

2 Via Modugno Place

TIA Report

prepared for: Campanale Homes 1187 Bank Street, Suite 200 Ottawa, ON, K1S 3X7

prepared by:

PARSONS

1223 Michael Street North Suite 100 Ottawa, ON K1J 7T2

October 11, 2019

477167 - 01000



Document Control Page

CLIENT:	Campanale Homes			
PROJECT NAME:	Transportation Impact Assessment – 2 Via Modugno Place			
REPORT TITLE:	2 Via Modugno Place – TIA Report			
PARSONS PROJECT NO.: 477167 - 01000				
VERSION:	Final			
DIGITAL MASTER:	H:\ISO\477167\1000\DOCS\STEP 5 - TIA Report\Strategy Report_2 Via Modugno PI_10.07.2019.docx			
ORIGINATOR	Basel Ansari/Rani Nahas			
REVIEWER:	Mark Baker			
AUTHORIZATION:	City of Ottawa			
CIRCULATION LIST:	Rosanna Baggs			
HISTORY:	TIA Step 1 Screening Form – Submitted June 7, 2019 TIA Step 2 Scoping Report – Submitted June 27, 2019 TIA Step 3 Forecasting Report – Submitted July 8, 2019 TIA Step 4 Strategy Report – Submitted Sep 10, 2019 TIA Step 5 Report – Submitted Oct 11, 2019			



TIA Plan Reports

On 14 June 2017, the Council of the City of Ottawa adopted new Transportation Impact Assessment (TIA) Guidelines. In adopting the guidelines, Council established a requirement for those preparing and delivering transportation impact assessments and reports to sign a letter of certification.

Individuals submitting TIA reports will be responsible for all aspects of development-related transportation assessment and reporting, and undertaking such work, in accordance and compliance with the City of Ottawa's Official Plan, the Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines.

By submitting the attached TIA report (and any associated documents) and signing this document, the individual acknowledges that s/he meets the four criteria listed below.

CERTIFICATION

- 1. I have reviewed and have a sound understanding of the objectives, needs and requirements of the City of Ottawa's Official Plan, Transportation Master Plan and the Transportation Impact Assessment (2017) Guidelines;
- 2. I have a sound knowledge of industry standard practice with respect to the preparation of transportation impact assessment reports, including multi modal level of service review;
- 3. I have substantial experience (more than 5 years) in undertaking and delivering transportation impact studies (analysis, reporting and geometric design) with strong background knowledge in transportation planning, engineering or traffic operations; and
- 4. I am either a licensed¹ or registered² professional in good standing, whose field of expertise [check $\sqrt{\text{appropriate field(s)}}$] is either transportation engineering \square or transportation planning \square .
- 1,2 License of registration body that oversees the profession is required to have a code of conduct and ethics guidelines that will ensure appropriate conduct and representation for transportation planning and/or transportation engineering works.

Dated at Ottawa (City)	this11 day ofOctober, 201 <u>9</u> .
Name:	Mark Baker (Please Print)
Professional Title:	Senior Project Manager
	Mark Bak
Signature	of Individual certifier that s/he meets the above four criteria

Office Contact Information (Please Print)
Address: 1223 Michael Street, Suite 100
City / Postal Code: Ottawa K1J 7T2
Telephone / Extension: 613-691-1524
E-Mail Address: Mark.Baker@Parsons.com

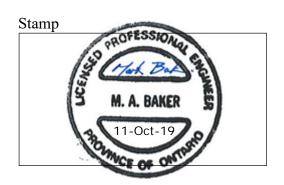




Table of Contents

1. 2.			1	
	2.1.		STING AND PLANNED CONDITIONS	
	2.1.		Proposed Development	
	2.1.		Existing Conditions	
	2.1.	.3.	Planned Conditions	6
	2.2. 2.3.		DY AREA AND TIME PERIODS	
3.	FOR	RECAS	TING	9
	3.1.	DEV	ELOPMENT GENERATED TRAVEL DEMAND	9
	3.2.	BAC	KGROUND NETWORK TRAFFIC	9
	3.2.	.1.	Transportation Network Plans	9
	3.2.		Background Growth	
	3.2.		Other Developments	
	3.2.		Total background traffic	
	3.3.	DEM	1AND RATIONALIZATION	12
4.	ANA	LYSIS	S	13
	4.1.	DEV	ELOPMENT DESIGN	13
	4.1.	.1.	Design for Sustainable Modes	13
	4.1.	.2.	Circulation and Access	13
	4.2.	PAR	KING	13
	4.2.	.1.	Parking Supply	13
	4.3.	BOU	NDARY STREET DESIGN	13
	4.4.	ACC	ESS INTERSECTION DESIGN	14
	4.4.	.1.	Location and Design of Access	14
	4.4.		Intersection Control	
	4.4.		Intersection Design	
			NSPORTATION DEMAND MANAGEMENT	
	4.6. 4.7.		GHBOURHOOD TRAFFIC MANAGEMENT NSIT	
	4.7. 4.8.		IEW OF NETWORK CONCEPT	
	4.9.		RSECTION DESIGN	
5.	FINI	DINGS	S, CONCLUSIONS AND RECOMMENDATIONS	16
Li	st of I	Figur	res	
Fig	gure 1:	Local	Context	1
Fi٤	gure 2:	Propo	osed Site Plan	2



Figure 3: Area Transit Network	5
Figure 4: Bus Stop Locations	
Figure 5: Existing Peak Hour Traffic Volumes	5
Figure 6: Other Area Developments	6
Figure 7: Study Area	8
Figure 8: Future Background 2020	9
Figure 9: Future Background 2025	10
Figure 10: Other Area Developments Traffic Volumes	
Figure 11: 619 Longfields Dr anticipated Traffic Volumes	11
Figure 12: Total Future Background 2020 Traffic Volumes	12
Figure 13: Total Future Background 2025 Traffic Volumes	12
List of Tables	
Table 1: Exemptions Review Summary	8
Table 2: Annual Background Growth Rate	9
Table 3: 619 Longfields Dr Vehicle Trips Generated	
Table 4: MMLOS - Boundary Road Analysis	
Table 5: Existing Conditions Intersection Performance	
Table 6: Total Future Background 2020 Intersection Performance	
Table 7: Total Future Background 2025 Intersection Performance	

List of Appendices

APPENDIX A - Screening Form

APPENDIX B - Truck Turning Templates

APPENDIX C - OC Transpo Route Maps

APPENDIX D - Traffic Counts

APPENDIX E - Collision Data

APPENDIX F - 619 Longfields Dr Site Plan

APPENDIX G - Trip Generation Calculations

APPENDIX H - Detailed Background Growth Analysis

APPENDIX I - Stopping Sight Distances

APPENDIX J - MMLoS Analysis

APPENDIX K - Synchro Analysis: Existing Conditions

APPENDIX L - Synchro Analysis: Total Future Background 2020 Conditions

APPENDIX M - Synchro Analysis: Total Future Background 2025 Conditions



TIA Report

1. SCREENING FORM

The screening form was completed for the subject development and included as part of the subsequent report. The Trip Generation, Location and Safety triggers were all checked against their respective criteria and it was determined that only the Location trigger is met. Since at least one of the triggers was met, a Transportation Impact Assessment (TIA) report must be completed based on the City of Ottawa's 2017 TIA Guidelines. The screening form, as well as responses to latest City comments, are provided in **Appendix A**.

2. SCOPING REPORT

2.1. EXISTING AND PLANNED CONDITIONS

2.1.1. PROPOSED DEVELOPMENT

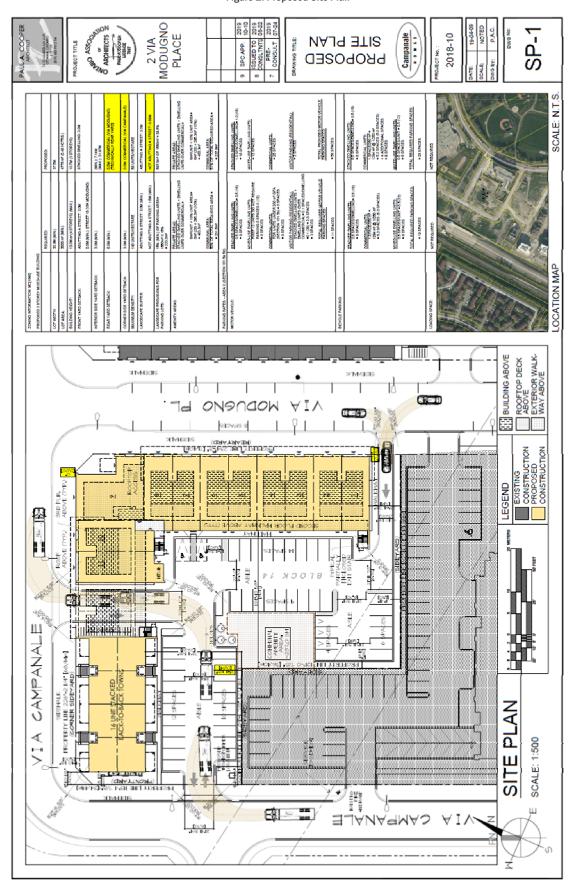
This study has been prepared as part of a Site Plan Application (SPA) of a proposed residential development taking place at 2 Via Modugno Pl, within Ward 3 (Barrhaven). Furthermore, this development forms part of the Campanale Homes' Longfields Station development lands in Barrhaven. The development site is zoned as a Mixed-Use Centre. **Figure 1** provides area context regarding the location of the planned future development and the surrounding road network.



Figure 1: Local Context

The residential development is expected to consist of 28 low-rise apartment units and approximately 15,000 ft² of commercial space and is anticipated to be constructed in a single phase by 2020. Parking spaces will be provided in the form of surface parking with a total of 55 spaces, which included two accessible and six visitor parking spaces. **Figure 2** shows the proposed site plan of the proposed future development.

Figure 2: Proposed Site Plan



As illustrated by the Site Plan, the following accesses will serve the site as a connection to the surrounding road network:

- One access will be located along Via Modugno PI, on the east side of the site, and will be restricted to inbound traffic only.
- The second access will be located along Via Campanale Ave, on the north side of the site, as a full-movement driveway.
- The third access will also be located along Via Campanale Ave, on the west side of the site, as a full-movement driveway. It should be noted that there is a slight offset between the third access and the existing Via Chianti Grove connection to Via Campanale Ave.

Garbage trucks are expected to enter through the west Via Campanale access and exit through the north Via Campanale access. Truck turning templates have been provided in **Appendix B**.

2.1.2. EXISTING CONDITIONS

Area Road Network

Longfields Dr is a municipal roadway in Barrhaven that extends from Bill Leathem Dr in the east to Jockvale Rd in the south. Between Bill Leathem Dr and Strandherd Dr, Longfields Dr is classified as a major collector roadway, with a two-lane cross-section. Whereas between Strandherd Dr and Jockvale Rd, it is classified as an arterial roadway, with a four-lane cross-section. Furthermore, auxiliary turn lanes are provided at major intersections. The posted speed limit of Longfields Dr within the study area is 40 km/h.

Via Modugno PI is a local municipal roadway in Barrhaven with a two-lane cross-section and on-street parking on both sides of the road. The roadway extends from Longfields Dr to Via Campanale Ave and is assumed to have a speed limit of 40 km/h.

Via Campanale Ave is a local municipal roadway in Barrhaven with a two-lane cross-section that extends from Via Mattino Way in the north to Longfields Dr in the east. The assumed speed limit of the roadway is 40 km/h.

Existing Study Area Intersections

Longfields/Via Modugno Pl

The Longfields/Via Modugno PI intersection is a 'T' intersection with signal control on the Longfields Dr approaches (north/south legs) and STOP control on the Via Modugno PI approach (west leg). All legs of the intersection consist of an unrestricted single, full-movement lane. The traffic signal provided is a pedestrian signal, that allows pedestrians to cross at the north leg of the intersection.



Longfields/Via Campanale Ave

The Longfields/Via Campanale Ave intersection is an unsignalized 'T' intersection with STOP control on the minor leg of the intersection. All legs of the intersection consist of an unrestricted single, full-movement lane. Pedestrians will be able to cross the intersection at the west leg only.



Existing Driveways to Adjacent Developments

Within 200 m of the proposed site accesses are several existing driveways along Via Campanale Ave. Two driveways serve the adjacent developments located at the municipal addresses 615 and 619 Longfields Dr. A few other driveways further down Via Campanale Ave are used by private residential house units. Furthermore, the proposed third access of the development will be located across from the Via Chianti Grove roadway.

Pedestrian/Cycling Network

With regards to pedestrians, sidewalk facilities and pathways are provided throughout Longfields Dr and along developed areas of Via Modugno Pl and Via Campanale Ave. A major pathway runs alongside the transitway, directly west of the development site. The major pathway also connects to other pathways that run along several sections of Longfields Dr and converge with the existing sidewalk facilities.

With regards to cyclists, on-road bicycle lanes are provided on Longfields Dr, along some sections of the roadway. Where on-street parking is provided, the bicycle lanes are not present.

Transit Network and Bus Services

The following OC Transpo bus routes operate within or near the study area:

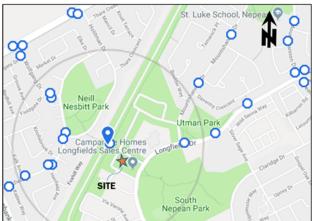
- Route #171 (Fallowfield <-> Barrhaven Centre): identified by OC Transpo as a "Local Route", this route operates at a rate of every 30 minutes during weekday morning and afternoon peak hour periods. The nearest bus stop to the future development site is at the intersection of Longfields/Mountshannon.
- Route #95 (Orleans & Trim <-> Cambrian): identified by OC Transpo as a "Rapid Route", this route operates along the transitway and stops at Longfields Station, located approximately 100 m west of the development site. Route #95 operates 7 days a week at a very rapid rate.
- Routes #271, #273 and #275 are identified as "Connexion Routes" that operate during weekday rush-hours only, at a rate of every 30 minutes. These routes run along the transitway and stop at Longfields Station.

The noted OC Transpo route maps are provided in **Appendix C. Figure 3** below illustrates the area transit network as portrayed in the OC Transpo System Map, while **Figure 4** provides the locations of bus stops nearest to the site.

Figure 3: Area Transit Network

Fallowfield Barrhaven Barrhaven Crossing 179 170 173 179 Larkin **171** 27) 278 (94) Baker Longfiel Longfields Longfields 170 SITE Mother Teresa (271)Longfields-Davidson (95) Heights Rideaucr 171

Figure 4: Bus Stop Locations



Existing Area Traffic Management

Some of the existing area traffic management measures within the study area include the following:

- Sidewalk facilities;
- Bike lanes:
- On-street parking;
- Streetscaping

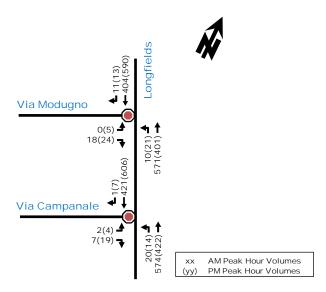
Peak hour travel demands

In order to establish a baseline for local traffic conditions, traffic counts were conducted by Parsons in June 2019 for the following intersections:

- Longfields Dr/Via Modugno Pl
- Longfields Dr/Via Campanale Ave

Figure 5 provides an illustration of the existing traffic volumes at intersections within the study area. All traffic count data is provided in **Appendix D**.

Figure 5: Existing Peak Hour Traffic Volumes



Existing Road Safety Conditions

A five-year (2014-2018, inclusive) collision history report was requested and obtained from the City of Ottawa (provided in **Appendix E**) for the following intersections and road segments:

- Longfields Dr/Via Modugno Pl
- Longfields Dr/Via Campanale Ave
- Longfields Dr between Via Modugno Pl and Campanale Ave

The collision data collected revealed that only two collisions have taken place at the above locations, within the past five-years. One collision occurred at the intersection of Longfields/Via Campanale in May 2015 and resulted in a non-fatal injury, while the second collision occurred at the intersection of Longfields/Via Modugno in June 2017 and resulted in property damage only. Based on the foregoing, there is no evidence of a systemic safety issue.

