

359 Kent Street

TIA Strategy Report

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Strategy Report

Parsons has been retained by 359 Kent Street Ltd. (c/o Taggart Realty Management) to prepare a Transportation Impact Assessment (TIA) in support of an Official Plan Amendment (OPA) and Zoning By-Law Amendment (ZBLA) applications for a residential development located at 359 Kent Street. This document follows the TIA process, as outlined in the City Transportation Impact Assessment (TIA) Guidelines (2017). The following report represents Step 4 – Strategy Report.

1. Screening Form

The screening form confirmed the need for a TIA Report based on the site meeting the trip generation, location and safety triggers. The trip generation trigger is met due to the number of person trips anticipated to be generated by the development exceeding 60 person trips per hour. The location trigger is met due to the development being located within a Design Priority Area (DPA). The safety trigger is met due to the proximity of the proposed site driveway within 150m of a signalized intersection. The Screening Form has been provided in Appendix A.

2. Scoping Report

2.1. Existing and Planned Conditions

2.1.1. Proposed Development

The proposed development borders both Kent St and Gilmour St, at the municipal address of 359 Kent St. The development is anticipated to consist of a 35-storey residential tower, housing 405 apartment units, 21,388 ft² office space and 7,833 ft² commercial space. A five-level underground parking lot is anticipated to be provided for vehicle and bicycle parking spaces, with an access provided along Gilmour St near the east end of the development site. The development may be constructed by 2031, however, for purposes of this report it is assumed that the site is constructed in a single phase by 2024. The site is currently zoned as R4UD[479]. Figure 1 illustrates the local site context and the direction of travel for one-way streets in the study area.



Figure 1: Local Context



Figure 2 illustrates the existing land uses and accesses of the site. The site is currently occupied mainly by a 6-storey office building and a surface parking lot. A section at the north end of the building is currently used as a parking and storage area by the two existing 2-storey buildings located north of the site. These parking spaces will be relocated to the underground parking lot of the proposed future development. Note that there are currently two left-in/left-out accesses into the existing parking lot, however, the future development will have a single left-in/left-out access to the underground parking lot near the east end of the site.

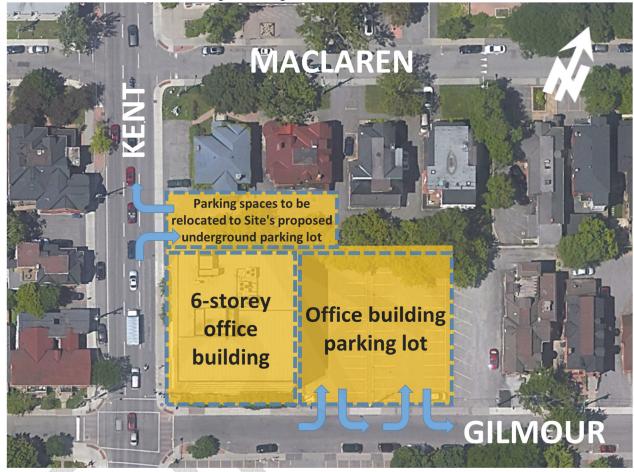
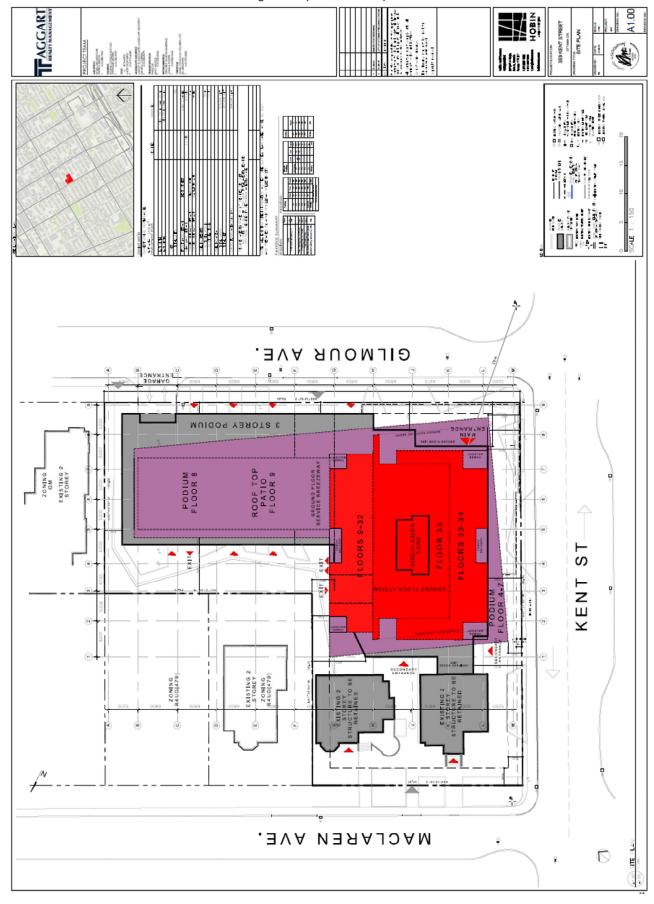


Figure 2: Existing Land Uses and Accesses on Site

The Site Concept of the proposed development is illustrated in Figure 3 below.



Figure 3: Proposed Site Concept





2.1.2. Existing Conditions

Area Road Network

Kent Street is a north-south municipal arterial road that extends from Chamberlain Ave in the south to Wellington St in the north. The roadway operates as a one-way northbound road with a three-lane cross-section and onstreet parking. The posted speed limit is 50km/h.

Bank St is a north-south municipal arterial road that extends from Wellington St in the north to past the City of Ottawa's limits in the south. Within the study area, the roadway consists of a two-way two-lane cross-section with a posted speed limit of 50km/h.

