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July 3, 2018 File: PH3333-LET.01

Mr. Abdo El-Arab 6175 Rockdale Road Vars, ON K0A 3H0 Geotechnical Engineering Environmental Engineering Archaeological Studies Hydrogeology Geological Engineering Materials Testing Building Science

Attention: Mr. Abdo El-Arab

www.patersongroup.ca

Subject: Water Supply Assessment for a

Proposed Site Plan Approval

6175 Rockdale Road -

Vars, Ontario

INTRODUCTION

Further to your request, this firm has conducted a Water Supply Assessment in support of site plan approval of a proposed re-development of the commercial property located at 6175 Rockdale Road, Vars, Ontario. The purpose of these works has been to determine the suitability of the water supply aquifer underlying the site to service a re-development of the existing commercial layout.

DESCRIPTION OF PROPOSED DEVELOPMENT

The subject property is located at the southeast corner of Rockdale Road and Russland Road/Highway Lane in Vars, Ontario. The property consists of approximately 0.9 ha. over two lots. The lot is occupied by two commercial businesses which are serviced by an on site sewage system and a drilled well. The businesses consist of an Esso fuel station and a used car sales lot. There are two parcels at the existing development with the municipal address of 6175 Rockdale Road.

The land is to be re-developed with a new configuration with an upgraded Esso fuel station and convenience store. A drive-through may be incorporated using paper service only. Refer to Figure 1 below showing the proposed site location.

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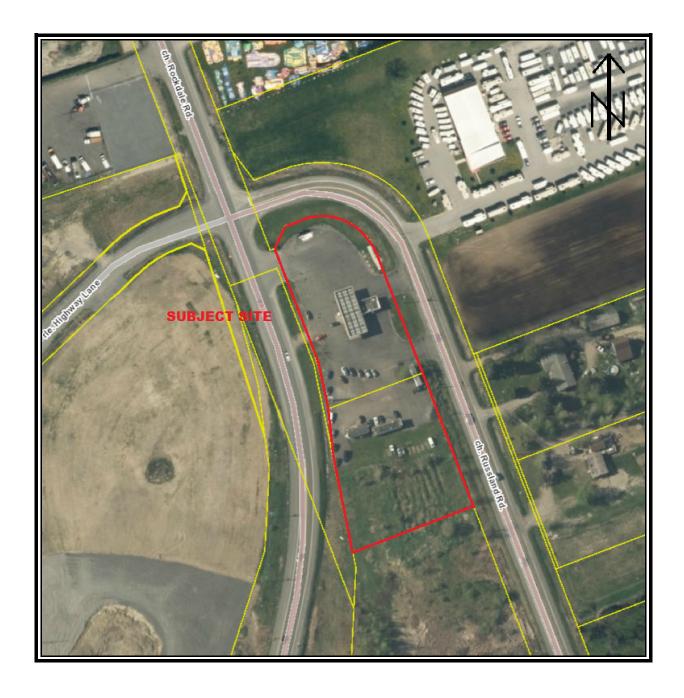


Figure 1: Key Plan

FIELDWORK PROGRAM

As a means to demonstrate the adequacy of the overburden aquifer underlying the subject lands, with respect to water quality and quantity, a shallow dug well was constructed by Maurice Cayer Ltd on March 16, 2018. The dug well (TW 1) was constructed adjacent to the northwest corner of the subject site and is located greater than 30 m from the proposed fuel tanks and the proposed sewage system. The Ministry of the Environment and Climate

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Change (MOECC) Water Well Record (WWR) indicates the well extends 4.9 m below the existing ground surface. The inside diameter of the well is 1.2 m and consisted of three tiles of 1.8 m height. See Paterson Drawing PH3333-3 for the approximate location of TW 1.

The tiles were set into limestone bedrock to a depth of approximately 1.2 m. Bedrock was encountered at 4.3 m depth and the casing extends down to approximately 5.3 m depth. The overburden material around the well casing consisted of a yellow sand to grey sand with some clay. A copy of the WWR can be found attached.

There is an existing drilled well on the site that is located west of the existing fuel bar and convenience store, and is located at the west edge of the asphalt parking area. The well is fully accessible with the 150 mm diameter steel casing extending approximately 200 - 300 mm above the existing ground surface. Due to the poor quality of water supply from the existing drilled well and the known poor water quality of the bedrock aquifer in the area, the owner elected to proceed with the installation of a dug well, similar to the adjacent development at 6135 Rockdale Road. As such, it is recommended the water well should be decommissioned by a certified well technician as per O. Reg. 903.

As a means to evaluate the water supply aquifer intercepted by the new well (TW 1), the well was subjected to a 6 hour constant rate pumping test. The pumping test was conducted on May 1, 2018 under the full-time supervision of Paterson.

Maurice Cayer Ltd. was retained to supply a submersible pump and generator for the pumping test. The submersible pump was placed approximately 0.3 m off the base of the well. The discharge hose was directed to the adjacent ditch along Russland Road in a downgradient direction based upon the slope of the existing landscaping and ditch. The discharge location was approximately 24 m downgradient of the well with the discharged water heading southward along Russland Road.

The pumping test (May 1, 2018) was carried out at a pumping rate of 22.5 L/min for a duration of 6 hours. Thereafter the pumping rate was lowered to 9 L/min for an additional 2.5 hours to determine if the turbidity level could be reduced. Additional pumping was performed on May 9, 2018, in an attempt, to reduce the turbidity level and to recover a bacteriological sample. During the pumping test, the pumping rate was periodically measured using the timed volume correlation method. The pump rate was maintained within 5% of the selected pump rate. The static water level was recorded and an electronic datalogger (Schlumberger Micro-Diver) was installed in the test well prior to the start of the pumping test. The data logger recorded water levels at 15 second intervals. In addition, manual water level readings were taken at periodic intervals during the test.

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Recovery data was collected for the well following the completion of the pumping. The well was noted to have achieved approximately 95% recovery approximately 85 minutes after the completion of the first pumping test. Further development of the well was performed and the water level was monitored for several days after the completion of the pumping.

Groundwater samples were collected at 3 hours and 8.5 hours after the start of pumping. Prior to collection of the groundwater samples, the free chlorine residual was tested and found to be within a range of 7.7 mg/L at the 3 hour mark and dropped to 0.06 mg/L by the end of the 8.5 hour period. The additional pumping, carried out on May 9, 2018, extended for a period of 7 hours, after which the free chlorine residual was verified to be non-detectable. The water samples were submitted for comprehensive testing of bacteriological, chemical and physical water quality parameters consistent with the standard 'Subdivision Supply' suite of parameters and additional parameters for VOCs and PHCs F1-F2.

An additional sample was taken of the raw water from the existing drilled well. There is no WWR available at the time of writing this report.

All samples were collected unfiltered and unchlorinated and were placed directly into clean bottles supplied by the analytical laboratory. Samples were placed immediately into a cooler with ice and were transported directly to the Eurofins laboratory in Ottawa. All samples were received by the laboratory within 24 hours of collection.

Furthermore, a series of field testing of the pumped water were carried out at the well head. The parameters tested at the well head included: pH, total dissolved solids, conductivity, turbidity and temperature.

