio: 0.75												
ntersection Signal Delay: 20.9				Int	ersection LC	S: C						
ntersection Capacity Utilization 6	2.2%				J Level of S							
Analysis Period (min) 15	,			.01	20.01010							
Splits and Phases: 3: Churchill	Avenue & Byro											
	Avenue à Dylu	II AVEIIUE										

Ø2 (R)	<u></u> 4	
42 s	38 s	
Ø6 (R)	<b>↓</b> Ø8	
42 s	38 s	

#### 6: Churchill Avenue & Ricmond Rd AM Peak

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	5	1.		- <b>N</b>	1.			୍ ଶ୍	1		4	1
Traffic Volume (vph)	295	384	37	53	188	21	26	301	103	17	274	139
Future Volume (vph)	295	384	37	53	188	21	26	301	103	17	274	139
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	30.0		0.0	25.0		0.0	15.0		15.0	15.0		15.0
Storage Lanes	1		0	1		0	0		1	0		1
Taper Length (m)	45.0			30.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.95	0.99		0.96	0.99			1.00	0.90		1.00	0.89
Frt	0.050	0.987		0.050	0.985			0.000	0.850		0.007	0.850
Flt Protected	0.950	4705	0	0.950	1000	٥	٥	0.996	4547	^	0.997	1400
Satd. Flow (prot)	1679	1765	0	1586	1690	0	0	1722	1517	0	1706	1488
Flt Permitted	0.374	4705	0	0.391	1000	0	0	0.952	1000	0	0.966	1017
Satd. Flow (perm)	629	1765	Yes	628	1690	Yes	0	1640	1369 Yes	0	1650	1317
Right Turn on Red Satd. Flow (RTOR)		7	res		7	res			180			Yes 180
		50			50			50	100		50	100
Link Speed (k/h)		135.1			50 164.7			123.1			131.3	
Link Distance (m) Travel Time (s)		9.7			164.7			8.9			131.3 9.5	
Confl. Peds. (#/hr)	50	9.1	47	47	11.9	50	38	0.9	30	30	9.5	38
Confl. Bikes (#/hr)	50		47	47		4	50		3	50		1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	1%	0.90	9%	5%	5%	8%	5%	2%	12%	0.90 6%	4%
Adj. Flow (vph)	328	427	41	59	209	23	29	334	114	12 /0	304	154
Shared Lane Traffic (%)	520	421	41	55	203	20	23	504	114	10	504	104
Lane Group Flow (vph)	328	468	0	59	232	0	0	363	114	0	323	154
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	Lon	3.7	ragin	Lon	3.7	ragin	Lon	3.7	rugitt	Lon	3.7	rugin
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.9			4.9			4.9			4.9	
Two way Left Turn Lane												
Headway Factor	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2		1	2	1	1	2	1
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5		6.1	30.5	6.1	6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8		6.1	1.8	6.1	6.1	1.8	6.1
Detector 1 Type	CI+Ex	CI+Ex		CI+Ex	CI+Ex		Cl+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA		Perm	NA	Perm	Perm	NA	Perm
Protected Phases	7	4			8			2			6	
Permitted Phases	4			8			2		2	6		6
Detector Phase	7	4		8	8		2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0		10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	11.1	31.1		31.1	31.1		24.2	24.2	24.2	24.2	24.2	24.2
Total Split (s)	14.0	40.1		31.1	31.1		30.0	30.0	30.0	30.0	30.0	30.0
Total Split (%)	17.5%	50.1%		38.8%	38.8%		37.5%	37.5%	37.5%	37.5%	37.5%	37.5%
Maximum Green (s)	7.9	34.0		25.0	25.0		23.8	23.8	23.8	23.8	23.8	23.8
Yellow Time (s)	3.3	3.3		3.3	3.3		3.6	3.6	3.6	3.6	3.6	3.6

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Lane Group	Ø1	Ø3	Ø5
Lane Configurations			
Traffic Volume (vph)			
Future Volume (vph)			
Ideal Flow (vphpl)			
Storage Length (m)			
Storage Lanes			
Taper Length (m)			
Lane Util. Factor			
Ped Bike Factor			
Frt			
Flt Protected			
Satd. Flow (prot)			
Flt Permitted			
Satd. Flow (perm)			
Right Turn on Red			
Satd. Flow (RTOR)			
Link Speed (k/h)			
Link Distance (m)			
Travel Time (s)			
Confl. Peds. (#/hr)			
Confl. Bikes (#/hr)			
Peak Hour Factor			
Heavy Vehicles (%)			
Adj. Flow (vph)			
Shared Lane Traffic (%)			
Lane Group Flow (vph)			
Enter Blocked Intersection			
Lane Alignment			
Median Width(m)			
Link Offset(m)			
Crosswalk Width(m)			
Two way Left Turn Lane			
Headway Factor			
Turning Speed (k/h)			
Number of Detectors			
Detector Template			
Leading Detector (m)			
Trailing Detector (m)			
Detector 1 Position(m)			
Detector 1 Size(m)			
Detector 1 Type			
Detector 1 Channel			
Detector 1 Extend (s)			
Detector 1 Queue (s)			
Detector 1 Delay (s)			
Detector 2 Position(m)			
Detector 2 Size(m)			
Detector 2 Type			
Detector 2 Channel			
Detector 2 Extend (s)			
Turn Type	4	^	-
Turn Type Protected Phases	1	3	5
Turn Type Protected Phases Permitted Phases	1	3	5
Turn Type Protected Phases Permitted Phases Detector Phase	1	3	5
Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase			
Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s)	3.0	3.0	3.0
Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s)	3.0 5.0	3.0 5.0	3.0 5.0
Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Total Split (s)	3.0 5.0 5.0	3.0 5.0 5.0	3.0 5.0 5.0
Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Total Split (s) Total Split (%)	3.0 5.0 5.0 6%	3.0 5.0 5.0 6%	3.0 5.0 5.0 6%
Turn Type Protected Phases Permitted Phases Detector Phase Switch Phase Minimum Initial (s) Minimum Split (s) Total Split (s)	3.0 5.0 5.0	3.0 5.0 5.0	3.0 5.0 5.0

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#### 6: Churchill Avenue & Ricmond Rd AM Peak

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
All-Red Time (s)	2.8	2.8		2.8	2.8		2.6	2.6	2.6	2.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0		0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1			6.2	6.2		6.2	6.2
Lead/Lag	Lead	Lag		Lag	Lag		Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None		None	None		C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)		16.0		16.0	16.0		7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		9.0		9.0	9.0		9.0	9.0	9.0	9.0	9.0	9.0
Pedestrian Calls (#/hr)		15		15	15		15	15	15	15	15	15
Act Effct Green (s)	31.6	26.6		17.6	17.6			29.2	29.2		29.2	29.2
Actuated g/C Ratio	0.39	0.33		0.22	0.22			0.36	0.36		0.36	0.36
v/c Ratio	0.93	0.79		0.43	0.62			0.61	0.19		0.54	0.26
Control Delay	55.1	33.8		34.7	33.6			28.0	1.6		25.8	3.7
Queue Delay	0.0	0.0		0.0	0.0			0.0	0.0		0.0	0.0
Total Delay	55.1	33.8		34.7	33.6			28.0	1.6		25.8	3.7
LOS	E	С		С	С			С	А		С	A
Approach Delay		42.5			33.8			21.7			18.7	
Approach LOS		D			С			С			В	
Queue Length 50th (m)	36.7	63.0		7.8	31.1			44.6	0.0		38.4	0.0
Queue Length 95th (m)	#62.2	83.2		17.0	46.4			#87.6	3.2		69.6	9.3
Internal Link Dist (m)		111.1			140.7			99.1			107.3	
Turn Bay Length (m)	30.0			25.0					15.0			15.0
Base Capacity (vph)	351	753		196	532			598	613		602	594
Starvation Cap Reductn	0	0		0	0			0	0		0	0
Spillback Cap Reductn	0	0		0	0			0	0		0	0
Storage Cap Reductn	0	0		0	0			0	0		0	0
Reduced v/c Ratio	0.93	0.62		0.30	0.44			0.61	0.19		0.54	0.26
Intersection Summary	Other											

Other Area Type: Cycle Length: 80.1 Actuated Cycle Length: 80.1 Offset: 43 (54%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green Natural Cycle: 75 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.93 Intersection Signal Delay: 30.8 Intersection LOS: C Intersection Capacity Utilization 91.2% ICU Level of Service F Analysis Period (min) 15 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. #

Splits and Phases: 6: Churchill Avenue & Ricmond Rd

5 s 30 s	5s 40.1s
	▶ <sub>Ø7</sub> <b>▼</b> <sub>Ø8</sub>
5 s 30 s	14s 31.1s

Lane Group	Ø1	Ø3	Ø5
All-Red Time (s)	0.0	0.0	0.0
Lost Time Adjust (s)			
Total Lost Time (s)			
Lead/Lag	Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0
Recall Mode	Min	Min	Min
Walk Time (s)			
Flash Dont Walk (s)			
Pedestrian Calls (#/hr)			
Act Effct Green (s)			
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (m)			
Queue Length 95th (m)			
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			

# 9: Highcroft Avenue & Byron Avenue AM Peak

	-	$\mathbf{r}$	<	-	•	1
Movement	EBT	EBR	• WBL	WBT	• NBL	• NBR
Lane Configurations				4	¥	
Traffic Volume (veh/h)	<b>1</b> 225	1	2	272	5	20
Future Volume (Veh/h)	225	1	2	272	5	20
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	250	1	2	302	6	22
Pedestrians					10	
Lane Width (m)					3.7	
Walking Speed (m/s)					1.1	
Percent Blockage					1	
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (m)	85					
pX, platoon unblocked			0.93		0.93	0.93
vC, conflicting volume			261		566	260
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			164		493	163
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		99	97
cM capacity (veh/h)			1310		494	814
Direction. Lane #	EB 1	WB 1	NB 1			
Volume Total	251	304	28			
Volume Left	0	2	6			
Volume Right	1	0	22			
cSH	1700	1310	715			
Volume to Capacity	0.15	0.00	0.04			
Queue Length 95th (m)	0.0	0.0	0.9			
Control Delay (s)	0.0	0.1	10.2			
Lane LOS	0.0	A	B			
Approach Delay (s)	0.0	0.1	10.2			
Approach LOS	0.0	0.1	B			
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utilization			26.8%	ICI	J Level of S	ervice
Analysis Period (min)			15			
			15			

# 3: Churchill Avenue & Byron Avenue PM Peak

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>1</b> 37			<b>4</b> 360		٦.	ĥ		5	î,	
Traffic Volume (vph)	45		61	95		75	33	298	94	28	321	69
Future Volume (vph)	45	137	61	95	360	75	33	298	94	28	321	69
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	0.0		0.0	15.0		0.0	15.0		0.0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (m)	7.6	4.00	4.00	7.6	1.00	4.00	25.0	4.00	4.00	25.0	4.00	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.99			0.99		0.98	0.98		0.99	0.99	_
Frt Elt Desta stard		0.966			0.981		0.050	0.964		0.050	0.973	
Fit Protected	0	0.991	0	٥	0.991	^	0.950	4000	0	0.950	4007	0
Satd. Flow (prot) Flt Permitted	0	1723 0.816	0	0	1752 0.874	0	1616 0.391	1663	0	1729 0.390	1667	0
Satd. Flow (perm)	0	1417	0	0	1543	0	652	1663	0	699	1667	0
Right Turn on Red	0	1417	Yes	0	1040	Yes	002	1005	Yes	099	1007	Yes
Satd. Flow (RTOR)		24	163		12	163		22	163		15	163
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		207.2			85.0			153.8			123.1	
Travel Time (s)		14.9			6.1			11.1			8.9	
Confl. Peds. (#/hr)	14		8	8	•	14	21		16	16	0.0	21
Confl. Bikes (#/hr)			11	-		6			14			6
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	0%	0%	0%	0%	2%	7%	5%	0%	0%	5%	4%
Adj. Flow (vph)	50	152	68	106	400	83	37	331	104	31	357	77
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	270	0	0	589	0	37	435	0	31	434	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.7			3.7	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.9			4.9			4.9			4.9	
Two way Left Turn Lane	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
Headway Factor	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
Turning Speed (k/h)	24 1	2	14	24 1	2	14	24 1	2	14	24 1	2	14
Number of Detectors Detector Template	Left	Z Thru		Left	Z Thru		Left	Z Thru		Left	Z	
	6.1	30.5		6.1	30.5		6.1	30.5		6.1	30.5	
Leading Detector (m) Trailing Detector (m)	0.0	0.0		0.1	0.0		0.1	0.0		0.1	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8		6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex		CI+Ex	Cl+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel	OT EX	OI' EX		OI · EX	OI! EX		OI · EX	OI · EX		OI' EX	OI' EX	
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	30.6	30.6		30.6	30.6		25.5	25.5		25.5	25.5	
Total Split (s)	45.0	45.0		45.0	45.0		45.0	45.0		45.0	45.0	
Total Split (%) Maximum Green (s) Yellow Time (s)	50.0% 39.4 3.3	50.0% 39.4 3.3		50.0% 39.4 3.3	50.0% 39.4 3.3		50.0% 39.6 3.3	50.0% 39.6 3.3		50.0% 39.6 3.3	Ę	50.0% 39.6 3.3

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# 3: Churchill Avenue & Byron Avenue PM Peak

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
All-Red Time (s)	2.3	2.3		2.3	2.3		2.1	2.1		2.1	2.1	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6		5.4	5.4		5.4	5.4	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		9.0	9.0		9.0	9.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0		0	0	
Act Effct Green (s)		36.9			36.9		42.1	42.1		42.1	42.1	
Actuated g/C Ratio		0.41			0.41		0.47	0.47		0.47	0.47	
v/c Ratio		0.45			0.92		0.12	0.55		0.09	0.55	
Control Delay		19.6			46.3		16.3	20.3		16.0	21.7	
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.6	
Total Delay		19.6			46.3		16.3	20.3		16.0	22.2	
LOS		В			D		В	С		В	С	
Approach Delay		19.6			46.3			20.0			21.8	
Approach LOS		В			D			В			С	
Queue Length 50th (m)		28.3			87.7		3.7	52.1		3.5	47.7	
Queue Length 95th (m)		48.4			#150.4		9.8	81.5		m6.2	m63.1	
Internal Link Dist (m)		183.2			61.0			129.8			99.1	
Turn Bay Length (m)							15.0			15.0		
Base Capacity (vph)		633			682		304	790		327	788	
Starvation Cap Reductn		0			0		0	0		0	111	
Spillback Cap Reductn		0			0		0	0		0	0	
Storage Cap Reductn		0			0		0	0		0	0	
Reduced v/c Ratio		0.43			0.86		0.12	0.55		0.09	0.64	
Intersection Summary	0.4											
Area Type:	Other											
Cycle Length: 90												
Actuated Cycle Length: 90			<u> </u>									
Offset: 40 (44%), Referenced to p	phase 2:NBTL a	nd 6:SBTL,	Start of Gre	en								
Natural Cycle: 60	4. J											
Control Type: Actuated-Coordina Maximum v/c Ratio: 0.92	lied											
Intersection Signal Delay: 29.0				Int	ersection LC	S C						
Intersection Capacity Utilization 7	78.6%				U Level of Se							
Analysis Period (min) 15	0.070											
# 95th percentile volume excee	ds canacity our	nie may he	longer									
Queue shown is maximum aft		ao may be	iongoi.									
m Volume for 95th percentile a		hy unstream	n signal									

m Volume for 95th percentile queue is metered by upstream signal.

#### Splits and Phases: 3: Churchill Avenue & Byron Avenue

Ø2 (R)	<u>↓</u> <sub>04</sub>
45 s	45 s
Ø6 (R)	₩ Ø8
45 s	45 s

# 6: Churchill Avenue & Ricmond Rd PM Peak

	٨	<b>→</b>	$\mathbf{r}$	4	+	*	1	1	1	1	Ļ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	î,		1	ĥ			ส์	1		<u>ل</u> اً	1
Traffic Volume (vph)	191	329	55	129	479	25	27	291	67	15	237	312
Future Volume (vph)	191	329	55	129	479	25	27	291	67	15	237	312
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	30.0		0.0	25.0		0.0	15.0		15.0	15.0		15.0
Storage Lanes	1		0	1		0	0		1	0		1
Taper Length (m)	45.0			30.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.98	0.98		0.95	0.99			1.00	0.89		1.00	0.87
Frt		0.979			0.992				0.850			0.850
Flt Protected	0.950			0.950				0.996			0.997	
Satd. Flow (prot)	1679	1740	0	1586	1710	0	0	1722	1517	0	1706	1488
Flt Permitted	0.170			0.514				0.871			0.821	
Satd. Flow (perm)	294	1740	0	818	1710	0	0	1499	1354	0	1402	1300
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		15			4				160			186
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		135.1			164.7			123.1			131.3	
Travel Time (s)		9.7			11.9			8.9			9.5	
Confl. Peds. (#/hr)	50		47	47		50	38		30	30		38
Confl. Bikes (#/hr)			6			4			3			1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	3%	1%	0%	9%	5%	5%	8%	5%	2%	12%	6%	4%
Adj. Flow (vph)	212	366	61	143	532	28	30	323	74	17	263	347
Shared Lane Traffic (%)												
Lane Group Flow (vph)	212	427	0	143	560	0	0	353	74	0	280	347
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.7			3.7			3.7			3.7	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.9			4.9			4.9			4.9	
Two way Left Turn Lane												
Headway Factor	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2		1	2	1	1	2	1
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5		6.1	30.5	6.1	6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8		6.1	1.8	6.1	6.1	1.8	6.1
Detector 1 Type	CI+Ex	CI+Ex		CI+Ex	CI+Ex		Cl+Ex	CI+Ex	CI+Ex	Cl+Ex	CI+Ex	CI+Ex
Detector 1 Channel		0.0		0.0	0.0		0.0	0.0	0.0	0.0		0.0
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel		0.0			0.0			0.0			• •	
Detector 2 Extend (s)		0.0		_	0.0		<b>_</b>	0.0	<b>_</b>	<b>_</b>	0.0	<b>_</b>
Turn Type	pm+pt	NA		Perm	NA		Perm	NA	Perm	Perm	NA	Perm
Protected Phases	7	4		0	8		0	2	~	^	6	
Permitted Phases	4			8	•		2	•	2	6	^	6
Detector Phase	7	4		8	8		2	2	2	6	6	6
Switch Phase		40.0		40.0	40.0		10.0	10.0	40.0	40.0	40.0	40.0
Minimum Initial (s)	5.0	10.0		10.0	10.0		10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	11.1	31.1		31.1	31.1		24.2	24.2	24.2	24.2	24.2	24.2
Total Split (s)	15.0	55.0		45.0	45.0		25.0	25.0	25.0	25.0	25.0	25.0
Total Split (%)	16.7%	61.1%		50.0%	50.0%		27.8%	27.8%	27.8%	27.8%	27.8%	27.8%
Maximum Green (s)	8.9	48.9		38.9	38.9		18.8	18.8	18.8	18.8	18.8	18.8
Yellow Time (s)	3.3	3.3		3.3	3.3		3.6	3.6	3.6	3.6	3.6	3.6