Gladstone Ave is an east-west municipal major collector road that extends from Parkdale Ave in the west to Cartier St in the east. Within the study area, the road consists of a two-way two-lane cross-section with on-street parking and a speed limit of 50km/h.

Somerset St W is an east-west municipal arterial road that extends from Queen Elizabeth Dr in the east to Garland St in the west, where it continues as Wellington St W. Within the study area, the roadway consists of a two-way two-lane cross-section, with on-street parking and an assumed speed limit of 50km/h.

Gilmour St is an east-west municipal local road that extends from Bronson Ave in the west to The Driveway in the east. The roadway operates as a one-way eastbound road between Bronson Ave and Elgin St. Within the study area, Gilmour St consists of one travel lane with on-street parking and an assumed speed limit of 50km/h.

MacLaren St is an east-west municipal local road that extends from The Driveway in the east to Bronson Ave in the west. The roadway operates as a one-way westbound road between Elgin St and Bronson Ave. Within the study area, MacLaren St consists of one travel lane with on-street parking and an assumed speed limit of 50km/h.

O'Connor Street is a north-south municipal road that extends from Wellington St in the north to Holmwood Ave in the south. Within the study area, O'Connor St is classified as an arterial road, which operates as a southbound only roadway with two-lane cross-section, on-street parking lane and a bi-directional cycle track on the east side. The speed limit is assumed to be 50km/h.

Existing Study Area Intersections

The following describes the existing physical geometry of the study area intersections.

Somerset/Kent

The Somerset/Kent intersection is a signalized four-legged intersection. Kent St operates as a one-way northbound road, with two through lanes, one shared through/right-turn lane and a left-turn lane on the south side and three receiving lanes on the north side. Somerset St provides one shared movement lane and one receiving lane on both sides, with an auxiliary left-turn lane. Westbound right-turn on red is prohibited. Painted crosswalks have been provided on all legs of this intersection. Cyclists operate in mixed traffic conditions.





MacLaren/Kent

The MacLaren/Kent intersection is an unsignalized four-legged intersection between two one-way streets, where Kent St operates northbound only and Maclaren St as westbound only. The northbound approach is uncontrolled, provides two through travel lanes and a shared through/left-turn lane. Westbound approach is composed of a single shared through/right-turn lane. Painted crosswalks have been provided crossing MacLaren St. Cyclists operate in mixed traffic conditions.

Gilmour/Kent

The Gilmour/Kent intersection is a signalized four-legged intersection connecting two one-way streets, where Kent St operates northbound only and Gilmour St as eastbound only. The northbound approach has two through lanes and a shared through/left-turn lane. The eastbound approach consists of a single shared through/right-turn lane. Painted crosswalks have been provided on all legs of this intersection. Cyclists operate in mixed traffic conditions.

Gladstone/Kent

The Gladstone/Kent intersection is a signalized four-legged intersection. Kent St operates in the northbound direction only. At the northbound approach, the roadway consists of two through lanes, one shared through/right-turn lane and a left-turn lane. The eastbound approach is composed of a through lane and an auxiliary left-turn lane. The westbound approach is comprised of a single shared through/right-turn lane. Painted crosswalks have been provided on all legs of this intersection. Cyclists operate in mixed traffic conditions.

MacLaren/Bank

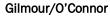
The MacLaren/Bank intersection is a signalized four-legged intersection. MacLaren St is a one-way road operating westbound only. The northbound approach consists of a single shared through/left-turn lane. The Southbound approach is composed of a shared through/right-turn lane and the westbound approach is comprised of a single shared through/right-turn/left-turn lane. Painted crosswalks have been provided on all legs of this intersection. Cyclists operate in mixed traffic conditions





Gilmour/Bank

The Gilmour/Bank intersection is a signalized four-legged intersection. Gilmour St is an eastbound only. The northbound approach consists of a single shared through/left-turn lane. The southbound approach is comprised of a single shared through/right-turn lane. The eastbound approach consists of a single shared through/right-turn/left-turn lane. Painted crosswalks have been provided on all legs of this intersection. Cyclists operate in mixed traffic conditions.



The Gilmour/O'Connor intersection is a signalized four-legged intersection. O'Connor St is a southbound only road and Gilmour St is an eastbound only road. The southbound approach consists of a shared through/left-turn lane and a through lane. The eastbound approach consists of a single shared through/right-turn lane. Painted crosswalks have been provided on all legs of this intersection. A cycle track is provided on the east side of O'Connor St, with bike signals and crossing on the east leg of the intersection.



Existing Driveways to Adjacent Developments

As mentioned previously, the proposed site access is located along Gilmour St, near the east end of the site. Figure 4 illustrates the location of adjacent driveways along Gilmour St, within 200 m of the proposed site access.



Figure 4: Adjacent Driveways to Proposed Site Access

West of Kent St:

- On the north side, there are nine driveway accesses to residential units.
- On the south side, there are five driveway accesses to residential units.

Between Kent St and Bank St:

• On the north side, there are a total of four driveways which provide access to small businesses, residential units and to an underground parking lot.



 On the south side, there are a total of five driveways which provide access to office/commercial parking, small businesses, residential units and a driveway connection between Gilmour St and James St where surface parking spaces and underground parking is provided for residential buildings.

East of Bank St:

- On the north side, there is one access for employee parking spaces at a commercial building.
- On the south side, there are three driveways providing access to commercial and office parking spaces and an access to a surface parking lot.

