REPORT 37543-5.2.2

SCOTIABANK RIDEAU AND WILLIAM STREET STORMWATER MANAGEMENT BRIEF

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Prepared for Scotiabank by IBI Group October 14, 2016 IBI GROUP REPORT SCOTIABANK RIDEAU STREET & WILLIAM STREET OTTAWA, ONTARIO STORMWATER MANAGEMENT BRIEF

Prepared for: SCOTIABANK

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1 INTRODUCTION

IBI Group has been retained by Scotiabank to design their new building at Rideau Street and William Street in the City of Ottawa. These lands are known as 117 Rideau Street and are zoned mixed use MD S 80.

The development also forms part of the City of Ottawa's LRT project, at this location commuters will be able to access the LRT tunnel which is located under the building. The development also provides the LRT a mechanical connection point to surface where heating and ventilation is provided to the LRT facility through the building. The LRT project has previously received approval for the relocation of municipal infrastructure, for information on site services, see Pageau and Morel Site Servicing Brief for details.

This report deals with addressing the on-site attenuation of stormwater for this development. This site is bounded by existing commercial buildings on two sides and William Street and Rideau Street on the other, see **Figure 1**. This report provides details on stormwater management to support the Site Plan for this development. No pre-consultation meetings were requested with the Rideau Valley Conservation Authority (RVCA), and Ministry of Environment (MOE) to solicit input on the detailed design as this is an infill parcel with connections to existing sewers.



				-		PROJECT TITLE SCOTIABANK - RIDEAU AND WILLIAM BRANCH	SCALE JOB NO. 37543	SKETCH NO. FIGURE 1	ISSUE
						SKETCH TITLE	DRAWN BY		
REVISION NO.	ISSUED WITH	STATUS	REVISION DATE	IBI	ibigroup.com	EXISTING AREA - WILLIAM AND RIDEAU STREET	DO NOT SCALE RESPONSIBLE F AND REPORTING IN WRITING BEF	THIS DRAWING. THE CONTRACTOR SHALL OR TAKING AND VERIFYING ALL THE DIMED 3 ERRORS AND/OR OMISSIONS TO THE ARC ORE PROCEEDING WITH THE WORK.	BE NSIONS CHITECT

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2 STORMWATER MANAGEMENT

The proposed development replaces a previous building which as per the proposed development, occupies 100% of the site. The existing building was connected to the municipal storm system and unattenuated roof drain discharged to the storm sewer system. Based on a roof area of 512 m², C=1.0, 100 year I = 178.6 mm/hr at a Tc of 10 min., the existing building would have a peak flow of 25.86 l/s during a 1:100 yr. rainfall event.

City of Ottawa design guidelines for the redevelopment of sites in the City core area (separated sewer areas) identified the preferred stormwater management, and provided on-site attenuation to achieve restricting peak flow from the site to mirror a 1:5 year rainfall event with C=0.5.

As noted previously, this building is serving dual function in that not only is it a commercial building but it provides access to grade for the LRT tunnel located below the building. This includes both LRT riders and mechanical systems. Figure 2 illustrates the proposed roof configuration for the building which includes features for the LRT's mechanical facilities. These features restrict the area available for roof top storage use. Alternative storage solutions include below grade storage, however, due to the presence of the LRT tunnel this is not possible, hence the stormwater management solution will need to work with the available roof area.

The proposed roof configuration provides for two roof drains on the north half and three on the south half. Utilizing Watts adjustable flow control roof drains (see attached spec sheet, or approved equal) at a discharge rate of 2.27 l/s (30gpm) for the north drains (2X2.27=4.54), and 1.51 l/s (20gpm) for the south drains (3X1.51=4.54), combining the roof drains, a total flow of 9.08l/s with a Tc of 10 min, for a 1:100 yr. rainfall event the effective runoff coefficient is C=0.76, which equates to a runoff coefficient of C=0.61 for a 1:5 yr. event.