2.1.3. PLANNED CONDITIONS

Planned Study Area Transportation Network Changes

According to the "2031 Affordable Network" (Map 11), 2015 revision, of the City of Ottawa's Transportation Master Plan, there are no notable transportation network changes planned within the study area by 2031. There are also no anticipated transit network changes.

Other Area Developments

Development applications for adjacent area developments located within 1 km of the proposed Campanale Homes development site were researched using the City of Ottawa's "In My Neighbourhood" tool. Descriptions of other area developments are provided below.

Campanale Homes

As previously mentioned, the subject development forms part of the Campanale Homes' Longfields Station development lands in Barrhaven. To illustrate the plan for the Longfields Station development lands, **Figure 6** was taken from the Campanale Homes website and was overlaid with the subject site's location.



Figure 6: Other Area Developments

A site visit to the study area revealed that, with the exception of the fully constructed building immediately south of the development site, all surrounding developments (represented by orange and red blocks) are either under construction or have yet to be constructed. Development applications for the following Campanale Homes developments were found:

- 605 Longfields Dr: illustrated in **Figure 6** by the "L" shaped block directly east of the development site, this development is currently under construction and is anticipated to consist of minor commercial uses. A TIA report was submitted by Parsons in December 2017, which determined that the development is expected to generate approximately 28 and 40 veh/h during the weekday morning and afternoon peak hour periods, respectively.
- <u>619 Longfields Dr</u>: illustrated in **Figure 6** by the five orange blocks immediately west of the development site, this development has almost reached full buildout. Based on its site plan (see **Appendix F**), this development consists of a total of 64 residential units housed within five mid-rise apartment buildings. Trips generated by this development will be manually calculated and assigned to the road network in **Section 3.2.3** of the report.

As such, the other area developments above will be included in the future background traffic volumes forecasted in this report. The remaining Campanale Homes developments that are located north of the development site and southwest of the 619 Longfields Dr development site are excluded due to uncertainty with regards to their use, size and status.

255 and 285 Mountshannon Dr

A 16-unit and 20-unit stacked dwellings are planned to be constructed at 255 and 285 Mountshannon Dr, respectively. This development is anticipated to generate minimal traffic. As such, it will be accounted for in the background growth rate determined in **Section 3.2.2**.

146 Mountshannon Dr

A residential development consisting of 22 townhomes and 160 condominium units are planned to be distributed in 9 low-rise apartment buildings at 146 Mountshannon Dr. Due to its proximity to the proposed Campanale Homes development site, traffic volumes generated by this development along Longfields Dr will be accounted for in the background growth rate determined in **Section 3.2.2.**

35 Highbury Park Dr

A development consisting of a church and accompanying café are planned at the above noted address. Due to its proximity to the proposed Campanale Homes development site, traffic volumes generated by this development along Longfields Dr will be accounted for in the background growth rate determined in **Section 3.2.2**

30 Highbury Park Dr

A development consisting of retail, a fast-food restaurant, a medical office, and a pharmacy are planned at the above noted address. Due to its proximity to the proposed Campanale Homes development site, traffic volumes generated by this development along Longfields Dr will be accounted for in the background growth rate determined in **Section 3.2.2**

2.2. STUDY AREA AND TIME PERIODS

The proposed study area intersections to be included in the analysis are outlined below and illustrated in Figure 7.

- Longfields Dr/Via Modugno Pl
- Longfields Dr/Via Campanale Ave

Figure 7: Study Area



Since the development consists of residential units, the critical time periods where the site-generated traffic volumes are expected to reach their peak is during the weekday morning and afternoon peak hour periods. This coincides with the peak periods of the background traffic of the road network. As such, operational analysis in this report will rely on these two peak time periods.

The TIA guidelines require that two horizon years be analyzed; those are the anticipated year of full build-out of the development (i.e. 2020) and five years after full build-out (i.e. 2025).

2.3. EXEMPTION REVIEW

Based on the City's TIA guidelines and the subject site, the following modules/elements of the TIA process, summarized in **Table 1**, are recommended to be exempt in the subsequent steps of the TIA process:

Table 1: Exemptions Review Summary

Module	Element	Exemption Consideration
3.1 Development Generated Travel Demand	All elements	Not required if Trip Generation trigger is not met (i.e. development does not generate more than 60 trips/h during peak hours).
4.1 Development Design	4.1.3 New Street Networks	Not required for applications involving site plans.
4.2 Parking	4.2.2 Spillover Parking	The parking is expected to meet By-Law requirements.
4.5 – 4.9 Network Impact Component	All elements	Not required if Trip Generation trigger is not met (i.e. development does not generate more than 60 trips/h during peak hours).

Summary of the trip generation calculations has been provided in **Appendix G**.

3. FORECASTING

3.1. DEVELOPMENT GENERATED TRAVEL DEMAND

Exempt - see Section 2.3.

3.2. BACKGROUND NETWORK TRAFFIC

3.2.1. TRANSPORTATION NETWORK PLANS

Refer to **Section 2.1.3**.

3.2.2. BACKGROUND GROWTH

In order to determine a background growth rate along Longfields Dr, regression analysis was conducted using historical traffic counts (2012, 2013 and 2019) at the intersection of Longfields/Mountshannon, approximately 190 m east of the Longfields/Via Modugno PI intersection. **Table 2** below provides a summary of the background growth rates determined using the AM and PM peak traffic counts, where east and west legs represent the approaches along Longfields Dr. The detailed background growth analysis is provided in **Appendix H**.

Table 2: Annual Background Growth Rate

Time Period	Percent Annual Change				
	East Leg	West Leg			
AM Peak	15.54%	11.55%			
PM Peak	17.43%	9.38%			

Based on the above, the average annual background growth rate is approximately 13.5% during both peak hours. However, this growth rate may be attributed to the construction of many new developments in the lands surrounding Longfields Dr in recent years, the majority of which consists of residential areas. As such, a background growth rate of 13.5% is not considered sustainable and would not be an accurate assumption for future background conditions. To account for the developments that have yet to be constructed, a 5% per year increase in through traffic was applied along Longfields Dr. The resulting future background traffic volumes for horizon years 2020 and 2025 are illustrated in **Figure 8** and **Figure 9**.

Figure 8: Future Background 2020

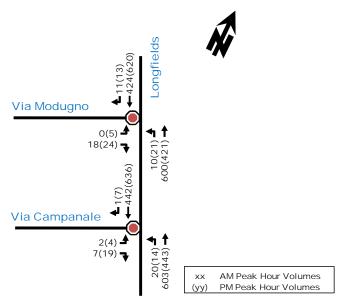
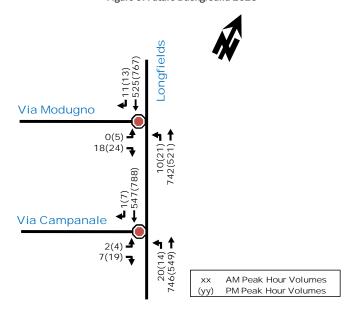


Figure 9: Future Background 2025



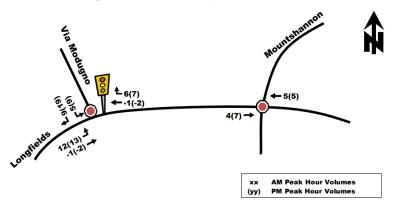
3.2.3. OTHER DEVELOPMENTS

Descriptions of adjacent developments in the surrounding area, that have initiated the development application process with the City, was provided in **Section 2.1.3: Other Area Developments**. Traffic volumes generated by the following future adjacent area developments will be included in the total future background traffic volumes of the study area:

605 Longfields Dr

Traffic volumes generated by the future development at 605 Longfields Dr are illustrated in **Figure 10**, which was taken directly from the TIA Report submitted by Parsons in December 2017. Note that the negative numbers refer to Pass-by traffic.

Figure 10: Other Area Developments Traffic Volumes



619 Longfields Dr

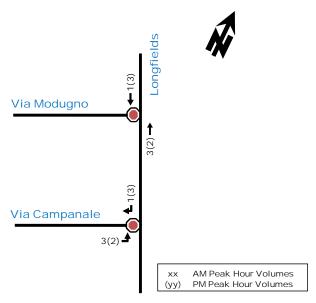
Using the 2009 TRANS Trip Generation Study Report, the trip rates, the percentage of inbound and outbound trips and the total person trips were determined for the development at 619 Longfields Dr. The total person trips were then separated into different travel modes, using the mode share percentages from the 2011 NCR Household Origin-Destination Survey (South Nepean District). **Table 3** provides the number of trips anticipated to be generated by auto vehicles during the morning and afternoon peak hour periods.

Table 3: 619 Longfields Dr Vehicle Trips Generated

	Data	D Illin at	AM Peak (veh/h)			PM Peak (veh/h)		
Land Use		Dwelling Units	In	Out	Total	In	Out	Total
	Source	Units	24%	76%		62%	38%	
Mid-rise apartments (4 floors)	TRANS	64 du	3	12	15	11	8	19

The total number of trips anticipated to be generated by the development at 619 Longfields Dr are 15 and 19 veh/h during the morning and afternoon peak hour periods, respectively. As shown in the site plan, there are two planned accesses to the development site. The majority of traffic generated by this development (75%) is expected to utilize the south access of the development site and arrive at the intersection of Longfields Dr/Via Chianti Grove, thus, it would not influence the study area intersections. The remaining traffic is assumed to use the west development access along Via Chianti Grove and arrive at the intersection of Longfields Dr/Via Campanale Ave. These volumes are shown in **Figure 11** below.

Figure 11: 619 Longfields Dr anticipated Traffic Volumes



3.2.4. TOTAL BACKGROUND TRAFFIC

Total background traffic represents the summation of background traffic growth (based on the 5% growth rate per annum) in each of **Figure 8** and **Figure 9**, with the adjacent development traffic in **Figure 10** and **Figure 11**. The resulting total background traffic volumes for horizon years 2020 and 2025 are illustrated in **Figure 12** and **Figure 13**.

Figure 12: Total Future Background 2020 Traffic Volumes

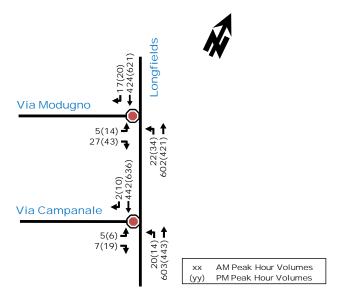
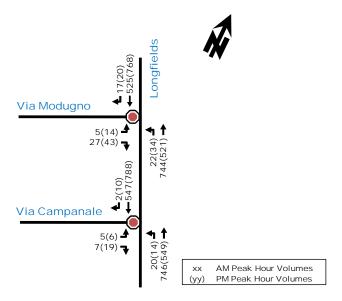


Figure 13: Total Future Background 2025 Traffic Volumes



3.3. DEMAND RATIONALIZATION

The study area road network is expected to accommodate projected volumes. There are currently no anticipated capacity issues. The capacity of the roadways will be further explored in a more detailed review of the total projected traffic volumes and intersection design in the ensuing Strategy Report.

4. ANALYSIS

4.1. DEVELOPMENT DESIGN

4.1.1. DESIGN FOR SUSTAINABLE MODES

Pedestrians can access the development site through the series of sidewalks that are currently provided throughout the study area. Sidewalks facilities will also be provided along both sides of Via Campanale Ave and Via Modugno Pl. There are no plans currently for bike lanes however there is an existing MUP on the south side of Longfields Dr.

With regards to transit, six bus routes currently operate within vicinity of the site, as previously mentioned in **Section 2.1.2**. Furthermore, the nearest bus stop to the development site is Longfields BRT Station, within approximately 175m walking distance. Transit stops along Longfields Dr are located approximately 400 – 550m walking distance from the site.

4.1.2. CIRCULATION AND ACCESS

There are three proposed accesses to the site: two all-movement accesses and an inbound-only access. According to the City's Private Approach By-law, the number of accesses and location are appropriate. **Section 4.4: Access Intersection Design** will go into further detail regarding the design, location, and control of these driveways.

The driveway aisle widths are noted to be 6.7 – 7.0m wide, meeting the minimum By-Law requirements. The single inbound entrance is noted to be 3.0m, also meeting minimum By-Law requirements. Stopping sight distances have been provided in **Appendix I**.

4.2. PARKING

4.2.1. PARKING SUPPLY

Vehicle Parking

A total of 55 vehicle parking spaces will be provided for the planned development, with 3 spaces provided for visitors and 2 barrier free parking spaces (i.e. accessible parking). The number of parking spaces provided exceeds the minimum parking space rates set by the City of Ottawa parking provisions (11 spaces minimum). The parking spaces are 5.2 m long and 2.6 m wide.

Bicycle Parking

A total of 25 bicycle parking spaces are provided, exceeding the requirement set by the City of Ottawa's bike parking provisions. Furthermore, the bike parking is located outside, near the north end of the site.

4.3. BOUNDARY STREET DESIGN

The boundary streets of the development are Via Campanale Ave and Via Modugno Pl. As these streets are not fully built, a Multi-Modal Level of Service (MMLOS) analysis was conducted for all three boundary streets, based on future conditions.

Via Campanale Ave, West Frontage:

- 1 vehicle travel lane in each direction;
- 2.0 m sidewalks along the site frontage:
- Less than 3000 avg. daily curb lane traffic volumes; and,
- Approximately 3.5 m wide lanes.

Via Campanale Ave, North Frontage:

- 1 vehicle travel lane in each direction;
- 2.0 m sidewalks along the site frontage;
- Parking along the north side of the roadway;
- Less than 3000 avg. daily curb lane traffic volumes; and,
- Approximately 3.5 m wide lanes.

Via Modugno PI, East Frontage:

- 1 vehicle travel lane in each direction;
- 2.0 m sidewalks along the site frontage;
- Parking along both sides of the roadway;
- Less than 3000 avg. daily curb lane traffic volumes; and,
- Approximately 3.5 m wide lanes.

The multi-modal level of service analysis for the adjacent road segments is summarized in **Table 4**, with detailed analysis provided in **Appendix J**. The table also identifies the target LOS, with respect to each mode, based on the land-use designation and road classification of the development site and the boundary streets. The Transportation Master Plan (TMP) identifies the land-use designation of the development site as General Urban Area.

	Level of Service								
Road Segment	Pedestrian (PLOS)		Bicycle (BLOS)		Transit (TLOS)		Truck (TkLOS)		
	PLOS	Target	BLOS	Target	TLOS	Target	TkLOS	Target	
Via Campanale (west frontage)	А	Α	Α	D	D	No target	В	No target	
Via Campanale (north frontage)	Α	Α	Α	D	D	No target	В	No target	
Via Modugno	Α	Α	А	D	D	No target	В	No target	

Table 4: MMLOS – Boundary Road Analysis

As shown in **Table 4**, all modes meet their respective level of service targets. Note that there are no targets for transit or trucks, as there are no transit facilities on boundary streets and the boundary streets do not make up part of the truck route network.

4.4. ACCESS INTERSECTION DESIGN

4.4.1. LOCATION AND DESIGN OF ACCESS

There are three proposed accesses to the site:

- West Access This access is proposed as an all-movement access. It is located on Via Campanale Ave and forms
 the forth leg with Via Chianti Grove to the west;
- North Access This access is proposed as an all-movement "T" access. It is located on Via Campanale Ave; and,
- East Access This access is proposed as an in-bound only "T" access, located on Via Modugno Pl.

4.4.2. INTERSECTION CONTROL

Based on the roadway design and projected vehicle volumes, the planned driveways would likely be proposed with STOP control on the minor approaches only.

4.4.3. INTERSECTION DESIGN

EXISTING CONDITIONS

SYNCHRO (V10) Trafficware was used to analyze intersections within the study area. The following **Table 5** provides a summary of the existing traffic operations based on the existing peak hour traffic volumes (**Figure 5**). Critical movements

at study area intersections were identified based on the movement with the highest average delay. The SYNCHRO model output for existing conditions is provided in **Appendix K**.