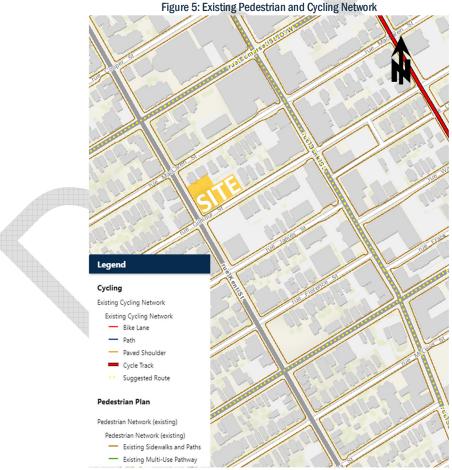
Existing Area Traffic Management Measures

Existing area traffic management measures within the study area include:

- · Speed humps;
- On-street parking;
- Textured crosswalks at study area intersections;
- Curb extensions at some locations; and,
- One-way traffic operations along study area roadways.

Pedestrian/Cycling Network

Figure 5 illustrates active transportation facilities within the study area. Sidewalks are provided on both sides of roadways throughout the study area. A cycle track is provided along O'Connor St, while Somerset St W, Bank St and Gladstone Ave are all suggested cycling routes. Gladstone Ave, O'Connor St and Somerset St W are classified as Spine Routes in the City of Ottawa Transportation Master Plan (TMP).



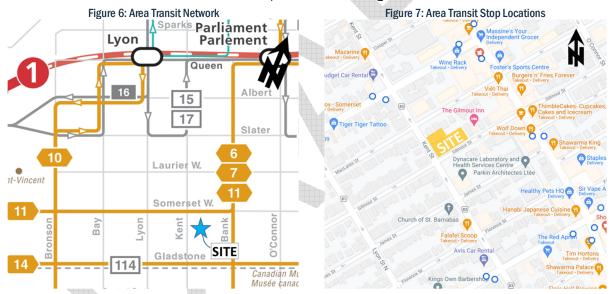
Transit Network

The existing transit network surrounding the proposed development site is illustrated in Figure 6. It is noted that no bus routes currently operate within the study area, along Kent St, MacLaren St, O'Connor St and Gilmour St.



Transit stop locations are shown highlighted blue in Figure 7. Brief descriptions of the nearby transit routes are provided below:

- O-Train Confederation Line: an east-west Light-Rail Transit (LRT) that runs from Blair Station in the east to Tunney's Pasture in the west, providing service to 13 stations. During peak hours, service is provided every 5 minutes or less and every 15 minutes or less at all other times. The O-Train can be accessed approximately 1.0 km north of the proposed development at Lyon Station.
- Bus route #6 (Greenboro <-> Rockcliffe): designated as a "frequent route" and operates 7 days a week, providing service every 15 minutes or less. The nearest bus stop to the site is along Kent St south of Gilmour St.
- Bus route #7 (Carleton <-> St. Laurent): designated as a "frequent route" and operates 7 days a week, providing service every 15 minutes or less. The nearest bus stop to the site is along Kent St south of Gilmour St.
- Bus route #11 (Bayshore <-> Parliament): designated as a "frequent route" and operates 7 days a week, providing service every 15 minutes or less. The nearest bus stop to the site is along Somerset St W at the Kent St intersection.
- Bus route #14 (Tunney's Pasture <-> St. Laurent): designated as a "frequent route" and operates 7 days a week, providing service every 15 minutes or less. The nearest bus stop to the site is along Gladstone Ave at the Kent St intersection.
- Bus route #114 (Carlington <-> Rideau): designated as a "local route" and operate Monday to Friday on a custom schedule. The nearest bus stop to the site is along Gladstone Ave at the Kent St intersection.



Peak Hour Travel Demands

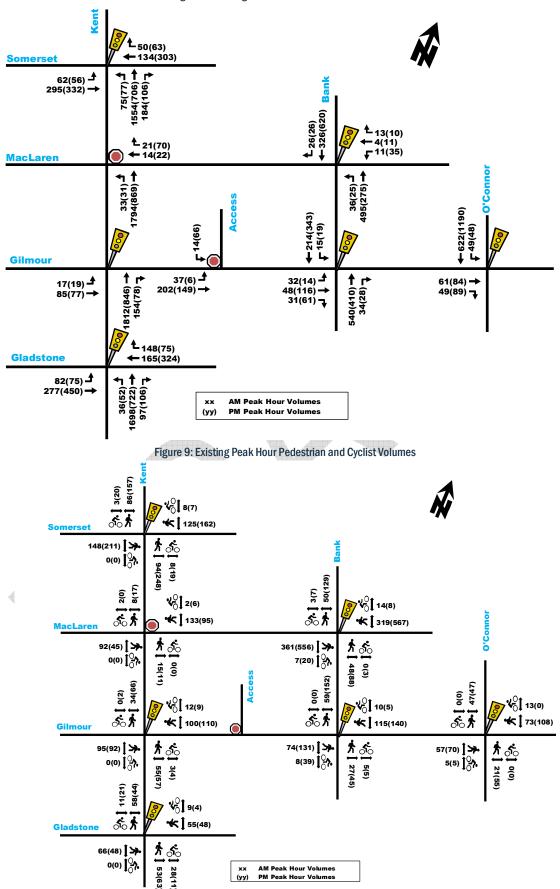
The existing peak hour traffic volumes within the study area were obtained from the City of Ottawa for the following intersections:

- Somerset/Kent conducted Wednesday, April 05, 2017.
- MacLaren/Kent conducted Tuesday, March 05, 2019.
- Gilmour/Kent conducted Wednesday, April 05, 2017.
- Gladstone/Kent conducted Tuesday, April 25, 2017.
- MacLaren/Bank conducted Tuesday, April 16, 2019.
- Gilmour/Bank conducted Tuesday, August 25, 2015
- Gilmour/O'Connor conducted Tuesday, March 21, 2017.