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AQUIFER ANALYSIS

Water Quantity

Pumping test data was analyzed using AquiferTest Pro (v. 2016.1) aquifer analysis software package by Schlumberger Water Services. Drawdown data was measured using an electronic water level tape and an electronic datalogger unit was also used to monitor drawdown in the test well.

| TABLE 1:SUMMARY OF WATER SUPPLY AQUIFER CHARACTERISTICS OF TW1 | | | | | |
|--|------------------------|--|--|--|--|
| AQUIFER PARAMETER | RESULT OF ANALYSIS | | | | |
| Transmissivity (m²/day) | 1.33 x 10 ² | | | | |
| Pumping Rate (L/min) | 22.5 | | | | |
| Pre-test Static Water Level (m) | 2.08 | | | | |
| Post-test Static Water level (m) | 2.83 | | | | |
| Available Drawdown (s) (m) | 3.41 | | | | |
| % Drawdown During Pumping Test | 22.0 | | | | |
| Specific Capacity (L/min/m drawdown) | 30.0 | | | | |

The drawdown data was analyzed using the Theis with Jacob Correction, and the Papadopulos & Cooper methods of analysis. Aquifer transmissivity is estimated to be approximately 133 m²/day.

The pumping test results show that test well TW1 has a high yield. Drawdown at a pumping rate of 22.5 L/min for 6 hours was 0.75 m. 95% recovery was achieved approximately 85 minutes after the end of pumping. The total volume of water pumped during the 6 hour pumping event was approximately 8,100 L.

The suitability of the aquifer to supply the proposed re-development was assessed based upon the methodology provided in MOECC Procedure D-5-5 (MOEE, 1996) and the proposed sewage daily design flows of 9,145 L/day. The usage of the water supply is proposed to be for the Service Station consisting of the gas bar at 560 L/day per gas nozzle (total of 12 nozzles), the convenience store at 155 m² at 5 L/day and two staff at 75 L/day. The water supply is intended to be used for hand washing and toilet supply only. The owner intends on importing bottled water for drinking purposes.

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Based on the information summarized in Table 1, it is readily apparent that the new water supply well has intercepted a strong water supply aquifer which has more than sufficient quantity to service a fuel service station's needs of pump servicing, hand washing and bathroom facilities. The transmissivity aquifer parameter suggests a strong aquifer which is able to transmit significant quantities of water relatively quickly. It should be noted that overburden aquifer's quantity may vary seasonally.

Water Quality

Field Data

Turbidity, electrical conductivity, total dissolved solids, pH and temperature were measured at the wellhead during the pumping test. The measurements and time intervals for each

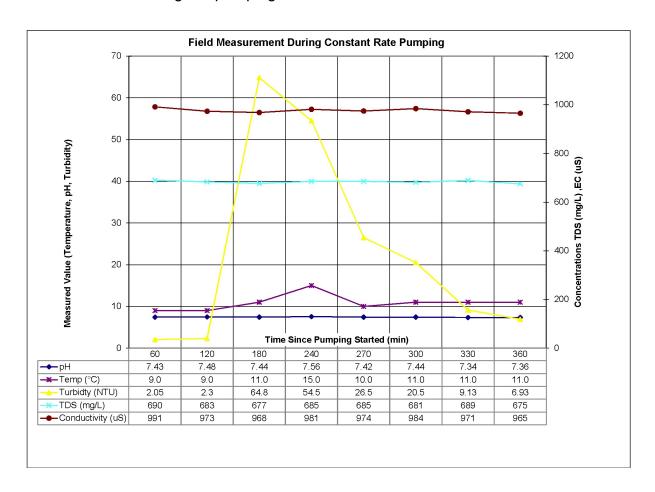


Figure 2: Field Measurement During Constant Rate Pumping

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of these parameters are summarized on the graphical representation in Figure 2. In addition, chlorine test strips and a Hach Colorimeter II were used to measure the chlorine residual level. No chlorine residual was detected in the discharge water prior to the collection of the bacteriological water sample recovered at the end of the May 9 pumping event.

Laboratory Data

The laboratory water quality, from the standard subdivision package, obtained from the pumping test of TW 1 is provided in Table 2 below and the full laboratory analyses reports that include the VOCs and PHC results can be found attached. The existing drilled well on the property was sampled to determine a comparison of the groundwater aquifer quality and the overburden aquifer quality with the sample WS#3 taken from the existing service station bathroom tap. There is no water treatment system within the existing Service Station.

The initial pumping test of TW1 contained a free chlorine residual at the completion of the pumping test and bacteriological testing was not performed on samples WS#1 or WS#2. Subsequent pumping to reduce turbidity and the free chlorine residual was performed with the bacteriological analysis results shown in Table 3 under WS#4.

The adjacent property at 6135 Rockdale Road is inferred to access the same or similar overburden aquifer. A sample was taken from an exterior tap to avoid the existing water treatment system (water softener). The results are presented below in Table 3 under SF#1.

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| TABLE 2: GROUNDWATER GEOCHEMISTRY (TW 1 AND EXISTING DRILLED WELL) | | | | | | |
|--|--------------|-----------|--------|------------|--------------|--------------|
| | | OD | WS | TW | Existing | |
| PARAMETER | AMETER UNITS | | | | | Drilled Well |
| 1 AIVAINETER | OMITO | LIMIT | TYPE | 1-May-18 | 1-May-18 | 1-May-18 |
| | | | | WS#1 (3hr) | WS#2 (8.5hr) | WS#3 |
| MICROBIOLOGICAL | | | | | | |
| Escherichia Coli (E.Coli) | ct/100mL | 0 | MAC | - | - | - |
| Total Coliforms | ct/100mL | 0 | MAC | - | - | - |
| GENERAL CHEMICAL - I | | | | | 1 | |
| Fluoride | mg/L | 1.5(2.4) | MAC | 0.15 | <0.10 | <0.10 |
| N-NO2 (Nitrite) | mg/L | 1 | MAC | <0.10 | <0.10 | <0.10 |
| N-NO3 (Nitrate) | mg/L | 10 | MAC | 0.35 | 0.22 | <0.10 |
| Turbidity (Laboratory) | NTU | 1.0 (5.0) | MAC/AO | 18.2 | 9 | 8.8 |
| Turbidity (Field) | NTU | 1.0 (5.0) | MAC/AO | 45.2 | 17.4 | - |
| N-NH3 (Ammonia) | mg/L | | | 0.03 | 0.10 | 1.2 |
| Total Kjeldahl Nitrogen | mg/L | | | 0.5 | 0.5 | 1.6 |
| GENERAL CHEMICAL - A | ESTHETIC R | ELATED | | | | |
| Hardness (as CaCO3) | mg/L | 100 | OG | 568 | 551 | 582 |
| Ion Balance | unitless | | | 0.92 | 0.9 | 0.9 |
| Total Dissolved Solids | mg/L | 500 | AO | 1,480 | 1,490 | 1,850 |
| Alkalinity (as CaCO3) | mg/L | 500 | OG | 330 | 418 | 503 |
| Chloride | mg/L | 250 | AO | 567 | 554 | 607 |
| Colour | TCU | 5 | AO | <2 | 2 | 5 |
| Conductivity | uS/cm | | | 2,270 | 2,290 | 2,850 |
| pH | unitless | 6.5-8.5 | AO | 7.82 | 7.79 | 7.74 |
| Sulphide | mg/L | 0.05 | AO | 0.03 | <0.02 | 0.47 |
| Sulphate | mg/L | 500 | AO | 54 | 82 | 53 |
| Calcium | mg/L | | | 196 | 191 | 177 |
| Iron | mg/L | 0.3 | AO | 0.36 | 0.22 | 0.77 |
| Potassium | mg/L | | | 14 | 7 | 9 |
| Magnesium | mg/L | | | 19 | 18 | 34 |
| Manganese | mg/L | 0.05 | AO | 0.5 | 0.42 | 1.22 |
| Sodium | mg/L | 200 | AO | 236 | 275 | 313 |
| Phenols | mg/L | | | <0.001 | <0.001 | <0.001 |
| Tannin & Lignin | mg/L | | | <0.1 | 0.1 | 0.5 |
| Dissolved Organic Carbon | mg/L | 5 | AO | 3.3 | 3.7 | 7.1 |