Table 5: Existing Conditions Intersection Performance

	Weekday AM Peak (PM Peak)					
Intersection		Intersection 'As a Whole'				
	LOS	avg. delay (s)	Movement	Delay (s)		
Longfields Dr/Via Modugno PI (U)	B(C)	11.4(16.0)	EB(EB)	0.4(0.8)		
Longfields Dr/Via Campanale Ave (U)	B(C)	13.5(15.2)	EB(EB)	0.4(0.5)		
Note: Analysis of signalized intersections assumes a PHF of 0.90 and a saturation flow rate of 1800 veh/h/lane.						

(U) - Unsignalized intersections assumes a Fire of 0.50 and a saturation flow rate of 1500 very by lane.

As shown in **Table 5**, critical movements at both study area intersections operate at a LOS 'C' or better during the morning and afternoon peak hour periods.

TOTAL FUTURE BACKGROUND 2020 CONDITIONS

Table 6 below provides a summary of SYNCHRO analysis at study area intersections, based on total future background 2020 traffic volumes (**Figure 12**). The SYNCHRO model output for total future background 2020 conditions is provided in **Appendix L**.

Table 6: Total Future Background 2020 Intersection Performance

	Weekday AM Peak (PM Peak)						
Intersection		Intersection 'As a Whole'					
	LOS	avg. delay (s)	Movement	Delay (s)			
Longfields Dr/Via Modugno PI (U)	B(C)	13.3(17.3)	EB(EB)	0.7(1.3)			
Longfields Dr/Via Campanale Ave (U)	C(C)	15.3(15.4)	EB(EB)	0.5(0.5)			
Note: Analysis of signalized intersections assumes a PHF of 1.0 and a saturation flow rate of 1800 veh/h/lane.							

As shown in **Table 6** above, intersections in total future background 2020 conditions are projected to operate similar to existing conditions, with slightly higher delays for critical movements and the intersection 'as a whole'.

TOTAL FUTURE BACKGROUND 2025 CONDITIONS

Table 7 below provides a summary of SYNCHRO analysis at study area intersections, based on total future background 2025 traffic volumes (**Figure 13**). The SYNCHRO model output for total future background 2025 conditions is provided in **Appendix M**.

Table 7: Total Future Background 2025 Intersection Performance

	Weekday AM Peak (PM Peak)					
Intersection		Intersection 'As a Whole'				
	LOS	avg. delay (s)	Movement	Delay (s)		
Longfields Dr/Via Modugno PI (U)	C(C)	15.4(22.0)	EB(EB)	0.7(1.3)		
Longfields Dr/Via Campanale Ave (U)	C(C)	18.8(18.8)	EB(EB)	0.5(0.5)		
Note: Analysis of signalized intersections assumes a PHF of 1.0 and a saturation flow rate of 1800 veh/h/lane.						

Note: Analysis of signalized intersections assumes a PHF of 1.0 and a saturation flow rate of 1800 veh/h/lane. (U) - Unsignalized intersection.

As shown in **Table 7** above, intersections in total future background 2025 conditions are projected to operate with higher delays than anticipated for the total future background 2020 conditions.

4.5. TRANSPORTATION DEMAND MANAGEMENT

Exempt - see Section 2.3.

4.6. NEIGHBOURHOOD TRAFFIC MANAGEMENT

Exempt - see Section 2.3.

4.7. TRANSIT

Exempt - see Section 2.3.

4.8. REVIEW OF NETWORK CONCEPT

Exempt - see Section 2.3.

4.9. INTERSECTION DESIGN

Exempt - see Section 2.3.

5. FINDINGS, CONCLUSIONS AND RECOMMENDATIONS

Based on the results summarized herein the following transportation related conclusions are offered:

- The proposed single-phase development will consist of 28 low-rise apartments and approximately 15,000 ft² of commercial space. The anticipated buildout year of the development is 2020.
- The development is planned to provide a total of 55 vehicle parking spaces and 25 bicycle parking spaces, exceeding what is required by the City of Ottawa's parking provisions.
- Three new accesses are proposed to serve the development. Two full-movement accesses will be located along Via Campanale Ave, and one in-bound-only access will be located along Via Modugno PI.
- The projected number of vehicle trips anticipated to be generated by the development is negligible.
- All MMLoS targets are met and there are no anticipated intersection capacity issues.
- SYNCHRO analysis conducted for all conditions (existing and total future background 2020 and 2025) resulted in a LOS 'C' or better during morning and afternoon peak hour periods.

Based on the foregoing, the proposed development can be well accommodated by the adjacent transportation network and is recommended to proceed from a transportation perspective.

Prepared By:

Basel Ansari, EIT. Transportation Planner 1 ans

Reviewed By:

Mark Baker, P.Eng. Senior Transportation Engineer





City of Ottawa 2017 TIA Guidelines

Date

31-May-19

TIA Screening Form

Project Campanale - Longfields (Block 14)

	Project Number	477167 - 01000
Results of Screening	Yes/No	
Development Satisfies the Trip Generation Trigger	No	
Development Satisfies the Location Trigger	Yes	
Development Satisfies the Safety Trigger	No	

Module 1.1 - Description of Proposed Development	
Municipal Address	2 Via Modugno Place
Description of location	Longfields Barrhaven
Land Use	Mixed use
Development Size	28 residental units and 15,000 square foot commercial space
Number of Accesses and Locations	2 proposed accesses (Via Modugno PI, Via Campanale Ave)
Development Phasing	1 Phase
Buildout Year	2020
Sketch Plan / Site Plan	See attached

Module 1.2 - Trip Generation Trigger		
Land Use Type	Townhomes or Apartments	/ Commercial Use
Development Size	28	15,000 sq ft commercial
Trip Generation Trigger Met?	No	

Module 1.3 - Location Triggers		
Development Proposes a new driveway to a boundary street that is designated as part of the City's Transit Priority, Rapid Transit, or Spine Bicycle Networks (See Sheet 3)	No	
Development is in a Design Priority Area (DPA) or Transit- oriented Development (TOD) zone.	Yes	
Location Trigger Met?	Yes	

Module 1.4 - Safety Triggers			
Posted Speed Limit on any boundary road	<80	km/h	
Horizontal / Vertical Curvature on a boundary street limits	No		
sight lines at a proposed driveway			
A proposed driveway is within the area of influence of an			
adjacent traffic signal or roundabout (i.e. within 300 m of			
intersection in rural conditions, or within 150 m of	No		
intersection in urban/ suburban conditions) or within auxiliary			
lanes of an intersection;			
A proposed driveway makes use of an existing median break	No		
that serves an existing site			
There is a documented history of traffic operations or safety			
concerns on the boundary streets within 500 m of the	No		
development			
The development includes a drive-thru facility	No		
Safety Trigger Met?	No		



Subject:

RE: 2 Via Modugno Pl Strategy Report

See comment responses in red below.

Transportation Engineering Services

1.

- a. Ensure that sidewalks along the entire frontage meet City standards, are continuous across accesses And;
- b. Ensure that there are no sight line issues for vehicles using the proposed access.
- a. Noted.
- b. Please see attached sightlines within the report and Figures 1-3 below. As shown in Figure 1, the north access provides sufficient stopping sight distances (SSD) for both east and westbound directions. As displayed in Figure 2, the northbound traffic approaching the west access will have sufficient SSD under normal operations. However, vehicles travelling southbound on Via Campanale Ave will have less than 50m SSD described in TAC, Table 2.5.2, with the assumed operating speed of 40km/h. Since vehicles travelling southbound on Via Campanale Ave will be making a 90 degree turn, operating speed are likely to be less than 40km/h, decreasing the minimum SSD. By assuming the operating speed is 30km/h, SSD decreases to 35m (as per TAC Table 2.5.2), which will give a sufficient SSD as shown in Figure 3.

Figure 1: North Access Stopping Sight Distances

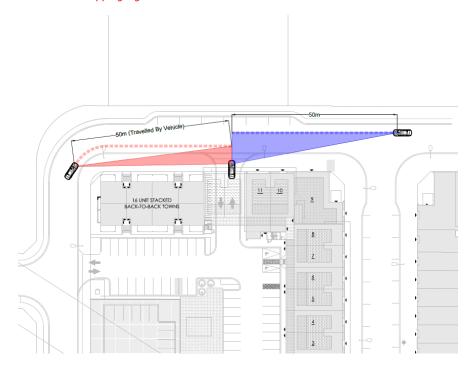
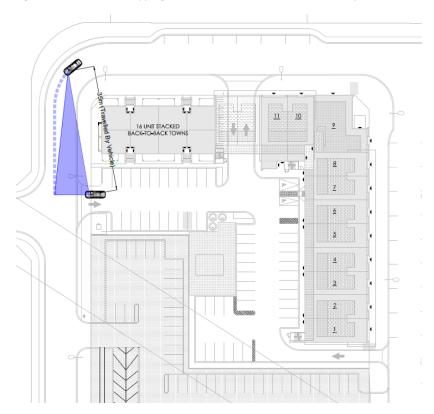


Figure 2: West Access Stopping Sight Distances



Figure 3: West Access Stopping Site Distance, with Assumed 30km/h Speed



Traffic Signal Operations

1. Correct the error in Element 4.1.1 second paragraph of the report. Error has been corrected.

Traffic Signal Design

No comments.

Noted.

Street Lighting

- 1. No comments with initial TIS for this circulation. Street Lighting reserves the right to make future comments based on subsequent submissions.
- 2. Future considerations are as follows:
 - a. If there are any proposed changes to the existing roadway geometry, the City of Ottawa Street Light Asset Management Group is required to provide a full street light design. Upon completion of proposed roadway geometry design changes, please submit digital Micro Station drawings with proposed roadway geometry changes to the Street Lighting Department, so that we may proceed with the detailed street light design and coordination with the Street Light maintenance provider and all necessary parties.
 - b. Be advised that the applicant will be 100% responsible for all costs associated with any Street Light design as a result of the roadway geometry change.
 - c. Alterations and/or repairs are required where the existing street light plant is directly, indirectly or adversely affected by the scope of work under this circulation, due to the proposed road reconstruction process. All street light plant alterations and/or repairs must be performed by the City of Ottawa's Street Light maintenance provider.
 - d. Be advised that the applicant will be 100% responsible for all costs associated with any relocations/modifications to the existing street light plant.

Noted.

Transit Services

On behalf of Transit Services there are no further comments.

Noted.

Development Review – Transportation Engineering Services

1. Remove Draft water mark.

Draft Water mark removed.

2. Include the Consultant Qualification Letter.

Qualification letter included.

From: Baggs, Rosanna < Rosanna. Baggs@ottawa.ca>

Sent: Thursday, October 3, 2019 1:58 PM

To: Ansari, Basel <Basel.Ansari@parsons.com>

Cc: Baker, Mark <Mark.Baker@parsons.com>; Christian Campanale <christian@campanale.com>; Giampa, Mike

<Mike.Giampa@ottawa.ca>

Subject: [EXTERNAL] RE: 2 Via Modugno PI Strategy Report

Hi Basel,

Please see the comments below:

Transportation Engineering Services

1. Ensure that sidewalks along the entire frontage meet City standards, are continuous across accesses and that there are no sight line issues for vehicles using the proposed access.

Traffic Signal Operations

1. Correct the error in Element 4.1.1 second paragraph of the report.

Traffic Signal Design

No comments.

Street Lighting

- 1. No comments with initial TIS for this circulation. Street Lighting reserves the right to make future comments based on subsequent submissions.
- 2. Future considerations are as follows:
 - a. If there are any proposed changes to the existing roadway geometry, the City of Ottawa Street Light Asset Management Group is required to provide a full street light design. Upon completion of proposed roadway geometry design changes, please submit digital Micro Station drawings with proposed roadway geometry changes to the Street Lighting Department, so that we may proceed with the detailed street light design and coordination with the Street Light maintenance provider and all necessary parties.
 - b. Be advised that the applicant will be 100% responsible for all costs associated with any Street Light design as a result of the roadway geometry change.
 - c. Alterations and/or repairs are required where the existing street light plant is directly, indirectly or adversely affected by the scope of work under this circulation, due to the proposed road reconstruction process. All street light plant alterations and/or repairs must be performed by the City of Ottawa's Street Light maintenance provider.
 - d. Be advised that the applicant will be 100% responsible for all costs associated with any relocations/modifications to the existing street light plant.

Transit Services

On behalf of Transit Services there are no further comments.

Development Review – Transportation Engineering Services

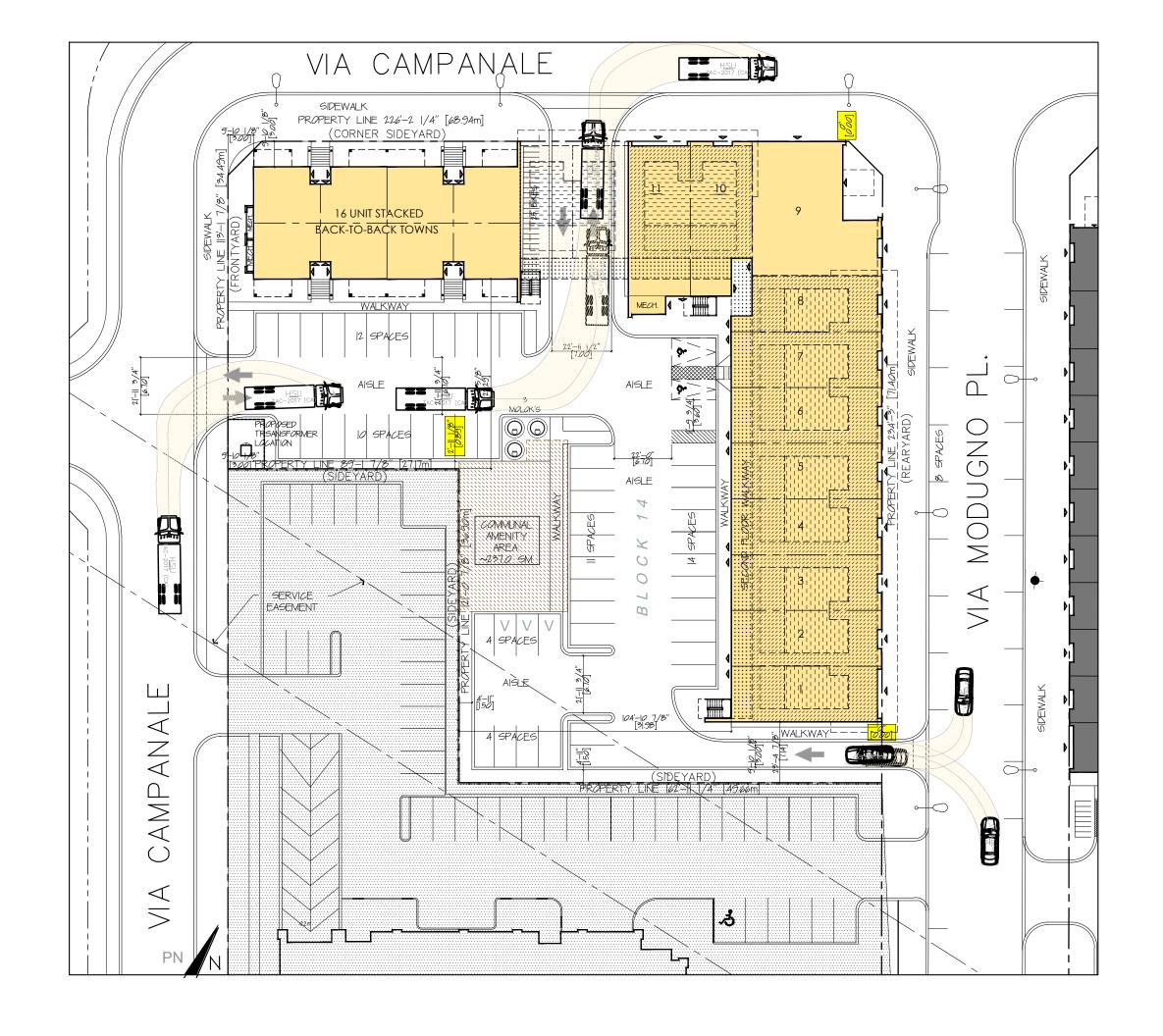
- 1. Remove Draft water mark.
- 2. Include the Consultant Qualification Letter.