The total area of the roof is 521 m^2 , of which 240 m^2 is available for use as stormwater roof storage, based on 0.15 m maximum depth ponding and the proposed roof configuration. The available roof top storage for the northern and southern halves of the roof is 2.1 m^3 and 7.42 m^3 respectively. The modified rational method notes for the roof drainage areas with C=1.0 (1:100), and discharge rate of 4.54l/s for each of the north and south section of roof, 1.9 m³ and 6.86 m³ of storage is required respectively, see attached calculation.

While the runoff coefficient does not meet the preferred C=0.5 (because of the limited storage due to the LRT requirements) it must be recognized the existing condition peak flow during a 1:100 event is 25.86 l/s and the proposed solution reduces that peak flow to 9.08 l/s, hence the post development condition is a significant improvement.



WATTS DRAINAGE	RD-100	Large Capacity Roof Drain
Components:	B2 B2-DM	Image: Space of the s
SPECIFICATION: Wats Drainage Provide served device with integral gravel stop and served dome strainer.	bolucts RD-100 epoxy coated cast iron ated flashing flange, flashing clamp elf-locking polyethylene (standard)	G G G G G G G G G G G G G G G G G G G
Underdeck Clamp (-BED and -D	options) are not available when -SO is sele	cted.
Job Nome	Contractor's P.O. N Contractor's P.O. N Representative	lo
WATTS Drainage reserves the right to modily or change produ- previously or subsequently sold. See your WATTS Drainage re- built Drainage 2004	ut design or construction without prior notice and without incom- presentative for any clarification. Dimensions are subject to mor wice Road, Burlington, ON, L7L 5H7 TEL: 905-332-6718	ng any abligation to make similar changes and modifications to products wfochring tolerooces. TOLL-FREE: 1-888-208-8927 Websile: www.waltsdrainage.co

(Dimension) Denotes Millimeters

WATTS®	Adjustable Accutrol Weir	Adjustable Flow Control		
DRAINAGE	Tag:	for Roof Drains		

ADJUSTABLE ACCUTROL(for Large Sump Roof Drains only)

For more flexibility in controlling flow with heads deeper than 2", Watts Drainage offers the Adjustable Accutrol. The Adjustable Accutrol Weir is designed with a single parabolic opening that can be covered to restrict flow above 2" of head to less than 5 gpm per inch, up to 6" of head. To adjust the flow rate for depths over 2" of head, set the slot in the adjustable upper cone according to the flow rate required. Refer to Table 1 below. Note: Flow rates are directly proportional to the amount of weir opening that is exposed.

EXAMPLE:

For example, if the adjustable upper cone is set to cover 1/2 of the weir opening, flow rates above 2" of head will be restricted to 2-1/2 gpm per inch of head.

Therefore, at 3" of head, the flow rate through the Accutrol Weir that has 1/2 the slot exposed will be: [5 gpm(per inch of head) x 2 inches of head] + 2-1/2 gpm(for the third inch of head) = 12-1/2 gpm.



TABLE 1. Adjustable Accutrol Flow Rate Settings

				Head of Wate	r		
W	Weir Opening Exposed	1"	2"	3''	4"	5"	6"
			Flow	Rate (gallons p	er minute)		
F		5	10	15	20	25	30
	3/4	5	10	13.75	17.5	21.25	25
	1/2	5	10	12.5	15	17.5	20
	1/4	5	10	11.25	12.5	13.75	15
-	Closed	5	10	10	10	10	10

ES-WD-RD-ACCUTROLADJ CANADA 0512

@ Wotts Droinage 2005

(Dimension) Denotes Millimeters



IBI 333 Preston St OTTAWA, ONTARIO K1S 5N4 ONSITE SWM 100yr design PROJECT: Scotia Bank Rideau/William CITY OF OTTAWA DEVELOPER : Scotiabank