^{1.} ODWS identifies the following types of parameters:

MAC=Maximum Allowable Concentration

AO = Aesthetic Objective

OG= Operational Guideline

2. Shaded Concentration Indicates an Exceedance of the ODWS Objective

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| TABLE 3: GROUNDWATER GEOCHEMISTRY (TW 1 / 6135 ROCKDALE RD) | | | | | | | | | |
|---|----------|-------------|------------|--------------------|------------------|--|--|--|--|
| | | OD | ws | | 6135 Rockdale | | | | |
| PARAMETER | UNITS | LIMIT | TYPE | TW # 1 9-May-18 | Road 9-May-18 | | | | |
| | | | | WS#4 (7 hr) | SF#1 | | | | |
| MICROBIOLOGICAL | | | | | 1 - | | | | |
| Escherichia Coli (E.Coli) | ct/100mL | 0 | MAC | 0 | 0 | | | | |
| Total Coliforms | ct/100mL | 0 | MAC | 0 | 4 | | | | |
| GENERAL CHEMICAL - HEALTH RELATED | | | | | | | | | |
| Fluoride | mg/L | 1.5(2.4) | MAC | - | <0.10 | | | | |
| N-NO2 (Nitrite) | mg/L | 1 | MAC | - | <0.10 | | | | |
| N-NO3 (Nitrate) | mg/L | 10 | MAC | - | 0.21 | | | | |
| Turbidity (Laboratory) | NTU | 1.0 (5.0) | MAC/AO | - | 0.4 | | | | |
| Turbidity (Field) | NTU | 1.0 (5.0) | MAC/AO | 1.2 | - | | | | |
| N-NH3 (Ammonia) | mg/L | | | - | 0.08 | | | | |
| Total Kjeldahl Nitrogen | mg/L | | | - | 0.3 | | | | |
| GENERAL CHEMICAL - | 1 | | | | | | | | |
| Hardness (as CaCO3) | mg/L | 100 | OG | - | 443 | | | | |
| Ion Balance | unitless | | | - | 1.06 | | | | |
| Total Dissolved Solids | mg/L | 500 | AO | - | 533 | | | | |
| Alkalinity (as CaCO3) | mg/L | 500 | OG | - | 353 | | | | |
| Chloride | mg/L | 250 | AO | - | 18 | | | | |
| Colour | TCU | 5 | AO | - | 7 | | | | |
| Conductivity | uS/cm | | | - | 820 | | | | |
| pH | unitless | 6.5-8.5 | AO | - | 7.58 | | | | |
| Sulphide | mg/L | 0.05 | AO | - | <0.02 | | | | |
| Sulphate | mg/L | 500 | AO | - | 70 | | | | |
| Calcium | mg/L | | | - | 146 | | | | |
| Iron | mg/L | 0.3 | AO | - | < 0.03 | | | | |
| Potassium | mg/L | | | - | 3 | | | | |
| Magnesium | mg/L | | | - | 19 | | | | |
| Manganese | mg/L | 0.05 | AO | - | 0.06 | | | | |
| Sodium | mg/L | 200 | AO | - | 14 | | | | |
| Phenols | mg/L | | | - | <0.001 | | | | |
| Tannin & Lignin | mg/L | | | - | <0.1 | | | | |
| Dissolved Organic | mg/L | 5 | AO | - | 3.2 | | | | |
| <u> </u> | | of paramete | | | | | | | |
| ODWS identifies the following types of parameters: MAC=Maximum Allowable Concentration | | | | | | | | | |
| AO = Aesthetic Objective | | | | | | | | | |
| OG= Operational Guideline | | | | | | | | | |
| 2. Shaded Concentration | | Evceedance | of the ODM | /S Objective | | | | | |

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Two water samples were recovered during the initial pumping test of the well and submitted for laboratory analyses. The laboratory groundwater geochemistry results can be found attached.

The water quality of the subject water supply well meets all the Ontario Drinking Water Standards maximum acceptable concentrations (MAC). Furthermore, the water meets all the aesthetic objectives (AO) and operational guidelines (OG) with the exception of the following:

| hardness; |
|----------------|
| TDS; |
| chloride; |
| iron; |
| manganese; and |
| sodium. |

Exceedances of the above parameters are typical of the water supply in the subject aquifer. Each of these groundwater parameters are discussed in detail below.

Hardness

Hardness, expressed as calcium carbonate, an operational guideline, does not appear in the ODWS. Rather, it appears in the Technical Support Documents for Drinking Water Standards, Objectives and Guidelines (Technical Support Documents) as a parameter with an operational guideline of 100 mg/L. At the measured concentration of 568 and 551 mg/L, the water is considered to be hard to very hard, however it is exceeding the reasonable treatable limit of 500 mg/L, specified in Table 3 of the MOECC guidance document Procedure D-5-5 (1996), by a small margin. The hardness concentration can be treated using modern conventional water softener technologies.

TDS

Total dissolved solids (TDS) refers to the concentration of inorganic substances dissolved in water. The main constituents are typically chloride, sulphates, calcium, magnesium and bicarbonates. Water with a TDS concentration above 500 mg/L of TDS may not be palatable. Procedure D-5-5 does not provide a 'treatability limit' for TDS, but it does require written rationale that corrosion, encrustation or taste problems will not occur.

The Langelier Saturation Index (Langelier, 1936) is used to predict the calcium carbonate stability of water. It indicates whether the water will precipitate, dissolve or be in equilibrium with calcium carbonate.

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The results of the Langelier calculation (LSI = 0.8) indicate the water is super saturated and tends to precipitate a scale layer of calcium carbonate (scale forming and non-corrosive). See attached Langelier calculations for further details.

The presence of TDS in drinking water contributes to the palatability of the water and is strictly an aesthetic parameter. Generally, water with TDS levels in excess of 1,200 mg/L is considered to be unacceptable, however, the palatability of the water is dependant upon the user. The TDS level in the subject water supply was measured to be 1,490 mg/L, which may impact the taste of the drinking water to some users. If desired, a point-of-use reverse osmosis treatment unit can be used to reduce the TDS levels at a designated drinking water tap. However, the proposed usage of the water supply is currently for hand washing and bathroom usage.

Chloride

Chloride (CI), an aesthetic parameter, was detected in the laboratory test sample at a concentration of 567 and 554 mg/L, which exceeds the ODWS aesthetic objective of 250 mg/L. The World Health Organization prepared a document "Chloride in Drinking-water" dated 1996 that concludes chloride concentrations in excess of 250 mg/L may potentially provide a detectable taste in the water. Consumers may become accustomed to chloride concentrations that exceed 250 mg/L. WHO noted that they would not be proposing limits for chlorides in drinking water. If desired, a reverse osmosis system would be able to reduce chloride levels.

Iron

An iron concentration of 0.36 and 0.22 mg/L was measured at the 3 and 8.5 hour interval, which is slightly above and below the aesthetic objectives in the ODWO. Concentrations exceeding the aesthetic objective of 0.3 mg/L may contribute to staining of plumbing fixtures and laundry. As per D-5-5, the results are below the level considered to be reasonably treatable. A conventional water softener can be used to reduce the levels of iron.