Rosanna Baggs, C.E.T.

Project Manager, Infrastructure Approvals | GPRJ Approbation demandes infrastructure Development Review West Branch | Dir Services d'exam des dem d'amgt

Tel |Tél.: 613-580-2424 ext. | poste 26388





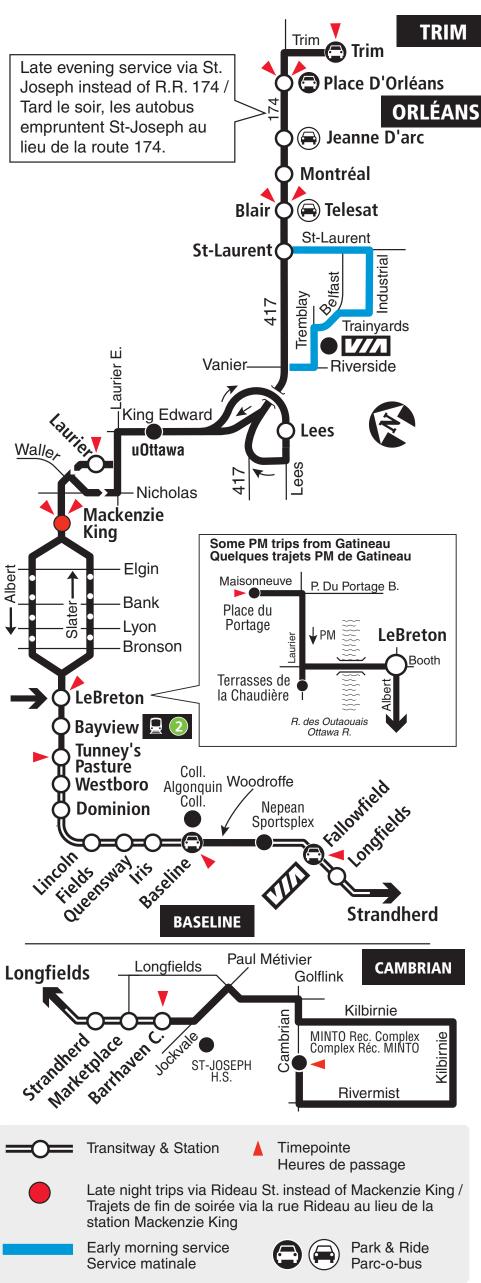




ORLÉANS & TRIM BARRHAVEN CAMBRIAN

7 days a week / 7 jours par semaine

All day service Service toute la journée



2018.10

Information / Renseignement......613-741-4390 **Customer Relations** Lost and Found / Objets perdus613-563-4011 Schedule / Horaire......613-560-1000 Text / Texto560560

plus your four digit bus stop number / plus votre numéro d'arrêt à quatre chiffres

Effective / En vigueur Oct. 15 oct. 2018

octranspo.com



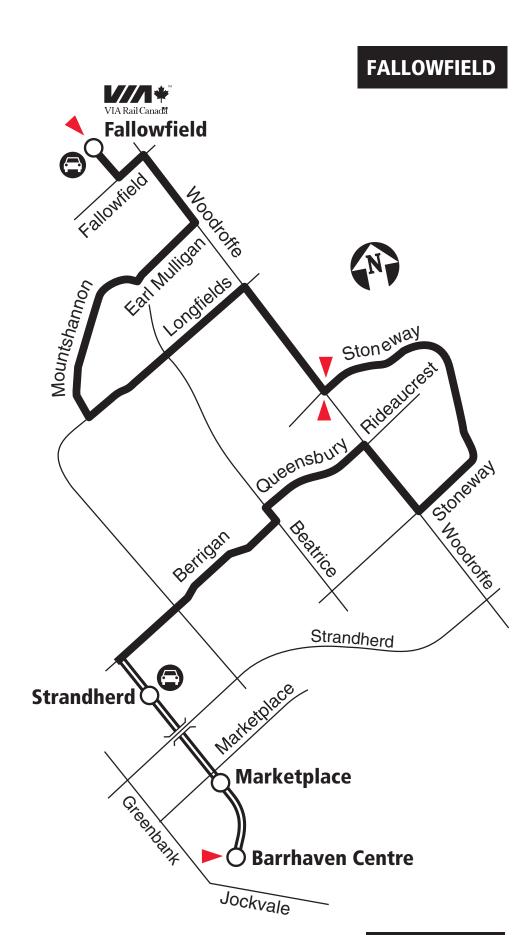
171

FALLOWFIELD BARRHAVEN CENTRE

Local

7 days a week / 7 jours par semaine

All day service Service toute la journée



BARRHAVEN CENTRE



Transitway & Station



Park & Ride / Parc-o-bus

A

Timepoint / Heures de passage

2017.01



plas your rour digit bas stop hamber i plas voite hamero a direct a quatre chimes

Effective December 23, 2012
En vigueur 23 décembre 2012
INFO 613-741-4390

octranspo.com

C Transpo



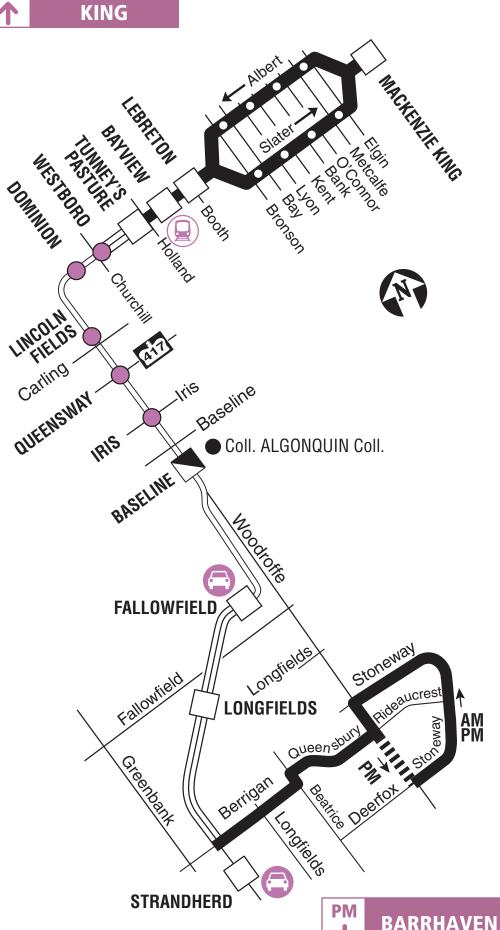
MACKENZIE KING BARRHAVEN

Monday to Friday / Lundi au vendredi

Peak periods only Périodes de pointe seulement



MACKENZIE



■ Legend • *Légende* **■**

Transitway & Station Limited stops: Off only in AM / No stop in PM

Arrêts limités : Débarquement en AM seulement / Aucun arrêt en PM Off only in AM / Full service in PM

Débarquement en AM seulement / Service complet en PM

Line 2 – O-Train Trillium Line Ligne 2 - O-Train LigneTrillium Park & Ride / Parc-o-bus

2016.12

Information / Renseignement 613-741-4390 **Customer Relations** 613-842-3600 Service à la clientèle. Lost and Found / Objets perdus..... 613-563-4011 Schedule / Horaire......613-560-1000560560 Text / Texto plus your four digit bus stop number / plus votre numéro d'arrêt à quatre chiffres

Effective December 25, 2016

En vigueur 25 décembre 2016

octranspo.com

C Transpo





MACKENZIE KING TARTAN

Connexion

Monday to Friday / Lundi au vendredi

Peak periods only Périodes de pointe seulement





Transitway & Station

Limited stops: Off only in AM / No stop in PM Arrêts limités : Débarquement en AM seulement / Aucun arrêt en PM

Limited stops: Off only in AM / Full service in PM Arrêts limités : Débarquement en AM seulement / Service complet en PM

Park & Ride / Parc-o-bus

2018.05

Cedarview



Customer Relations613-842-3600 Service à la clientèle Lost and Found / Objets perdus...... **613-563-4011**

Security / Sécurité 613-741-2478

Effective December 25, 2016 En vigueur 25 décembre 2016

C Transpo

INFO 613-741-4390 octranspo.com



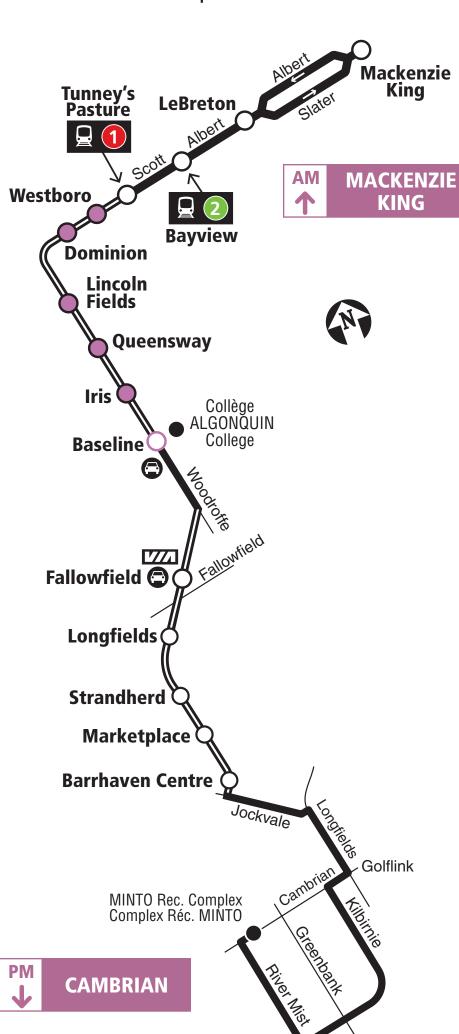


CAMBRIAN

MACKENZIE KING

Monday to Friday / Lundi au vendredi

Peak periods only Périodes de pointe seulement



Transitway & Station

Limited stops: Off only in AM / No stop in PM
Arrêts limités : Débarquement en AM seulement /
Aucun arrêt en PM

Limited stops: Off only in AM / Full service in PM Arrêts limités : Débarquement en AM seulement / Service complet en PM

Park & Ride / Parc-o-bus

Effective December 23, 2018 En vigueur 23 décembre 2018

CC Transpo

INFO 613-741-4390 octranspo.com

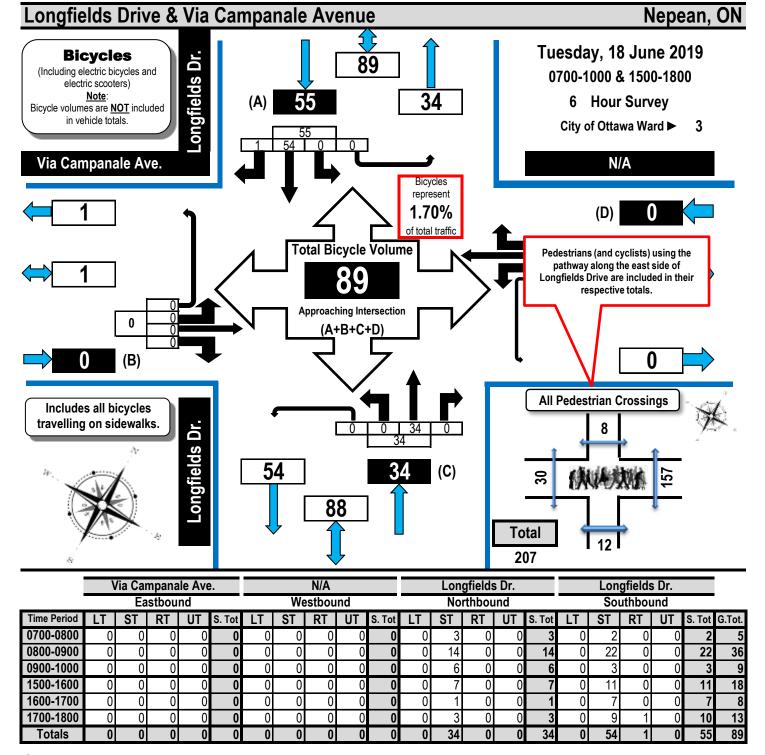




Turning Movement Count

Bicycle Summary Flow Diagram





Comments:

Printed on: 6/20/2019

New housing construction underway west of Longfields Drive. Although this is a 'T' intersection, pedestrians and cyclists were counted on all quadrants.

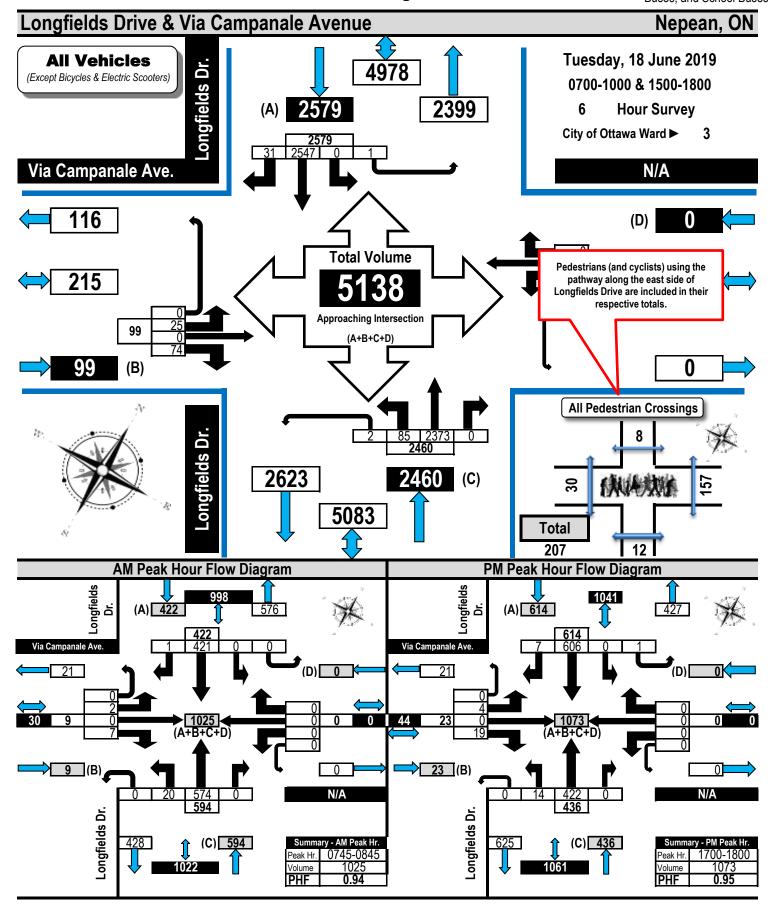


Printed on: 6/20/2019

Turning Movement Count Summary, AM and PM Peak Hour Flow Diagrams

Automobiles, Taxis, Light Trucks, Vans, SUV's, Motorcycles, Heavy Trucks, Buses, and School Buses