Figure 8 displays the existing vehicle traffic volumes while Figure 9 shows the existing pedestrian and cyclist volumes. Peak hour count data is provided in Appendix B.



Figure 8: Existing Peak Hour Vehicle Traffic Volumes





Existing Road Safety Conditions

A five-year collision history data (2015-2019, inclusive) was requested and obtained from the City of Ottawa for all intersections and road segments within the study area. Upon analyzing the collision data, the total number of collisions occurring between two or more vehicles within the study area was determined to be 112 collisions within the past five-years. Of the reported collisions, 90 (80%) resulted in property damage only, 21 (19%) resulted in non-fatal injury and 1 (1%) was classified as "non-reportable". The types of impact were broken down into the following: 24 (21%) sideswipe, 20 (18%) angled, 19 (17%) rear end, 19 (17%) Single Unattended Vehicle, 17 (15%) turning movement, 9 (8%) Single Vehicle, 3 (3%) "other" and 1 (1%) approaching.

To help quantify the relative safety risk at intersections within the study area, an industry standard unit of measure for assessing collisions at an intersection was used based on the number of collisions per million entering vehicles (MEV). An MEV value greater than 1.00 indicates a relatively high frequency of collisions. Furthermore, the City of Ottawa TIA Guidelines identifies more than six collisions of the same nature occurring within a five-year period to be a collision pattern. Reported collisions at study area intersections have historically taken place at a rate of:

- <u>Bank/Gilmour:</u> 0.58 collisions/MEV, with a total of 11 collisions occurring within the five-year period. No particular collision pattern is present.
- Bank/MacLaren: 0.05 collisions/MEV, with only 1 collision occurring within the five-year period. No
 particular collision pattern is present.
- <u>Gilmour/Kent:</u> 0.17 collisions/MEV, with 5 collisions occurring within the five-year period. No particular collision pattern is present.
- <u>Kent/MacLaren:</u> 0.38 collisions/MEV, with a total of 11 collisions occurring within the five-year period. No particular collision pattern is present.
- <u>Kent/Gladstone</u>: 0.69 collisions/MEV, with a total of 29 collisions occurring within the five-year period. Although there are 7 rear end collisions at this intersection, the collisions occurred between different movements and travel directions. On the other hand, 14 angled collisions have occurred at this intersection, where 7 of the collisions were the result of northbound and eastbound vehicles "going ahead". As such, this may indicate a collision pattern is present with regards to angled collisions between northbound and southbound vehicles. However, it is noted that the collisions/MEV at this intersection is well below 1.00.
- Kent/Somerset: 0.59 collisions/MEV, with a total of 23 collisions occurring within the five-year period.
 Although there are 7 turning movement collisions at this intersection, the collisions occurred between different movements and travel directions. As such, there are no particular collision patterns identified.

With regards to road segments within the study area, the following collision data is provided:

- Bank St, between MacLaren St and Gilmour St: a total of 4 collisions occurred along this road segment within the five-year period.
- Kent St, between Gladstone Ave and Somerset St: in total, 17 collisions occurred in the five-year period. As this is a relatively long road segment, with 4 intersections between the two roads, there are no concerns with regards to the number of collisions.
- <u>MacLaren St, between Bank St and Kent St</u>: a total of 10 collisions have occurred in the five-year period.
 Of the total, 8 collisions involved single unattended vehicles, where 5 collisions recorded "unknown" circumstances. The single unattended vehicle collisions likely occurred between a moving vehicle on MacLaren St and an on-street parked vehicle.
- <u>Gilmour St. between Kent St and Bank St:</u> no recorded collisions data was available for the five-year period, indicating no collisions occurred.

A total of 8 pedestrian collisions have occurred in the study area out of the total 112 collisions. 6 of the pedestrian collisions occurred at the intersection of Kent/Somerset, while 1 occurred at Gladstone/Kent and 1 occurred at Gilmour/Kent. Additionally, a total of 2 bicycle collisions occurred within the study area, with 1 occurring at Kent/Somerset and 1 at Bank/Gilmour. All pedestrian and bicycle collisions resulted in non-fatal injuries.



The source collision data as provided by the City of Ottawa and related analysis are provided as Appendix C.

2.1.3. Planned Conditions

Planned Study Area Transportation Network Changes

LRT Stage 2

Stage 2 of the City of Ottawa LRT system is currently under construction. Stage 2, as shown in Figure 10, is a combination of three extensions – south, east and west – totaling 44 km of new rail and 24 new LRT stations. As mentioned previously, the proposed development site is within 600m of the LRT's Parliament Station



Figure 10: Stage 2 LRT System Map

Centretown Community Design Plan (CDP)

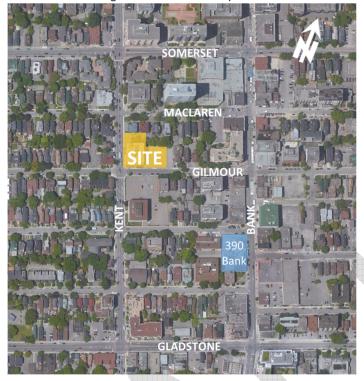
The purpose of the CDP is to create a comprehensive design plan to guide and manage future growth in the Centretown area of Ottawa. The study area is generally square in shape and is bounded by Elgin St to the east, Kent St to the west, Highway 417 to the south and Gloucester St to the north. The CDP was completed in 2013 and discussed the potential conversion of Kent St to a two-way road.