Manganese

The manganese concentration results of 0.5 and 0.42 mg/L is above the aesthetic objectives in the ODWO. Concentrations exceeding the aesthetic objective of 0.05 mg/L may contribute to staining of plumbing fixtures and laundry. As per D-5-5, the results are well below the level considered to be reasonably treatable (1.0 mg/L). A conventional water softener can be used to reduce the levels of manganese.

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Sodium

Sodium (Na), an aesthetic parameter, was detected in the laboratory test sample at a concentration of 236 and 275 mg/L, which exceeds the ODWS aesthetic objective of 200 mg/L. Although sodium is not toxic and no maximum acceptable concentration has been set, concentrations above 20 mg/L require that the Medical Officer of Health be notified of the water quality results, so that this information may be passed on to local physicians for use in treatment of those requiring a sodium-restricted diet.

Turbidity

Turbidity, which is generally an aesthetic parameter, was detected in the laboratory test samples at values of 18.2 and 9.0 NTU at the 3 and 8.5 hour tests. The field results showed that turbidity increased around the 2.0 to 6.5 hour period of the initial test. Continued pumping showed a steady decrease towards the end of the pumping test. Field tests initially showed values of 1.15 to 2.05 NTU during the second pumping test over a seven hour period.

The ODWS maximum acceptable concentration for turbidity in drinking water entering the distribution system is 1 NTU. The Aesthetic Objective for turbidity in drinking water reaching the consumer is 5 NTU. In accordance with Procedure D-5-5, Table 2 does not reflect a maximum concentration considered reasonably treatable for turbidity. Rather, Procedure D-5-5 indicates that "particular care must be taken during testing to ensure that the bacteria requirements of Table 1 are met." Based on the test results, the bacteria requirements of Table 1 of D-5-5 have been met (E.Coli = 0 and Total Coliforms = 0).

It should be noted that the field turbidity testing indicated that the turbidly level reduced significantly during the pumping event. The field turbidity of 70.3 and 17.4 NTU was measured at the well head at approximately the 6 hour and 8.5 hour interval, respectively. Approximately 1 week after completion of the pumping test, the turbidity level was measured at 1.15 to 2.05 NTU. The high turbidity levels are related to sediment being mobilized and flushing into the well from the initial well construction. Further development of the well is expected to further reduce the turbidity levels.

It should be noted that the turbidity levels were 0.4 NTU at the adjacent property (6135 Rockdale Road) that also utilizes a shallow dug well.

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EXISTING DRILLED WATER WELL SUPPLY

The existing drilled water well supply at the subject site is currently used for only hand washing and bathroom needs. When comparing the existing drilled well water supply to the proposed supply (TW 1), the existing drilled water well samples show exceedances for the same categories (Turbidity, Hardness, TDS, Chloride, Iron, Manganese and Sodium) as the proposed water well supply and also for Dissolved Organic Carbon (DOC) and Hydrogen Sulphide. The higher concentration of hydrogen sulphide was found in the existing drilled supply well. It was noted that the existing drilled supply has been in use for many years and is exhibiting worse water quality than the proposed water supply. There is no treatment system in place and the Service Station bathroom has a distinct odour of hydrogen sulphide (rotten eggs). This odour is common in other drilled wells in the area. Table 2 provides a comparison of the existing supply and TW 1 results.

6135 ROCKDALE ROAD

The homeowner at the residence of 6135 Rockdale Road was interviewed and the following information was obtained. The current water supply consists of a shallow dug well that was constructed in 2004 to a depth of approximately 3.65 m below ground surface. Previously, 3 different drilled wells were located onsite and reported to exhibit poorer water quality than the current water supply. The drilled wells were then decommissioned by a licensed well technician. The existing treatment system consists of a water softener only.

The homeowner reports that they have never had any issues with quantity or quality. This includes using the water supply to top up their pool and to rinse/wash numerous (>100) large bouncy castle type inflatable structures.

The result of 0 E.Coli and 4 Total Coliforms was found in the samples taken from the outside tap. Notification was provided to the homeowner with instructions to re-test the treated water and how to properly disinfect the water supply.

CONCLUSIONS

Based on the information contained within the body of this report, the following conclusions can be drawn:

The water supply aquifer intercepted by TW 1 is considered to be adequate to support the proposed service station in the long term. However, seasonal variations of water quality and quantity in shallow aquifers may occur. It is recommended that a the dug well water supply be used for hand washing and toilet use only. Disinfection (Ultraviolet treatment) is recommended and signs indicating the water is to be used for hand washing and toilet use only must be posted.

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- 2. The preferred water supply aquifer intercepted by TW 1 contains a water supply that contains only elevated concentrations of aesthetic parameters (Hardness, TDS, chloride, iron, manganese and sodium). Some of the concentrations are above the reasonable treatable limits of D-5-5, but they can be removed by readily available water conditioning equipment.
- 3. A water softener is recommended to facilitate the removal of the hardness, iron and manganese concentrations.
- 4. Turbidity had reduced to below 2 NTU during the further development of the well and it is expected to reduce upon further development of the well until they reach concentrations similar to the value of 0.4 NTU found at the adjacent property (6135 Rockdale Road).
- 5. The sodium concentrations were measured to be above the 20 mg/L reporting limit and, as such, the Medical Officer of Health for the City of Ottawa should be informed to assist area physicians in the treatment of local residents on sodium reduced diets.
- 6. The results of the water supply assessment have provided satisfactory evidence that the water supply aquifer underlying the subject lands can support the redeveloped property with respect to water quality and quantity.
- 7. If no longer required, it is recommended that the existing drilled well should be decommissioned in accordance with O. Reg. 903 by a qualified well technician.

We trust that this satisfies your present requirements. Should you have any questions regarding this submission, please do not hesitate to contact the undersigned.

Yours truly,

PATERSON GROUP INC.

Michael S. Killam, P. Eng.

Attachments:

- MOECC Water Well Record
- Eurofins Certificate of Analysis
- AquiferTest Pro Pumping Test Analysis Reports
- Langelier Saturation Index Calculation
- Paterson Drawing PH3333-3



Ministry of the Environment Well Record Tag#: A 238439 (ow) and Climate Change Regulation 903 Ontario Water Resources Act 238439 Measurements recorded in: ☐ Metric ☐ Imperial Page **Well Owner's Information** Last Name / Organization E-mail Address First Name ABDO Mailing Address (Street Number/N EL-Arab by Well Owner Municipality Province Telephone No. (inc. area code) 6175 ROCKdale Road KIGA3 406/13/8/35/35/25 ottawa **Well Location** Address of Well Location (Street Number/Name) Concession County/District/Municipality O TTO Wa Postal Code KON3HO Ontario UTM Coordinates Zone Easting Municipal Plan and Sublot Number Other NAD | 8 | 3 | 18 | 18 | 18 | 5 | 02 | 08 | 8 | 3 |