Flow Diagrams: AM PM Peak

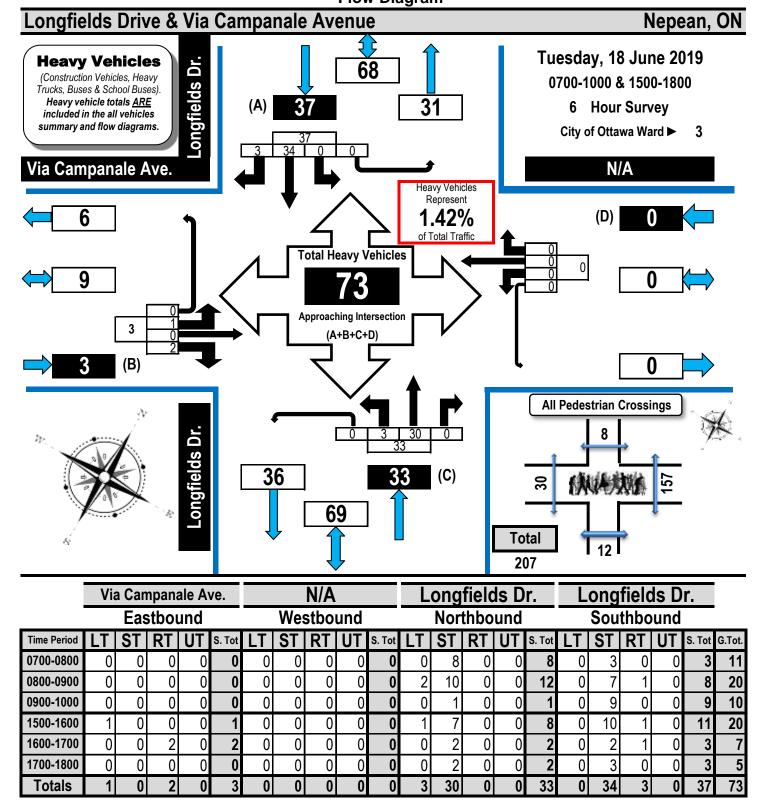




Turning Movement Count

Heavy Trucks, Buses, and School Buses

Heavy Vehicle Summary Flow Diagram



Comments:

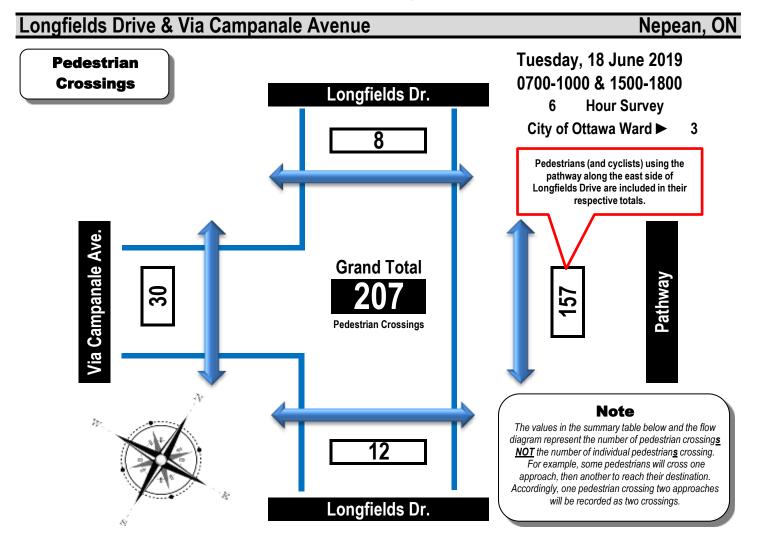
Printed on: 6/20/2019

New housing construction underway west of Longfields Drive. Although this is a 'T' intersection, pedestrians and cyclists were counted on all quadrants.



Turning Movement Count Pedestrian Crossings Summary and Flow Diagram





Time Period	West Side Crossing	East Side on	Street	South Side Crossing	North Side Crossing	Street	Grand
Time Period	Via Campanale Ave.	Ped/Bike Path	Total	Longfields Dr.	Longfields Dr.	Total	Total
0700-0800	0	9	9	0	0	0	9
0800-0900	5	34	39	2	4	6	45
0900-1000	5	29	34	1	2	3	37
1500-1600	3	37	40	3	1	4	44
1600-1700	12	14	26	6	1	7	33
1700-1800	5	34	39	0	0	0	39
Totals	30	157	187	12	8	20	207

Comments:

New housing construction underway west of Longfields Drive. Although this is a 'T' intersection, pedestrians and cyclists were counted on all quadrants.



Turning Movement Count Summary Report AADT and Expansion Factors

Automobiles, Taxis, Light Trucks, Vans, SUV's, Motorcycles, Heavy Trucks, Buses, and School Buses

Longfields Drive & Via Campanale Avenue

Nepean, ON

Survey Date: Tuesday, 18 June 2019 Start Time: 0700 AADT Factor: 0.9

Weather AM: Clear +12°C Survey Duration: 6 Hrs. Survey Hours: 0700-1000 & 1500-1800

Weather PM: Partly Cloudy +26°C Surveyor(s): Carmody

											-		· / · \	- 1									
	Via	Can	npan	ale A	ve.			N/A				L	ong	field	ds D	r.	L	ong	field	ds D	r.		
		Ea	stbou	ınd			We	estbou	ınd		_		Noi	rthbo	und			Sou	ıthbo	und			
Time	LT	ST	RT	UT	E/B	LT	ST	RT	UT	W/B	Street	LT	ST	RT	UT	N/B	ΙT	ST	RT	UT	S/B		Grand
Period		<u> </u>		0.	Tot	_	•		٠.	Tot	Total	-	0.		•	Tot	_	•		•	Tot	Total	Total
0700-0800	2	0	5	0	7	0	0	0	0	0	7	10	476	0	0	486	0	264	4	0	268	754	761
0800-0900	5	0	8	0	13	0	0	0	0	0	13	20	555	0	0	575	0	396	2	0	398	973	986
0900-1000	3	0	9	0	12	0	0	0	0	0	12	11	247	0	0	258	0	268	6	0	274	532	544
1500-1600	4	0	17	0	21	0	0	0	0	0	21	12	319	0	2	333	0	505	6	0	511	844	865
1600-1700	7	0	16	0	23		0	0	0	0	23	18	354	0	0	372	0	508	6	0	514	886	
1700-1800	4	0	19	0	23	0	0	0	0	0	23	14	422	0	0	436	0	606	7	1	614	1050	1073
Totals	25	0	74	0	99	0	0	0	0	0	99	85	2373	0	2	2460	0	2547	31	1	2579	5039	5138

Equivalent 12 & 24-hour Vehicle Volumes Including the Annual Average Daily Traffic (AADT) Factor Applicable to the Day and Month of the Turning Movement Count

Expansion factors are applied exclusively to standard <u>weekday</u> 8-hour turning movement counts conducted during the hours of 0700h - 1000h, 1130h - 1330h and 1500h - 1800h

Equ. 12 Hr	Eq n/a	uivalen n/a	nt 12-ho n/a	our veh n/a		These n/a			alculate n/a						expans n/a			n/a	n/a
AADT 12-hr	n/a	. '	ge dail n/a								•	 -	•		•		tor of: (n/a
AADT 24 Hr	24-H o	our AAE n/a	OT. The n/a		re calc n/a		y mult n/a	i plying n/a	the ave n/a	•	•		olumes n/a	•		actor o	f 1.31 n/a	n/a	n/a

AADT and expansion factors provided by the City of Ottawa

AM Peak Ho	our Fac	tor =		0.9)4								High	est H	ourly \	/ehic	le Volu	ıme B	Betwe	en 0700h & 1	1000h
AM Peak Hr	LT	ST	RT	UT	TOT	LT	ST	RT	UT	TOT S.TOT	LT	ST	RT	UT	TOT	LT	ST	RT	UT	TOT S.TOT	G.TOT
0745-0845	2	0	7	0	9	0	0	0	0	0 9	20	574	0	0	594	0	421	1	0	422 1016	1025

PM Peak Ho	our Fac	tor =)	0.9)5								High	est H	ourly `	Vehic	le Volι	ıme B	etwe	en 150	00h &	1800h
PM Peak Hr	LT	ST	RT	UT	TOT	LT	ST	RT	UT	TOT S.TOT	LT	ST	RT	UT	TOT	LT	ST	RT	UT	TOT	S.TOT	G.TOT
1700-1800	4	0	19	0	23	0	0	0	0	0 23	14	422	0	0	436	0	606	7	1	614	1050	1073

Comments:

New housing construction underway west of Longfields Drive. Although this is a 'T' intersection, pedestrians and cyclists were counted on all quadrants.

Notes:

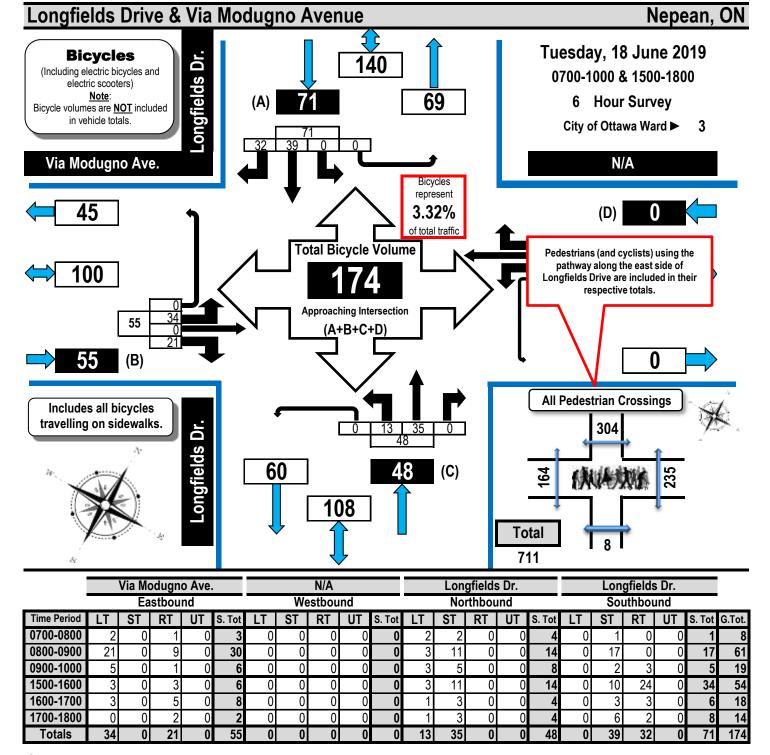
- 1. Includes all vehicle types except bicycles, electric bicycles, and electric scooters.
- 2. When expansion and AADT factors are applied, the results will differ slightly due to rounding.



Turning Movement Count

Bicycle Summary Flow Diagram





Comments:

Printed on: 6/20/2019

New housing construction underway west of Longfields Drive. Although this is a 'T' intersection, pedestrians and cyclists were counted on all quadrants. There was a school field trip between 0945 & 1000 resulting in a substantial increase in pedestrian crossings during that time period. Several drivers completely ignored the red pedestrian signal regardless of whether adults or children were waiting to cross Longfields Drive.

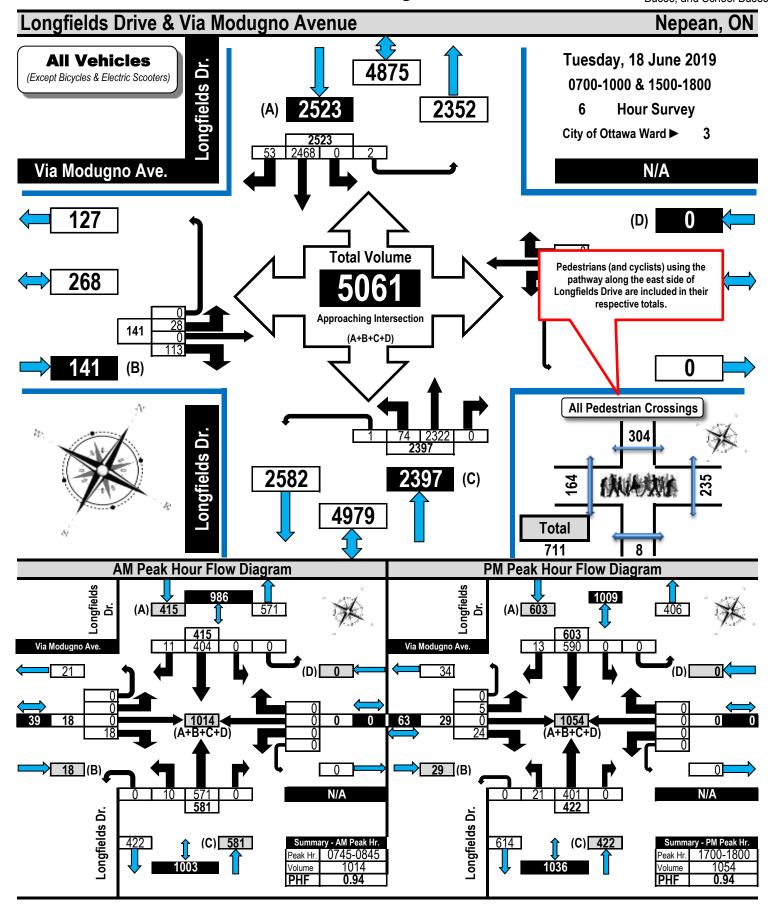


Printed on: 6/20/2019

Turning Movement Count Summary, AM and PM Peak Hour Flow Diagrams

Automobiles, Taxis, Light Trucks, Vans, SUV's, Motorcycles, Heavy Trucks, Buses, and School Buses

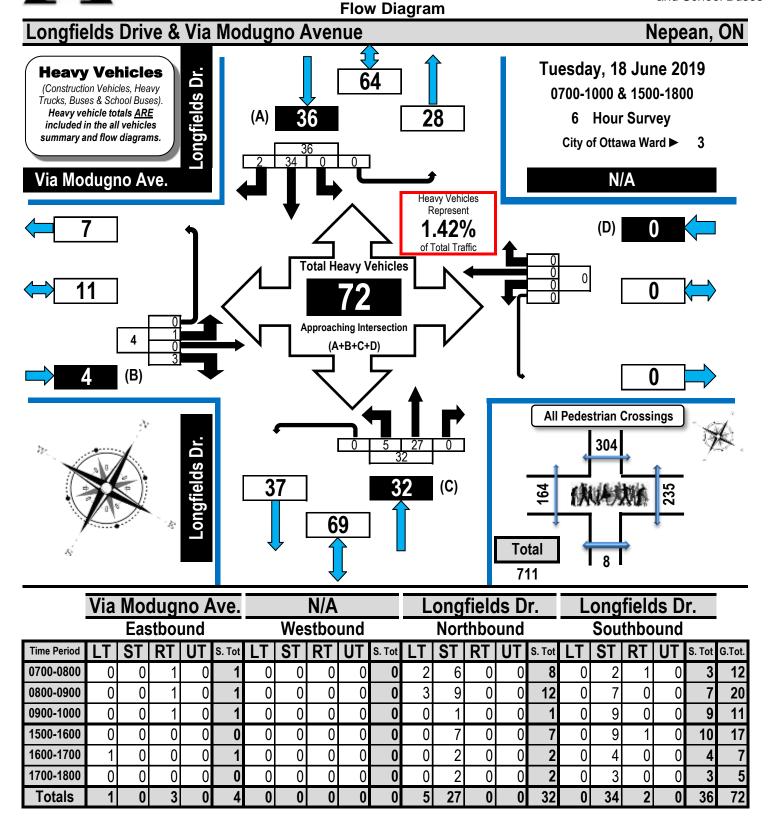
Flow Diagrams: AM PM Peak





Turning Movement Count Heavy Vehicle Summary

Heavy Trucks, Buses, and School Buses



Comments:

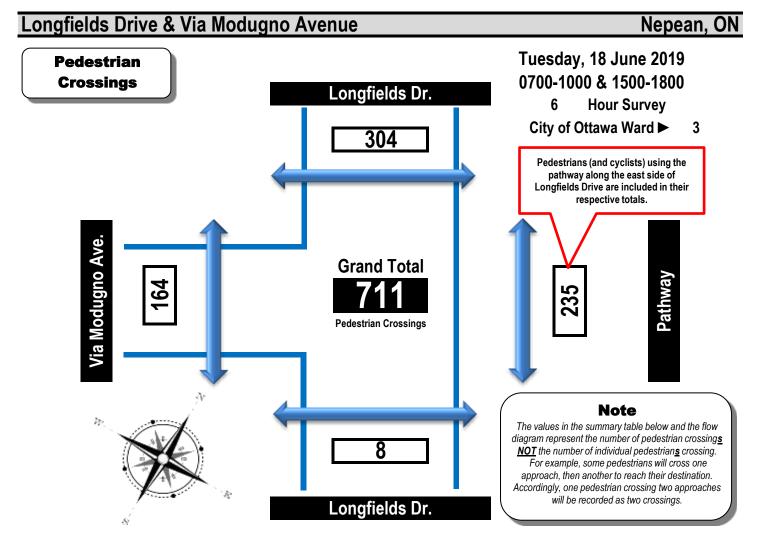
Printed on: 6/20/2019

New housing construction underway west of Longfields Drive. Although this is a 'T' intersection, pedestrians and cyclists were counted on all quadrants. There was a school field trip between 0945 & 1000 resulting in a substantial increase in pedestrian crossings during that time period. Several drivers completely ignored the red pedestrian signal regardless of whether adults or children were waiting to cross Longfields Drive.