Other Area Developments

The following section outlines adjacent developments in the general area that were considered in the TIA. The criteria for inclusion of other area developments are the proximity to the proposed development site and the potential impact to study area intersections. Developments that are either approved or have an active planning application in the City are included below, with the location illustrated in Figure 11.



Figure 11: Other Area Developments



390 Bank Street

A TIA was prepared by CGH Transportation in June 2021, for a residential development consisting of 127 apartment units and 6,828 ft² of retail space. The buildout of the development was anticipated to be constructed by 2024. The development was anticipated to generate 16 and 19 veh/h during the morning and afternoon peak hours, respectively.

2.2. Study Area and Time Periods

The development may be constructed by 2031, however, for purposes of this report it is assumed that full buildout of the proposed residential development will be 2024. As such, the horizon years being analyzed in this report are 2024 and 2029 (five years after full buildout) horizon years, using the weekday morning and afternoon peak hour time periods.

Proposed study area intersections and boundary roads are outlined below and highlighted in Figure 12.

- Somerset/Kent (signalized)
- MacLaren/Kent (unsignalized)
- Gilmour/Kent (signalized)
- Gladstone/Kent (signalized)
- Gilmour/O'Connor (signalized)

- Gilmour/Bank (signalized)
- MacLaren/Bank (signalized)
- Kent St, at site frontage
- Gilmour St, at site frontage



Figure 12: Study Area Intersections



2.3. Exemption Review

The following modules/elements of the TIA process recommended to be exempt in the subsequent steps of the TIA process, based on the City's TIA guidelines and the subject site:

Table 1: Exemptions Review Summary

Module	Element	Exemption Consideration
4.1 – 4.4 Design Review Component	All elements	Not required for applications involving ZBLA or OPA. However, a brief description may be provided.
4.8 Network Concept	4.8 Network Concept	Only required if proposed development is anticipated to generate more than 200 person-trips over the permitted zoning

3. Forecasting Report

3.1. Development Generated Travel Demand

3.1.1. Trip Generation and mode shares

Existing Development Trips

As mentioned previously, the site is currently occupied by an office building that is 6-storeys high. The trips that are currently generated by the office building are accounted for as they reduce the number of 'new' trips that will be generated by the proposed development within the study area. Appropriate trip rates for an office building have been obtained from the ITE Trip Generation Manual (10th edition) and provided as shown in Table 2.

Table 2: Existing Office Building Trip Rates

Land Use	Data	Trip Rates				
Land Use	Source	AM Peak Hour	PM Peak Hour			
Office Building (7-Storey)	ITE 710	T = 1.16(x); T = 0.94(x) + 26.49;	T = 1.15(x); Ln(T) = 0.95Ln(x) + 0.36;			
Notes: T = Average Vehicle Trip Ends x = Gross Floor Area (GFA) (1						



The gross floor area used for the office building was determined using the GeoOttawa measuring tool, as shown in Figure 13, which indicates a total area of approximately 8,668 ft² per floor (i.e. a total area of approximately 52,000 ft² for 6-storeys).



Figure 13: Existing Office Building Area Measurement

Using the total gross floor area and the office building trip rates, the person trips generated by the existing office building can be calculated. Note that the trip rates are multiplied by a factor of 1.28, as per TIA standards, to account for typical North American auto occupancy values of approximately 1.15 and combined transit and non-motorized modal shares of less than 10%. The resulting total person trips/hour for the existing office building are provided in Table 3. The inbound and outbound percentages were also obtained from the ITE Manual.

Table 3: Existing Office Building Peak Hour Person Trip Generation

A	Land Usa	Area (ft²) AM Peak (Person Trips/				s/h) PM Peak (Person Trips/h)			
	Land Use	Area (112)	In (86%)	Out (14%)	Total	In (16%)	Out (84%)	Total	
	Office Building (6-Storey)	52,000	82	14	96	12	66	78	

As shown in Table 3, the existing office building generates a total of 96 and 78 person trips during the morning and afternoon peak hours. Mode shares for different travel modes were obtained from the 2020 TRANS Trip Generation Manual for Employment Generators in the Ottawa Inner Area district. As such, a breakdown of the trips generated by the different travel modes is provided in Table 4 below.

Table 4: Existing Office Building Morning and Afternoon Travel Mode Breakdown

Travel Mode	Mode	AM Peak (Person Trips/h)			Mode	PM Pe	ak (Person Ti	rips/h)
Travel Mode	Shares	In (86%)	Out (14%)	Total	Shares	In (16%)	Out (84%)	Total
Auto Driver	45%	37	7	44	45%	6	30	36
Passenger	7%	6	1	7	7%	1	5	6
Transit	29%	24	4	28	29%	3	19	22
Bike	8%	6	1	7	8%	1	5	6
Walk	11%	9	1	10	11%	1	7	8
Total Person Trips	100%	82	14	96	100%	12	66	78



The existing office building generates a total of 44 and 36 vehicle trips during the morning and afternoon peak hours, respectively.

Proposed Development Trips

The proposed development will consist of 405 residential units, $21,388 \text{ ft}^2$ office space and $7,833 \text{ ft}^2$ commercial space within a 35-storey high-rise apartment building. The sections below determine trips generated by each land use.

Residential Building Trip Generation

The appropriate trip generation rates for a high-rise apartment land use were obtained from the 2020 TRANS Trip Generation Manual. Table 3 in the Manual provides person-trip rates during the peak AM and PM periods (7am-9:30am and 3:30PM-6PM). The trip rates are summarized in Table 5 below.