NAD | 8 | 3 | 18 | 18 | 18 | 5 | 02 | 08 | 8 | 3 |

Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form) Depth (m/ft) General Description From 0 **Results of Well Yield Testing** Annular Space Type of Sealant Used Volume Placed After test of well yield, water was: Depth Set at (m/ft)
From To Time | Water Level (m3/ft3) Clear and sand free (m/ft) (min) (m/ft) Other, specify seolants If pumping discontinued, give reason: 8-8 Level 3 1 1 8.8 Pump intake set at (m/ft) 7 2 7 **Method of Construction** Well Use 4 Commercial Diamond Public Not used Duration of pumping Jetting Rotary (Conventional) Domestic Municipal Dewatering 5 ☐ Rotary (Reverse) ☐ Driving ☐ Livestock Test Hole ☐ Monitoring Final water level end of pumping (m/j) ☐ Boring Digging ☐ Irrigation Cooling & Air Conditioning 4 10 ☐ Industrial Air percussion Other, specify Other, specify 15 15 **Construction Record - Casing** Status of Well 20 20 Open Hole OR Material (Galvanized, Fibreglass, Concrete, Plastic, Steel) Water Supply Inside Wall Depth (m/ft) Diamete (cm/in) Thickness Replacement Well 25 From To (cm/in) Test Hole 30 8.0 18 +2 Recharge Well concrete Dewatering Well 40 40 Observation and/or Monitoring Hole Well production (I/min (GPM) 50 Alteration (Construction) 7, 60 60 Yes Abandoned. Insufficient Supply Map of Well Location **Construction Record - Screen** Abandoned, Poor Water Quality Please provide a map below following instructions on the back. Depth (m/ft) Abandoned, other, reland RD (cm/in) specify Other, specify **Water Details Hole Diameter** Water found at Depth Depth (m/ft) Kind of Water: Fresh Untested Diameter 14 (m/tt) Gas Other, specify From Water found at Depth Kind of Water: Fresh Untested (m/ft) Gas Other, specify Water found at Depth Kind of Water: Fresh Untested **Well Contractor and Well Technician Information** Business Name of Well Contractor 1 | 5 Municipality Comment est Casselman Business E-mail Address Well owner's information Ministry Use Only Date Package Delivered 20180366 Name of Well Technician (Last Name, First Name) package delivered Date Work Completed tor Date Submitted 20180366 No Received © Queen's Printer for Ontario, 2014

CERTIFICATE OF WELL COMPLIANCE

| I, Regean Cayer | DO HEREBY CERTLFY that I am licensed |
|---|--|
| to drill water wells in the Province of Oniario, and | that I have supervised the drilling of a well on the |
| property of ABDO EL-A sab | (Name of Landowner), located at |
| 6175 Rockdale Road Von | (Legal Description Lot/Plan No.) |
| property of ABDO EL-Arab (4175 Rockdale Roed V on in the Township of Osgoode Lot 27 | CONC 6 KOA 3HO |
| | |
| I CERTIFY FURTHER that, I am aware of | the well drilling requirements the guidelines |
| recommendations and regulations of the Ministry of | of the Environment governing well installations in |
| the Province of Ontario, and the standards | specified in any subdivision agreement and |
| hydrogeological report applicable to this site and | Township Standards |
| | |
| AND DO HEREBY CERTIFY THAT the said | well has been drilled, cased, grouted (cement or |
| bentonite) as applicable and constructed in strict | conformity with the standards required |
| | y sand station to particular. |
| SIGNED this _ q _ day of _ A pull | 2018 |
| | <u> </u> |
| | |
| Maurice Cayer ITD Well Driller/ Company | |
| Well Driller/ Company | • |
| | |
| The Engineer on behalf of the landowner set out a | have CEDIVIEIEG A. A. A. |
| well and it was constructed in accordance with the | specifications in O.P. as 002. It is |
| Hydrogeological Report with regards to casing len | igh and grouting requirements |
| | g and grouting requirements. |
| SIGNED this 23rd day of April | ,2018 |
| | PROFESSIONAL EL |
| -91V. II | 73/04/18 PROFESSIONAL SILVER 100221103 |
| Engineer | M. S. KILLAM H |
| | |
| | NCE OF ONTER |
| | |



Environment Testing

Client: Paterson Group

154 Colonnade Rd. South

Nepean, ON K2E 7T7

Attention: Mr. Mike Killam

PO#: 10449

Invoice to: Paterson Group

 Report Number:
 1806662

 Date Submitted:
 2018-05-02

 Date Reported:
 2018-05-10

 Project:
 PH 3333

 COC #:
 197584

| | | | | Lab I.D. Sample Matrix Sample Type Sampling Date Sample I.D. | 1356872 Water 2018-05-01 WS #1 | 1356873 Water 2018-05-01 WS #2 | 1356874 Water 2018-05-01 WS #3 |
|-------------------|-------------------------|-------|-------|--|---|---|---|
| Group | Analyte | MRL | Units | Guideline | | | |
| Calculations | Hardness as CaCO3 | 1 | mg/L | OG 100 | 568* | 551* | 582* |
| | Ion Balance | 0.01 | | | 0.92 | 0.90 | 0.90 |
| | TDS (COND - CALC) | 1 | mg/L | AO 500 | 1480* | 1490* | 1850* |
| General Chemistry | Alkalinity as CaCO3 | 5 | mg/L | OG 500 | 330 | 418 | 503* |
| | Chlorine (free) | 0.04 | mg/L | | | 0.17 | |
| | CI | 1 | mg/L | AO 250 | 567* | 554* | 607* |
| | Colour | 2 | TCU | AO 5 | <2 | 2 | 5 |
| | Conductivity | 5 | uS/cm | | 2270 | 2290 | 2850 |
| | F | 0.10 | mg/L | MAC 1.5 | 0.15 | <0.10 | <0.10 |
| | N-NO2 | 0.10 | mg/L | MAC 1.0 | <0.10 | <0.10 | <0.10 |
| | N-NO3 | 0.10 | mg/L | MAC 10.0 | 0.35 | 0.22 | <0.10 |
| | рН | 1.00 | | 6.5-8.5 | 7.82 | 7.79 | 7.74 |
| | SO4 | 1 | mg/L | AO 500 | 54 | 82 | 53 |
| | Turbidity | 0.1 | NTU | AO 5.0 | 18.2* | 9.0* | 8.8* |
| Metals | Ca | 1 | mg/L | | 196 | 191 | 177 |
| | Fe | 0.03 | mg/L | AO 0.3 | 0.36* | 0.22 | 0.77* |
| | K | 1 | mg/L | | 14 | 7 | 9 |
| | Mg | 1 | mg/L | | 19 | 18 | 34 |
| | Mn | 0.01 | mg/L | AO 0.05 | 0.50* | 0.42* | 1.22* |
| | Na | 2 | mg/L | AO 200 | 236* | 275* | 313* |
| Nutrients | Total Kjeldahl Nitrogen | 0.1 | mg/L | | 0.5 | 0.5 | 1.6 |
| Others | F1 (C6-C10) | 20 | ug/L | | <20 | <20 | |
| | F2 (C10-C16) | 20 | ug/L | | 60 | <20 | |
| Phenols | Phenols | 0.001 | mg/L | | <0.001 | <0.001 | <0.001 |
| Subcontract | DOC | 0.5 | mg/L | AO 5 | 3.3 | 3.7 | 7.1* |

Guideline = ODWSOG

Results relate only to the parameters tested on the samples submitted. Methods references and/or additional QA/QC information available on request.