Turning Movement Count Pedestrian Crossings Summary and Flow Diagram





Time Period	West Side Crossing	East Side on	Street	South Side Crossing	North Side Crossing	Street	Grand
Time Period	Via Modugno Ave.	Ped/Bike Path	Total	Longfields Dr.	Longfields Dr.	Total	Total
0700-0800	4	7	11	2	14	16	27
0800-0900	16	25	41	0	39	39	80
0900-1000	17	81	98	1	77	78	176
1500-1600	90	77	167	3	122	125	292
1600-1700	24	20	44	2	17	19	63
1700-1800	13	25	38	0	35	35	73
Totals	164	235	399	8	304	312	711

Comments:

New housing construction underway west of Longfields Drive. Although this is a 'T' intersection, pedestrians and cyclists were counted on all quadrants. There was a school field trip between 0945 & 1000 resulting in a substantial increase in pedestrian crossings during that time period. Several drivers completely ignored the red pedestrian signal regardless of whether adults or children were waiting to cross Longfields Drive.



Turning Movement Count Summary Report AADT and Expansion Factors

Automobiles, Taxis, Light Trucks, Vans, SUV's, Motorcycles, Heavy Trucks, Buses, and School Buses

Longfields Drive & Via Modugno Avenue

Nepean, ON

Survey Date: Tuesday, 18 June 2019 Start Time: 0700 AADT Factor: 0.9

Weather AM: Clear +12°C Survey Duration: 6 Hrs. Survey Hours: 0700-1000 & 1500-1800

Weather PM: Partly Cloudy +26°C Surveyor(s): Carmody

	Via	Мо	dug	no A	ve.			N/A				L	ong		ds D	r.	Ĺ	ong	field	ds D	r.		
		Ea	stbou	ınd			We	stbou	ınd		•		Noi	rthbo	und			Sou	ıthbo	und		l	
Time Period	LT	ST	RT	UT	E/B Tot	LT	ST	RT	UT	W/B Tot	Street Total	LT	ST	RT	UT	N/B Tot	LT	ST	RT	UT	S/B Tot	Street Total	Grand Total
0700-0800	1	0	15	0	16	0	0	0	0	0	16	8	469	0	0	477	0	253	9	0	262	739	755
0800-0900	0	0	15	0	15	0	0	0	0	0	15	10	555	0	0	565	0	383	6	0	389	954	969
0900-1000	2	0	13	0	15	0	0	0	0	0	15	12	247	0	1	260	0	261	5	0	266	526	541
1500-1600	12	0	27	0	39	0	0	0	0	0	39	14	305	0	0	319	0	486	11	1	498	817	856
1600-1700	8	0	19	0	27	0	0	0	0	0	27	9	345	0	0	354	0	495	9	1	505	859	886
1700-1800	5	0	24	0	29	0	0	0	0	0	29	21	401	0	0	422	0	590	13	0	603	1025	1054
Totals	28	0	113	0	141	0	0	0	0	0	141	74	2322	0	1	2397	0	2468	53	2	2523	4920	5061

Equivalent 12 & 24-hour Vehicle Volumes Including the Annual Average Daily Traffic (AADT) Factor Applicable to the Day and Month of the Turning Movement Count

Expansion factors are applied exclusively to standard <u>weekday</u> 8-hour turning movement counts conducted during the hours of 0700h - 1000h, 1130h - 1330h and 1500h - 1800h

Equ. 12 Hr	Eq n/a	uivalen n/a	nt 12-ho n/a	our veh n/a	icle vol n/a	umes. n/a		volume n/a		alculate n/a	d by m n/a				by the 8 n/a			ion fac n/a	tor of 1 n/a	.39 n/a	n/a	n/a
AADT 12-hr	n/a	Averaç n/a	ge dail n/a	y 12-ho n/a	ur vehi n/a	cle vol n/a		These v n/a	olumes n/a	s are ca n/a	_	•	 •	•			tals by n/a		DT fact n/a	or of: 0	.9 n/a	n/a
AADT 24 Hr	24-H o	our AAD n/a	T. The	se volu n/a	mes ar n/a	re calc n/a	ulated k n/a	y mult n/a	iplying n/a	the ave	rage da n/a	-	hicle v n/a	olumes n/a	by the	12 ⇒ : n/a	24 expa n/a		factor o n/a	f 1.31 n/a	n/a	n/a

AADT and expansion factors provided by the City of Ottawa

AM Peak Ho	our Fac	tor •	>	0.9	94								High	est H	ourly '	Vehic	le Volu	ıme B	etwe	en 070	0h &	1000h
AM Peak Hr	LT	ST	RT	UT	TOT	LT	ST	RT	UT	TOT S.TOT	LT	ST	RT	UT	TOT	LT	ST	RT	UT	TOT S	S.TOT	G.TOT
0745-0845	0	0	18	0	18	0	0	0	0	0 18	10	571	0	0	581	0	404	11	0	415	996	1014

PM Peak Ho	our Fac	tor =)	0.9	94								High	est H	ourly \	Vehic	le Volu	ıme B	etwe	en 150	00h &	1800h
PM Peak Hr	LT	ST	RT	UT	TOT	LT	ST	RT	UT	TOT S.TOT	LT	ST	RT	UT	TOT	LT	ST	RT	UT	TOT	S.TOT	G.TOT
1700-1800	5	0	24	0	29	0	0	0	0	0 29	21	401	0	0	422	0	590	13	0	603	1025	1054

Comments:

New housing construction underway west of Longfields Drive. Although this is a 'T' intersection, pedestrians and cyclists were counted on all quadrants. There was a school field trip between 0945 & 1000 resulting in a substantial increase in pedestrian crossings during that time period. Several drivers completely ignored the red pedestrian signal regardless of whether adults or children were waiting to cross Longfields Drive.

Notes:

- 1. Includes all vehicle types except bicycles, electric bicycles, and electric scooters.
- 2. When expansion and AADT factors are applied, the results will differ slightly due to rounding.





City Operations - Transportation Services

Collision Details Report - Public Version

From: January 1, 2013 **To:** December 31, 2017

Location: LONGFIELDS DR @ VIA CAMPANALE AVE

Traffic Control: Stop sign Total Collisions: 1

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2015-May-05, Tue,19:04	Clear	Approaching	Non-fatal injury	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	
					East	Slowing or stoppin	g Automobile, station wagon	Other motor vehicle	

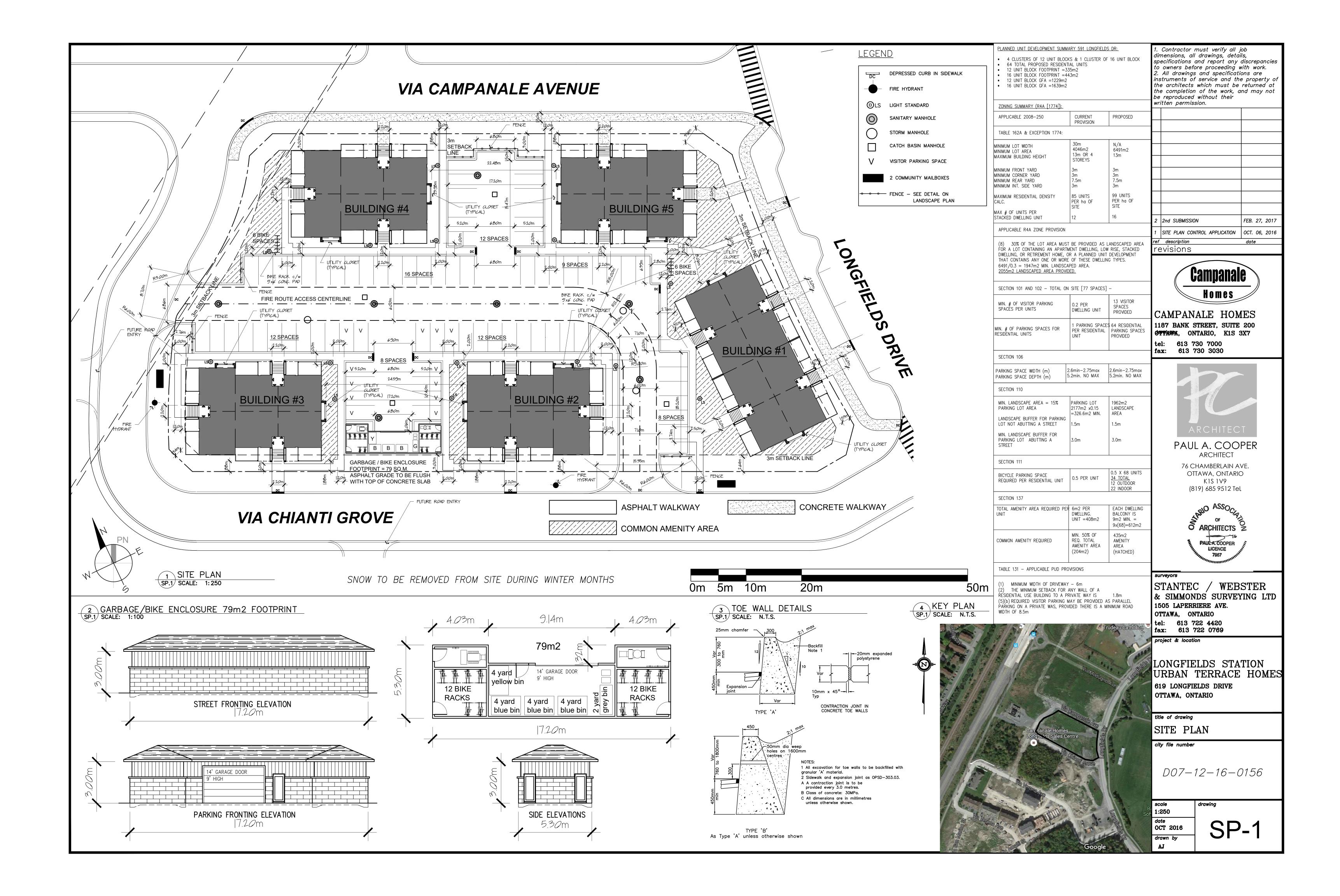
Location: LONGFIELDS DR @ VIA MODUGNO PL

Traffic Control: Stop sign Total Collisions: 1

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2017-Jun-15, Thu,09:12	Clear	SMV other	P.D. only	Dry	East	Turning left	Truck - tractor	Other	

Monday, June 17, 2019 Page 1 of 1







TRANS Trip Generation for Low-Rise Apartments (2 floors) with a Total of 28 Dwelling Units

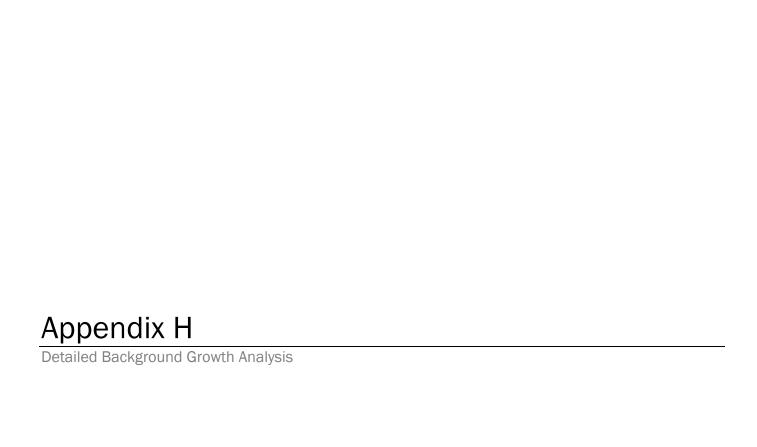
Travel Mode	AM Mode Share	AM	Peak (persons	s/h)	PM Mode	PM	Peak (persons	s/h)
Travel Mode	AW Wode Share	In	Out	Total	Share	In	Out	Total
Auto Driver	35%	1	7	8	35%	6	4	10
Auto Passenger	15%	1	2	3	15%	3	2	5
Transit	35%	1	7	8	35%	6	4	10
Non-motorized	15%	1	3	4	15%	3	2	5
Total People Trips	4	19	23	100%	18	12	30	
Total 'New' TRANS Trip Generation for L with a Total	1	7	8		6	4	10	

ITE Trip Generation for 15,000 sq. ft. Commercial Use

Travel Mode	Mode Share	AM Pea	ak (Person T	rips/hr)	PM Peak (Person Trips/hr)			
Travel Mode	Widde Share	In	Out	Total	In	Out	Total	
Auto Driver	15%	0	0	0	5	7	12	
Auto Passenger	5%	0	0	0	2	2	4	
Transit	15%	0	0	0	5	6	11	
Non-motorized	65%	0	0	0	20	27	47	
Total Person Trips	100%	0	0	0	32	42	74	
	Less Pass-by (0%)	0	0	0	0	0	0	
Total 'New' ITE Trip Generation for 15,	0	0	0	5	7	12		

Total Trips Generated by Proposed Future Development

Travel Mode	A	M Peak (persons,	/h)	PM Peak (persons/h)			
Travel Mode	In	Out	Total	In	Out	Total	
Auto Driver	1	7	8	11	11	22	
Auto Passenger	1	2	3	5	4	9	
Transit	1	7	8	11	10	21	
Non-motorized	1	3	4	23	29	52	
Total People Trips	4	19	23	50	54	104	
Total 'New' Auto Trips	1	7	8	11	11	22	



Longfields/Mountshannon AM Peak

Year	Date	North Leg		Sout	South Leg		East Leg		West Leg	
rear	Date	SB	NB	NB	SB	WB	EB	EB	WB	Total
2012	30-Aug	125	73	16	142	185	242	318	187	1288
2013	31-May	80	92	11	36	96	262	347	144	1068
2019	18-Jun					415	571	581	422	1989

North Leg

Year		Cou	ınts		% Change				
Year	NB	SB	NB+SB	INT	NB	SB	NB+SB	INT	
2012	73	125	198	1288					
2013	92	80	172	1068	26.0%	-36.0%	-13.1%	-17.1%	
2019				1989				86.2%	

Regression Estimate Regression Estimate
Average Annual Change

2012 2019

West Leg

Year		Co	unts		% Change				
real	EB	WB	EB+WB	INT	EB	WB	EB+WB	INT	
2012	318	187	505	1288					
2013	347	144	491	1068	9.1%	-23.0%	-2.8%	-17.1%	
2019	581	422	1003	1989	67.4%	193.1%	104.3%	86.2%	

Regression Estimate Regression Estimate

2012 314 2019 580

150 416 996 15.72%

Average Annual Change

9.18%

11.55%

East Leg

Year		Cou	unts		% Change					
real	EB	WB	EB+WB	INT	EB	WB	EB+WB	INT		
2012	242	185	427	1288						
2013	262	96	358	1068	8.3%	-48.1%	-16.2%	-17.1%		
2019	571	415	986	1989	117.9%	332.3%	175.4%	86.2%		

Regression Estimate Regression Estimate
Average Annual Change 2012 229 2019 569

13.89%

125 354 405 974 15.54% 18.22%

South Leg

Year		Co	unts		% Change				
rear	NB	SB	NB+SB	INT	NB	SB	NB+SB	INT	
2012	16	142	158	1288					
2013	11	36	47	1068	-31.3%	-74.6%	-70.3%	-17.1%	
2019				1989				86.2%	