Table 5: High-Rise Apartments Trip Rates

Land Use	Data	Trip I	Rates
Land USE	Source	AM Peak Period (7-9:30am)	PM Peak Period (3:30-6pm)
High-Rise Apartments (35 floors)	TRANS 2020	T = 0.8(du);	T = 0.9(du);
Notes: T = Average Vehicle Trip Ends	;		
du = Dwelling unit			

Using the trip rates provided in Table 5, the total number of person trips generated during the morning and afternoon peak periods can be found in Table 6.

Table 6: Apartment Building Peak Period Person Trip Generation

Land Use	Dwelling	AM Peak Period	PM Peak Period
	Units	Person Trips	Person Trips
High-Rise Apartments (35 floors)	405	329	370

The proposed development is anticipated to generate 329 and 370 person trips during the morning and afternoon peak periods, respectively. The total peak period person trips in Table 6 are then divided into different travel modes, as shown in Table 7, using mode share percentages obtained from the 2020 TRANS Manual for the "Ottawa Inner Area" district.

Table 7: Apartment Building Peak Period Trips Mode Shares Breakdown

Travel Mode	Mode Share	AM Peak Period Person Trip	Mode Share	PM Peak Period Person Trips		
Auto Driver	27%	89	26%	96		
Auto Passenger	6%	20	8%	30		
Transit	28%	92	21%	78		
Cycling	5%	16	6%	22		
Walking	34%	112	39%	144		
Total Person Trips	100%	329	100%	370		

Standard traffic analysis is usually conducted using the morning and afternoon peak hour trips as they represent a worst-case scenario. In the 2020 TRANS Manual, Table 4 provides conversions rates from peak period to peak hours for different mode shares. The conversion rates are provided in Table 8 below.

Table 8: Peak Period to Peak Hour Conversion Factors (2020 TRANS Manual)

Travel Mode	Peak Period to Peak Hour Conversion Factors			
Travel Mode	AM	PM		
Auto Driver	0.48	0.44		
Passenger	0.31	0.29		
Transit	0.55	0.47		
Bike	0.58	0.48		
Walk	0.58	0.52		

Note that conversion factors for auto passenger trips are not available in the 2020 TRANS Manual. To obtain the passenger trip factor it was assumed that the total person trip peak hour conversion factor is the average of the



provided adjustment factors minus the passenger trip peak hour conversion factor and has been calculated as shown in the example below:

$$0.5 = \frac{x + 0.48 + 0.55 + 0.58 + 0.58}{5}$$

$$x = 2.5 - 0.48 - 0.55 - 0.58 - 0.58$$

$$x = 0.31 \rightarrow AM \ passenger \ trip \ peak \ hour \ conversion \ factor$$

Using the conversion rates in Table 8 and the peak period person trips for different travel modes in Table 7, the peak hour trips for different travel modes can be calculated as shown in Table 9. The peak hour mode share percentages were calculated using the percentage of each travel mode to the total person trips.

10010 0.71	rable 5.7 parametre banding reak from 111ps, with 7 octain would office reaching 65							
Travel Mode	Peak Hour Mode Share Percentages	AM Peak Hour Trips	Peak Hour Mode Share Percentages	PM Peak Hour Trips				
Auto Driver	24%	43	25%	42				
Auto Passenger	4%	6	5%	9				
Transit	29%	51	21%	37				
Cycling	6%	9	6%	11				
Walking	37%	65	43%	75				
Total Person Trips	100%	174	100%	174				

Table 9: Apartment Building Peak Hour Trips, with Actual Mode Share Percentages

As shown in Table 9, the proposed development is anticipated to generate a total of approximately 174 person trips during both the morning and afternoon peak hours. Vehicle trips are anticipated to be 43 veh/h during both the morning and afternoon peak hours. Active transportation mode shares (bike and walk) generate the highest number of trips for the proposed development (74 to 86 trips during peak hours), which is expected given the location of the development in a core sector of the City of Ottawa. As shown in Table 10, these trips are divided into inbound and outbound trips using percentages obtained from Table 9 of the 2020 TRANS Manual.

	Table 10. Apartification and Traver Mode Trips											
Travel Made	AM P	eak (Person T	rips/h)	PM Peak (Person Trips/h)								
Travel Mode	In (31%)	Out (69%)	Total	In (58%)	Out (42%)	Total						
Auto Driver	13	30	43	24	18	42						
Passenger	2	4	6	5	4	9						
Transit	16	35	51	21	16	37						
Bike	3	6	9	6	5	11						
Walk	20	45	65	44	32	75						
Total Person Trips	54	120	174	100	75	174						

Table 10: Apartment Building Peak Hour Travel Mode Trips

Office and Commercial Space Trip Generation

The appropriate trip generation rates for the office and commercial land uses were obtained from the ITE Trip Generation Manual and summarized in Table 11 below.

Table 11: Office and Commercial Spaces Trip Rates

	Land Use	Data	Trip Rates				
	Land Use	Source	AM Peak Hour	PM Peak Hour			
	Office Space	ITE 710	T = 1.16(x); T = 0.94(x) + 26.49;	T = 1.15(x); Ln(T) = 0.95Ln(x) + 0.36;			
	Commercial Space	ITE 820	T = 0.94(x);	T = 3.81(x);			
Notes:	T = Average Vehicle Trip Ends x = Gross Floor Area (GFA) (1)						

Using the total gross floor areas of the office and commercial spaces and their respective trip rates, the person trips generated are calculated as shown in Table 12. Note that the trip rates are multiplied by a factor of 1.28, as per TIA standards, to account for typical North American auto occupancy values of approximately 1.15 and combined transit and non-motorized modal shares of less than 10%. The inbound and outbound percentages were also obtained from the ITE Manual.