^{* =} Guideline Exceedence



Environment Testing

Client: Paterson Group

154 Colonnade Rd. South

Nepean, ON K2E 7T7

Attention: Mr. Mike Killam

PO#: 10449

Invoice to: Paterson Group

 Report Number:
 1806662

 Date Submitted:
 2018-05-02

 Date Reported:
 2018-05-10

 Project:
 PH 3333

 COC #:
 197584

| | | | | Lab I.D. Sample Matrix Sample Type Sampling Date Sample I.D. | 1356872 Water 2018-05-01 WS #1 | 1356873 Water 2018-05-01 WS #2 | 1356874 Water 2018-05-01 WS #3 |
|-------------|-----------------------------------|------|-------|--|---|---|---|
| Group | Analyte | MRL | Units | Guideline | 0.00 | 0.40 | |
| Subcontract | N-NH3 | 0.01 | mg/L | | 0.03 | 0.10 | 4.00 |
| | 00 | 0.04 | mg/L | 10.005 | 0.00 | 0.00 | 1.20 |
| | S2- | 0.02 | mg/L | AO 0.05 | 0.03 | <0.02 | 0.47* |
| \(\omega) | Tannin & Lignin | 0.1 | mg/L | | <0.1 | 0.1 | 0.5 |
| VOCs | 1,1,1,2-tetrachloroethane | 0.5 | ug/L | | <0.5 | <0.5 | |
| | 1,1,1-trichloroethane | 0.4 | ug/L | | <0.4 | <0.4 | |
| | 1,1,2,2-tetrachloroethane | 0.5 | ug/L | | <0.5 | <0.5 | |
| | 1,1,2-trichloroethane | 0.4 | ug/L | | <0.4 | <0.4 | |
| | 1,1-dichloroethane | 0.4 | ug/L | | <0.4 | <0.4 | |
| | 1,1-dichloroethylene | 0.5 | ug/L | MAC 14 | <0.5 | <0.5 | |
| | 1,2-dichlorobenzene | 0.4 | ug/L | MAC 200 | <0.4 | <0.4 | |
| | 1,2-dichloroethane | 0.2 | ug/L | IMAC 5 | <0.2 | <0.2 | |
| | 1,2-dichloropropane | 0.5 | ug/L | | <0.5 | <0.5 | |
| | 1,3,5-trimethylbenzene | 0.3 | ug/L | | <0.3 | <0.3 | |
| | 1,3-dichlorobenzene | 0.4 | ug/L | | <0.4 | <0.4 | |
| | 1,3-Dichloropropylene (cis+trans) | 0.3 | ug/L | | <0.3 | <0.3 | |
| | 1,4-dichlorobenzene | 0.4 | ug/L | MAC 5 | <0.4 | <0.4 | |
| | Acetone | 30 | ug/L | | <30 | <30 | |
| | Benzene | 0.5 | ug/L | MAC 1 | <0.5 | <0.5 | |
| | Bromodichloromethane | 0.3 | ug/L | | 32.0 | 8.8 | |
| | Bromoform | 0.4 | ug/L | | 1.8 | 1.5 | |
| | Bromomethane | 0.5 | ug/L | | <0.5 | <0.5 | |
| | c-1,2-Dichloroethylene | 0.4 | ug/L | | <0.4 | <0.4 | |
| | c-1,3-Dichloropropylene | 0.2 | ug/L | | <0.2 | <0.2 | |
| | Carbon Tetrachloride | 0.2 | ug/L | MAC 2 | <0.2 | <0.2 | |

Guideline = ODWSOG

Results relate only to the parameters tested on the samples submitted. Methods references and/or additional QA/QC information available on request.

^{* =} Guideline Exceedence



Environment Testing

Client: Paterson Group

154 Colonnade Rd. South

Nepean, ON K2E 7T7

Attention: Mr. Mike Killam

PO#: 10449

Invoice to: Paterson Group

 Report Number:
 1806662

 Date Submitted:
 2018-05-02

 Date Reported:
 2018-05-10

 Project:
 PH 3333

 COC #:
 197584

| | | | | Lab I.D. Sample Matrix Sample Type Sampling Date Sample I.D. | 1356872 Water 2018-05-01 WS #1 | 1356873 Water 2018-05-01 WS #2 | 1356874 Water 2018-05-01 WS #3 |
|----------------|--------------------------------|-----|-------|--|---|---|---|
| Group | Analyte | MRL | Units | Guideline | | | |
| VOCs | Chloroethane | 0.2 | ug/L | | <0.2 | <0.2 | |
| | Chloroform | 0.5 | ug/L | | 54.8 | 9.6 | |
| | Dibromochloromethane | 0.3 | ug/L | | 12.9 | 5.7 | |
| | Dichlorodifluoromethane | 0.5 | ug/L | | <0.5 | <0.5 | |
| | Dichloromethane | 4.0 | ug/L | MAC 50 | <4.0 | <4.0 | |
| | Ethylbenzene | 0.5 | ug/L | MAC 140 | <0.5 | <0.5 | |
| | Ethylene Dibromide | 0.2 | ug/L | | <0.2 | <0.2 | |
| | Hexane | 5 | ug/L | | <5 | <5 | |
| | m/p-xylene | 0.4 | ug/L | | <0.4 | <0.4 | |
| | Methyl Ethyl Ketone (MEK) | 10 | ug/L | | <10 | <10 | |
| | Methyl Isobutyl Ketone (MIBK) | 10 | ug/L | | <10 | <10 | |
| | Methyl Tert Butyl Ether (MTBE) | 2 | ug/L | AO 15 | <2 | <2 | |
| | Monochlorobenzene | 0.5 | ug/L | MAC 80 | <0.5 | <0.5 | |
| | o-xylene | 0.4 | ug/L | | <0.4 | <0.4 | |
| | Styrene | 0.5 | ug/L | | <0.5 | <0.5 | |
| | t-1,2-Dichloroethylene | 0.4 | ug/L | | <0.4 | <0.4 | |
| | t-1,3-Dichloropropylene | 0.2 | ug/L | | <0.2 | <0.2 | |
| | Tetrachloroethylene | 0.3 | ug/L | MAC 10 | <0.3 | <0.3 | |
| | Toluene | 0.5 | ug/L | MAC 60 | <0.5 | <0.5 | |
| | Trichloroethylene | 0.3 | ug/L | MAC 5 | <0.3 | <0.3 | |
| | Trichlorofluoromethane | 0.5 | ug/L | | <0.5 | <0.5 | |
| | Vinyl Chloride | 0.2 | ug/L | MAC 1 | <0.2 | <0.2 | |
| | Xylene; total | 0.5 | ug/L | MAC 90 | <0.5 | <0.5 | |
| OCs Surrogates | 1,2-dichloroethane-d4 | 0 | % | | 103 | 105 | |
| (%REC) | 4-bromofluorobenzene | 0 | % | | 117 | 121 | |

Guideline = ODWSOG

* = Guideline Exceedence

Results relate only to the parameters tested on the samples submitted. Methods references and/or additional QA/QC information available on request.



Environment Testing

Client: Paterson Group

154 Colonnade Rd. South

Nepean, ON K2E 7T7

Attention: Mr. Mike Killam

PO#: 10449

Invoice to: Paterson Group

Report Number: 1806662 Date Submitted: 2018-05-02 Date Reported: 2018-05-10 Project: PH 3333

COC #: 197584

| Crown | Analyte | MDI | Unito | Lab I.D. Sample Matrix Sample Type Sampling Date Sample I.D. | 1356872 Water 2018-05-01 WS #1 | 1356873 Water 2018-05-01 WS #2 | 1356874 Water 2018-05-01 WS #3 |
|--------------------|------------|-----|-------|--|---|---|---|
| Group | Analyte | MRL | Units | Guideline | | | |
| VOCs Surrogates (% | Toluene-d8 | 0 | % | | 83 | 85 | |

Guideline = ODWSOG

* = Guideline Exceedence

Results relate only to the parameters tested on the samples submitted. Methods references and/or additional QA/QC information available on request.