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Lane Group	Ø1	Ø3	Ø5
Lane 🕶 nfigurations			
Traffic Volume (vph)			
Future Volume (vph)			
Ideal Flow (vphpl)			
Storage Length (m)			
Storage Lanes			
Taper Length (m)			
Lane Util. Factor			
Ped Bike Factor			
Frt			
Flt Protected			
Satd. Flow (prot)			
Flt Permitted			
Satd. Flow (perm)			
Right Turn on Red			
Satd. Flow (RTOR)			
Link Speed (k/h)			
Link Distance (m)			
Travel Time (s)			
Confl. Peds. (#/hr)			
Confl. Bikes (#/hr)			
Peak Hour Factor			
Heavy Vehicles (%)			
Adj. Flow (vph)			
Shared Lane Traffic (%)			
Lane Group Flow (vph)			
Enter Blocked Intersection			
Lane Alignment			
Median Width(m)			
Link Offset(m)			
Crosswalk Width(m)			
Two way Left Turn Lane			
Headway Factor			
Turning Speed (k/h)			
Number of Detectors			
Detector Template			
Leading Detector (m)			
Trailing Detector (m)			
Detector 1 Position(m)			
Detector 1 Size(m)			
Detector 1 Type			
Detector 1 Channel			
Detector 1 Extend (s)			
Detector 1 Queue (s)			
Detector 1 Delay (s)			
Detector 2 Position(m)			
Detector 2 Size(m)			
Detector 2 Type			
Detector 2 Channel			
Detector 2 Extend (s)			
Turn Type			
Protected Phases	1	3	5
Permitted Phases			
Detector Phase			
Switch Phase			
Minimum Initial (s)	3.0	3.0	3.0
Minimum Split (s)	5.0	5.0	5.0
Total Split (s)	5.0	5.0	5.0
Total Split (%)	6%	6%	6%
Maximum Green (s)	3.0	3.0	3.0
Yellow Time (s)	2.0	2.0	2.0

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#### 6: Churchill Avenue & Ricmond Rd PM Peak

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
All-Red Time (s)	2.8	2.8		2.8	2.8		2.6	2.6	2.6	2.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0		0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1			6.2	6.2		6.2	6.2
Lead/Lag	Lead	Lag		Lag	Lag		Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None		None	None		C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)		16.0		16.0	16.0		7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		9.0		9.0	9.0		9.0	9.0	9.0	9.0	9.0	9.0
Pedestrian Calls (#/hr)		15		15	15		15	15	15	15	15	15
Act Effct Green (s)	49.2	44.2		34.2	34.2			22.0	22.0		22.0	22.0
Actuated g/C Ratio	0.55	0.49		0.38	0.38			0.24	0.24		0.24	0.24
v/c Ratio	0.71	0.50		0.46	0.86			0.96	0.16		0.82	0.76
Control Delay	24.9	16.3		25.1	39.1			72.2	4.7		55.2	27.9
Queue Delay	0.0	0.0		0.0	0.0			0.0	0.0		0.0	0.0
Total Delay	24.9	16.3		25.1	39.1			72.2	4.7		55.2	27.9
LOS	С	В		С	D			E	А		E	С
Approach Delay		19.1			36.2			60.5			40.1	
Approach LOS		В			D			E			D	
Queue Length 50th (m)	16.9	43.5		17.5	83.6			~71.9	1.0		47.9	26.9
Queue Length 95th (m)	#28.6	62.2		32.4	119.2			m#119.2	m4.5		#95.4	#73.5
Internal Link Dist (m)		111.1			140.7			99.1			107.3	
Turn Bay Length (m)	30.0			25.0					15.0			15.0
Base Capacity (vph)	297	952		353	741			366	452		342	458
Starvation Cap Reductn	0	0		0	0			0	0		0	0
Spillback Cap Reductn	0	0		0	0			0	0		0	0
Storage Cap Reductn	0	0		0	0			0	0		0	0
Reduced v/c Ratio	0.71	0.45		0.41	0.76			0.96	0.16		0.82	0.76
Intersection Summary												
Area Type:	Other											
Cycle Length: 90												
Actuated Cycle Length: 90												
Offset: 0 (0%), Referenced to Natural Cycle: 90	phase 2:NBTL and	16:SBTL, St	art of Greer	1								
Control Type: Actuated-Coord	dinated											
Maximum v/c Ratio: 0.96												
	•											_

Intersection Signal Delay: 37.0 Intersection Capacity Utilization 91.9% Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles. m Volume for 95th percentile queue is metered by upstream signal.

#### Splits and Phases: 6: Churchill Avenue & Ricmond Rd

1 g 1 g 2 (R)	<b>k</b> k <sub>Ø3</sub> - <sup>▲</sup> <sub>Ø4</sub>
5 s 25 s	5 s 55 s
5 s 25 s	15 s 45 s

Intersection LOS: D

ICU Level of Service F

Lane Group	Ø1	Ø3	Ø5
All-Red Time (s)	0.0	0.0	0.0
Lost Time Adjust (s)			
Total Lost Time (s)			
Lead/Lag	Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0
Recall Mode	Min	Min	Min
Walk Time (s)			
Flash Dont Walk (s)			
Pedestrian Calls (#/hr)			
Act Effct Green (s)			
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (m)			
Queue Length 95th (m)			
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			

# 9: Highcroft Avenue & Byron Avenue PM Peak

	-	$\mathbf{r}$	1	-	1	1
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	τ.			ਵੀ	W.	
Traffic Volume (veh/h)	<b>1</b> 233	0	1	589	1	8
Future Volume (Veh/h)	233	0	1	589	1	8
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	259	0	1	654	1	9
Pedestrians	200	Ū		001	10	Ű
Lane Width (m)					3.7	
Walking Speed (m/s)					1.1	
Percent Blockage					1	
Right turn flare (veh)						
Median type	None			None		
Median storage veh)	NULLE			NUNC		
Upstream signal (m)	85					
pX, platoon unblocked	05					
vC, conflicting volume			269		925	269
vC1, stage 1 conf vol			209		920	209
vC1, stage 1 conf vol						
vC2, stage 2 cont vol			269		925	269
			269 4.1		925 6.4	269 6.2
tC, single (s)			4.1		0.4	0.2
tC, 2 stage (s)			2.2		2 5	3.3
tF (s)					3.5	
p0 queue free %			100		100	99
cM capacity (veh/h)			1294		298	767
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	259	655	10			
Volume Left	0	1	1			
Volume Right	0	0	9			
cSH	1700	1294	663			
Volume to Capacity	0.15	0.00	0.02			
Queue Length 95th (m)	0.0	0.0	0.3			
Control Delay (s)	0.0	0.0	10.5			
Lane LOS		А	В			
Approach Delay (s)	0.0	0.0	10.5			
Approach LOS			В			
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utilization			43.6%	ICI	J Level of S	ervice
Analysis Period (min)			15	10		
			10			

# 3: Churchill Avenue & Byron Avenue AM Peak

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		1	ĥ		2	ĥ	
Traffic Volume (vph)	36	171	46	40	<b>4</b> 142	64	29	331	42	37	287	24
Future Volume (vph)	36	171	46	40	142	64	29	331	42	37	287	24
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	0.0		0.0	15.0		0.0	15.0		0.0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (m)	7.6			7.6			25.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.99			0.99		0.97	0.99		0.98	0.99	
Frt		0.975			0.965			0.983			0.988	
Flt Protected		0.993			0.992		0.950			0.950		
Satd. Flow (prot)	0	1748	0	0	1712	0	1616	1701	0	1729	1705	0
Flt Permitted		0.889			0.857		0.566			0.519		
Satd. Flow (perm)	0	1562	0	0	1477	0	938	1701	0	929	1705	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		17			27			11			7	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		207.2			85.0			153.8			123.1	
Travel Time (s)		14.9	•	•	6.1			11.1	10	10	8.9	0.1
Confl. Peds. (#/hr)	14		8	8		14	21		16	16		21
Confl. Bikes (#/hr)			11			6			14			6
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	0%	0%	0%	0%	0%	2%	7%	5%	0%	0%	5%	4%
Adj. Flow (vph)	36	171	46	40	142	64	29	331	42	37	287	24
Shared Lane Traffic (%)	•	050	0	0	0.40	•	00	070	0	07	044	•
Lane Group Flow (vph)	0	253	0	0	246	0	29	373	0	37	311	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.7			3.7	
Link Offset(m) Crosswalk Width(m)		0.0 4.9			0.0 4.9			0.0 4.9			0.0 4.9	
		4.9			4.9			4.9			4.9	
Two way Left Turn Lane	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
Headway Factor	24	1.00	1.06	24	1.00	1.06	24	1.00	1.06	24	1.00	1.06
Turning Speed (k/h) Number of Detectors	24	2	14	24	2	14	24	2	14	24	2	14
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5		6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.1	0.0		0.1	0.0		0.0	0.0		0.1	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8		6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex		CI+Ex	CI+Ex		Cl+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel	OITEX			OILX				OFLX		OFLX	OFLX	
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)	0.0	28.7		0.0	28.7		0.0	28.7		0.0	28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel		0. 2.			0. 2.4			0. 2.4			0. 2.	
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8	•		2	_		6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase				-			_	_		-		
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	30.6	30.6		30.6	30.6		25.5	25.5		25.5	25.5	
Total Split (s)	38.0	38.0		38.0	38.0		42.0	42.0		42.0	42.0	
Total Split (%)	47.5%	47.5%		47.5%	47.5%		52.5%	52.5%		52.5%	52.5%	
Maximum Green (s)	32.4	32.4		32.4	32.4		36.6	36.6		36.6	36.6	

Brad Byvelds, Novatech

# 3: Churchill Avenue & Byron Avenue AM Peak

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
All-Red Time (s)	2.3	2.3		2.3	2.3		2.1	2.1		2.1	2.1	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6		5.4	5.4		5.4	5.4	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		9.0	9.0		9.0	9.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0		0	0	
Act Effct Green (s)		17.1			17.1		51.9	51.9		51.9	51.9	
Actuated g/C Ratio		0.21			0.21		0.65	0.65		0.65	0.65	
v/c Ratio		0.73			0.73		0.05	0.34		0.06	0.28	
Control Delay		38.9			38.1		7.0	8.2		7.1	7.7	
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.0	
Total Delay		38.9			38.1		7.0	8.2		7.1	7.7	
LOS		D			D		А	А		А	А	
Approach Delay		38.9			38.1			8.1			7.6	
Approach LOS		D			D			А			A	
Queue Length 50th (m)		33.5			31.1		1.4	21.5		1.8	17.4	
Queue Length 95th (m)		51.5			49.2		5.3	45.5		6.4	37.3	
Internal Link Dist (m)		183.2			61.0			129.8			99.1	
Turn Bay Length (m)							15.0			15.0		
Base Capacity (vph)		642			614		608	1106		602	1108	
Starvation Cap Reductn		0			0		0	0		0	0	
Spillback Cap Reductn		0			0		0	0		0	0	
Storage Cap Reductn		0			0		0	0		0	0	
Reduced v/c Ratio		0.39			0.40		0.05	0.34		0.06	0.28	
Intersection Summary												
Area Type:	Other											
Cycle Length: 80												
Actuated Cycle Length: 80												
Offset: 74 (93%), Referenced to	phase 2:NBTL a	nd 6:SBTL,	Start of Gre	een								
Natural Cycle: 60												
Control Type: Actuated-Coordina	ated											
Maximum v/c Ratio: 0.73												
ntersection Signal Delay: 20.1					ersection LC							
ntersection Capacity Utilization 6	62.2%			IC	J Level of S	ervice B						
Analysis Period (min) 15												
Splits and Phases: 3: Churchil	I Avenue & Byro	n Avenue										
						<b>A</b>						

Ø2 (R)	<u>⊿_</u>	
42 s	38 s	
Ø6 (R)	<b>₩</b> Ø8	
42 s	38 s	

#### 6: Churchill Avenue & Ricmond Rd AM Peak

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	5	1,		5	ĥ			្ឋ	1		<del>ب</del> ا ا	1
Traffic Volume (vph)	295	387	37	53	203	21	26	<b>4</b> 301	103	17	274	139
Future Volume (vph)	295	387	37	53	203	21	26	301	103	17	274	139
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	30.0		0.0	25.0		0.0	15.0		15.0	15.0		15.0
Storage Lanes	1		0	1		0	0		1	0		1
Taper Length (m)	45.0			30.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.95	0.99		0.96	0.99			1.00	0.90		1.00	0.89
Frt		0.987			0.986				0.850			0.850
Flt Protected	0.950			0.950				0.996			0.997	
Satd. Flow (prot)	1679	1765	0	1586	1693	0	0	1722	1517	0	1706	1488
Flt Permitted	0.373			0.462				0.957			0.970	
Satd. Flow (perm)	626	1765	0	739	1693	0	0	1648	1369	0	1656	1317
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		7			7				180			180
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		135.1			164.7			123.1			131.3	
Travel Time (s)		9.7			11.9			8.9			9.5	
Confl. Peds. (#/hr)	50		47	47		50	38		30	30		38
Confl. Bikes (#/hr)			6			4			3			1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	1%	0%	9%	5%	5%	8%	5%	2%	12%	6%	4%
Adj. Flow (vph)	295	387	37	53	203	21	26	301	103	17	274	139
Shared Lane Traffic (%)												
Lane Group Flow (vph)	295	424	0	53	224	0	0	327	103	0	291	139
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.7			3.7			3.7			3.7	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.9			4.9			4.9			4.9	
Two way Left Turn Lane	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
Headway Factor	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
Turning Speed (k/h)	24	0	14	24	0	14	24	0	14	24	0	14
Number of Detectors	1	2		1	2		1	2	1	1	2	1
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5		6.1	30.5	6.1	6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0 6.1	0.0 1.8		0.0 6.1	0.0 1.8		0.0 6.1	0.0 1.8	0.0 6.1	0.0 6.1	0.0 1.8	0.0 6.1
Detector 1 Size(m)	CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Type Detector 1 Channel	CI+EX	CI+EX		UI+EX	CI+EX		CI+EX	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX
	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s) Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	0.0	28.7		0.0	28.7		0.0	28.7	0.0	0.0	28.7	0.0
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA		Perm	NA	Perm	Perm	NA	Perm
Protected Phases	7	4		I CIIII	8		I CIIII	2	I CIIII	I CIIII	6	I CIIII
Permitted Phases	4	4		8	0		2	2	2	6	U	6
Detector Phase	4	4		8	8		2	2	2	6	6	6
Switch Phase		4		0	0		2	2	2	0	0	0
Minimum Initial (s)	5.0	10.0		10.0	10.0		10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	11.1	31.1		31.1	31.1		24.2	24.2	24.2	24.2	24.2	24.2
Total Split (s)	14.0	40.1		31.1	31.1		30.0	30.0	30.0	30.0	30.0	30.0
								37.5%		37.5%	37.5%	37.5%
	17 5%	50.1%		.18 X%	.10 0%		.1/ ""^					
Total Split (%) Maximum Green (s)	17.5% 7.9	50.1% 34.0		38.8% 25.0	38.8% 25.0		37.5% 23.8	23.8	37.5% 23.8	23.8	23.8	23.8

Brad Byvelds, Novatech

Lane Group	Ø1	Ø3	Ø5
Lane 🕶 nfigurations			
Traffic Volume (vph)			
Future Volume (vph)			
Ideal Flow (vphpl)			
Storage Length (m)			
Storage Lanes			
Taper Length (m)			
Lane Util. Factor			
Ped Bike Factor			
Frt			
Flt Protected			
Satd. Flow (prot)			
Flt Permitted			
Satd. Flow (perm)			
Right Turn on Red			
Satd. Flow (RTOR)			
Link Speed (k/h)			
Link Distance (m)			
Travel Time (s)			
Confl. Peds. (#/hr)			
Confl. Bikes (#/hr)			
Peak Hour Factor			
Heavy Vehicles (%)			
Adj. Flow (vph)			
Shared Lane Traffic (%)			
Lane Group Flow (vph)			
Enter Blocked Intersection			
Lane Alignment			
Median Width(m)			
Link Offset(m)			
Crosswalk Width(m)			
Two way Left Turn Lane			
Headway Factor			
Turning Speed (k/h)			
Number of Detectors			
Detector Template			
Leading Detector (m)			
Trailing Detector (m)			
Detector 1 Position(m)			
Detector 1 Size(m)			
Detector 1 Type			
Detector 1 Channel			
Detector 1 Extend (s)			
Detector 1 Queue (s)			
Detector 1 Delay (s)			
Detector 2 Position(m)			
Detector 2 Size(m)			
Detector 2 Type			
Detector 2 Channel			
Detector 2 Extend (s)			
Turn Type			
Protected Phases	1	3	5
Permitted Phases			
Detector Phase			
Switch Phase			
Minimum Initial (s)	3.0	3.0	3.0
Minimum Split (s)	5.0	5.0	5.0
Total Split (s)	5.0	5.0	5.0
Total Split (%)	6%	6%	6%
Maximum Green (s)	3.0	3.0	3.0
Yellow Time (s)	2.0	2.0	2.0