Regression Estimate Regression Estimate

2012 2019

Average Annual Change

Longfields/Mountshannon PM Peak

Year	Date	North Leg		Sout	South Leg		East Leg		West Leg	
rear	Date	SB	NB	NB	SB	WB	EB	EB	WB	Total
2012	30-Aug	164	153	9	114	199	160	327	272	1398
2013	31-May	121	111	4	8	207	177	261	297	1186
2019	18-Jun					603	406	422	614	2045

North Leg

Year		Cou	ınts		% Change				
Year	NB	SB	NB+SB	INT	NB	SB	NB+SB	INT	
2012	153	164	317	1398					
2013	111	121	232	1186	-27.5%	-26.2%	-26.8%	-15.2%	
2019				2045				72.4%	

Regression Estimate Regression Estimate
Average Annual Change

2012 2019

West Leg

Year		Cou	unts		% Change				
real	EB	WB	EB+WB	INT	EB	WB	EB+WB	INT	
2012	327	272	599	1398					
2013	261	297	558	1186	-20.2%	9.2%	-6.8%	-15.2%	
2019	422	614	1036	2045	61.7%	106.7%	85.7%	72.4%	

Regression Estimate Regression Estimate

2012 2019 288 416

548 1028

326

Average Annual Change

5.37%

612 12.99% 9.38%

260

East Leg

Year		Cou	unts		% Change				
real	EB	WB	EB+WB	INT	EB	WB	EB+WB	INT	
2012	160	199	359	1398					
2013	177	207	384	1186	10.6%	4.0%	7.0%	-15.2%	
2019	406	603	1009	2045	129.4%	191.3%	162.8%	72.4%	

Regression Estimate Regression Estimate
Average Annual Change

151 405

175 599 1003

15.10% 17.43% 19.24%

South Leg

Year		Cou	unts			% Cr	nange	
real	NB	SB	NB+SB	INT	NB	SB	NB+SB	INT
2012	9	114	123	1398				
2013	4	8	12	1186	-55.6%	-93.0%	-90.2%	-15.2%
2019				2045				72.4%

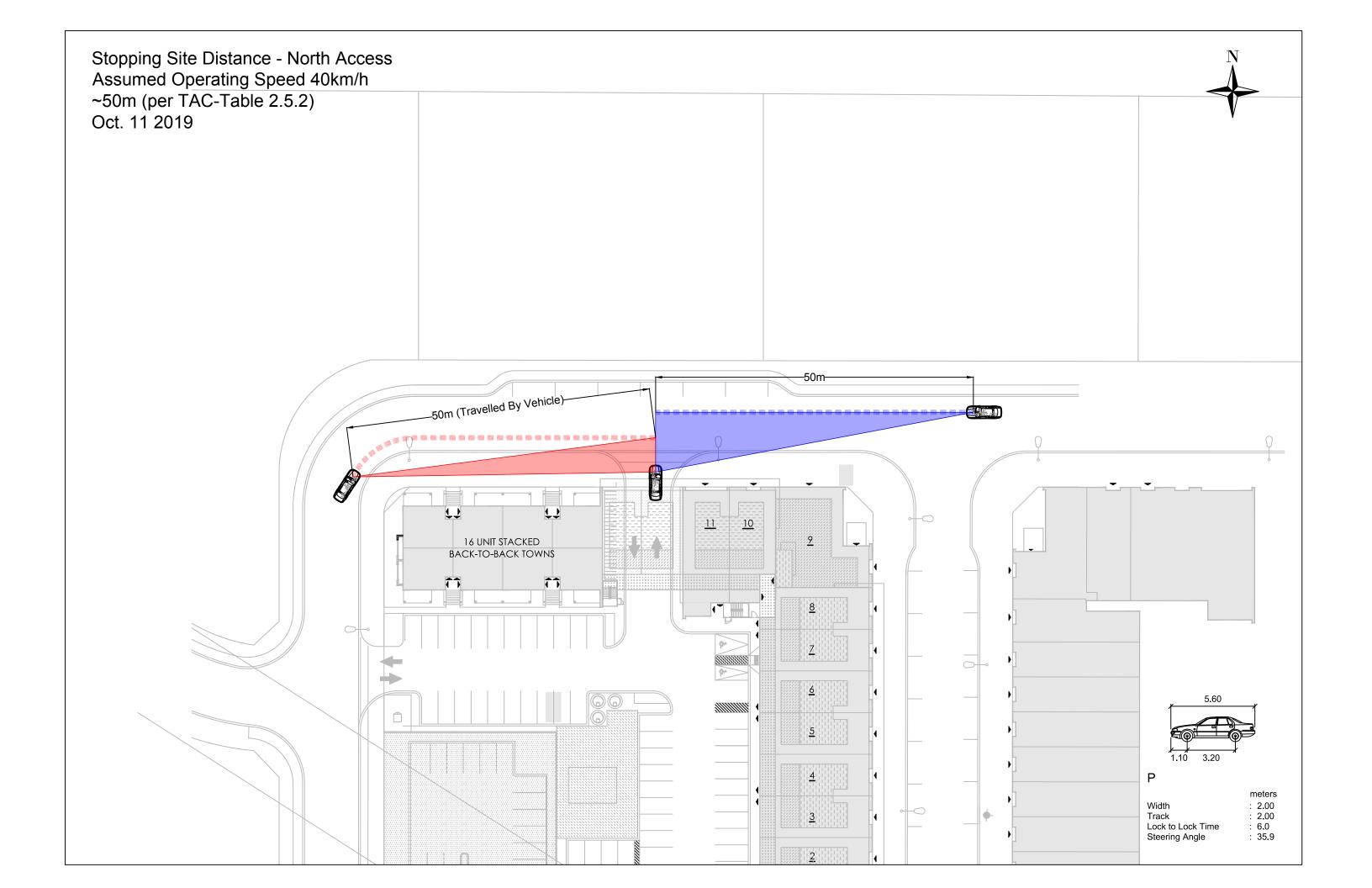
Regression Estimate Regression Estimate Average Annual Change

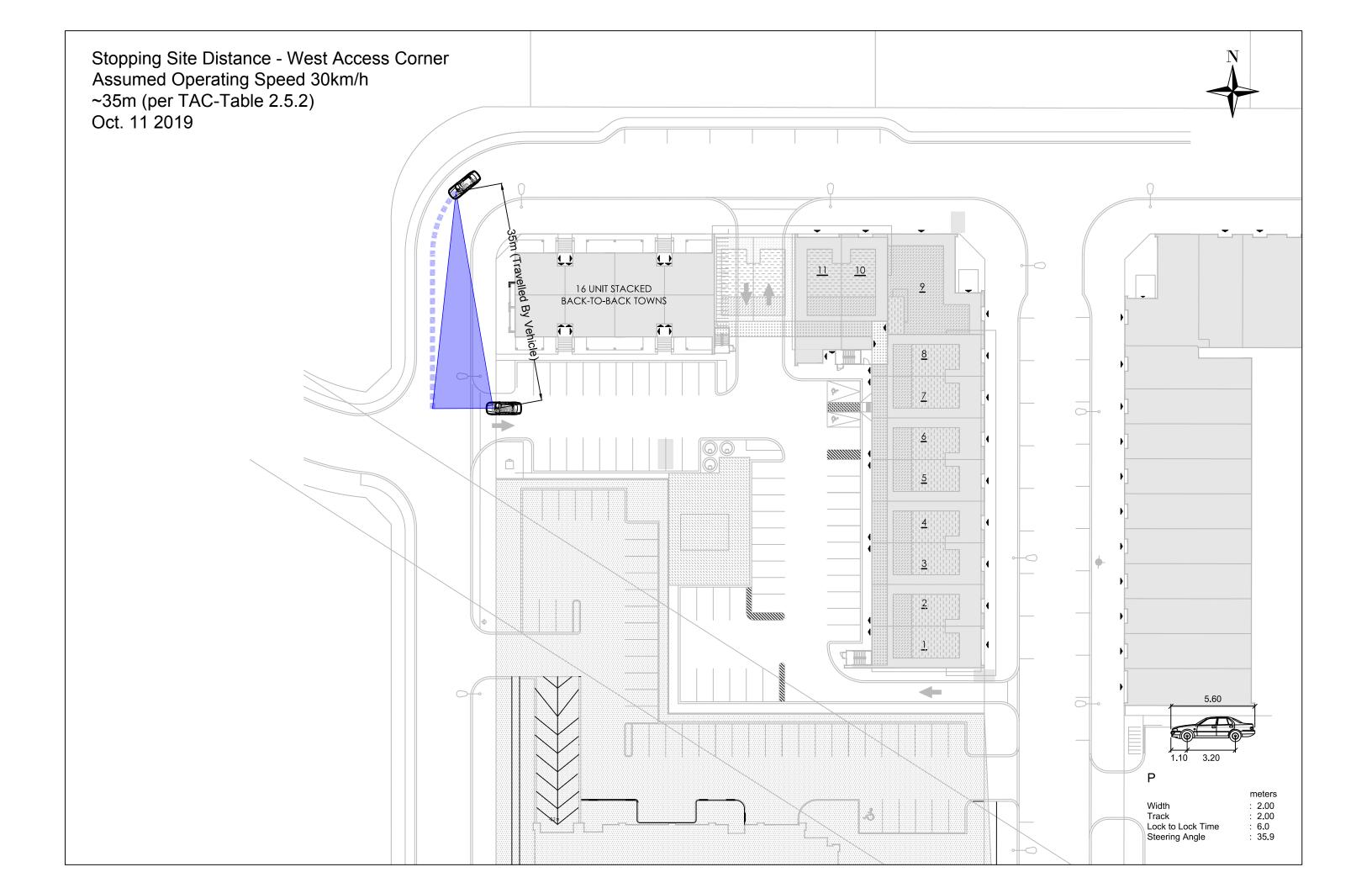
2012 2019

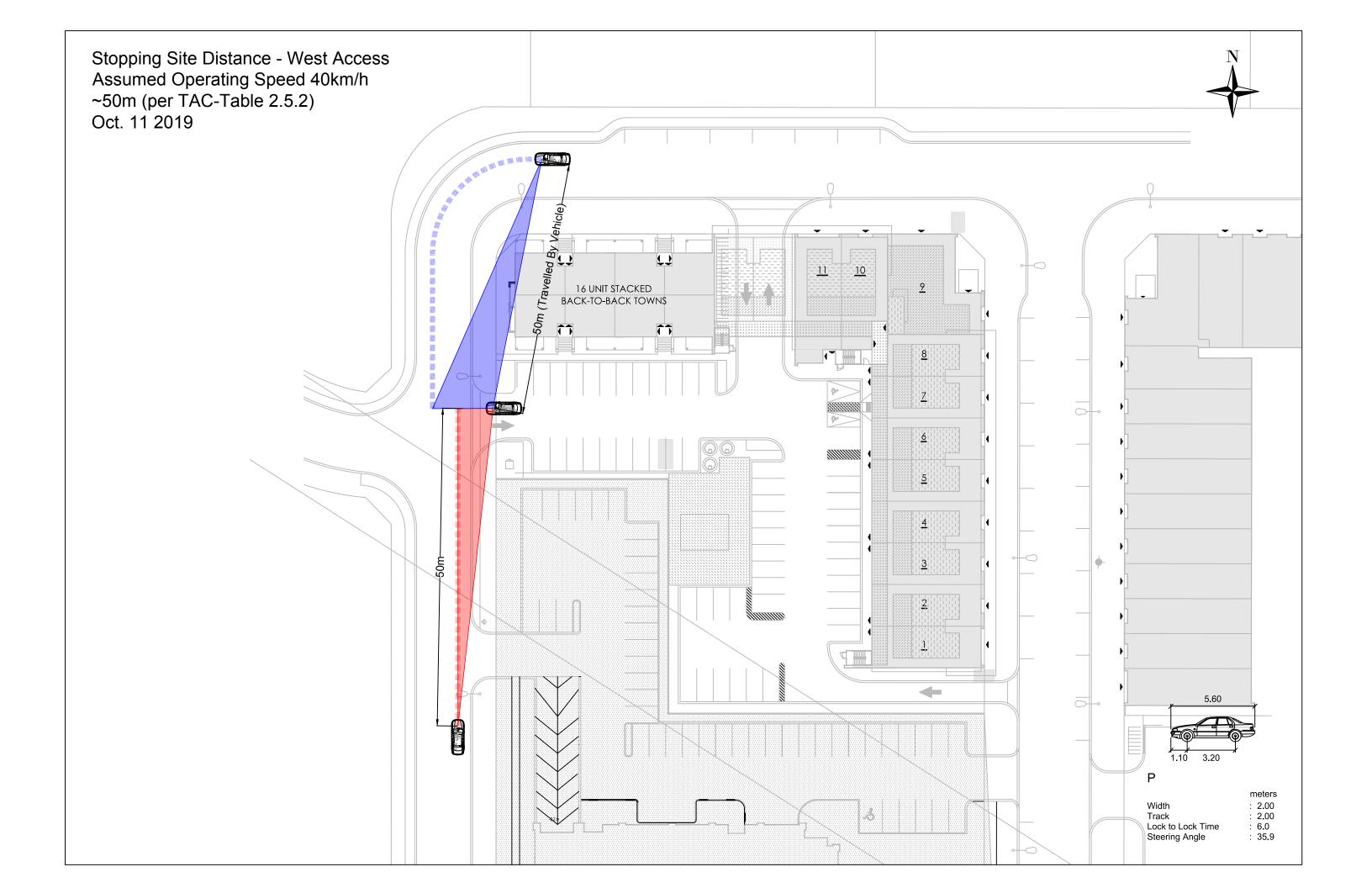
2012

2019











Multi-Modal Level of Service - Segments Form

Consultant
Scenario
Comments

PARSONS	Project
ΓΙΑ Step 4	Date
Block 14	

SEGMENTS		Street A	Via Campanale	Via Campanale	Via Modugno
OEGINER 10		Olicel A	West Frontage	North Frontage	East Frontage
	Sidewalk Width Boulevard Width		≥ 2 m < 0.5	≥ 2 m < 0.5	≥ 2 m < 0.5
	Avg Daily Curb Lane Traffic Volume		≤ 3000	≤ 3000	≤ 3000
_	Operating Speed		≤ 30 km/h	≤ 30 km/h	≤ 30 km/h
Pedestrian	On-Street Parking		no	no	yes
est	Exposure to Traffic PLoS	-	Α	Α	Α
þ	Effective Sidewalk Width				
A.	Pedestrian Volume				
	Crowding PLoS		-	-	-
	Level of Service		-	-	-
	Type of Cycling Facility		Mixed Traffic	Mixed Traffic	Mixed Traffic
	Number of Travel Lanes		≤ 2 (no centreline)	≤ 2 (no centreline)	≤ 2 (no centreline)
	Operating Speed		≤ 40 km/h	≤ 40 km/h	≤ 40 km/h
	# of Lanes & Operating Speed LoS		Α	Α	Α
Bicycle	Bike Lane (+ Parking Lane) Width				
۶	Bike Lane Width LoS	Α	-	-	-
<u>B</u>	Bike Lane Blockages				
	Blockage LoS			- 10 6	- 10 - 1
	Median Refuge Width (no median = < 1.8 m) No. of Lanes at Unsignalized Crossing		< 1.8 m refuge ≤ 3 lanes	< 1.8 m refuge ≤ 3 lanes	< 1.8 m refuge ≤ 3 lanes
	Sidestreet Operating Speed		≤ 40 km/h	≤ 40 km/h	≤ 40 km/h
	Unsignalized Crossing - Lowest LoS		A	A	A
	Level of Service		Α	Α	Α
±	Facility Type		Mixed Traffic	Mixed Traffic	Mixed Traffic
Transit	Friction or Ratio Transit:Posted Speed	D	Vt/Vp ≥ 0.8	Vt/Vp ≥ 0.8	Vt/Vp ≥ 0.8
Ë	Level of Service		D	D	D
	Truck Lane Width		> 3.7 m	> 3.7 m	> 3.7 m
ick S	Travel Lanes per Direction	В	1	1	1
Truck	Level of Service	В	В	В	В