PAGE: 1 OF 1 JOB #: 37543 DATE: Oct 14, 2016 DESIGN: DY Rev#0

100yr design

MAXIMUM ALLOWABLE FLOW - Flow Restricted to 5yr C= 0.61

Time of concentration = 10 minutes

Area (ha) =	0.051
post C Average =	0.61
post C Average =	0.90

10 min Tc i5yr = 998.071/(T+6.053)^0.814= 104.2 mm/hr

 Unrestricted Flowrate (Q5)

 10 min Tc
 Qpre-devo = 2.78*A*Cw*i =
 9.08 l/s

Intensity - 100 year event storm							
10 min Tc	i100yr = 1735.688/(T+6.014)^0.82=	178.6 mm/hr					

Unrestricted Flowrate (Q100)10 min TcQpost-devo = 2.78*A*Cw*i =

STORM WATER MANAGEMENT - Post-Development Controlled

(5 year post-development with 100yr inlets)

ROOF AREA (n	orth)		1				
153.	5 sm						
100 -YR FLOW							
Qp (l/s)							
Area(ha)=	0.0154						
Cw =	1.00	STORMWATER MAN	AGEMENT Qm =		4.54	l/s	
Tc		Qp	Qn	n Qp-Qm	Volume		-
variable	(2.78 x Area x c x l	44-) (1/-)	(0)		
(min)	(mm/nour)	(I/S)	(I/S) (I/S)	(m3)		
5	242.7	10.4	4.5	4 5.8	1.75		
6	226.0	9.6	4.5	4 5.1	1.84		
7	211.7	9.0	4.5	4 4.5	1.89		
8	199.2	8.5	4.5	4 4.0	1.90	<====	Required volume
9	188.3	8.0	4.5	4 3.5	1.89		for roof storage
10	178.6	7.6	4.5	4 3.1	1.85		-
11	169.9	7.3	4.5	4 2.7	1.79		
12	162.1	6.9	4.5	4 2.4	1.71		
13	155.1	6.6	4.5	4 2.1	1.62		
14	148.7	6.3	4.5	4 1.8	1.52		
15	142.9	6.1	4.5	4 1.6	1.40		
16	137.5	5.9	4.5	4 1.3	1.28		

25.42 l/s

ROOF AREA (s	south)							
305								
100 -YR FLOW								
Qp (l/s)								
Area(ha)=	0.0306							
Cw =	1.00	STORMWATER MAN	NAGEMENT Qm	=		4.54	l/s	
Tc		Qp		Qm	Qp-Qm	Volume		
Variable	i	2.78 x Area x c x i			-			
(min)	(mm/hour)	(l/s)		(l/s)	(l/s)	(m3)		
14	148.7	12.6		4.54	8.1	6.80		
15	142.9	12.1		4.54	7.6	6.84		
16	137.5	11.7		4.54	7.1	6.86		
17	132.6	11.3		4.54	6.7	6.86	<===	Required volume
18	128.1	10.9		4.54	6.3	6.84	1	for roof storage
19	123.9	10.5		4.54	6.0	6.82	1	
20	120.0	10.2		4.54	5.6	6.78		
21	116.3	9.9		4.54	5.3	6.72		
22	112.9	9.6		4.54	5.0	6.66		
23	109.7	9.3		4.54	4.8	6.59		
24	106.7	9.1		4.54	4.5	6.51		
25	103.8	8.8		4.54	4.3	6.42		

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3 CONCLUSIONS

Stormwater sewers required to support the development are adjacent and available to service this site. The stormwater management approach noted in this report demonstrates the 1:100 yr post development flows from the site will be significantly reduced from pre development levels, and will not negatively impact the existing municipal storm sewer system.

The detail design is subject to governmental approval prior to construction, including but not limited to the following:

Commence Work Order: City of Ottawa

Report prepared by:

Demetrius Yahnoulopoulos, P. Eng. Associate Director