Brad Byvelds, Novatech

#### 6: Churchill Avenue & Ricmond Rd AM Peak

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Lane Group	EBL	EBT	EBR	- WBL	WBT	WBR	NBL	NBT	- NBR	SBL	SBT	SBR
All-Red Time (s)	2.8	2.8		2.8	2.8		2.6	2.6	2.6	2.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0		0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1			6.2	6.2		6.2	6.2
Lead/Lag	Lead	Lag		Lag	Lag		Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None		None	None		C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)		16.0		16.0	16.0		7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		9.0		9.0	9.0		9.0	9.0	9.0	9.0	9.0	9.0
Pedestrian Calls (#/hr)		15		15	15		15	15	15	15	15	15
Act Effct Green (s)	30.6	25.6		16.6	16.6			30.2	30.2		30.2	30.2
Actuated g/C Ratio	0.38	0.32		0.21	0.21			0.38	0.38		0.38	0.38
v/c Ratio	0.86	0.75		0.35	0.63			0.53	0.16		0.47	0.23
Control Delay	43.6	31.8		31.2	34.9			24.8	1.0		23.5	2.7
Queue Delay	0.0	0.0		0.0	0.0			0.0	0.0		0.0	0.0
Total Delay	43.6	31.8		31.2	34.9			24.8	1.0		23.5	2.7
LOS	D	С		С	С			С	А		С	A
Approach Delay		36.6			34.2			19.1			16.8	
Approach LOS		D			С			В			В	
Queue Length 50th (m)	33.7	57.2		7.0	30.8			37.4	0.0		32.4	0.0
Queue Length 95th (m)	#49.0	73.4		14.9	44.8			70.5	1.5		61.9	6.9
Internal Link Dist (m)		111.1			140.7			99.1			107.3	
Turn Bay Length (m)	30.0			25.0					15.0			15.0
Base Capacity (vph)	343	753		230	533			620	627		623	608
Starvation Cap Reductn	0	0		0	0			0	0		0	0
Spillback Cap Reductn	0	0		0	0			0	0		0	0
Storage Cap Reductn	0	0		0	0			0	0		0	0
Reduced v/c Ratio	0.86	0.56		0.23	0.42			0.53	0.16		0.47	0.23
Intersection Summary	Other											

Area Type: Cycle Length: 80.1 Other Actuated Cycle Length: 80.1 Offset: 43 (54%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green Natural Cycle: 75 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.86 Intersection Signal Delay: 27.6 Intersection Capacity Utilization 91.4% Analysis Period (min) 15 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. #

#### Splits and Phases: 6: Churchill Avenue & Ricmond Rd

	<u>∦k<sub>Ø3</sub> →<sub>Ø4</sub></u>
5 s 30 s	5s 40.1s
	▶ <sub>Ø7</sub> ₩ <sub>Ø8</sub>
5 s 30 s	14s 31.1s

Intersection LOS: C

ICU Level of Service F

Lane Group	Ø1	Ø3	Ø5
All-Red Time (s)	0.0	0.0	0.0
Lost Time Adjust (s)			
Total Lost Time (s)			
Lead/Lag	Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0
Recall Mode	Min	Min	Min
Walk Time (s)			
Flash Dont Walk (s)			
Pedestrian Calls (#/hr)			
Act Effct Green (s)			
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (m)			
Queue Length 95th (m)			
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			

# 9: Highcroft Avenue & Byron Avenue AM Peak

	-	$\mathbf{i}$	-	-	•	/
Movement	EBT	EBR	WBL	WBT	NBL	• NBR
Lane Configurations	1,	LBIX	IIDE	4	¥	NDIX.
Traffic Volume (veh/h)	225	1	2	272	5	20
Future Volume (Veh/h)	225	1	2	272	5	20
Sign Control	Free		_	Free	Stop	20
Grade	0%			0%	0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	225	1.00	2	272	5	20
Pedestrians	220		-	212	10	20
Lane Width (m)					3.7	
Walking Speed (m/s)					1.1	
Percent Blockage					1	
Right turn flare (veh)						
Median type	None			None		
Median storage veh)	None			None		
Upstream signal (m)	85					
pX, platoon unblocked	00		0.94		0.94	0.94
vC, conflicting volume			236		512	236
vC1, stage 1 conf vol			200		512	200
vC2, stage 2 conf vol						
vCu, unblocked vol			157		449	156
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)			4.1		0.4	0.2
tF (s)			2.2		3.5	3.3
p0 queue free %			100		99	98
cM capacity (veh/h)			1338		531	834
					551	004
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	226	274	25			
Volume Left	0	2	5			
Volume Right	1	0	20			
cSH	1700	1338	749			
Volume to Capacity	0.13	0.00	0.03			
Queue Length 95th (m)	0.0	0.0	0.8			
Control Delay (s)	0.0	0.1	10.0			
Lane LOS		А	А			
Approach Delay (s)	0.0	0.1	10.0			
Approach LOS			А			
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utilization			26.8%	IC	U Level of S	ervice
Analysis Period (min)			15			
			10			

# 3: Churchill Avenue & Byron Avenue PM Peak

EBL 45 1800 0.0 0 7.6 1.00 0 0 0 0 0 14 14	EBT 137 137 1800 1.00 0.99 0.966 0.991 1723 0.831 1443 24 50 207.2 14.9	EBR 61 61 1800 0.0 0 1.00 0 1.00 0 Yes	WBL 95 95 1800 0.0 0 7.6 1.00 0 0	WBT 360 360 1800 1.00 0.99 0.981 0.991 1752 0.886 1564 12	WBR 75 75 1800 0.0 0 1.00 1.00 0 0 Yes	NBL 33 33 1800 15.0 1 25.0 1.00 0.98 0.950 1616 0.446 742	NBT 298 298 1800 1.00 0.98 0.964 1662 1662	NBR 94 94 1800 0.0 0 1.00 0 0 Yes	SBL 28 28 1800 15.0 1 25.0 1.00 0.98 0.950 1729 0.444 794	SBT 321 321 1800 1.00 0.99 0.973 1667 1667	SBR 69 69 1800 0.0 0 1.00 0 0 Yes
45 1800 0.0 0 7.6 1.00 0 0 0 0	137 137 1800 1.00 0.99 0.966 0.991 1723 0.831 1443 24 50 207.2	61 1800 0.0 0 1.00 0 0	95 1800 0.0 7.6 1.00	360 360 1800 1.00 0.99 0.981 0.991 1752 0.886 1564 12	75 1800 0.0 0 1.00 0 0	33 33 1800 15.0 1 25.0 1.00 0.98 0.950 1616 0.446	298 298 1800 1.00 0.98 0.964 1662	94 1800 0.0 0 1.00 0 0	28 28 1800 15.0 1 25.0 1.00 0.98 0.950 1729 0.444	321 321 1800 1.00 0.99 0.973 1667	69 1800 0.0 0 1.00
45 1800 0.0 0 7.6 1.00 0 0 0 0	137 137 1800 1.00 0.99 0.966 0.991 1723 0.831 1443 24 50 207.2	61 1800 0.0 0 1.00 0 0	95 1800 0.0 7.6 1.00	360 360 1800 1.00 0.99 0.981 0.991 1752 0.886 1564 12	75 1800 0.0 0 1.00 0 0	33 1800 15.0 1 25.0 1.00 0.98 0.950 1616 0.446	298 298 1800 1.00 0.98 0.964 1662	94 1800 0.0 0 1.00 0 0	28 1800 15.0 1 25.0 1.00 0.98 0.950 1729 0.444	321 321 1800 1.00 0.99 0.973 1667	69 1800 0.0 0 1.00
1800 0.0 7.6 1.00 0 0 0 14 14	1800 1.00 0.99 0.966 0.991 1723 0.831 1443 24 50 207.2	1800 0.0 0 1.00 0 0	1800 0.0 7.6 1.00	1800 1.00 0.99 0.981 0.991 1752 0.886 1564 12	1800 0.0 0 1.00 0	1800 15.0 1 25.0 1.00 0.98 0.950 1616 0.446	1800 1.00 0.98 0.964 1662	1800 0.0 0 1.00 0 0	1800 15.0 1 25.0 1.00 0.98 0.950 1729 0.444	1800 1.00 0.99 0.973 1667	1800 0.0 0 1.00 0
0.0 0 7.6 1.00 0 0 0 14	1.00 0.99 0.966 0.991 1723 0.831 1443 24 50 207.2	0.0 0 1.00 0 0	0.0 0 7.6 1.00	1.00 0.99 0.981 0.991 1752 0.886 1564 12	0.0 0 1.00 0 0	15.0 1 25.0 1.00 0.98 0.950 1616 0.446	1.00 0.98 0.964 1662	0.0 0 1.00 0 0	15.0 1 25.0 1.00 0.98 0.950 1729 0.444	1.00 0.99 0.973 1667	0.0 0 1.00 0
0 7.6 1.00 0 0 0 14	0.99 0.966 0.991 1723 0.831 1443 24 50 207.2	0 1.00 0 0	0 7.6 1.00	0.99 0.981 0.991 1752 0.886 1564 12	0 1.00 0 0	1 25.0 1.00 0.98 0.950 1616 0.446	0.98 0.964 1662	0 1.00 0 0	1 25.0 1.00 0.98 0.950 1729 0.444	0.99 0.973 1667	0 1.00 0 0
7.6 1.00 0 0 14 1.00	0.99 0.966 0.991 1723 0.831 1443 24 50 207.2	1.00 0 0	7.6 1.00 0	0.99 0.981 0.991 1752 0.886 1564 12	1.00 0 0	25.0 1.00 0.98 0.950 1616 0.446	0.98 0.964 1662	1.00 0 0	25.0 1.00 0.98 0.950 1729 0.444	0.99 0.973 1667	1.00 0 0
1.00 0 0 14 1.00	0.99 0.966 0.991 1723 0.831 1443 24 50 207.2	0	1.00 0	0.99 0.981 0.991 1752 0.886 1564 12	0	1.00 0.98 0.950 1616 0.446	0.98 0.964 1662	0	1.00 0.98 0.950 1729 0.444	0.99 0.973 1667	0
0 0 14 1.00	0.99 0.966 0.991 1723 0.831 1443 24 50 207.2	0	0	0.99 0.981 0.991 1752 0.886 1564 12	0	0.98 0.950 1616 0.446	0.98 0.964 1662	0	0.98 0.950 1729 0.444	0.99 0.973 1667	0
0 14 1.00	0.966 0.991 1723 0.831 1443 24 50 207.2	0		0.981 0.991 1752 0.886 1564 12	0	0.950 1616 0.446	0.964 1662	0	0.950 1729 0.444	0.973 1667	0
0 14 1.00	0.991 1723 0.831 1443 24 50 207.2	0		0.991 1752 0.886 1564 12	0	1616 0.446	1662	0	1729 0.444	1667	0
0 14 1.00	1723 0.831 1443 24 50 207.2	0		1752 0.886 1564 12	0	1616 0.446		0	1729 0.444		0
0 14 1.00	0.831 1443 24 50 207.2	0		0.886 1564 12	0	0.446		0	0.444		0
14	1443 24 50 207.2		0	1564 12			1662			1667	
14	24 50 207.2		0	12		742	1662		794	1667	
1.00	50 207.2	Yes			Yes			Yes			Yes
1.00	50 207.2							100			100
1.00	207.2						23			15	
1.00				50			50			50	
1.00	14.9			85.0			153.8			123.1	
1.00				6.1			11.1			8.9	
		8	8		14	21		16	16		21
		11			6			14			6
<b>∩</b> 0/	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
U 70	0%	0%	0%	0%	2%	7%	5%	0%	0%	5%	4%
45	137	61	95	360	75	33	298	94	28	321	69
0	243	0	0	530	0	33	392	0	28	390	0
No		No	No		No			No			No
											Right
1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
											14
	2	••	1	2			2		1	2	
			-						-		
0. 2.4	0. 2.		01 2/	0. 2.4		0. 2.4	0. 2.		0. 2.	0. 2.4	
0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
0.0			0.0			0.0			0.0		
	OI' EX			OI! EX			OI LA			OI' EX	
	0.0			0.0			0.0			0.0	
Perm			Perm			Porm			Perm		
1 CIIII			T CHIII			T CITI			T CIIII		
Δ	4		8	0		2	2		6	0	
	1			8			2			6	
4	4		0	0		2	2		0	U	
10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
	45	45         137           0         243           No         No           Left         Left           0.0         0.0           0.0         4.9           1.06         1.06           24         1           1         2           Left         Thru           6.1         30.5           0.0         0.0           0.0         0.0           0.0         0.0           0.0         0.0           0.0         0.0           0.0         0.0           0.0         0.0           0.0         0.0           0.0         0.0           0.0         0.0           0.0         0.0           0.0         0.0           0.0         0.0           0.0         0.0           28.7         1.8           CI+Ex         0.0           Perm         NA           4         4           4         4           10.0         10.0           30.6         30.6           45.0         50.0%           39.4	45         137         61           0         243         0           No         No         No           Left         Left         Right           0.0         0.0           0.0         4.9           1.06         1.06         1.06           24         14         12           Left         Thru         6.1         30.5           0.0         0.0         0.0           0.0         0.0         0.0           0.0         0.0         0.0           0.0         0.0         0.0           0.0         0.0         0.0           0.0         0.0         0.0           0.0         0.0         0.0           0.0         0.0         0.0           0.0         0.0         0.0           0.0         0.0         0.0           0.0         0.0         0.0           0.0         0.0         0.0           0.0         0.0         0.0           0.0         0.0         0.0           0.0         0.0         0.0           0.0         0.0         0.0	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	45         137         61         95         360         75         33         298           0         243         0         0         530         0         33         392           No           Left         Left         Right         Left         Left         Right         Left         Left         Left         Left         Left         Left         Left         Left         Left         1.06	45       137       61       95       360       75       33       298       94         0       243       0       0       530       0       33       392       0         No       No	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$

Brad Byvelds, Novatech

# 3: Churchill Avenue & Byron Avenue PM Peak

	٦	+	$\mathbf{F}$	•	ł	•	1	1	1	1	ŧ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
All-Red Time (s)	2.3	2.3		2.3	2.3		2.1	2.1		2.1	2.1	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6		5.4	5.4		5.4	5.4	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		9.0	9.0		9.0	9.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0		0	0	
Act Effct Green (s)		34.4			34.4		44.6	44.6		44.6	44.6	
Actuated g/C Ratio		0.38			0.38		0.50	0.50		0.50	0.50	
v/c Ratio		0.43			0.88		0.09	0.47		0.07	0.47	
Control Delay		19.9			41.5		15.1	17.6		15.6	20.0	
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.4	
Total Delay		19.9			41.5		15.1	17.6		15.6	20.4	
LOS		В			D		В	В		В	С	
Approach Delay		19.9			41.5			17.4			20.0	
Approach LOS		В			D			В			С	
Queue Length 50th (m)		26.2			78.9		3.0	41.7		3.1	41.6	
Queue Length 95th (m)		42.6			#115.5		8.9	70.9		m6.8	60.5	
Internal Link Dist (m)		183.2			61.0			129.8			99.1	
Turn Bay Length (m)							15.0			15.0		
Base Capacity (vph)		645			691		367	835		393	834	
Starvation Cap Reductn		0			0		0	0		0	134	
Spillback Cap Reductn		0			0		0	0		0	0	
Storage Cap Reductn		0			0		0	0		0	0	
Reduced v/c Ratio		0.38			0.77		0.09	0.47		0.07	0.56	
Intersection Summary												
Area Type:	Other											
Cycle Length: 90												
Actuated Cycle Length: 90												
Offset: 40 (44%), Referenced t	o phase 2:NBTL a	nd 6:SBTL,	Start of Gre	en								
Natural Cycle: 60												
Control Type: Actuated-Coordi	nated											
Maximum v/c Ratio: 0.88												
Intersection Signal Delay: 26.4				Int	ersection LC	S: C						
Intersection Capacity Utilization	n 78.6%			IC	U Level of S	ervice D						
Analysis Period (min) 15												
# 95th percentile volume exc	eeds capacity, que	eue may be	longer.									
Queue shown is maximum												
m Volume for 95th percentile	auque in motored	hu unatroa	maianal									

m Volume for 95th percentile queue is metered by upstream signal.

#### Splits and Phases: 3: Churchill Avenue & Byron Avenue

Ø2 (R)	<u>→</u> <sub>Ø4</sub>
45 s	45 s
Ø6 (R)	₩ Ø8
45 s	45 s

### 6: Churchill Avenue & Ricmond Rd PM Peak

	٦	<b>→</b>	$\mathbf{F}$	4	+	*	1	Ť	1	1	Ŧ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	<u>۲</u>	1.		- <b>N</b>	1.			<b>4</b> 291	1		च	1
Traffic Volume (vph)	191	342	55	129	485	25	27		67	15	237	312
Future Volume (vph)	191	342	55	129	485	25	27	291	67	15	237	312
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	30.0		0.0	25.0		0.0	15.0		15.0	15.0		15.0
Storage Lanes	1		0	1		0	0		1	0		1
Taper Length (m)	45.0			30.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.97	0.99		0.95	0.99			0.99	0.89		1.00	0.87
Frt		0.979			0.993				0.850			0.850
Flt Protected	0.950			0.950				0.996			0.997	
Satd. Flow (prot)	1679	1740	0	1586	1712	0	0	1722	1517	0	1706	1488
Flt Permitted	0.193			0.528				0.953			0.967	
Satd. Flow (perm)	332	1740	0	839	1712	0	0	1639	1354	0	1651	1300
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		14			4				160			186
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		135.1			164.7			123.1			131.3	
Travel Time (s)		9.7			11.9			8.9			9.5	
Confl. Peds. (#/hr)	50		47	47		50	38		30	30		38
Confl. Bikes (#/hr)			6			4			3			1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	1%	0%	9%	5%	5%	8%	5%	2%	12%	6%	4%
Adj. Flow (vph)	191	342	55	129	485	25	27	291	67	15	237	312
Shared Lane Traffic (%)												
Lane Group Flow (vph)	191	397	0	129	510	0	0	318	67	0	252	312
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	2011	3.7		2011	3.7		2011	3.7		2011	3.7	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.9			4.9			4.9			4.9	
Two way Left Turn Lane												
Headway Factor	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2		1	2	1	1	2	1
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5		6.1	30.5	6.1	6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8		6.1	1.8	6.1	6.1	1.8	6.1
Detector 1 Type	CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel	OFEX			OFEX					OILEX	OILEX	OFER	OILX
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	0.0	28.7		0.0	28.7		0.0	28.7	0.0	0.0	28.7	0.0
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
· · · · · · · · · · · · · · · · · · ·		NA		Perm	NA		Dorm	NA	Dorm	Perm		Perm
Turn Type	pm+pt			Feilii			Perm		Perm	Feilii	NA	Feilii
Protected Phases Permitted Phases	7 4	4		8	8		0	2	2	6	6	C
	4	4		8	8		2	0		6	C	6
Detector Phase	- 1	4		ð	õ		2	2	2	6	6	6
Switch Phase	<b>F A</b>	10.0		40.0	40.0		10.0	10.0	40.0	40.0	40.0	40.0
Minimum Initial (s)	5.0	10.0		10.0	10.0		10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	11.1	31.1		31.1	31.1		24.2	24.2	24.2	24.2	24.2	24.2
Total Split (s)	15.0	55.0		45.0	45.0		25.0	25.0	25.0	25.0	25.0	25.0
Total Split (%)	16.7%	61.1%		50.0%	50.0%		27.8%	27.8%	27.8%	27.8%	27.8%	27.8%
Maximum Green (s)	8.9	48.9		38.9	38.9		18.8	18.8	18.8	18.8	18.8	18.8
Yellow Time (s)	3.3	3.3		3.3	3.3		3.6	3.6	3.6	3.6	3.6	3.6