Environment Testing

Client: Paterson Group

154 Colonnade Rd. South

Nepean, ON K2E 7T7

Attention: Mr. Mike Killam

PO#: 23839

Invoice to: Paterson Group

 Report Number:
 1807217

 Date Submitted:
 2018-05-09

 Date Reported:
 2018-05-11

 Project:
 PH3333

 COC #:
 82891

| Group | Analyte | MRL | Units | Lab I.D. Sample Matrix Sample Type Sampling Date Sample I.D. Guideline | 1358330 Water 2018-05-09 SF#1 | 1358331 Water 2018-05-09 WS#3 |
|--------|------------------|-----|----------|--|--|--|
| Others | Escherichia Coli | 0 | ct/100mL | MAC 0 | 0 | 0 |
| | Total Coliforms | 0 | ct/100mL | MAC 0 | 4* | 0 |

Guideline = ODWSOG

* = Guideline Exceedence

Results relate only to the parameters tested on the samples submitted. Analytical Method: AMBCOLM1

additional QA/QC information available on request.



Environment Testing

Client: Paterson Group

154 Colonnade Rd. South

Nepean, ON K2E 7T7

Attention: Mr. Mike Killam

PO#: 23839

Invoice to: Paterson Group

 Report Number:
 1807216

 Date Submitted:
 2018-05-09

 Date Reported:
 2018-05-16

 Project:
 PH3333

 COC #:
 82891

| Group | Analyte | MRL | Units | Lab I.D. Sample Matrix Sample Type Sampling Date Sample I.D. Guideline | 1358328 Water 2018-05-09 SF#1 |
|-------------------|-------------------------|-------|-------|---|--|
| Calculations | Hardness as CaCO3 | 1 | mg/L | OG 100 | 443* |
| | Ion Balance | 0.01 | | | 1.06 |
| | TDS (COND - CALC) | 1 | mg/L | AO 500 | 533* |
| General Chemistry | Alkalinity as CaCO3 | 5 | mg/L | OG 500 | 353 |
| | Cl | 1 | mg/L | AO 250 | 18 |
| | Colour | 2 | TCU | AO 5 | 7* |
| | Conductivity | 5 | uS/cm | | 820 |
| | F | 0.10 | mg/L | MAC 1.5 | <0.10 |
| | N-NO2 | 0.10 | mg/L | MAC 1.0 | <0.10 |
| | N-NO3 | 0.10 | mg/L | MAC 10.0 | 0.21 |
| | рН | 1.00 | | 6.5-8.5 | 7.58 |
| | SO4 | 1 | mg/L | AO 500 | 70 |
| | Turbidity | 0.1 | NTU | AO 5.0 | 0.4 |
| Metals | Ca | 1 | mg/L | | 146 |
| | Fe | 0.03 | mg/L | AO 0.3 | <0.03 |
| | K | 1 | mg/L | | 3 |
| | Mg | 1 | mg/L | | 19 |
| | Mn | 0.01 | mg/L | AO 0.05 | 0.06* |
| | Na | 2 | mg/L | AO 200 | 14 |
| Nutrients | Total Kjeldahl Nitrogen | 0.1 | mg/L | | 0.3 |
| Phenols | Phenols | 0.001 | mg/L | | <0.001 |
| Subcontract | DOC | 0.5 | mg/L | AO 5 | 3.2 |
| | N-NH3 | 0.01 | mg/L | | 0.08 |
| | S2- | 0.02 | mg/L | AO 0.05 | <0.02 |
| | Tannin & Lignin | 0.1 | mg/L | | <0.1 |

Guideline = ODWSOG

Results relate only to the parameters tested on the samples submitted. Methods references and/or additional QA/QC information available on request.

^{* =} Guideline Exceedence

154 Colonnade Road South Ottawa, ON

patersongroup K2E 7J5

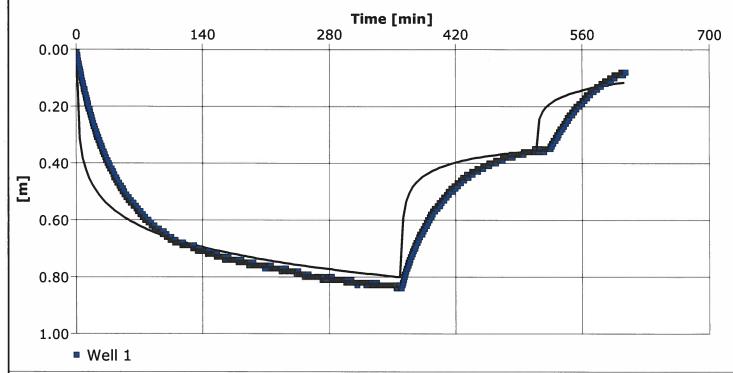
Pumping Test Analysis Report

Project: Vars Esso

Number: PH3333

Client: Abdo El Arab

| Location: 6175 Rockdale Road, Vars | Pumping Test: Pumping Test with full data | Pumping Well: Well 1 | | |
|---|---|---------------------------|--|--|
| Test Conducted by: MK | | Test Date: 01/05/2018 | | |
| Analysis Performed by: MK | Theis with Jacob Correction | Analysis Date: 28/05/2018 | | |
| Aquifer Thickness: 4.30 m | Discharge: variable, average rate 17.824 [l/s] | | | |
| Pumping rate was reduced to lower turbidity | . Part of recovery consists of pumping at a lower output. | | | |



Calculation using Theis with Jacob Correction

| Observation Well | Transmissivity | Hydraulic Conductivity | Storage coefficient | Radial Distance to PW | |
|------------------|------------------------|------------------------|---------------------|-----------------------|--|
| | [m²/d] | [m/d] | | [m] | |
| Well 1 | 1.64 × 10 ³ | 3.80 × 10 ² | | 0.6 | |

154 Colonnade Road South Ottawa, ON patersongroup K2E 7J5

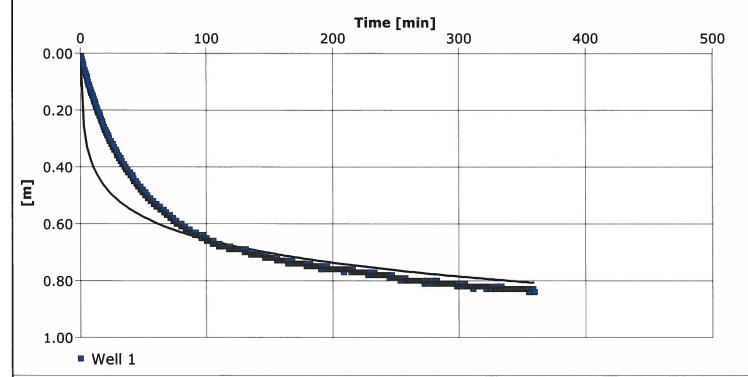
Pumping Test Analysis Report

Project: Vars Esso

Number: PH3333

Client: Abdo El Arab

| Location: 6175 Rockdale Road, Vars Pumping Test: Pumping Test with full data | | Pumping Well: Well 1 |
|--|---|---------------------------|
| Test Conducted by: MK | | Test Date: 01/05/2018 |
| Analysis Performed by: MK | Papadopulos & Cooper | Analysis Date: 30/05/2018 |
| Aquifer Thickness: 4.30 m | Discharge: variable, average rate 17.824 [l/s] | |
| Pumping rate was reduced to lower turbidity. Part | recovery consists of pumping at a lower output. | |



Calculation using Theis with Jacob Correction

| Observation Well | Transmissivity | Hydraulic Conductivity | Storage coefficient Radial Distance to PW | | |
|------------------|------------------------|------------------------|---|-----|--|
| | [m²/d] | [m/d] | | [m] | |
| Well 1 | 1.07 × 10 ³ | 2.49 × 10 ² | | 0.6 | |