Existing AM 1: Longfields Dr & Via Modugno Pl

	٠	•	1	†	†	4
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			ર્ન	f)	
Traffic Volume (veh/h)	0	18	10	571	404	11
Future Volume (Veh/h)	0	18	10	571	404	11
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	20	11	634	449	12
Pedestrians	16				39	
Lane Width (m)	3.7				3.7	
Walking Speed (m/s)	1.1				1.1	
Percent Blockage	1				4	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1166	471	477			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1166	471	477			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	97	99			
cM capacity (veh/h)	201	584	1069			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	20	645	461			
Volume Left	0	11	0			
Volume Right	20	0	12			
cSH	584	1069	1700			
Volume to Capacity	0.03	0.01	0.27			
Queue Length 95th (m)	0.03	0.01	0.27			
Control Delay (s)	11.4	0.2	0.0			
Lane LOS	В	0.3 A	0.0			
Approach Delay (s)	11.4	0.3	0.0			
Approach LOS	11.4 B	0.3	0.0			
Approach LOS	Б					
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utiliza	ation		50.2%	IC	CU Level c	of Service
Analysis Period (min)			15			

Existing AM 2: Longfields Dr & Via Campanale Ave

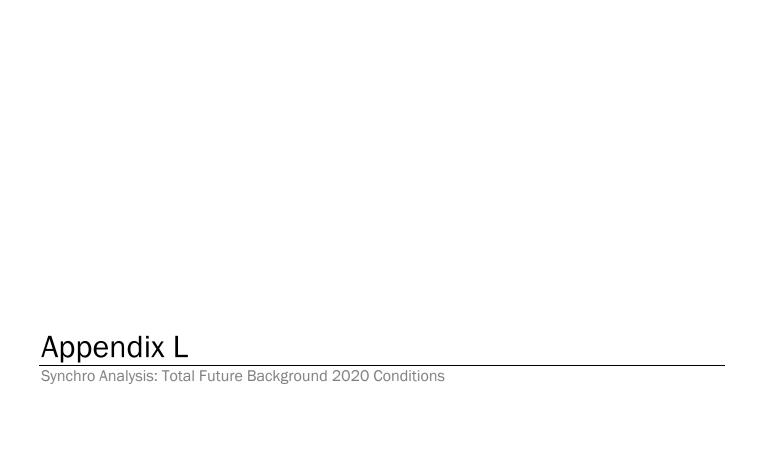
	۶	•	1	†	†	4
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			ર્ન	-f	
Traffic Volume (veh/h)	2	7	20	574	421	1
Future Volume (Veh/h)	2	7	20	574	421	1
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	2	8	22	638	468	1
Pedestrians	5			2	4	
Lane Width (m)	3.7			3.7	3.7	
Walking Speed (m/s)	1.1			1.1	1.1	
Percent Blockage	0			0	0	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1160	476	474			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1160	476	474			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	99	98			
cM capacity (veh/h)	210	585	1083			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	10	660	469			
Volume Left	2	22	0			
Volume Right	8	0	1			
cSH	431	1083	1700			
Volume to Capacity	0.02	0.02	0.28			
Queue Length 95th (m)	0.5	0.5	0.0			
Control Delay (s)	13.5	0.5	0.0			
Lane LOS	В	Α				
Approach Delay (s)	13.5	0.5	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utiliza	ition		59.6%	IC	CU Level o	of Service
Analysis Period (min)			15		. 5 25 07 0	
raidiyələ i Gilod (illili)			13			

Existing PM 1: Longfields Dr & Via Modugno Pl

	۶	•	4	†	†	4
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	eĵ.	
Traffic Volume (veh/h)	5	24	21	401	590	13
Future Volume (Veh/h)	5	24	21	401	590	13
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	6	27	23	446	656	14
Pedestrians	13				35	
Lane Width (m)	3.7				3.7	
Walking Speed (m/s)	1.1				1.1	
Percent Blockage	1				3	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1203	676	683			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1203	676	683			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	97	94	97			
cM capacity (veh/h)	190	448	899			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	33	469	670			
Volume Left	33 6	23	0/0			
Volume Right	27	0	14			
cSH	359	899	1700			
Volume to Capacity	0.09	0.03	0.39			
Queue Length 95th (m)	2.3	0.03	0.39			
	16.0					
Control Delay (s)	16.0 C	0.7	0.0			
Lane LOS	16.0	A 0.7	0.0			
Approach LOS	16.0 C	0.7	0.0			
Approach LOS	C					
Intersection Summary						
Average Delay			0.8			
Intersection Capacity Utiliza	tion		50.4%	IC	CU Level c	f Service
Analysis Period (min)			15			

Existing PM 2: Longfields Dr & Via Campanale Ave

	۶	•	1	†	Ţ	4
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			ર્ન	ĵ»	
Traffic Volume (veh/h)	4	19	14	422	606	7
Future Volume (Veh/h)	4	19	14	422	606	7
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	4	21	16	469	673	8
Pedestrians	5					
Lane Width (m)	3.7					
Walking Speed (m/s)	1.1					
Percent Blockage	0					
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1183	682	686			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1183	682	686			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	98	95	98			
cM capacity (veh/h)	205	448	903			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	25	485	681			
Volume Left	4	16	0			
Volume Right	21	0	8			
cSH	376	903	1700			
Volume to Capacity	0.07	0.02	0.40			
Queue Length 95th (m)	1.6	0.4	0.0			
Control Delay (s)	15.2	0.5	0.0			
Lane LOS	С	А				
Approach Delay (s)	15.2	0.5	0.0			
Approach LOS	С					
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utiliza	ition		45.4%	IC	CU Level o	of Service
Analysis Period (min)			15		23.3.0	3 3. 1.30
rinary sis i crioù (illiii)			10			

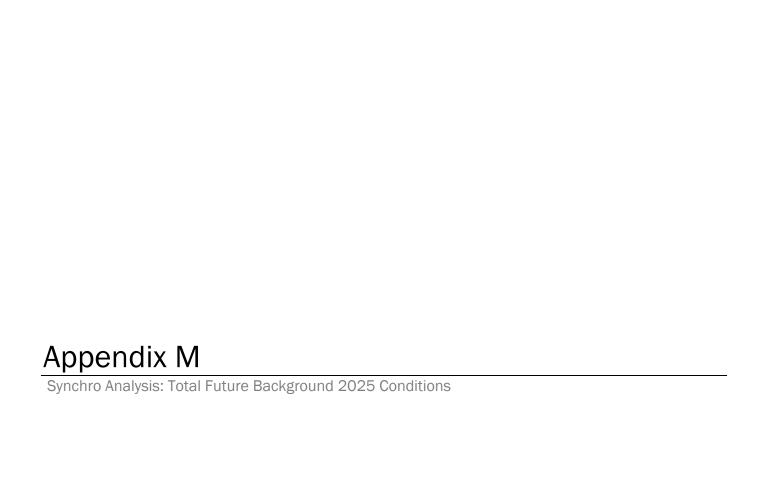


	٠	•	4	†	ļ	4
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			ર્ન	f)	
Traffic Volume (veh/h)	5	27	22	602	424	17
Future Volume (Veh/h)	5	27	22	602	424	17
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	5	27	22	602	424	17
Pedestrians	16				39	
Lane Width (m)	3.7				3.7	
Walking Speed (m/s)	1.1				1.1	
Percent Blockage	1				4	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1134	448	457			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1134	448	457			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	98	96	98			
cM capacity (veh/h)	209	601	1087			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	32	624	441			
Volume Left	5	22	0			
Volume Right	27	0	17			
cSH	465	1087	1700			
Volume to Capacity	0.07	0.02	0.26			
Queue Length 95th (m)	1.7	0.5	0.0			
Control Delay (s)	13.3	0.5	0.0			
Lane LOS	В	A	0.0			
Approach Delay (s)	13.3	0.5	0.0			
Approach LOS	В	0.0	0.0			
Intersection Summary						
Average Delay			0.7			
Intersection Capacity Utiliza	ation		62.2%	IC	CU Level c	of Service
Analysis Period (min)	auon		15	ıc	JO LOVOI C	, JOI VICE
Analysis r chou (IIIII)			10			

	۶	•	4	†	+	4
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	î,	
Traffic Volume (veh/h)	5	7	20	603	442	2
Future Volume (Veh/h)	5	7	20	603	442	2
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	5	7	20	603	442	2
Pedestrians	5			2	4	
Lane Width (m)	3.7			3.7	3.7	
Walking Speed (m/s)	1.1			1.1	1.1	
Percent Blockage	0			0	0	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1095	450	449			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1095	450	449			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	98	99	98			
cM capacity (veh/h)	230	605	1106			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	12	623	444			
Volume Left	5	20	0			
Volume Right	7	0	2			
cSH	361	1106	1700			
Volume to Capacity	0.03	0.02	0.26			
Queue Length 95th (m)	0.03	0.02	0.20			
	15.3	0.4	0.0			
Control Delay (s)	15.3	0.5 A	0.0			
Lane LOS Approach Dolay (s)	15.3	0.5	0.0			
Approach LOS	15.3 C	0.5	0.0			
Approach LOS	C					
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utilization	n		61.2%	IC	CU Level c	of Service
Analysis Period (min)			15			

	۶	•	1	†	†	1
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			4	eĵ.	
Traffic Volume (veh/h)	14	43	34	421	621	20
Future Volume (Veh/h)	14	43	34	421	621	20
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	14	43	34	421	621	20
Pedestrians	13				35	
Lane Width (m)	3.7				3.7	
Walking Speed (m/s)	1.1				1.1	
Percent Blockage	1				3	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1168	644	654			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1168	644	654			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	93	91	96			
cM capacity (veh/h)	197	467	922			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	57	455	641			
Volume Left	14	34	041			
Volume Right	43	0	20			
cSH	349	922	1700			
Volume to Capacity	0.16	0.04	0.38			
Queue Length 95th (m)	4.4	0.04	0.0			
Control Delay (s)	17.3	1.1	0.0			
Lane LOS	17.3 C	Α	0.0			
Approach Delay (s)	17.3	1.1	0.0			
Approach LOS	17.3 C	1.1	0.0			
Approach LOS	C					
Intersection Summary						
Average Delay			1.3			
Intersection Capacity Utilizat	tion		63.3%	IC	CU Level c	f Service
Analysis Period (min)			15			

	٠	•	4	†	↓	4
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			ર્ન	ĵ»	
Traffic Volume (veh/h)	6	19	14	443	636	10
Future Volume (Veh/h)	6	19	14	443	636	10
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	6	19	14	443	636	10
Pedestrians	5					
Lane Width (m)	3.7					
Walking Speed (m/s)	1.1					
Percent Blockage	0					
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1117	646	651			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1117	646	651			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	97	96	98			
cM capacity (veh/h)	225	469	931			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	25	457	646			
Volume Left	6	14	0			
Volume Right	19	0	10			
cSH	372	931	1700			
Volume to Capacity	0.07	0.02	0.38			
Queue Length 95th (m)	1.6	0.3	0.0			
Control Delay (s)	15.4	0.4	0.0			
Lane LOS	С	Α				
Approach Delay (s)	15.4	0.4	0.0			
Approach LOS	С					
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utiliza	ation		46.5%	IC	CU Level o	of Service
Analysis Period (min)			15		, ,,,,,	
rinary sis i onou (illiii)			10			



	•	\rightarrow	•	†	ţ	4
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			4	ĵ _e	
Traffic Volume (veh/h)	5	27	22	744	525	17
Future Volume (Veh/h)	5	27	22	744	525	17
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	5	27	22	744	525	17
Pedestrians	16				39	
Lane Width (m)	3.7				3.7	
Walking Speed (m/s)	1.1				1.1	
Percent Blockage	1				4	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1376	550	558			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1376	550	558			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	97	95	98			
cM capacity (veh/h)	148	527	998			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	32	766	542			
Volume Left	5	22	0			
Volume Right	27	0	17			
cSH	377	998	1700			
Volume to Capacity	0.08	0.02	0.32			
Queue Length 95th (m)	2.1	0.5	0.0			
Control Delay (s)	15.4	0.6	0.0			
Lane LOS	С	Α				
Approach Delay (s)	15.4	0.6	0.0			
Approach LOS	С					
Intersection Summary						
Average Delay			0.7			
Intersection Capacity Utiliza	tion		70.1%	IC	CU Level o	f Service
Analysis Period (min)			15			

-	۶	•	4	†	+	4
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	1>	
Traffic Volume (veh/h)	5	7	20	746	547	2
Future Volume (Veh/h)	5	7	20	746	547	2
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	5	7	20	746	547	2
Pedestrians	5			2	4	
Lane Width (m)	3.7			3.7	3.7	
Walking Speed (m/s)	1.1			1.1	1.1	
Percent Blockage	0			0	0	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1343	555	554			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1343	555	554			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)		<u> </u>				
tF (s)	3.5	3.3	2.2			
p0 queue free %	97	99	98			
cM capacity (veh/h)	163	528	1012			
Direction, Lane # Volume Total	EB 1	NB 1 766	SB 1 549			
Volume Left	5	20	0			
	5 7		2			
Volume Right		1012				
cSH Valuma to Compositu	273	1012	1700			
Volume to Capacity	0.04	0.02	0.32			
Queue Length 95th (m)	1.0	0.5	0.0			
Control Delay (s)	18.8	0.5	0.0			
Lane LOS	C	A				
Approach Delay (s)	18.8	0.5	0.0			
Approach LOS	С					
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utiliza	tion		69.1%	IC	CU Level o	of Service
Analysis Period (min)			15			
J						

	۶	•	4	†	†	4
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	(Î	
Traffic Volume (veh/h)	14	43	34	521	768	20
Future Volume (Veh/h)	14	43	34	521	768	20
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	14	43	34	521	768	20
Pedestrians	13				35	
Lane Width (m)	3.7				3.7	
Walking Speed (m/s)	1.1				1.1	
Percent Blockage	1				3	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1415	791	801			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1415	791	801			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	90	89	96			
cM capacity (veh/h)	139	385	812			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	<u> </u>	555	788			
Volume Left	14	34	0			
Volume Right	43	0	20			
cSH	268	812	1700			
Volume to Capacity	0.21	0.04	0.46			
Queue Length 95th (m)	6.0	1.0	0.0			
Control Delay (s)	22.0	1.1	0.0			
Lane LOS	C C	A	0.0			
Approach Delay (s)	22.0	1.1	0.0			
Approach LOS	C C	1.1	0.0			
	C					
Intersection Summary						
Average Delay			1.3			
Intersection Capacity Utiliza	ation		68.7%	IC	CU Level o	of Service
Analysis Period (min)			15			

	۶	•	4	†	†	4
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			ર્ન	f)	
Traffic Volume (veh/h)	6	19	14	549	788	10
Future Volume (Veh/h)	6	19	14	549	788	10
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	6	19	14	549	788	10
Pedestrians	5					
Lane Width (m)	3.7					
Walking Speed (m/s)	1.1					
Percent Blockage	0					
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1375	798	803			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1375	798	803			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	96	95	98			
cM capacity (veh/h)	157	384	817			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	25	563	798			
Volume Left	6	14	0			
Volume Right	19	0	10			
cSH	285	817	1700			
Volume to Capacity	0.09	0.02	0.47			
Queue Length 95th (m)	2.2	0.02	0.47			
Control Delay (s)	18.8	0.4	0.0			
Lane LOS	10.0 C	0.5 A	0.0			
Approach Delay (s)	18.8	0.5	0.0			
Approach LOS	10.0 C	0.5	0.0			
Approach LOS	C					
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utilizat	ion		54.4%	IC	CU Level c	f Service
Analysis Period (min)			15			