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Lane Group	Ø1	Ø3	Ø5
Lane 🖚 nfigurations			
Traffic Volume (vph)			
Future Volume (vph)			
Ideal Flow (vphpl)			
Storage Length (m)			
Storage Lanes			
Taper Length (m)			
Lane Util. Factor			
Ped Bike Factor			
Frt			
Flt Protected			
Satd. Flow (prot)			
Flt Permitted			
Satd. Flow (perm)			
Right Turn on Red			
Satd. Flow (RTOR)			
Link Speed (k/h)			
Link Distance (m)			
Travel Time (s)			
Confl. Peds. (#/hr)			
Confl. Bikes (#/hr)			
Peak Hour Factor			
Heavy Vehicles (%)			
Adj. Flow (vph)			
Shared Lane Traffic (%)			
Lane Group Flow (vph)			
Enter Blocked Intersection			
Lane Alignment			
Median Width(m)			
Link Offset(m)			
Crosswalk Width(m)			
Two way Left Turn Lane			
Headway Factor			
Turning Speed (k/h)			
Number of Detectors			
Detector Template			
Leading Detector (m)			
Trailing Detector (m)			
Detector 1 Position(m)			
Detector 1 Size(m)			
Detector 1 Type			
Detector 1 Channel			
Detector 1 Extend (s)			
Detector 1 Queue (s)			
Detector 1 Delay (s)			
Detector 2 Position(m)			
Detector 2 Size(m)			
Detector 2 Type			
Detector 2 Channel			
Detector 2 Extend (s)			
Turn Type			
Protected Phases	1	3	5
Permitted Phases			
Detector Phase			
Switch Phase			
Minimum Initial (s)	3.0	3.0	3.0
Minimum Split (s)	5.0	5.0	5.0
Total Split (s)	5.0	5.0	5.0
Total Split (%)	6%	6%	6%
Maximum Green (s)	3.0	3.0	3.0
Yellow Time (s)	2.0	2.0	2.0
	2.0	2.0	2.0

Brad Byvelds, Novatech

## 6: Churchill Avenue & Ricmond Rd PM Peak

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
All-Red Time (s)	2.8	2.8		2.8	2.8		2.6	2.6	2.6	2.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0		0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1			6.2	6.2		6.2	6.2
Lead/Lag	Lead	Lag		Lag	Lag		Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None		None	None		C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)		16.0		16.0	16.0		7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		9.0		9.0	9.0		9.0	9.0	9.0	9.0	9.0	9.0
Pedestrian Calls (#/hr)		15		15	15		15	15	15	15	15	15
Act Effct Green (s)	47.3	42.3		32.3	32.3			23.6	23.6		23.6	23.6
Actuated g/C Ratio	0.53	0.47		0.36	0.36			0.26	0.26		0.26	0.26
v/c Ratio	0.62	0.48		0.43	0.83			0.74	0.14		0.58	0.65
Control Delay	19.7	17.0		25.2	37.7			44.6	4.3		37.6	20.9
Queue Delay	0.0	0.0		0.0	0.0			0.0	0.0		0.0	0.0
Total Delay	19.7	17.0		25.2	37.7			44.6	4.3		37.6	20.9
LOS	В	В		С	D			D	А		D	С
Approach Delay		17.9			35.2			37.6			28.4	
Approach LOS		В			D			D			С	
Queue Length 50th (m)	16.3	42.6		16.4	77.3			51.6	0.7		38.9	18.7
Queue Length 95th (m)	23.4	57.0		28.8	104.3			m#101.7	m4.9		#73.6	#58.2
Internal Link Dist (m)		111.1			140.7			99.1			107.3	
Turn Bay Length (m)	30.0			25.0					15.0			15.0
Base Capacity (vph)	307	951		362	742			429	472		432	477
Starvation Cap Reductn	0	0		0	0			0	0		0	0
Spillback Cap Reductn	0	0		0	0			0	0		0	0
Storage Cap Reductn	0	0		0	0			0	0		0	0
Reduced v/c Ratio	0.62	0.42		0.36	0.69			0.74	0.14		0.58	0.65

Intersection Summary		
Area Type: Other		
Cycle Length: 90		
Actuated Cycle Length: 90		
Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SE	3TL, Start of Green	
Natural Cycle: 75		
Control Type: Actuated-Coordinated		
Maximum v/c Ratio: 0.83		
Intersection Signal Delay: 29.2	Intersection LOS: C	
Intersection Capacity Utilization 92.2%	ICU Level of Service F	
Analysis Period (min) 15		
# 95th percentile volume exceeds capacity, queue r	nay be longer.	
Oucus about is maximum offer two evolus		

Queue shown is maximum after two cycles. m Volume for 95th percentile queue is metered by upstream signal.

#### Splits and Phases: 6: Churchill Avenue & Ricmond Rd

1 02 (R)	<b>k</b> k <sub>Ø3</sub> → <sub>Ø4</sub>
5 s 25 s	5 s 55 s
5 s 25 s	15 s 45 s

Lane Group	Ø1	Ø3	Ø5
All-Red Time (s)	0.0	0.0	0.0
Lost Time Adjust (s)			
Total Lost Time (s)			
Lead/Lag	Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0
Recall Mode	Min	Min	Min
Walk Time (s)			
Flash Dont Walk (s)			
Pedestrian Calls (#/hr)			
Act Effct Green (s)			
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (m)			
Queue Length 95th (m)			
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			

# 9: Highcroft Avenue & Byron Avenue PM Peak

	-	$\mathbf{r}$	1	+	•	1
Movement	EBT	EBR	• WBL	WBT	NBL	• NBR
Lane Configurations	τ.			ની	W.	
Traffic Volume (veh/h)	<b>1</b> 233	0	1	589	1	8
Future Volume (Veh/h)	233	0	1	589	1	8
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	233	0	1	589	1	8
Pedestrians	200	, i i i i i i i i i i i i i i i i i i i			10	Ŭ
Lane Width (m)					3.7	
Walking Speed (m/s)					1.1	
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)	None			None		
Upstream signal (m)	85					
pX, platoon unblocked	00					
vC, conflicting volume			243		834	243
vC1, stage 1 conf vol			243		0.04	245
vC2, stage 2 conf vol						
vCu, unblocked vol			243		834	243
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)			4.1		0.4	0.2
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	99
cM capacity (veh/h)			1323		337	793
,						100
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	233	590	9			
Volume Left	0	1	1			
Volume Right	0	0	8			
cSH	1700	1323	690			
Volume to Capacity	0.14	0.00	0.01			
Queue Length 95th (m)	0.0	0.0	0.3			
Control Delay (s)	0.0	0.0	10.3			
Lane LOS		А	В			
Approach Delay (s)	0.0	0.0	10.3			
Approach LOS			В			
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utilization			43.6%	IC	U Level of S	ervice
Analysis Period (min)			15			
· · · · · · · · · · · · · · · · · · ·						

# 3: Churchill Avenue & Byron Avenue AM Peak

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EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
	4			4		<b>N</b>	î.		5	۵.	
36	172	46	43	146	69	29	331	43	40	287	24
36	172	46	43	146	69	29	331	43	40	287	24
1800	1800	1800	1800	1800	1800		1800	1800	1800	1800	1800
0.0		0.0	0.0		0.0	15.0		0.0	15.0		0.0
0		0	0		0	1		0	1		0
									25.0		
1.00	1.00	1.00	1.00		1.00			1.00		1.00	1.00
						0.97			0.98		
							0.983			0.988	
0		0	0		0		1701	0		1705	0
0	1557		0	1464		936	1701		927	1705	0
		Yes			Yes			Yes			Yes
	14.9			6.1			11.1			8.9	
14			8			21			16		21
											6
											1.00
											4%
36	172	46	43	146	69	29	331	43	40	287	24
							-				0
											No
Left		Right	Left		Right	Left		Right	Left		Right
	4.9			4.9			4.9			4.9	
4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
	1.06			1.06			1.06			1.06	1.06
	0	14		0	14		0	14		0	14
									-		
UI+EX	CI+EX		CI+EX	CI+EX		CI+EX	CI+EX		CI+EX	CI+EX	
0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
0.0			0.0			0.0			0.0		
	0.0			0.0			0.0			0.0	
Dorm			Dorm			Dorm			Dorm		
reiiii			Feilii			renn			Feilii		
1	4		Q	0		2	2		6	0	
	Λ			Q			2			6	
4	4		0	0		2	2		0	0	
10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
17 50/	17 60/		17 60/	17 60/							
47.5% 32.4	47.5% 32.4		47.5% 32.4	47.5% 32.4		52.5% 36.6	52.5% 36.6		52.5% 36.6	52.5% 36.6	
	EBL 36 36 1800 0.0	EBL         EBT           36         172           36         172           1800         1800           0         0           7.6         1.00           1.00         1.00           0.993         0.976           0.993         0.1750           0.885         0           0         1557           17         50           207.2         14.9           14         1.00           0         254           No         No           Left         Left           0         254           No         No           Left         Left           0         254           No         No           Left         Left           0.0         0.0           0.0         0.0           0.0         0.0           0.1.06         1.06           24         1           2         Left           Thru         6.1           0.0         0.0           0.0         0.0           0.0         0.0           0.0 <td>EBL         EBT         EBR           36         172         46           36         172         46           1800         1800         1800           0.0         0.0         0.0           0         0.0         0.0           7.6        </td> <td>EBL         EBT         EBR         WBL           36         172         46         43           36         172         46         43           1800         1800         1800         1800           0.0         0.0         0.0         0.0           0.0         0.0         0.0         0.0           0.0         0.0         1.00         1.00           0.993         -         0         0           0.993         0         0         0           0.993         0         0         0           0.993         0         0         0           0.1557         0         0         0           207.2         -         -         14           14         8         8         11           1.00         1.00         1.00         1.00           0%         0%         0%         0%           14         8         8         11           1.00         1.00         1.00         1.00           0         254         0         0         0           No         No         No         No      &lt;</td> <td>EBL         EBT         EBR         WBL         WBT           36         172         46         43         146           36         172         46         43         146           1800         1800         1800         1800         1800         1800           0         0         0         0         0         0           7.6         7.6         7.6         1.00         1.00         1.00           0.99         0.99         0.99         0.992         0         1709         0.885         0.851           0         1557         0         0         1464         Yes         17         28         50         50         207.2         85.0         250         207.2         85.0         14.9         6.1         144         8         8         11         100         1.</td> <td>EBL         EBT         EBR         WBL         WBT         WBR           <math>36</math>         172         46         43         146         69           36         172         46         43         146         69           1800         1800         1800         1800         1800         1800           0.0         0.0         0.0         0.0         0.0           0         0.00         1.00         1.00         1.00         1.00           0.976         0.993         0.993         0.993         0.992           0         1750         0         0         1464         0           0         1557         0         0         1464         0           0         1557         0         0         1464         0           14.9         6.1         141         6         1         100           14.9         6.1         141         6         1         100           0         254         0         0         258         0           0         254         0         0         258         0           0         0         254         0&lt;</td> <td>EBL         EBT         EBR         WBL         WBT         WBR         NBL           36         172         46         43         146         69         29           36         172         46         43         146         69         29           1800         1800         1800         1800         1800         1800         1800           0         0         0         0         0         100         1.00         1.00           1.00         1.00         1.00         1.00         1.00         1.00         1.00           0.976         0.994         0.993         0.992         0.950           0         1750         0         0         1709         0         1616           0.885         0         0.851         0.565         0         150         207.2         85.0         1         1.00</td> <td>EBL         EBT         EBR         WBL         WBT         WBR         NBL         NBT           36         172         46         43         146         69         29         331           36         172         46         43         146         69         29         331           1800         1800         1800         1800         1800         1800         1800         1800         1800           0         0         0         0         0         1</td> <td>EBL         EBT         EBR         WBL         WBT         WBR         NBL         NBT         NBR           36         172         46         43         146         69         29         331         43           36         172         46         43         146         69         29         331         43           1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         100         1.00</td> <td>EBL         EBT         EBR         WBL         WBT         WBR         NBL         NBT         NBR         SEL           36         172         46         43         146         69         29         331         43         40           1800         100         1.00</td> <td>EBL         EBT         EBR         WBL         WBT         WBL         NBT         NBT         NBR         SBL         SBT           36         172         46         43         146         69         29         331         43         40         287           1800</td>	EBL         EBT         EBR           36         172         46           36         172         46           1800         1800         1800           0.0         0.0         0.0           0         0.0         0.0           7.6	EBL         EBT         EBR         WBL           36         172         46         43           36         172         46         43           1800         1800         1800         1800           0.0         0.0         0.0         0.0           0.0         0.0         0.0         0.0           0.0         0.0         1.00         1.00           0.993         -         0         0           0.993         0         0         0           0.993         0         0         0           0.993         0         0         0           0.1557         0         0         0           207.2         -         -         14           14         8         8         11           1.00         1.00         1.00         1.00           0%         0%         0%         0%           14         8         8         11           1.00         1.00         1.00         1.00           0         254         0         0         0           No         No         No         No      <	EBL         EBT         EBR         WBL         WBT           36         172         46         43         146           36         172         46         43         146           1800         1800         1800         1800         1800         1800           0         0         0         0         0         0           7.6         7.6         7.6         1.00         1.00         1.00           0.99         0.99         0.99         0.992         0         1709         0.885         0.851           0         1557         0         0         1464         Yes         17         28         50         50         207.2         85.0         250         207.2         85.0         14.9         6.1         144         8         8         11         100         1.	EBL         EBT         EBR         WBL         WBT         WBR $36$ 172         46         43         146         69           36         172         46         43         146         69           1800         1800         1800         1800         1800         1800           0.0         0.0         0.0         0.0         0.0           0         0.00         1.00         1.00         1.00         1.00           0.976         0.993         0.993         0.993         0.992           0         1750         0         0         1464         0           0         1557         0         0         1464         0           0         1557         0         0         1464         0           14.9         6.1         141         6         1         100           14.9         6.1         141         6         1         100           0         254         0         0         258         0           0         254         0         0         258         0           0         0         254         0<	EBL         EBT         EBR         WBL         WBT         WBR         NBL           36         172         46         43         146         69         29           36         172         46         43         146         69         29           1800         1800         1800         1800         1800         1800         1800           0         0         0         0         0         100         1.00         1.00           1.00         1.00         1.00         1.00         1.00         1.00         1.00           0.976         0.994         0.993         0.992         0.950           0         1750         0         0         1709         0         1616           0.885         0         0.851         0.565         0         150         207.2         85.0         1         1.00	EBL         EBT         EBR         WBL         WBT         WBR         NBL         NBT           36         172         46         43         146         69         29         331           36         172         46         43         146         69         29         331           1800         1800         1800         1800         1800         1800         1800         1800         1800           0         0         0         0         0         1	EBL         EBT         EBR         WBL         WBT         WBR         NBL         NBT         NBR           36         172         46         43         146         69         29         331         43           36         172         46         43         146         69         29         331         43           1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         1800         100         1.00	EBL         EBT         EBR         WBL         WBT         WBR         NBL         NBT         NBR         SEL           36         172         46         43         146         69         29         331         43         40           1800         100         1.00	EBL         EBT         EBR         WBL         WBT         WBL         NBT         NBT         NBR         SBL         SBT           36         172         46         43         146         69         29         331         43         40         287           1800

Brad Byvelds, Novatech

Synchro 10 Report

# 3: Churchill Avenue & Byron Avenue AM Peak

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
All-Red Time (s)	2.3	2.3		2.3	2.3		2.1	2.1		2.1	2.1	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6		5.4	5.4		5.4	5.4	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		9.0	9.0		9.0	9.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0		0	0	
Act Effct Green (s)		17.4			17.4		51.6	51.6		51.6	51.6	
Actuated g/C Ratio		0.22			0.22		0.64	0.64		0.64	0.64	
v/c Ratio		0.72			0.76		0.05	0.34		0.07	0.28	
Control Delay		38.2			39.7		7.3	8.4		7.3	7.9	
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.0	
Total Delay		38.2			39.7		7.3	8.4		7.3	7.9	
LOS		D			D		А	А		А	А	
Approach Delay		38.2			39.7			8.3			7.8	
Approach LOS		D			D			А			А	
Queue Length 50th (m)		33.6			32.9		1.4	21.8		2.0	17.5	
Queue Length 95th (m)		50.9			51.2		5.4	46.8		7.0	38.1	
Internal Link Dist (m)		183.2			61.0			129.8			99.1	
Turn Bay Length (m)							15.0			15.0		
Base Capacity (vph)		640			609		603	1100		597	1101	
Starvation Cap Reductn		0			0		0	0		0	0	
Spillback Cap Reductn		0			0		0	0		0	0	
Storage Cap Reductn		0			0		0	0		0	0	
Reduced v/c Ratio		0.40			0.42		0.05	0.34		0.07	0.28	
Intersection Summary	•											
Area Type:	Other											
Cycle Length: 80												
Actuated Cycle Length: 80			~									
Offset: 74 (93%), Referenced to	phase 2:NBTL a	nd 6:SBTL,	Start of Gre	een								
Natural Cycle: 60												
Control Type: Actuated-Coordin	ated											
Maximum v/c Ratio: 0.76												
Intersection Signal Delay: 20.6	04.00/				ersection LC							
Intersection Capacity Utilization Analysis Period (min) 15	64.9%			IC	J Level of S	ervice C						
Splits and Phases: 3: Church	ill Avenue & Byro	n Avenue										
	,					A						