154 Colonnade Road South Ottawa, ON patersongroup K2E 7J5

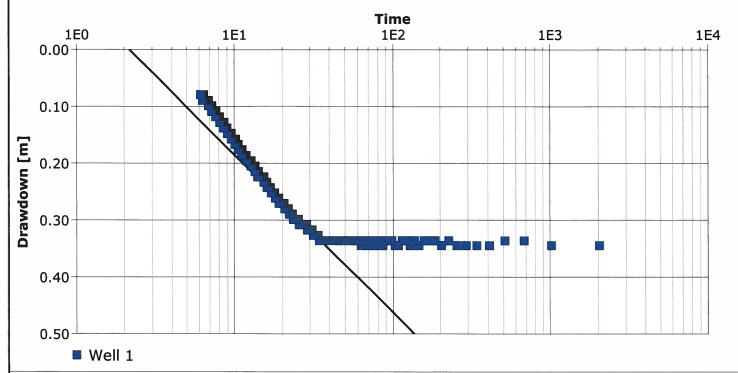
Project: Vars Esso

Pumping Test Analysis Report

Number: PH3333

Client: Abdo El Arab

| Location: 6175 Rockdale Road, Vars | Pumping Test: Pumping Test with full data | Pumping Well: Well 1 | | | |
|--|---|---------------------------|--|--|--|
| Test Conducted by: MK | | Test Date: 01/05/2018 | | | |
| Analysis Performed by: | Theis Recovery | Analysis Date: 31/05/2018 | | | |
| Aquifer Thickness: 4.30 m | Discharge: variable, average rate 17.824 [l/s] | | | | |
| Pumping rate was reduced to lower turbidity. | Part of recovery consists of pumping at a lower output. | | | | |



Calculation using THEIS & JACOB

| Observation Well | Observation Well Transmissivity | | Radial Distance to PW | |
|------------------|---------------------------------|------------------------|-----------------------|--|
| | [m²/d] | [m/d] | [m] | |
| Well 1 | 1.02 × 10 ³ | 2.36 × 10 ² | 0.6 | |

154 Colonnade Road South Ottawa, ON patersongroup K2E 7J5

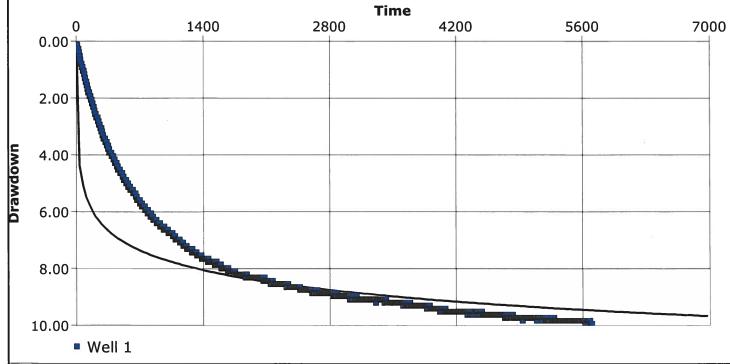
Project: Vars Esso

Pumping Test Analysis Report

Number: PH3333

Client: Abdo El Arab

| Location: 6175 Rockdale Road, Vars Pumping Test: Pumping Test with full data | | Pumping Well: Well 1 | |
|--|--|---------------------------|--|
| Test Conducted by: MK | | Test Date: 01/05/2018 | |
| Analysis Performed by: MK Theis with Jacob Correction (6 hours) | | Analysis Date: 31/05/2018 | |
| Aquifer Thickness: 4.30 m Discharge: variable, average rate 17.824 [l/s] | | | |
| Pumping rate was reduced to lower turbidity | Part of recovery consists of numping at a lower output | | |



Calculation using Theis with Jacob Correction

| Observation Well | Transmissivity | Hydraulic Conductivity | Storage coefficient | Radial Distance to PW |
|------------------|------------------------|------------------------|---------------------|-----------------------|
| | [m²/d] | [m/d] | | [m] |
| Well 1 | 1.61 × 10 ³ | 3.73 × 10 ² | | 0.6 |

154 Colonnade Road South Ottawa, ON

patersongroup K2E 7J5

Pumping Test Analysis Report

Project: Vars Esso

Number: PH3333

| | | | | Client: Abdo | El Arab | | | |
|------|-----------------------------|--------------------------|------------------|-----------------------------|---------|------------------|-----------------------------|---|
| Loca | ation: 6175 Rockdale Roa | ad, Vars | Pumping Test: F | Pumping Test with full dat | ta Pump | ing Well: Well 1 | | |
| Tes | t Conducted by: MK | | | | Test [| Date: 01/05/2018 | | |
| Aqu | ifer Thickness: 4.30 m | F: | Discharge: varia | able, average rate 17.824 | [l/s] | | | |
| Pun | nping rate was reduced to | lower turbidity. Part of | recovery consist | s of pumping at a lower o | output. | | | |
| | Analysis Name | Analysis Performed by | Analysis Date | Method name | Well | T [m²/d] | K [m/d] | s |
| 1 | Theis with Jacob Correction | nMK | 28/05/2018 | Theis with Jacob Correction | nWell 1 | 1.64 × 10 | 3.80×10^2 | |
| 2 | Papadopulos & Cooper | мк | 30/05/2018 | Theis with Jacob Correction | nWell 1 | 1.07 × 10 | 3 2.49 × 10 ² | |
| 3 | Theis Recovery | | 31/05/2018 | Theis Recovery | Well 1 | 1.02 × 10 | 3 2.36 × 10 ² | |
| 4 | Theis with Jacob Correction | nMKhours) | 31/05/2018 | Theis with Jacob Correction | nWell 1 | 1.61 × 10 | 3.73×10^2 | |
| ši. | <u>*</u> | | • | | Α | verage 1.33 × 10 | 3.10×10^2 | |

patersongroup

6175 Rockdale Road PH3333

| ΓW1 | inputs | | |
|------------|--------|-------|-------------|
| pН | 7.79 | A | 0.22 |
| TDS | 1490 | В | 2.40 |
| Hardness | 551 | С | 2.34 |
| Alkalinity | 418 | D | 2.62 |
| Temp. | 9 | | |
| | | pHs = | 6.957722445 |
| | | | |

| Langelier Saturation Index (LSI) Calcu | lation (Langelier, 1936) |
|---|---|
| pHs = (9.3 + A + B) - (C + D) Where: | A = (Log10 [TDS] - 1) / 10 B = -13.12 x Log10 (oC + 273) + 34.55 C = Log10 [Ca2+ as CaCO3] - 0.4 D = Log10 [alkalinity as CaCO3] |
| | LSI = 0.8 |

| | | L01 = | 0.0 | |
|------------|---|---------------------------------|--------------------------------|------------|
| LSI | Effect | | | |
| 0.5 to 2 | Water is super saturated and tends to precipitate a scale layer | er of calcium carbonate (scale | forming but non-corrosive) | |
| 0 to 0.5 | Water is super saturated and tends to precipitate a scale layer | r of calcium carbonate (slightl | y scale forming and corrosive) | |
| 0 | Water is saturated (in equilibrium) with calcium carbonate. A | scale layer of calcium carbona | te is neither precipitated nor | dissolved. |
| 0 to -0.5 | Water is under saturated and tends to dissolve solid calcium c | arbonate (slightly corrosivebu | it non-scale forming). | |
| -0.5 to -2 | Water is under saturated and tends to dissolve solid calcium c | arbonate (seriously corrosive |). | |