Ø2 (R)	<u> ≁</u> ₀₄	
42 s	38 s	
Ø6 (R)	<b>↓</b> Ø8	
42 s	38 s	

#### 6: Churchill Avenue & Ricmond Rd AM Peak

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲,	1,		<b>N</b>	1,			<u>ل</u> اً	1		<del>ب</del> ا ا	1
Traffic Volume (vph)	295	387	38	53	203	21	28	304	103	17	276	139
Future Volume (vph)	295	387	38	53	203	21	28	304	103	17	276	139
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	30.0		0.0	25.0		0.0	15.0		15.0	15.0		15.0
Storage Lanes	1		0	1		0	0		1	0		1
Taper Length (m)	45.0			30.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.95	0.99		0.96	0.99			1.00	0.90		1.00	0.89
Frt		0.987			0.986				0.850			0.850
Flt Protected	0.950			0.950				0.996			0.997	
Satd. Flow (prot)	1679	1764	0	1586	1693	0	0	1722	1517	0	1706	1488
Flt Permitted	0.374			0.462				0.954			0.970	
Satd. Flow (perm)	628	1764	0	739	1693	0	0	1642	1369	0	1656	1317
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		8			7				180			180
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		135.1			164.7			123.1			131.3	
Travel Time (s)		9.7			11.9			8.9			9.5	
Confl. Peds. (#/hr)	50		47	47		50	38		30	30		38
Confl. Bikes (#/hr)			6			4			3			1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	1%	0%	9%	5%	5%	8%	5%	2%	12%	6%	4%
Adj. Flow (vph)	295	387	38	53	203	21	28	304	103	17	276	139
Shared Lane Traffic (%)												
Lane Group Flow (vph)	295	425	0	53	224	0	0	332	103	0	293	139
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.7			3.7			3.7			3.7	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.9			4.9			4.9			4.9	
Two way Left Turn Lane												
Headway Factor	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2	••	1	2	••	1	2	1	1	2	1
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5		6.1	30.5	6.1	6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	6.1	1.8		6.1	1.8		6.1	1.8	6.1	6.1	1.8	6.1
Detector 1 Type	CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel	0. 2.	01 2.4		0. 2.4	0. 2.		0. 2/	0. 2.4	0. 2.4	0. 2.4	0. 2.0	0. 2.4
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	010	28.7		0.0	28.7		0.0	28.7	0.0	0.0	28.7	0.0
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			Cl+Ex	
Detector 2 Channel		OI! EX			OI LA			OI LA			OI' EX	
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA		Perm	NA	Perm	Perm	NA	Perm
Protected Phases	7	4		1 0111	8			2	1 0111	1 0111	6	i cim
Permitted Phases	4	т		8	0		2	2	2	6	U	6
Detector Phase	7	4		8	8		2	2	2	6	6	6
Switch Phase	1	7		0	0		2	2	2	0	U	0
Minimum Initial (s)	5.0	10.0		10.0	10.0		10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	11.1	31.1		31.1	31.1		24.2	24.2	24.2	24.2	24.2	24.2
Total Split (s)	11.1	40.1		31.1	31.1		24.2 30.0	24.2 30.0	24.2 30.0	24.2 30.0	24.2 30.0	24.2 30.0
Total Split (%)	14.0	40.1 50.1%		31.1	31.1		30.0	30.0 37.5%	30.0 37.5%	30.0 37.5%	30.0 37.5%	30.0
Maximum Green (s)	17.5% 7.9	50.1% 34.0		38.8% 25.0	38.8% 25.0		23.8	23.8	37.5% 23.8	37.5% 23.8	37.5% 23.8	37.5% 23.8
Yellow Time (s)	3.3	3.3		3.3	3.3		3.6	3.6	3.6	3.6	3.6	3.6

Brad Byvelds, Novatech

Synchro 10 Report

Lane Group	Ø1	Ø3	Ø5
Lane 🕶 nfigurations			
Traffic Volume (vph)			
Future Volume (vph)			
Ideal Flow (vphpl)			
Storage Length (m)			
Storage Lanes			
Taper Length (m)			
Lane Util. Factor			
Ped Bike Factor			
Frt			
Flt Protected			
Satd. Flow (prot)			
Flt Permitted			
Satd. Flow (perm)			
Right Turn on Red			
Satd. Flow (RTOR)			
Link Speed (k/h)			
Link Distance (m)			
Travel Time (s)			
Confl. Peds. (#/hr)			
Confl. Bikes (#/hr)			
Peak Hour Factor			
Heavy Vehicles (%)			
Adj. Flow (vph)			
Shared Lane Traffic (%)			
Lane Group Flow (vph)			
Enter Blocked Intersection			
Lane Alignment			
Median Width(m)			
Link Offset(m)			
Crosswalk Width(m)			
Two way Left Turn Lane			
Headway Factor			
Turning Speed (k/h)			
Number of Detectors			
Detector Template			
Leading Detector (m)			
Trailing Detector (m)			
Detector 1 Position(m)			
Detector 1 Size(m)			
Detector 1 Type			
Detector 1 Channel			
Detector 1 Extend (s)			
Detector 1 Queue (s)			
Detector 1 Delay (s)			
Detector 2 Position(m)			
Detector 2 Size(m)			
Detector 2 Type			
Detector 2 Channel			
Detector 2 Extend (s)			
Turn Type			
Protected Phases	1	3	5
Permitted Phases			
Detector Phase			
Switch Phase			
Minimum Initial (s)	3.0	3.0	3.0
Minimum Split (s)	5.0	5.0	5.0
Total Split (s)	5.0	5.0	5.0
Total Split (%)	6%	6%	6%
Maximum Green (s)	3.0	3.0	3.0
Yellow Time (s)	2.0	2.0	2.0

Brad Byvelds, Novatech

#### 6: Churchill Avenue & Ricmond Rd AM Peak

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Lane Group	EBL	EBT	EBR	• WBL	WBT	WBR	NBL	NBT	• NBR	SBL	• SBT	SBR
All-Red Time (s)	2.8	2.8		2.8	2.8		2.6	2.6	2.6	2.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0		0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1			6.2	6.2		6.2	6.2
Lead/Lag	Lead	Lag		Lag	Lag		Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None		None	None		C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)		16.0		16.0	16.0		7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		9.0		9.0	9.0		9.0	9.0	9.0	9.0	9.0	9.0
Pedestrian Calls (#/hr)		15		15	15		15	15	15	15	15	15
Act Effct Green (s)	30.7	25.7		16.7	16.7			30.1	30.1		30.1	30.1
Actuated g/C Ratio	0.38	0.32		0.21	0.21			0.38	0.38		0.38	0.38
v/c Ratio	0.86	0.75		0.35	0.63			0.54	0.16		0.47	0.23
Control Delay	43.2	31.7		31.2	34.9			25.1	1.0		23.6	2.7
Queue Delay	0.0	0.0		0.0	0.0			0.0	0.0		0.0	0.0
Total Delay	43.2	31.7		31.2	34.9			25.1	1.0		23.6	2.7
LOS	D	С		С	С			С	А		С	A
Approach Delay		36.4			34.2			19.4			16.9	
Approach LOS		D			С			В			В	
Queue Length 50th (m)	33.7	57.3		7.0	30.8			38.2	0.0		32.6	0.0
Queue Length 95th (m)	#48.9	73.4		14.9	44.8			72.0	1.5		62.4	6.9
Internal Link Dist (m)		111.1			140.7			99.1			107.3	
Turn Bay Length (m)	30.0			25.0					15.0			15.0
Base Capacity (vph)	344	753		230	533			617	627		623	607
Starvation Cap Reductn	0	0		0	0			0	0		0	0
Spillback Cap Reductn	0	0		0	0			0	0		0	0
Storage Cap Reductn	0	0		0	0			0	0		0	0
Reduced v/c Ratio	0.86	0.56		0.23	0.42			0.54	0.16		0.47	0.23
Intersection Summary												
Area Type:	Other											

 Area Type:
 Other

 Cycle Length: 80.1
 Actuated Cycle Length: 80.1

 Offset: 43 (54%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

 Natural Cycle: 75

 Control Type: Actuated-Coordinated

 Maximum v/c Ratio: 0.86

 Intersection Signal Delay: 27.6

 Intersection Capacity Utilization 92.0%

 Analysis Period (min) 15

 # 95th percentile volume exceeds capacity, queue may be longer.

 Queue shown is maximum after two cycles.

ICU Level of Service F

Splits and Phases: 6: Churchill Avenue & Ricmond Rd

	1 a a a a a a a a a a a a a a a a a a a	
5 s 30 s	5s 40.1s	
	✓ <sub>Ø7</sub>	₩ Ø8
5 s 30 s	14 s	31.1s

Intersection LOS: C

Lane Group	Ø1	Ø3	Ø5
All-Red Time (s)	0.0	0.0	0.0
Lost Time Adjust (s)			
Total Lost Time (s)			
Lead/Lag	Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0
Recall Mode	Min	Min	Min
Walk Time (s)			
Flash Dont Walk (s)			
Pedestrian Calls (#/hr)			
Act Effct Green (s)			
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (m)			
Queue Length 95th (m)			
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			

# 9: Highcroft Avenue & Byron Avenue AM Peak

Movement         EBT         EBR         WBL         WBL         NBL         NBR           Lane Configurations         1         1         1         1         1         1         1         1         1         1         1         1         1         1         1         1         1         26         5         1         1         26         5         1         1         26         5         1         1         26         5         1         1         26         5         1         1         26         5         1         1         1         26         5         6         4         272         1         7         26         5         6         4         272         1         7         26         1         10         1         10         1         10         1         10         1         10         1         10         1         10         1         10         1         10         1         10         1         10         1         10         1         10         1         10         1         10         1         1         1         1         1         1         1         1		-	$\mathbf{r}$	<	+	•	1
Lane Configurations         Image: Configuration of the second of th	Movement	EBT	EBR	- WBL	WBT	NBL	- NBR
Traffic Volume (veh/h)       225       6       4       272       17       26         Future Volume (Veh/h)       225       6       4       272       17       26         Sign Control       Free       Step       0%       0%       0%         Grade       0%       0%       0%       0%       0%         Peak Hour Factor       1.00       1.00       1.00       1.00       1.00       1.00         Houry flow rate (vph)       225       6       4       272       17       26         Pedestrians       10       1.00       1.00       1.00       1.00       1.00         Lane Width (m)       3.7       Walking Speed (m/s)       1.1       Percent Blockage       1         Welian type       None       None       None       Median type       Vone       Median type         Volume face (weh)       85       9X, platoon unblocked       0.94       0.94       0.94       0.94         V2, stage 1 conf vol       vC2, stage 1 conf vol       vC2, stage 1 conf vol       vC2, stage 2 conf vol       vC2       3.5       3.3         V0 queue free %       100       97       97       97       97       26       832							
Future Volume (Veh/h)         225         6         4         272         17         26           Sign Control         Free         Stop         Grade         0%		225	6	4			26
Sign Control         Free         Free         Stop           Grade         0%         0%         0%         0%           Peak Hour Factor         1.00         1.00         1.00         1.00         1.00           Hourly flow rate (vph)         225         6         4         272         17         26           Pedestrians         10         10         10         10         10         10           Lane Width (m)         3.7         Walking Speed (m/s)         1.1         Pecestrians         1           Right turn flare (veh)         Median storage (m/s)         1.1         Pecestrians         1           Wedian storage veh)         Upstream signal (m)         85         PX         Platoon unblocked         0.94         0.94         0.94           VC2, stage 1 conf vol         vC2, stage 2 conf vol         vC1, stage 1 conf vol         159         454         156         156         212         3.5         3.3           p0 queue free %         100							
Grade       0%       0%       0%       0%         Peak Hour Factor       1.00       1.00       1.00       1.00       1.00       1.00         Houry flow rate (vph)       225       6       4       272       17       26         Pedestrians       10       10       10       10       10       10         Lane Width (m)       3.7       Walking Speed (m/s)       1.1       1         Percent Blockage       1       1       1         Right turn flare (veh)       None       None       None         Median storage veh)       Upstream signal (m)       85       0.94       0.94       0.94         VC2, stage 1 conf vol       vC2, stage 2 conf vol       vC4, stage 1 conf vol       vC2, stage 2 conf vol       vC4, stage 1 conf vol       vC2, stage 2 conf vol       vC4, stage 1 conf vol       vC2, stage 2 conf vol       vC4, stage 1 conf vol       97       97       97       CM capacity (veh/h)       1333       526       832       28       22       3.5       3.3       33       90 queue free %       100       97       97       97       CM capacity (veh/h)       1333       526       832       28       28       26       832       28       26       832							
Peak Hour Factor         1.00         1.00         1.00         1.00         1.00         1.00         1.00           Hourly flow rate (vph)         225         6         4         272         17         26           Pedestrians         10         10         10         10         10           Lane Width (m)         3.7         Valking Speed (m/s)         1.1         1           Percent Blockage         1         1         1           Right turn flare (veh)         Median storage veh)         1         1           Upstream signal (m)         85         94         0.94         0.94         0.94           VC, conflicting volume         241         518         238         vC1, stage 1 conf vol         vC2, stage 2 conf vol         vC2, stage 2 conf vol         vC4, unblocked vol         159         454         156         156         156         156         156         156         157         333         526         832         22         3.5         3.3         90         queue free %         100         97         97         97         cd capacity (veh/h)         1333         526         832         22         3.5         3.3         23         22         3.5         3.3							
Hourly flow rate (vph)       225       6       4       272       17       26         Pedestrians       10       3.7       10       11       10       11       10       10       11       10       10       11       10       11       10       11       11       11       10       11 </td <td></td> <td></td> <td>1 00</td> <td>1 00</td> <td></td> <td></td> <td>1 00</td>			1 00	1 00			1 00
Pedestrians         10           Lane Width (m)         3.7           Walking Speed (m/s)         1.1           Percent Blockage         1           Right turn flare (veh)         Median storage veh)           Median storage veh)         Upstream signal (m)           85         0.94         0.94         0.94           Vpstream signal (m)         85         0.94         0.94         0.94           vC2, stage 1 conf vol         241         518         238         vC1, stage 1 conf vol         vC2, stage 2 conf vol         vC4, unblocked vol         159         454         156           tC, single (s)         2.2         3.5         3.3         p0 queue free %         100         97         97           cM capacity (veh/h)         1333         526         832         Direction, Lane #         EB 1         WB 1         NB 1           Volume Total         231         276         43         Volume Left         0         4         17           Volume Right         6         0         26         cSH         100         10.33         677           Volume Left         0         4				4			
Lane Width (m)       3.7         Walking Speed (m/s)       1.1         Percent Blockage       1         Right tum flare (veh)       1         Median storage veh)       Upstream signal (m)         VS, platoon unblocked       0.94       0.94         VC, conflicting volume       241       518       238         vC2, stage 1 conf vol       vC2, stage 2 conf vol       vC2, stage 2 conf vol       vC2, stage 2 conf vol         vC2, stage 2 conf vol       159       454       156         tC, single (s)       4.1       6.4       6.2         tC, stage (s)       tf (s)       2.2       3.5       3.3         p0 queue free %       100       97       97         cM capacity (veh/h)       1333       526       832         Direction, Lane #       EB 1       WB 1       NB 1         Volume Total       231       276       43         Volume Right       6       0       26       cSH         Volume Right       6       0       26       cSH       1700       1333       677         Volume Left       0       4       17       Volume Kight       0.0       0.1       1.5       Control Delay (s)       <		220	Ŭ	•			-•
Walking Speed (m/s)       1.1         Percent Blockage       1         Right turn flare (veh)       None         Median storage veh)       Upstream signal (m)         Upstream signal (m)       85         pX, platoon unblocked       0.94       0.94         VC, conflicting volume       241       518       238         vC1, stage 1 conf vol       vC2, stage 2 conf vol       vC2, stage 2 conf vol       vC2, stage 2 conf vol         vC2, stage (s)       4.1       6.4       6.2       tC, 2 stage (s)       tr         tF (s)       2.2       3.5       3.3       p0 queue free %       100       97       97         cM capacity (veh/h)       1333       526       832       20       20       832       20         Direction, Lane #       EB 1       WB 1       NB 1       Volume Total       231       276       43         Volume Total       231       276       43       20       20       20         Volume Edf       0       4       17       20       20       20       20         Volume Right       6       0       26       26       25       27       27       26       23       26       26							
Percent Blockage         1           Right turn flare (veh)         None         None           Median type         None         None           Median storage veh)         Upstream signal (m)         85           pX, platoon unblocked         0.94         0.94         0.94           vC, conflicting volume         241         518         238           vC1, stage 1 conf vol         vC2, stage 2 conf vol         vC2, stage 2 conf vol         vC2, stage 2 conf vol           vC2, unblocked vol         159         454         156           tC, single (s)         4.1         6.4         6.2           tC, 2 stage (s)         tr         tr         f(s)         3.3           p0 queue free %         100         97         97           cd capacity (veh/h)         1333         526         832           Direction, Lane #         EB 1         WB 1         NB 1           Volume Total         231         276         43           Volume Right         6         0         26           cSH         1700         1333         677           Volume to Capacity         0.14         0.00         0.06           Queue Length 95th (m)         0.0 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>							
Right turn flare (veh)         None         None           Median storage veh)         Upstream signal (m)         85           pX, platoon unblocked         0.94         0.94         0.94           vC, conflicting volume         241         518         238           vC1, stage 1 conf vol         vC2, stage 2 conf vol         vC2, stage 2 conf vol         vC2, unblocked vol         159         454         156           tC, single (s)         4.1         6.4         6.2         C, 2 stage (s)         tF (s)         2.2         3.5         3.3           p0 queue free %         100         97         97         cM capacity (veh/h)         1333         526         832           Direction, Lane #         EB 1         WB 1         NB 1         Volume Total         231         276         43           Volume Total         231         276         43         Volume Left         0         4         17           Volume Right         6         0         26         cSH         1333         677         Volume to Capacity         0.14         0.00         0.06         Queue Length 95th (m)         0.0         0.1         1.5         Control Delay (s)         0.0         0.1         10.7         Lane LOS							
Median type         None         None           Median storage veh)         Upstream signal (m)         85           pX, platoon unblocked         0.94         0.94         0.94           vC, conflicting volume         241         518         238           vC1, stage 1 conf vol         241         518         238           vC2, stage 2 conf vol         vC2, stage 2 conf vol         vC2, unblocked vol         159         454         156           tC, single (s)         4.1         6.4         6.2         2.2         3.5         3.3           p0 queue free %         100         97         97         97         cM capacity (veh/h)         1333         526         832           Direction, Lane #         EB 1         WB 1         NB 1         Volume Total         231         276         43           Volume Total         231         276         43         Volume Left         0         4         17           Volume Right         6         0         26         cSH         1333         677           Volume to Capacity         0.14         0.00         0.06         Queue Length 95th (m)         0.0         1.1         5.2           Control Delay (s)         0.0 </td <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>							
Median storage veh)         85           Upstream signal (m)         85           pX, platoon unblocked         0.94         0.94         0.94           vC, conflicting volume         241         518         238           vC1, stage 1 conf vol         241         518         238           vC2, stage 2 conf vol         vCu, unblocked vol         159         454         156           tC, single (s)         4.1         6.4         6.2         2.2         3.5         3.3           p0 queue free %         100         97         97         97         27         26         832           Direction, Lane #         EB 1         WB 1         NB 1         Volume Total         231         276         43         43         Volume Total         231         276         43         Volume Right         6         0         26         cSH         1700         1333         677         Volume Right         0.0         26         cSH         100         0.1         1.5         Control Delay (s)         0.0         0.1         10.7         Lane LOS         A         B         Approach Delay (s)         0.0         0.1         10.7		None			None		
Upstream signal (m)         85           pX, platoon unblocked         0.94         0.94         0.94           vC, conflicting volume         241         518         238           vC1, stage 1 conf vol         vC2, stage 2 conf vol         vC2, stage 2 conf vol         vC2, stage 2 conf vol           vC2, sigle (s)         454         156         156           tC, single (s)         4.1         6.4         6.2           tC, 2 stage (s)         518         235         3.3           p0 queue free %         100         97         97           cM capacity (veh/h)         1333         526         832           Direction, Lane #         EB 1         WB 1         NB 1         Volume Total         231         276         43           Volume Total         231         276         43         Volume Right         6         0         26         cSH         1700         1333         677         Volume Right         0.0         26         cSH         27         26         26         CSH         27         27         27         27         27         27         26         28         27         27         26         832         27         27         26 <t< td=""><td></td><td>None</td><td></td><td></td><td>None</td><td></td><td></td></t<>		None			None		
pX, platoon unblocked       0.94       0.94       0.94       0.94         vC, conflicting volume       241       518       238         vC1, stage 1 conf vol       vc       vc       vc         vC2, stage 2 conf vol       vc       vc       vc         vCu, unblocked vol       159       454       156         tC, single (s)       4.1       6.4       6.2         tC, 2 stage (s)       tr       tr       (s)       3.5       3.3         p0 queue free %       100       97       97         cM capacity (veh/h)       1333       526       832         Direction, Lane #       EB 1       WB 1       NB 1       Volume Total       231       276       43         Volume Total       231       276       43       Volume Left       0       4       17         Volume Right       6       0       26       CSH       1700       1333       677         Volume to Capacity       0.14       0.00       0.06       Queue Length 95th (m)       0.0       0.1       1.5         Control Delay (s)       0.0       0.1       10.7       Lane LOS       A       B         Approach Delay (s)       0.0 </td <td></td> <td>85</td> <td></td> <td></td> <td></td> <td></td> <td></td>		85					
vC, conflicting volume       241       518       238         vC1, stage 1 conf vol       vC2, stage 2 conf vol       vC4, unblocked vol       159       454       156         vC2, stage 2 conf vol       159       454       156       6.2       6.2       6.2       7.2       3.5       3.3       90       97       <		00		0 94		0 94	0 94
vC1, stage 1 conf vol       vC2, stage 2 conf vol         vCu, unblocked vol       159       454       156         tC, single (s)       4.1       6.4       6.2         tC, 2 stage (s)        2.2       3.5       3.3         p0 queue free %       100       97       97         cM capacity (veh/h)       1333       526       832         Direction, Lane #       EB 1       WB 1       NB 1         Volume Total       231       276       43         Volume Total       0       4       17         Volume Right       6       0       26         cSH       1700       1333       677         Volume to Capacity       0.14       0.00       0.06         Queue Length 95th (m)       0.0       0.1       1.5         Control Delay (s)       0.0       0.1       10.7         Lane LOS       A       B       Approach Delay (s)       0.0       0.1							
vC2, stage 2 conf vol vCu, unblocked vol 159 454 156 tC, single (s) 4.1 6.4 6.2 tC, 2 stage (s) tF (s) 2.2 3.5 3.3 p0 queue free % 100 97 97 cM capacity (veh/h) 1333 526 832 Direction, Lane # EB 1 WB 1 NB 1 Volume Total 231 276 43 Volume Left 0 4 17 Volume Right 6 0 26 cSH 1700 1333 677 Volume to Capacity 0.14 0.00 0.06 Queue Length 95th (m) 0.0 0.1 1.5 Control Delay (s) 0.0 0.1 10.7 Lane LOS A B Approach Delay (s) 0.0 0.1 10.7				271		010	200
vCu, unblocked vol         159         454         156           tC, single (s)         4.1         6.4         6.2           tC, 2 stage (s)							
tC, single (s)       4.1       6.4       6.2         tC, single (s)				159		454	156
tC, 2 stage (s)       2.2       3.5       3.3         p0 queue free %       100       97       97         cM capacity (veh/h)       1333       526       832         Direction, Lane #       EB 1       WB 1       NB 1         Volume Total       231       276       43         Volume Total       0       4       17         Volume Left       0       4       17         Volume Right       6       0       26         cSH       1700       1333       677         Volume to Capacity       0.14       0.00       0.06         Queue Length 95th (m)       0.0       0.1       1.5         Control Delay (s)       0.0       0.1       10.7         Lane LOS       A       B       Approach Delay (s)       0.0       0.1							
tF (s)       2.2       3.5       3.3         p0 queue free %       100       97       97         cM capacity (veh/h)       1333       526       832         Direction, Lane #       EB 1       WB 1       NB 1         Volume Total       231       276       43         Volume Total       0       4       17         Volume Right       6       0       26         cSH       1700       1333       677         Volume to Capacity       0.14       0.00       0.06         Queue Length 95th (m)       0.0       0.1       1.5         Control Delay (s)       0.0       0.1       10.7         Lane LOS       A       B       A         Approach Delay (s)       0.0       0.1       10.7						••••	v
D0 queue free %         100         97         97           cM capacity (veh/h)         1333         526         832           Direction, Lane #         EB 1         WB 1         NB 1           Volume Total         231         276         43           Volume Left         0         4         17           Volume Right         6         0         26           cSH         1700         1333         677           Volume to Capacity         0.14         0.00         0.06           Queue Length 95th (m)         0.0         0.1         1.5           Control Delay (s)         0.0         0.1         10.7           Lane LOS         A         B         A           Approach Delay (s)         0.0         0.1         10.7				2.2		3.5	3.3
cM capacity (veh/h)         1333         526         832           Direction, Lane #         EB 1         WB 1         NB 1           Volume Total         231         276         43           Volume Total         231         276         43           Volume Left         0         4         17           Volume Right         6         0         26           cSH         1700         1333         677           Volume to Capacity         0.14         0.00         0.06           Queue Length 95th (m)         0.0         0.1         1.5           Control Delay (s)         0.0         0.1         10.7           Lane LOS         A         B           Approach Delay (s)         0.0         0.1         10.7							
Direction, Lane #         EB 1         WB 1         NB 1           Volume Total         231         276         43           Volume Left         0         4         17           Volume Right         6         0         26           cSH         1700         1333         677           Volume to Capacity         0.14         0.00         0.06           Queue Length 95th (m)         0.0         0.1         1.5           Control Delay (s)         0.0         0.1         10.7           Lane LOS         A         B           Approach Delay (s)         0.0         0.1         10.7							
Volume Total         231         276         43           Volume Left         0         4         17           Volume Right         6         0         26           cSH         1700         1333         677           Volume to Capacity         0.14         0.00         0.06           Queue Length 95th (m)         0.0         0.1         1.5           Control Delay (s)         0.0         0.1         10.7           Lane LOS         A         B           Approach Delay (s)         0.0         0.1         10.7		FR 1	WR 1			520	
Volume Left         0         4         17           Volume Right         6         0         26           cSH         1700         1333         677           Volume to Capacity         0.14         0.00         0.06           Queue Length 95th (m)         0.0         0.1         1.5           Control Delay (s)         0.0         0.1         10.7           Lane LOS         A         B           Approach Delay (s)         0.0         0.1         10.7							
Volume Right         6         0         26           cSH         1700         1333         677           Volume to Capacity         0.14         0.00         0.06           Queue Length 95th (m)         0.0         0.1         1.5           Control Delay (s)         0.0         0.1         10.7           Lane LOS         A         B           Approach Delay (s)         0.0         0.1         10.7							
cSH         1700         1333         677           Volume to Capacity         0.14         0.00         0.06           Queue Length 95th (m)         0.0         0.1         1.5           Control Delay (s)         0.0         0.1         10.7           Lane LOS         A         B           Approach Delay (s)         0.0         0.1         10.7							
Volume to Capacity         0.14         0.00         0.06           Queue Length 95th (m)         0.0         0.1         1.5           Control Delay (s)         0.0         0.1         10.7           Lane LOS         A         B           Approach Delay (s)         0.0         0.1         10.7							
Queue Length 95th (m)         0.0         0.1         1.5           Control Delay (s)         0.0         0.1         10.7           Lane LOS         A         B           Approach Delay (s)         0.0         0.1         10.7							
Control Delay (s)         0.0         0.1         10.7           Lane LOS         A         B           Approach Delay (s)         0.0         0.1         10.7							
Lane LOS         A         B           Approach Delay (s)         0.0         0.1         10.7							
Approach Delay (s) 0.0 0.1 10.7		0.0					
		0.0		_			
Apploach 200		0.0	0.1				
				D			
Intersection Summary							
Average Delay 0.9							
Intersection Capacity Utilization 28.5% ICU Level of Service	Intersection Capacity Utilization				IC	U Level of S	ervice
Analysis Period (min) 15	Analysis Period (min)			15			

#### 11: Highcroft Avenue & Access AM Peak

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	5			⊿	1.	
Traffic Volume (veh/h)	18	0	0	<b>4</b> 25	<b>1</b>	7
Future Volume (Veh/h)	18	0	0	25	2	7
Sign Control	Stop	, , , , , , , , , , , , , , , , , , ,	•	Free	Free	•
Grade	0%			0%	0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	18	0	0	25	2	7
Pedestrians	10	U	0	25	2	,
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				NOTE	NUTE	
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	30	6	9			
	30	0	9			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol	20	C	0			
vCu, unblocked vol	30	6	9			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	<u> </u>	0.0	0.0			
tF (s)	3.5	3.3	2.2			
p0 queue free %	98	100	100			
cM capacity (veh/h)	984	1077	1611			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	18	25	9			
Volume Left	18	0	0			
Volume Right	0	0	7			
cSH	984	1611	1700			
Volume to Capacity	0.02	0.00	0.01			
Queue Length 95th (m)	0.4	0.0	0.0			
Control Delay (s)	8.7	0.0	0.0			
Lane LOS	A	0.0	0.0			
Approach Delay (s)	8.7	0.0	0.0			
Approach LOS	A	0.0	0.0			
Intersection Summary						
Average Delay			3.0			
Intersection Capacity Utilization			13.3%		U Level of Serv	ico
Analysis Period (min)			13.3%	iC	o Level OI Selv	
Analysis Period (min)			15			

# 3: Churchill Avenue & Byron Avenue PM Peak

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		Υ.	ĥ		5	î,	
Traffic Volume (vph)	45	141	61	97	362	79	33	298	97	34	321	69
Future Volume (vph)	45	141	61	97	362	79	33	298	97	34	321	69
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	0.0		0.0	0.0		0.0	15.0		0.0	15.0		0.0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (m)	7.6			7.6			25.0			25.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.99			0.99		0.98	0.98		0.98	0.99	
Frt		0.967			0.980			0.963			0.973	
Flt Protected		0.991			0.991		0.950			0.950		
Satd. Flow (prot)	0	1725	0	0	1750	0	1616	1661	0	1729	1667	0
Flt Permitted		0.832			0.883		0.444			0.440		
Satd. Flow (perm)	0	1446	0	0	1556	0	738	1661	0	787	1667	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		23			12			23			15	
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		207.2			85.0			153.8			123.1	
Travel Time (s)		14.9			6.1			11.1			8.9	
Confl. Peds. (#/hr)	14		8	8		14	21		16	16		21
Confl. Bikes (#/hr)			11			6			14			6
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	0%	0%	0%	0%	0%	2%	7%	5%	0%	0%	5%	4%
Adj. Flow (vph)	45	141	61	97	362	79	33	298	97	34	321	69
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	247	0	0	538	0	33	395	0	34	390	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			3.7			3.7	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.9			4.9			4.9			4.9	
Two way Left Turn Lane												
Headway Factor	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	6.1	30.5		6.1	30.5		6.1	30.5		6.1	30.5	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	6.1	1.8		6.1	1.8		6.1	1.8		6.1	1.8	
Detector 1 Type	CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)		28.7			28.7			28.7			28.7	
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	30.6	30.6		30.6	30.6		25.5	25.5		25.5	25.5	
Total Split (s)	45.0	45.0		45.0	45.0		45.0	45.0		45.0	45.0	
Total Split (%)	50.0%	50.0%		50.0%	50.0%		50.0%	50.0%		50.0%	50.0%	
Maximum Green (s)	39.4	39.4		39.4	39.4		39.6	39.6		39.6	39.6	
Yellow Time (s)	3.3	3.3		3.3	3.3		3.3	3.3		3.3	3.3	

Brad Byvelds, Novatech

Synchro 10 Report

# 3: Churchill Avenue & Byron Avenue PM Peak

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
All-Red Time (s)	2.3	2.3		2.3	2.3		2.1	2.1		2.1	2.1	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.6			5.6		5.4	5.4		5.4	5.4	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	10.0	10.0		10.0	10.0		10.0	10.0		10.0	10.0	
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		9.0	9.0		9.0	9.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0		0	0	
Act Effct Green (s)		34.7			34.7		44.3	44.3		44.3	44.3	
Actuated g/C Ratio		0.39			0.39		0.49	0.49		0.49	0.49	
v/c Ratio		0.43			0.89		0.09	0.48		0.09	0.47	
Control Delay		19.9			42.3		15.2	17.9		15.9	20.2	
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.4	
Total Delay		19.9			42.3		15.2	17.9		15.9	20.6	
LOS		В			D		В	В		В	С	
Approach Delay		19.9			42.3			17.7			20.3	
Approach LOS		В			D			В			С	
Queue Length 50th (m)		26.6			79.9		3.1	42.7		3.7	41.8	
Queue Length 95th (m)		43.6			#122.5		8.9	71.8		m8.0	60.9	
Internal Link Dist (m)		183.2			61.0			129.8			99.1	
Turn Bay Length (m)							15.0			15.0		
Base Capacity (vph)		645			687		362	828		387	827	
Starvation Cap Reductn		0			0		0	0		0	132	
Spillback Cap Reductn		0			0		0	0		0	0	
Storage Cap Reductn		0			0		0	0		0	0	
Reduced v/c Ratio		0.38			0.78		0.09	0.48		0.09	0.56	
Intersection Summary												
Area Type:	Other											
Cycle Length: 90												
Actuated Cycle Length: 90												
Offset: 40 (44%), Referenced to	phase 2:NBTL a	nd 6:SBTL,	Start of Gre	een								
Natural Cycle: 60												
Control Type: Actuated-Coordin	ated											
Maximum v/c Ratio: 0.89												
Intersection Signal Delay: 26.8					ersection LC							
Intersection Capacity Utilization	80.4%			IC	U Level of S	ervice D						
Analysis Period (min) 15												
# 95th percentile volume exce		eue may be	longer.									
Queue shown is maximum a												
m Volume for 95th percentile (	auque in motored	huunotroor	n aignal									

m Volume for 95th percentile queue is metered by upstream signal.

#### Splits and Phases: 3: Churchill Avenue & Byron Avenue

Ø2 (R)	<u>↓</u> <sub>04</sub>
45 s	45 s
Ø6 (R)	₩ Ø8
45 s	45 s

# 6: Churchill Avenue & Ricmond Rd PM Peak

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	5	ĥ		1	٦,			ส์	1		<u>ل</u>	1
Traffic Volume (vph)	191	342	57	129	485	25	28	294	67	15	241	312
Future Volume (vph)	191	342	57	129	485	25	28	294	67	15	241	312
Ideal Flow (vphpl)	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Length (m)	30.0		0.0	25.0		0.0	15.0		15.0	15.0		15.0
Storage Lanes	1		0	1		0	0		1	0		1
Taper Length (m)	45.0			30.0			15.0			15.0		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.97	0.98		0.95	0.99			0.99	0.89		1.00	0.87
Frt		0.979			0.993				0.850			0.850
Flt Protected	0.950	4740	•	0.950	4740	0	0	0.996	4547	•	0.997	4.400
Satd. Flow (prot)	1679	1740	0	1586	1712	0	0	1722	1517	0	1706	1488
Flt Permitted	0.193	4740	0	0.527 837	4740	0	0	0.951	1354	0	0.967	4000
Satd. Flow (perm)	332	1740	0 Yes	837	1712	0 Yes	U	1635	Yes	0	1651	1300 Yes
Right Turn on Red Satd. Flow (RTOR)		15	res		4	res			160			183
Link Speed (k/h)		15 50			4 50			50	100		50	105
Link Distance (m)		135.1			164.7			123.1			131.3	
Travel Time (s)		9.7			11.9			8.9			9.5	
Confl. Peds. (#/hr)	50	5.1	47	47	11.5	50	38	0.5	30	30	0.0	38
Confl. Bikes (#/hr)			6			4	00		3	00		1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles (%)	3%	1%	0%	9%	5%	5%	8%	5%	2%	12%	6%	4%
Adj. Flow (vph)	191	342	57	129	485	25	28	294	67	15	241	312
Shared Lane Traffic (%)												
Lane Group Flow (vph)	191	399	0	129	510	0	0	322	67	0	256	312
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.7			3.7			3.7			3.7	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.9			4.9			4.9			4.9	
Two way Left Turn Lane												
Headway Factor	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
Turning Speed (k/h)	24		14	24		14	24		14	24		14
Number of Detectors	1	2		1	2		1	2	1	1	2	1
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	6.1	30.5		6.1	30.5		6.1	30.5	6.1	6.1	30.5	6.1
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m) Detector 1 Type	6.1 Cl+Ex	1.8 Cl+Ex		6.1 Cl+Ex	1.8 Cl+Ex		6.1 Cl+Ex	1.8 Cl+Ex	6.1 Cl+Ex	6.1 Cl+Ex	1.8 Cl+Ex	6.1 Cl+Ex
Detector 1 Channel	CI+EX	CI+EX		CI+EX	CI+EX		CI+EX	CI+EX	CI+EX	CI+EX	CI+EX	CI+EX
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	0.0	28.7		0.0	28.7		0.0	28.7	0.0	0.0	28.7	0.0
Detector 2 Size(m)		1.8			1.8			1.8			1.8	
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex			Cl+Ex	
Detector 2 Channel		0. 2.1			0. 2.1			0			0. 2.	
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA		Perm	NA		Perm	NA	Perm	Perm	NA	Perm
Protected Phases	7	4			8			2			6	
Permitted Phases	4			8			2		2	6		6
Detector Phase	7	4		8	8		2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	5.0	10.0		10.0	10.0		10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	11.1	31.1		31.1	31.1		24.2	24.2	24.2	24.2	24.2	24.2
Total Split (s)	15.0	55.0		45.0	45.0		25.0	25.0	25.0	25.0	25.0	25.0
Total Split (%)	16.7%	61.1%		50.0%	50.0%		27.8%	27.8%	27.8%	27.8%	27.8%	27.8%
Maximum Green (s)	8.9	48.9		38.9	38.9		18.8	18.8	18.8	18.8	18.8	18.8
Yellow Time (s)	3.3	3.3		3.3	3.3		3.6	3.6	3.6	3.6	3.6	3.6

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Synchro 10 Report

Lane Group	Ø1	Ø3	Ø5
Lane 🗱 nfigurations			
Traffic Volume (vph)			
Future Volume (vph)			
Ideal Flow (vphpl)			
Storage Length (m)			
Storage Lanes			
Taper Length (m)			
Lane Util. Factor			
Ped Bike Factor			
Frt			
Flt Protected			
Satd. Flow (prot)			
Flt Permitted			
Satd. Flow (perm)			
Right Turn on Red			
Satd. Flow (RTOR)			
Link Speed (k/h)			
Link Distance (m)			
Travel Time (s)			
Confl. Peds. (#/hr)			
Confl. Bikes (#/hr)			
Peak Hour Factor			
Heavy Vehicles (%)			
Adj. Flow (vph)			
Shared Lane Traffic (%)			
Lane Group Flow (vph)			
Enter Blocked Intersection			
Lane Alignment			
Median Width(m)			
Link Offset(m)			
Crosswalk Width(m)			
Two way Left Turn Lane			
Headway Factor			
Turning Speed (k/h)			
Number of Detectors			
Detector Template			
Leading Detector (m)			
Trailing Detector (m)			
Detector 1 Position(m)			
Detector 1 Size(m)			
Detector 1 Type			
Detector 1 Channel			
Detector 1 Extend (s)			
Detector 1 Queue (s)			
Detector 1 Delay (s)			
Detector 2 Position(m)			
Detector 2 Size(m)			
Detector 2 Type			
Detector 2 Channel			
Detector 2 Extend (s)			
Turn Type			
Protected Phases	1	3	5
Permitted Phases			
Detector Phase			
Switch Phase			
Minimum Initial (s)	3.0	3.0	3.0
Minimum Split (s)	5.0	5.0	5.0
Total Split (s)	5.0	5.0	5.0
Total Split (%)	6%	6%	6%
Maximum Green (s)	3.0	3.0	3.0
Yellow Time (s)	2.0	2.0	2.0

Brad Byvelds, Novatech

## 6: Churchill Avenue & Ricmond Rd PM Peak

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
All-Red Time (s)	2.8	2.8		2.8	2.8		2.6	2.6	2.6	2.6	2.6	2.6
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0		0.0	0.0
Total Lost Time (s)	6.1	6.1		6.1	6.1			6.2	6.2		6.2	6.2
Lead/Lag	Lead	Lag		Lag	Lag		Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None		None	None		C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)		16.0		16.0	16.0		7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		9.0		9.0	9.0		9.0	9.0	9.0	9.0	9.0	9.0
Pedestrian Calls (#/hr)		15		15	15		15	15	15	15	15	15
Act Effct Green (s)	47.3	42.3		32.3	32.3			23.6	23.6		23.6	23.6
Actuated g/C Ratio	0.53	0.47		0.36	0.36			0.26	0.26		0.26	0.26
v/c Ratio	0.62	0.48		0.43	0.83			0.75	0.14		0.59	0.66
Control Delay	19.7	17.0		25.2	37.7			45.2	4.2		37.9	21.3
Queue Delay	0.0	0.0		0.0	0.0			0.0	0.0		0.0	0.0
Total Delay	19.7	17.0		25.2	37.7			45.2	4.2		37.9	21.3
LOS	В	В		С	D			D	А		D	С
Approach Delay		17.9			35.2			38.1			28.8	
Approach LOS		В			D			D			С	
Queue Length 50th (m)	16.3	42.7		16.4	77.3			52.3	0.7		39.7	19.2
Queue Length 95th (m)	23.4	57.1		28.8	104.3			m#102.6	m4.7		#75.4	#59.2
Internal Link Dist (m)		111.1			140.7			99.1			107.3	
Turn Bay Length (m)	30.0			25.0					15.0			15.0
Base Capacity (vph)	307	952		361	742			427	472		432	475
Starvation Cap Reductn	0	0		0	0			0	0		0	0
Spillback Cap Reductn	0	0		0	0			0	0		0	0
Storage Cap Reductn	0	0		0	0			0	0		0	0
Reduced v/c Ratio	0.62	0.42		0.36	0.69			0.75	0.14		0.59	0.66

Intersection Summary			
Area Type:	Other		
Cycle Length: 90			
Actuated Cycle Length: 90	)		
Offset: 0 (0%), Reference	d to phase 2:NBTL and 6:SBTL, Start of Green		
Natural Cycle: 75			
Control Type: Actuated-Co	bordinated		
Maximum v/c Ratio: 0.83			
Intersection Signal Delay:	29.4	Intersection LOS: C	
Intersection Capacity Utiliz	zation 92.6%	ICU Level of Service F	
Analysis Period (min) 15			
# 95th percentile volume	e exceeds capacity, queue may be longer.		
Oucus chown is mavin	and after the analysis		

Queue shown is maximum after two cycles. m Volume for 95th percentile queue is metered by upstream signal.

#### Splits and Phases: 6: Churchill Avenue & Ricmond Rd

1 mg 1 mg 2 (R)	# <b>k</b> ø3 - 04	
5 s 25 s	5 s 55 s	
1 a a a a a a a a a a a a a a a a a a a	▶ <sub>Ø7</sub> ▼ <sub>Ø8</sub>	
5 s 25 s	15 s 45 s	

Lane Group	Ø1	Ø3	Ø5
All-Red Time (s)	0.0	0.0	0.0
Lost Time Adjust (s)			
Total Lost Time (s)			
Lead/Lag	Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0
Recall Mode	Min	Min	Min
Walk Time (s)			
Flash Dont Walk (s)			
Pedestrian Calls (#/hr)			
Act Effct Green (s)			
Actuated g/C Ratio			
v/c Ratio			
Control Delay			
Queue Delay			
Total Delay			
LOS			
Approach Delay			
Approach LOS			
Queue Length 50th (m)			
Queue Length 95th (m)			
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			

# 9: Highcroft Avenue & Byron Avenue PM Peak

ane Configurations         Image: Configuration straffic Volume (veh/h)         233         12         7         589         9         13           raffic Volume (Veh/h)         233         12         7         589         9         13           gin Control         Free         Free         Stop         13           rade         0%         0%         0%         0%           outry flow rate (vph)         233         12         7         589         9         13           ade Strians         10         1.00         1.00         1.00         1.00         1.00         1.00           ane Width (m)         3.7         589         9         13         37         13           ane Width (m)         3.7         11         ercent Blockage         1         11         ercent Blockage         1         1         11         ercent Blockage         1 <th></th> <th>-</th> <th><math>\mathbf{r}</math></th> <th>1</th> <th>-</th> <th>1</th> <th>1</th>		-	$\mathbf{r}$	1	-	1	1
ane Configurations         1         7         589         9         13           ardfic Volume (veh/h)         233         12         7         589         9         13           gin Control         Free         Free         Stop         9         13           gin Control         Free         Stop         9         13           gin Control         Free         Stop         9         13           gin Control         1.00         1.00         1.00         1.00         1.00           outry flow rate (vph)         233         12         7         589         9         13           odestrians         10         1.00         1.00         1.00         1.00         1.00           ane Width (m)         3.7         ////////////////////////////////////	Movement	EBT	EBR	WBL	WBT	NBL	NBR
raffic Volume (veh/h) 233 12 7 589 9 13 ruture Volume (Veh/h) 233 12 7 589 9 13 gin Control Free Free Stop rade 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0%	Lane Configurations						
uture Volume (Veh/h)       233       12       7       589       9       13         ign Control       Free       Stop	Traffic Volume (veh/h)	233	12	7	589	9	13
Free         Free         Stop           rade         0%         0%         0%         0%           eak Hour Factor         1.00         1.00         1.00         1.00         1.00           outry flow rate (vph)         233         12         7         589         9         13           edestrians         10         3.7         10         10         100         100         1.01         10           ane Width (m)         3.7         1         11	Future Volume (Veh/h)			-			
rade 0% 0% 0% eak Hour Factor 1.00 1.00 1.00 1.00 1.00 1.00 ourly flow rate (vph) 233 12 7 589 9 13 dedestrians 10 ane Width (m) 3.7 falking Speed (m/s) 1.1 ercent Blockage 1 1 ight turn flare (veh) edian type None None edian storage veh) pstream signal (m) 85 K, platoon unblocked 2, conflicting volume 255 852 249 C1, stage 1 conf vol 22, stage 2 conf vol 22, stage 2 conf vol 22, stage 2 conf vol 22, stage 2 conf vol 22, stage 3 conf vol 23, single (s) 2.2 3.5 3.3 0 queue free % 99 97 98 M capacity (veh/h) 1309 328 787 irrection, Lane # EB 1 WB 1 NB 1 olume Total 245 596 22 olume Left 0 7 9 olume Right 12 0 13 SH 1700 1309 500 olume Capacity 0.14 0.01 0.04 ueue Length 95th (m) 0.0 0.1 1.0 ontrol Delay (s) 0.0 0.2 12.5 pproach LOS A B pproach LOS A A B	Sign Control	Free			Free	Stop	
Beak Hour Factor         1.00	Grade	0%			0%		
State         233         12         7         589         9         13           addestrians         10         3.7         10           ane Width (m)         3.7         11           ane Width (m)         3.7         11           arecent Blockage         1         11           edian storage veh)         11         11           petream signal (m)         85         10           X, platoon unblocked         255         852         249           C, conflicting volume         255         852         249           C1, stage 1 conf vol         225         852         249           C2, stage 2 conf vol         22         3.5         3.3           O queue free %         99         97         98           M capacity (veh/h)         1309         328         787           irection, Lane #         EB 1         WB 1         NB 1           olume Total         245         596         22           olume Right         12         0         13           SH         1700         1309         500           olume Left         0         7         9           ourotol Delay (s) <td< td=""><td>Peak Hour Factor</td><td>1.00</td><td>1.00</td><td>1.00</td><td></td><td>1.00</td><td>1.00</td></td<>	Peak Hour Factor	1.00	1.00	1.00		1.00	1.00
ane Width (m)       3.7         ane Width (m)       3.7         Ialking Speed (m/s)       1.1         ercent Blockage       1         ight turn flare (veh)       1         edian type       None         edian storage veh)       85         K, platoon unblocked       255         2, conflicting volume       255         2, stage 1 conf vol       255         2, stage 2 conf vol       22         2, single (s)       4.1         (s)       2.2         2, single (s)       4.1         (s)       2.2         3.3       3.0         0 queue free %       99         99       97         M capacity (veh/h)       1309         3.28       787         irection, Lane #       EB 1       WB 1         VB 1       10         olume Right       12       0         3.9       0.4       10         outme to Capacity       0.14       0.01         outme Left       0       7         oproach Delay (s)       0.0       0.2         optract LOS       A       B         pproach LOS       B		233	12	7	589	9	13
ane Width (m)       3.7         falking Speed (m/s)       1.1         ercent Blockage       1         ight tum flare (veh)       11         edian type       None         conflicting volume       255         X, platoon unblocked       255         C2, stage 1 conf vol       255         C3, stage 1 conf vol       255         C4, stage 2 conf vol       255         C4, stage (s)       22         (s)       309         Outume free %       99         99       97         98       98         Olume Left       0         0       7         99       97         98       100         99       101         0.0<	Pedestrians					10	
/alking Speed (m/s)       1.1         ercent Blockage       1         ight turn flare (veh)       1         edian type       None       None         conflicting volume       255       852       249         C1, stage 1 conf vol       255       852       249         C2, stage 1 conf vol       255       852       249         C3, stage 1 conf vol       255       852       249         C4, atop 1       6.4       6.2       2       3.5         C4, atop 2       3.5       3.3       3.0       3.0       3.0         Outpue free %       99       97       9.8       <							
ercent Blockage       1         ight turn flare (veh)       edian storage veh)         edian storage veh)       pstream signal (m)         pstream signal (m)       85         X, platoon unblocked       255         C, conflicting volume       255         22, stage 1 conf vol       255         Cu, unblocked vol       255         Stage 2 conf vol       22         Cu, unblocked vol       255         S, single (s)       4.1         C, single (s)       2.2         S, single (s)       309         O queue free %       99         99       97         Varaacity (veh/h)       1309         1309       328         Olume Total       245         Olume Right       12         12       0         SH       1700         Olume Kight       1.0         Olume Kight       0.0         0.14       0.01       0.04         ueue Length 95th (m)       0.0       2.2         Dip	Walking Speed (m/s)					1.1	
ight turn flare (veh) edian type None None edian storage veh) pstream signal (m) 85 X, platoon unblocked C, conflicting volume 255 852 249 C1, stage 1 conf vol C2, stage 2 conf vol C2, stage 2 conf vol C2, unblocked vol 255 852 249 , single (s) 4.1 6.4 6.2 C2, 2 stage (s) (s) 2.2 3.5 3.3 O queue free % 99 97 98 M capacity (veh/h) 1309 328 787 irection, Lane # EB 1 WB 1 NB 1 olume Total 245 596 22 olume Left 0 7 9 olume Right 12 0 13 SH 1700 1309 500 olume to Capacity 0.14 0.01 0.04 ueue Length 95th (m) 0.0 0.1 1.0 ontrol Delay (s) 0.0 0.2 12.5 pproach Delay (s) 0.0 0.2 12.5 pproach LOS B tersection Summary verage Delay 0.4 tersection Capacity Utilization 48.6% ICU Level of Service	Percent Blockage					1	
edian type         None         None           edian storage veh)         pstream signal (m)         85           X, platoon unblocked         255         852         249           C1, stage 1 conf vol         255         852         249           C1, stage 2 conf vol         225         852         249           C2, stage 2 conf vol         22         255         852         249           C2, stage 2 conf vol         22         3.5         3.3         2           C2, stage (s)         2.2         3.5         3.3         3           C, upublocked vol         2.22         3.5         3.3           O queue free %         99         97         98           M capacity (veh/h)         1309         328         787           irrection, Lane #         EB 1         WB 1         NB 1         1           olume Total         245         596         22         0         1           olume Total         0         7         9         9         1         1         1         0         1         1         0         1         1         0         1         1         0         1         1         1         1	Right turn flare (veh)						
edian storage veh)       85         ypstream signal (m)       85         X, platoon unblocked       255       852       249         C1, stage 1 conf vol       255       852       249         C2, stage 2 conf vol       200       200       200         C2, stage 2 conf vol       255       852       249         C2, stage 2 conf vol       200       255       852       249         C3, single (s)       4.1       6.4       6.2       200	Median type	None			None		
Bit         85           X, platoon unblocked         255         852         249           C, conflicting volume         255         852         249           C1, stage 1 conf vol         255         852         249           C2, stage 2 conf vol         255         852         249           C1, unblocked vol         255         852         249           C, single (s)         4.1         6.4         6.2           C, 2 stage (s)         5         3.3         0           (s)         2.2         3.5         3.3           O queue free %         99         97         98           M capacity (veh/h)         1309         328         787           irrection, Lane #         EB 1         WB 1         NB 1           olume Total         245         596         22           olume Left         0         7         9           olume Right         12         0         13           SH         1700         1309         500           olume to Capacity         0.14         0.01         0.04           ueue Length 95th (m)         0.0         0.2         12.5           ane LOS <t< td=""><td>Median storage veh)</td><td></td><td></td><td></td><td></td><td></td><td></td></t<>	Median storage veh)						
X, platoon unblocked       255       852       249         C1, stage 1 conf vol       255       852       249         C1, stage 1 conf vol       255       852       249         C1, unblocked vol       255       852       249         C2, stage 2 conf vol       4.1       6.4       6.2         C2, stage (s)	Upstream signal (m)	85					
C, conflicting volume       255       852       249         C1, stage 1 conf vol       255       852       249         C2, stage 2 conf vol       255       852       249         Cu, unblocked vol       255       852       249         C, single (s)       4.1       6.4       6.2         C, 2 stage (s)       22       3.5       3.3         C queue free %       99       97       98         M capacity (veh/h)       1309       328       787         irection, Lane #       EB 1       WB 1       NB 1         olume Total       245       596       22         olume Right       12       0       13         SH       1700       1309       500         olume to Capacity       0.14       0.01       0.04         ueue Length 95th (m)       0.0       0.2       12.5         ane LOS       A       B       9         pproach Delay (s)       0.0       0.2       12.5         pproach LOS       B       E       12.5         pproach LOS       B       12.5       12.5         pproach LOS       B       12.5         pproach LOS							
C1, stage 1 conf vol       22, stage 2 conf vol         C2, stage 2 conf vol       255       852       249         Cu, unblocked vol       255       852       249         C, single (s)       4.1       6.4       6.2         C, 2 stage (s)       2.2       3.5       3.3         O queue free %       99       97       98         M capacity (veh/h)       1309       328       787         irection, Lane #       EB 1       WB 1       NB 1         olume Total       245       596       22         olume Right       12       0       13         SH       1700       1309       500         olume to Capacity       0.14       0.01       0.04         ueue Length 95th (m)       0.0       0.1       1.0         ontrol Delay (s)       0.0       0.2       12.5         ane LOS       A       B       5         pproach Delay (s)       0.0       0.2       12.5         pproach LOS       B       5       5         tersection Summary       0.4       102       102         verage Delay       0.4       102       102         tersection Capacity Uti				255		852	249
C2, stage 2 conf vol       255       852       249         Cu, unblocked vol       255       852       249         C, single (s)       4.1       6.4       6.2         C, 2 stage (s)       2.2       3.5       3.3         O queue free %       99       97       98         M capacity (veh/h)       1309       328       787         irection, Lane #       EB 1       WB 1       NB 1         olume Total       245       596       22         olume Right       12       0       13         SH       1700       1309       500         olume to Capacity       0.14       0.01       0.04         ueue Length 95th (m)       0.0       0.2       12.5         ane LOS       A       B       9         pproach Delay (s)       0.0       0.2       12.5         pproach LOS       B       E       12.5         pproach LOS       B       12.5       12.5         pproach LOS	vC1, stage 1 conf vol			200			2.0
Cu, unblocked vol         255         852         249           c, single (s)         4.1         6.4         6.2           c, 2 stage (s)         2.2         3.5         3.3           0 queue free %         99         97         98           M capacity (veh/h)         1309         328         787           irection, Lane #         EB 1         WB 1         NB 1           olume Total         245         596         22           olume Right         12         0         13           SH         1700         1309         500           olume to Capacity         0.14         0.01         0.04           ueue Length 95th (m)         0.0         0.2         12.5           ane LOS         A         B            pproach Delay (s)         0.0         0.2         12.5           pproach LOS         B         E         E           tersection Summary         0.4         102 Level of Service	vC2, stage 2 conf vol						
c, single (s)       4.1       6.4       6.2         c, 2 stage (s)       2.2       3.5       3.3         0 queue free %       99       97       98         M capacity (veh/h)       1309       328       787         irection, Lane #       EB 1       WB 1       NB 1       VIII NB 1         olume Total       245       596       22       VIIII NB 1         olume Right       12       0       13       VIIII NB 1         olume to Capacity       0.14       0.01       0.04         ueue Length 95th (m)       0.0       0.2       12.5         ane LOS       A       B       VIIIIIIII NB 1       VIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII	vCu, unblocked vol			255		852	249
c), 2 stage (s)       2.2       3.5       3.3         0 queue free %       99       97       98         M capacity (veh/h)       1309       328       787         irection, Lane #       EB 1       WB 1       NB 1         olume Total       245       596       22         olume Right       12       0       13         SH       1700       1309       500         olume to Capacity       0.14       0.01       0.04         ueue Length 95th (m)       0.0       0.2       12.5         ane LOS       A       B         pproach Delay (s)       0.0       0.2       12.5         pproach LOS       B       Etersection Summary       Verage Delay       0.4         tersection Capacity Utilization       48.6%       ICU Level of Service	tC, single (s)						
(s)       2.2       3.5       3.3         D queue free %       99       97       98         M capacity (veh/h)       1309       328       787         irection, Lane #       EB 1       WB 1       NB 1       1         olume Total       245       596       22       22         olume Right       12       0       13       3         SH       1700       1309       500       3       3         olume to Capacity       0.14       0.01       0.04       4       4         ueue Length 95th (m)       0.0       0.2       12.5       3       3       5         ane LOS       A       B       B       5						••••	
O queue free %         99         97         98           M capacity (veh/h)         1309         328         787           irrection, Lane #         EB 1         WB 1         NB 1           olume Total         245         596         22           olume Left         0         7         9           olume Right         12         0         13           SH         1700         1309         500           olume to Capacity         0.14         0.01         0.04           ueue Length 95th (m)         0.0         0.1         1.0           ontrol Delay (s)         0.0         0.2         12.5           ane LOS         A         B           pproach Delay (s)         0.0         0.2         12.5           pproach LOS         B         Etersection Summary           verage Delay         0.4         ICU Level of Service	tF (s)			2.2		3.5	3.3
I capacity (veh/h)         1309         328         787           irrection, Lane #         EB 1         WB 1         NB 1                328         787           irrection, Lane #         EB 1         WB 1         NB 1 <td>p0 queue free %</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	p0 queue free %						
irrection, Lane #         EB 1         WB 1         NB 1           olume Total         245         596         22           olume Left         0         7         9           olume Right         12         0         13           SH         1700         1309         500           olume to Capacity         0.14         0.01         0.04           ueue Length 95th (m)         0.0         0.1         1.0           ontrol Delay (s)         0.0         0.2         12.5           ane LOS         A         B           pproach Delay (s)         0.0         0.2         12.5           pproach Delay (s)         0.0         0.2         12.5           pproach LOS         B         B         12.5           pproach LOS         B         12.5         12.5           pproach LOS         B         12.5         12.5           pproach LOS         B         12.5         12.5           pproach LOS         0.4         12.5         12.5           percentin Summary         0.4         12.5         12.5           output         0.4         12.5         12.5           output <t< td=""><td>cM capacity (veh/h)</td><td></td><td></td><td></td><td></td><td></td><td></td></t<>	cM capacity (veh/h)						
olume Total         245         596         22           olume Left         0         7         9           olume Right         12         0         13           SH         1700         1309         500           olume to Capacity         0.14         0.01         0.04           ueue Length 95th (m)         0.0         0.1         1.0           ontrol Delay (s)         0.0         0.2         12.5           ane LOS         A         B           pproach Delay (s)         0.0         0.2         12.5           pproach LOS         B         B           tersection Summary         Verage Delay         0.4           tersection Capacity Utilization         48.6%         ICU Level of Service		FR 1	W/R 1				
olume Left         0         7         9           olume Right         12         0         13           SH         1700         1309         500           olume to Capacity         0.14         0.01         0.04           ueue Length 95th (m)         0.0         0.1         1.0           ontrol Delay (s)         0.0         0.2         12.5           ane LOS         A         B           pproach Delay (s)         0.0         0.2         12.5           pproach LOS         B         B           tersection Summary         0.4         LCU Level of Service							
Image: Dolume Right         12         0         13           SH         1700         1309         500           olume to Capacity         0.14         0.01         0.04           ueue Length 95th (m)         0.0         0.1         1.0           ontrol Delay (s)         0.0         0.2         12.5           ane LOS         A         B           pproach Delay (s)         0.0         0.2         12.5           pproach LOS         B         B           tersection Summary         0.4         tersection Capacity Utilization							
Initial SH         Initial Transmission         Initearest and transmission         Initial Tran			-	-			
olume to Capacity         0.14         0.01         0.04           ueue Length 95th (m)         0.0         0.1         1.0           ontrol Delay (s)         0.0         0.2         12.5           ane LOS         A         B           pproach Delay (s)         0.0         0.2         12.5           pproach LOS         B         B           tersection Summary         0.4         E           verage Delay         0.4         ICU Level of Service	cSH						
ueue Length 95th (m)         0.0         0.1         1.0           ontrol Delay (s)         0.0         0.2         12.5           ane LOS         A         B           pproach Delay (s)         0.0         0.2         12.5           pproach LOS         B         B           tersection Summary         0.4         ICU Level of Service							
ontrol Delay (s)         0.0         0.2         12.5           ane LOS         A         B           pproach Delay (s)         0.0         0.2         12.5           pproach LOS         B         B           tersection Summary         0.4         1CU Level of Service							
ane LOS A B pproach Delay (s) 0.0 0.2 12.5 pproach LOS B tersection Summary verage Delay 0.4 tersection Capacity Utilization 48.6% ICU Level of Service							
pproach Delay (s) 0.0 0.2 12.5 pproach LOS B tersection Summary verage Delay 0.4 tersection Capacity Utilization 48.6% ICU Level of Service	Control Delay (s)	0.0					
pproach LOS B tersection Summary verage Delay 0.4 tersection Capacity Utilization 48.6% ICU Level of Service		0.0		-			
tersection Summary verage Delay tersection Capacity Utilization 48.6% ICU Level of Service		0.0	0.2				
verage Delay 0.4 tersection Capacity Utilization 48.6% ICU Level of Service	Approach LOS			В			
tersection Capacity Utilization 48.6% ICU Level of Service	Intersection Summary						
	Average Delay						
nalvsis Period (min) 15	Intersection Capacity Utilization				IC	U Level of S	ervice
	Analysis Period (min)			15			

#### 11: Highcroft Avenue & Access PM Peak

	≯	$\mathbf{i}$	•	1	Ļ	∢
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	<b>5</b>			ដ	1,	
Traffic Volume (veh/h)	13	0	0	9	1	19
Future Volume (Veh/h)	13	0	0	9	1	19
Sign Control	Stop	-	-	Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	13	0	0	9	1.00	19
Pedestrians	10	0	0	5	I	15
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
				None	None	
Median type				NOTE	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	20	10	20			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	20	10	20			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	100	100			
cM capacity (veh/h)	998	1071	1596			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	13	9	20			
Volume Left	13	0	0			
Volume Right	0	0	19			
cSH	998	1596	1700			
Volume to Capacity	0.01	0.00	0.01			
Queue Length 95th (m)	0.3	0.0	0.0			
Control Delay (s)	8.7	0.0	0.0			
Lane LOS	0.7 A	0.0	0.0			
		0.0	0.0			
Approach Delay (s)	8.7	0.0	0.0			
Approach LOS	А					
Intersection Summary						
Average Delay			2.7			
Intersection Capacity Utilization			13.3%	IC	U Level of Serv	vice
Analysis Period (min)			15			

### APPENDIX K

Intersection MMLOS Analysis

### Pedestrian Level of Service (PLOS)

Criteria	North Approach		South Approach		East Approach		West Approach	
Churchill Avenue/Byron A	Venue							
			PETSI SCORE					
CROSSING DISTANCE CONDITIO	DNS							
Median > 2.4m in Width	No	00	No	00	No	00	No	00
Lanes Crossed (3.5m Lane Width)	4	88	4	88	4	88	4	88
SIGNAL PHASING AND TIMING	·							
Left Turn Conflict	Permissive	-8	Permissive	-8	Permissive	-8	Permissive	-8
Right Turn Conflict	Permissive or Yield	-5						
Right Turn on Red	RTOR Allowed	-3						
Leading Pedestrian Interval	No	-2	No	-2	No	-2	No	-2
CORNER RADIUS								
Parallel Radius	> 5m to 10m	-5						
Parallel Right Turn Channel	No Right Turn Channel	-4						
Perpendicular Radius	N/A	0	N/A	0	N/A	0	N/A	0
Perpendicular Right Turn Channel	N/A	0	N/A	0	N/A	0	N/A	0
CROSSING TREATMENT								
Treatment	Zebra Stripe	-4						
	PETSI SCORE	57		57		57		57
	LOS	D		D		D		D
			DELAY SCORE					
Cycle Length		80		80		80		80
Pedestrian Walk Time		17.4		17.4		27.6		27.6
	DELAY SCORE	24.5		24.5		17.2		17.2
	LOS	С		С		В		В
	OVERALL	D		D		D		D

Criteria	North Approach		South Approach		East Approach		West Approach	
Churchill Avenue/Richmo	nd Road							
			PETSI SCORE					
CROSSING DISTANCE CONDITIO	DNS							
Median > 2.4m in Width	No	88	No	88	No	88	No	88
Lanes Crossed (3.5m Lane Width)	4	00	4	00	4	00	4	00
SIGNAL PHASING AND TIMING								
Left Turn Conflict	Perm + Prot	-8	Permissive	-8	Permissive	-8	Permissive	-8
Right Turn Conflict	Permissive or Yield	-5						
Right Turn on Red	RTOR Allowed	-3						
Leading Pedestrian Interval	Yes	0	Yes	0	Yes	0	Yes	0
CORNER RADIUS								
Parallel Radius	> 5m to 10m	-5						
Parallel Right Turn Channel	No Right Turn Channel	-4						
Perpendicular Radius	N/A	0	N/A	0	N/A	0	N/A	0
Perpendicular Right Turn Channel	N/A	0	N/A	0	N/A	0	N/A	0
CROSSING TREATMENT								
Treatment	Zebra Stripe	-4						
	PETSI SCORE	59		59		59		59
	LOS	D		D		D		D
			DELAY SCORE					
Cycle Length		80		80		80		80
Pedestrian Walk Time		15.9		24.9		14.8		14.8
	DELAY SCORE	25.7		19		26.6		26.6
	LOS	С		В		С		С
	OVERALL	D		D		D		D

### Bicycle Level of Service (BLOS)

Approach	Bikeway Facility Type	Criteria	Travel Lanes and/or Speed	BLOS
Churchill Avenu	e/Byron Avenu	le		
North Approach	Mixed Traffic	Right Turn Lane Characteristics	No impact on LTS	А
		Left Turn Accommodation	One lane crossed, 60 km/hr	F
South Approach	Mixed Traffic	Right Turn Lane Characteristics	No impact on LTS	А
		Left Turn Accommodation	One lane crossed, 60 km/hr	F
East Approach	Mixed Traffic	Right Turn Lane Characteristics	No impact on LTS	А
		Left Turn Accommodation	No lanes crossed, 60 km/hr	D
West Approach	Bike Lane	Right Turn Lane Characteristics	No impact on LTS	А
	Dike Lane	Left Turn Accommodation	No lanes crossed, 60 km/hr	С
Churchill Avenu	e/Richmond R	oad		
North Approach	Pocket Bike	Right Turn Lane Characteristics	No impact on LTS	А
	Lane	Left Turn Accommodation	No lanes crossed, 60 km/hr	D
South Approach	Bike Lane	Right Turn Lane Characteristics	No impact on LTS	А
	Dire Lane	Left Turn Accommodation	No lanes crossed, 60 km/hr	D
East Approach	Mixed Traffic	Right Turn Lane Characteristics	No impact on LTS	А
		Left Turn Accommodation	One lane crossed, 60 km/hr	F
West Approach	Mixed Traffic	Right Turn Lane Characteristics	No impact on LTS	А
West Approach		Left Turn Accommodation	One lane crossed, 60 km/hr	F

### Transit Level of Service (TLOS)

Approach	Facility Type	Delay <sup>1</sup>	TLOS
Churchill Avenue	/Byron Avenue		
North Approach	Mixed Traffic (No TSP)	20 seconds	С
South Approach	Mixed Traffic (No TSP)	19 seconds	С
East Approach	Mixed Traffic (No TSP)	N/A <sup>2</sup>	N/A <sup>2</sup>
West Approach	Mixed Traffic (No TSP)	N/A <sup>2</sup>	N/A <sup>2</sup>
Churchill Avenue	Richmond Road		
North Approach	Mixed Traffic (No TSP)	40 seconds	F
South Approach	Mixed Traffic (No TSP)	61 seconds	F
East Approach	Mixed Traffic (No TSP)	36 seconds	E
West Approach	Mixed Traffic (No TSP)	19 seconds	С

1. Mixed traffic delay based on the critical approach delay in Synchro analysis

2. No east-west transit along Byron Avenue

#### Truck Level of Service (TkLOS)

Approach	Effective Corner Radius	Number of Receiving Lanes on Departure from Intersection	LOS								
Churchill Avenue	Churchill Avenue/Byron Avenue										
North Approach	< 10m	One	F								
South Approach	< 10m	One	F								
East Approach	< 10m	One	F								
West Approach	< 10m	One	F								
Churchill Avenue	/Richmond Road										
North Approach	< 10m	One	F								
South Approach	< 10m	One	F								
East Approach	< 10m	One	F								
West Approach	< 10m	One	F								

		AM Peak		PM Peak			
Approach	V/C or Delay	LOS	Mvmt	V/C or Delay	LOS	Mvmt	
Churchill Aven	ue/Byron	Avenue					
North Approach	0.32	А	SBT/R	0.55	А	SBT/R	
South Approach	0.39	А	NBT/R	0.55	A	NBT/R	
East Approach	0.75	С	WB	0.92	D	WB	
West Approach	0.75	С	EB	0.45	А	EB	
Churchill Aven	ue/Richm	ond Road	l				
North Approach	0.54	А	SB	0.82	D	SB	
South Approach	0.61	В	NB	0.96	Е	NB	
East Approach	0.62	В	WBT/R	0.86	D	WBT/R	
West Approach	0.93	Е	EBL	0.71	С	EBL	

#### Auto LOS

### MMLOS Summary Table

			Churchill Avenu	e/Bvron Avenue			Churchill Avenue	Richmond Road	
	Intersection	North Approach	South Approach		West Approach	North Approach	South Approach		West Approach
	Median > 2.4m in Width	No	No	No	No	No	No	No	No
	Lanes (3.5m Lane Width)	Four	Four	Four	Four	Four	Four	Four	Four
	Conflicting Left Turns	Permissive	Permissive	Permissive	Permissive	Perm + Prot	Permissive	Permissive	Permissive
	Conflicting Right Turns	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive	Permissive
	Right Turn on Red	Permitted	Permitted	Permitted	Permitted	Permitted	Permitted	Permitted	Permitted
	Pedestrian Leading Interval	No	No	No	No	Yes	Yes	Yes	Yes
Pedestrian	Parallel Radius	5m to 10m	5m to 10m	5m to 10m	5m to 10m	5m to 10m	5m to 10m	5m to 10m	5m to 10m
stri	Parallel Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel	No Channel
les	Perpendicular Radius	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
e e	Perpendicular Channel	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
	Crosswalk Type	Zebra Stripe	Zebra Stripe	Zebra Stripe	Zebra Stripe	Zebra Stripe	Zebra Stripe	Zebra Stripe	Zebra Stripe
	PETSI Score	57	57	57	57	59	59	59	59
	Delay Score	24.5	24.5	17.2	17.2	25.7	19	26.6	26.6
		D	D	D	D	D	D	D	D
	Level of Service		[	)	1		[	)	•
	Target		(	;			(	)	
	Type of Bikeway	Mixed Traffic	Mixed Traffic	Mixed Traffic	Bike Lane	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic
	Turning Speed	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
	Right Turn Storage	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
	Dual Right Turn Lanes	No	No	No	No	No	No	No	No
ц.	Shared Through-Right Lane	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Cyclist	Bike Box	No	No	No	No	No	No	No	No
Š	Lanes Crossed for Left Turns	One	One	None	None	None	None	One	One
0	Dual Left Turn Lanes	No	No	No	No	No	No	No	No
	Approach Speed	60 km/hr	60 km/hr	60 km/hr	60 km/hr	60 km/hr	60 km/hr	60 km/hr	60 km/hr
		F	F	D	С	D	D	F	F
	Level of Service			-			F		
	Target		Ε	3					
	Facility Type	Mixed Traffic	Mixed Traffic	N/A	N/A	Mixed Traffic	Mixed Traffic	Mixed Traffic	Mixed Traffic
sit	Average Signal Delay	20 Seconds	19 Seconds	N/A	N/A	40 Seconds	61 Seconds	36 Seconds	19 Seconds
ransit		С	С	-	-	F	F	E	С
Tr:	Level of Service		(	)	1		F	-	•
	Target								
	Turning Radius	< 10m	< 10m	< 10m	< 10m	> 15m	> 15m	10m to 15m	10m to 15m
<u>×</u>	Receiving Lanes	One	One	One	One	One	One	One	One
Truck		F	D	F	F	С	С	E	E
É.	Level of Service		F	:	1		E		•
	Target			)				)	
	Volume to Capacity Ratio	0.55	0.55	0.92	0.75	0.82	0.96	0.86	0.93
2		А	А	E	С	D	Е	D	E
Auto	Level of Service		Ē				E		
	Target								