Table 12: Office and Commercial Peak Hour Person Trip Generation

Land Use	Area (ft²)	AM Pe	ak (Perso	n Trips/h)	PM Peak (Person Trips/h)		
Land USE	Area (IL-)	In	Out	Total	In	Out	Total
Office Space	21,338	51	9	60	5	29	34
Commercial Space	7,833	5	4	9	18	20	38
	Total	56	13	69	23	49	72

As shown in Table 12, the office space is anticipated to generate a total of 60 and 34 person trips, while the commercial space is anticipated to generate a total of 9 and 38 person trips during the morning and afternoon peak hours. Mode shares for different travel modes were obtained from the 2020 TRANS Trip Generation Manual for Employment Generators and Commercial Generators in the Ottawa Inner Area district. As such, a breakdown of the trips generated by different travel modes for the office space and the commercial space is provided in Table 13 and Table 14 below.

Table 13: Office Space Morning and Afternoon Travel Mode Breakdown

Travel Mode	Mode	AM Pe	eak (Person T	rips/h)	Mode	PM Peak (Person Trips/h)		
Travel Mode	Shares	In (86%)	Out (14%)	Total	Shares	In (16%)	Out (84%)	Total
Auto Driver	45%	23	5	28	45%	3	14	17
Passenger	7%	4	1	5	7%	1	2	3
Transit	29%	15	2	17	29%	1	8	9
Bike	8%	4	0	4	8%	0	2	2
Walk	11%	5	1	6	11%	0	3	3
Total Person Trips	100%	51	9	60	100%	5	29	34

Table 14: Commercial Space Morning and Afternoon Travel Mode Breakdown

			<u> </u>		Velocities (CA)			
Travel Mode	Mode	AM Pe	eak (Person 1	rips/h)	Mode	PM Peak (Person Trips/h)		
Travel Wode	Shares	In (62%)	Out (38%)	Total	Shares	In (48%)	Out (52%)	Total
Auto Driver	39%	2	2	4	22%	4	5	9
Passenger	2%	1	1	2	4%	1	1	2
Transit	16%	0	0	0	12%	2	2	4
Bike	3%	0	0	0	4%	1	1	2
Walk	40%	2	1	3	58%	10	11	21
Total Person Trips	100%	5	4	9	100%	18	20	38

The office space is anticipated to generate up to 28 vehicle trips during peak hours, while the commercial space is anticipated to generate up to 9 vehicle trips during peak hours.

Total Trips Generated

The total trips anticipated to be generated by the proposed development are calculated by summing the trips generated by the residential (Table 10), office (Table 13) and commercial (Table 14) land uses. The total trips generated are provided in Table 15 below.

Table 15: Total Trips Generated by Proposed Development

Travel Mode	AM Pe	eak (Person 1	rips/h)	PM Peak (Person Trips/h)			
Traver Mode	In	Out	Total	In	Out	Total	
Auto Driver	38	37	75	31	37	68	
Passenger	7	6	13	7	7	14	
Transit	31	37	68	24	26	50	
Bike	7	6	13	7	8	15	
Walk	27	47	74	54	46	99	
Total Person Trips	110	133	243	123	124	246	

The proposed development is anticipated to generate approximately 245 total person trips during peak hours. Up to 75 vehicle trips, 68 transit trips and 114 active transportation trips are expected during the peak hours.

Net Total Trips Generated

The 'new' number of trips that are anticipated to be generated by the proposed development are provided in Table 16, which reflect the difference between the total trips anticipated to be generated by the proposed development in Table 15 and the existing office building trips in Table 4. The resulting net new trips are summarized in Table 16.



Table 16: 'New' Trips Generated by the Proposed Development

Travel Mode	AM Pe	eak (Person T	rips/h)	PM Peak (Person Trips/h)			
Travel Mode	In	Out	Total	In	Out	Total	
Auto Driver	1	30	31	25	7	32	
Passenger	1	5	6	6	2	8	
Transit	7	33	40	21	7	28	
Bike	1	5	6	6	3	9	
Walk	18	46	64	53	39	91	
Total Person Trips	28	119	147	111	58	168	

The total number of 'new' person trips anticipated to be generated by the proposed development are 147 and 168 person trips during the morning and afternoon peak hours, respectively. The proposed development is anticipated to generate 31 to 32 'new' vehicle trips, 28 to 40 'new' transit trips and 70 to 100 new active transportation trips during the peak hours.

3.1.2. Trip Distribution and Assignment

Based on the 2011 OD Survey (Ottawa Inner Area district) and the location of adjacent arterial roadways and neighbourhoods, the distribution of site-generated traffic volumes was estimated as follows:

- 25% to/from the north;
- 25% to/from the south;
- 20% to/from the east; and,
- 30% to/from the west.

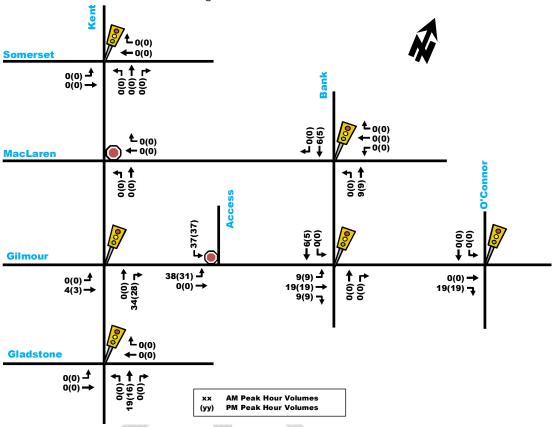
The anticipated site-generated auto trips for the proposed development from Table 15 were then assigned to the road networks as shown in Figure 14. Since the proposed development access located along Gilmour St permits left-in/left-out movements only and the road network is composed of one-way travel, the following trip distribution assumptions were made:

- Traffic to/from the north:
 - Arriving traffic:
 - If travelling southbound on a north-south road east of the site, take a circuitous route by travelling southbound on Bank St, turn right and head westbound on James St, then turn right on Kent St, where traffic can head northbound to turn right onto Gilmour St and finally turn left into the site's driveway.
 - If travelling southbound on a north-south road west of the site, approach from the west of the Kent St/Gilmour St intersection and turn left into the site's driveway.
 - Departing traffic will travel east on Gilmour St away from the site driveway and turn left onto Bank St to head north.
- Traffic to/from the south:
 - Arriving traffic may use Bank St to travel northbound turn left and head westbound on James St, then turn right on Kent St, where traffic can head northbound to turn right onto Gilmour St and finally turn left into the site's driveway.
 - Departing traffic will travel east on Gilmour St away from the site driveway and turn right onto Bank St to head south.
- Traffic to/from the east:
 - Arriving traffic is assumed to use Hwy 417 WB primarily and take the Metcalfe St exit, travelling westbound on Catherine St, then northbound on Kent St to turn right onto Gilmour St and finally turn left into the site's driveway.
 - Departing traffic is assumed to use Hwy 417 EB primarily by travelling east on Gilmour St away from the site driveway and turning right on O'Connor St to travel southbound to Isabella St and travel eastbound to access the highway.
- Traffic to/from the west:
 - Arriving traffic is assumed to use Hwy 417 EB primarily and take the Kent St exit to travel northbound, then turn right onto Gilmour St and finally turn left into the site's driveway.



 Departing traffic is assumed to use Hwy 417 WB primarily by travelling east on Gilmour St away from the site driveway and turning right on O'Connor St to travel southbound to Catherine St, then turning right onto the highway ramp.

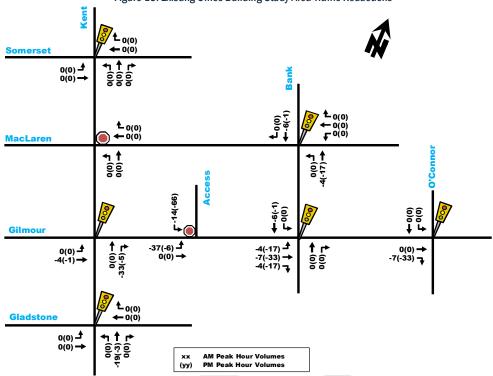
Figure 14: 2024 Site-Generated Traffic



Based on the site-generated vehicle trips of the existing office building (provided in Table 4), study area traffic volumes are expected to decrease as shown in Figure 15. A similar trip distribution and assignment has been assumed for the existing office building's vehicle trips as the proposed residential development. This reduction in traffic volumes will be applied to the total projected traffic volumes for horizon years 2024 and 2029.



Figure 15: Existing Office Building Study Area Traffic Reductions



3.2. Background Network Traffic

3.2.1. Transportation network plans

Refer to Section 2.1.3: Planned Study Area Transportation Network Changes.

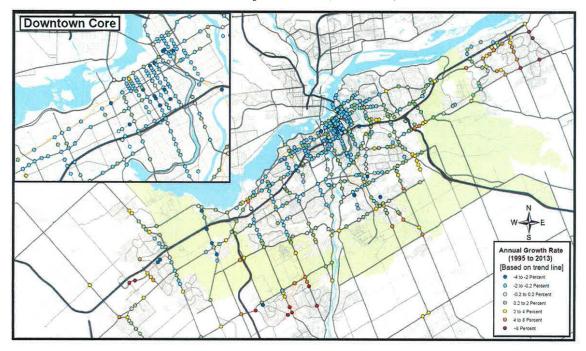
3.2.2. Background Growth

Historically, traffic within the study area has seen a decline in growth, as illustrated by the growth rates map obtained from the City of Ottawa in Figure 16.

Figure 16: Vehicle Growth Rates at Intersections within the City of Ottawa

INTERSECTION TRAFFIC GROWTH RATES, AM PEAK PERIOD (0700 to 0900)

Total Vehicular Volume Entering the Intersection, 1995 to 2013, Scenario F AM 2





Given that the proposed development will be located in the well-developed core downtown area of the city of Ottawa, where there is a high level of transit and active transportation connectivity, traffic along study area roadways is not anticipated to increase drastically within the future horizon years. Additionally, although the development is outside of the 600 m TOD distance of the LRT stations, the LRT can be accessed approximately 1.0 km north of the development site via frequent bus routes on Bank St.

Nonetheless, a conservative background growth rate of 1% has been applied to through movements of major study area roadways (Bank St, Kent St, Gladstone Ave, Somerset St W and O'Connor St) to account for trips that may be generated by future other area developments that are minor or located outside the scope of the study area. Figure 17 provides the future background traffic at horizon year 2024 and Figure 18 provides the future background traffic at horizon year 2029.

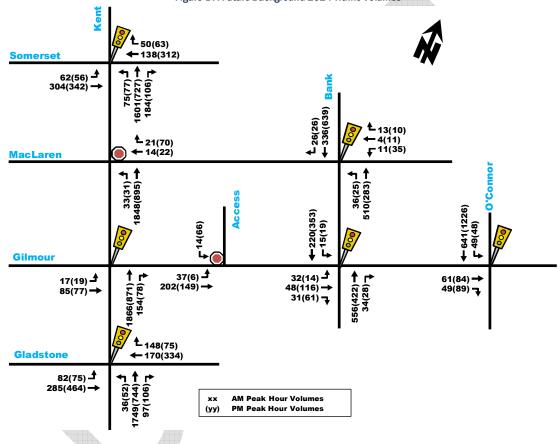
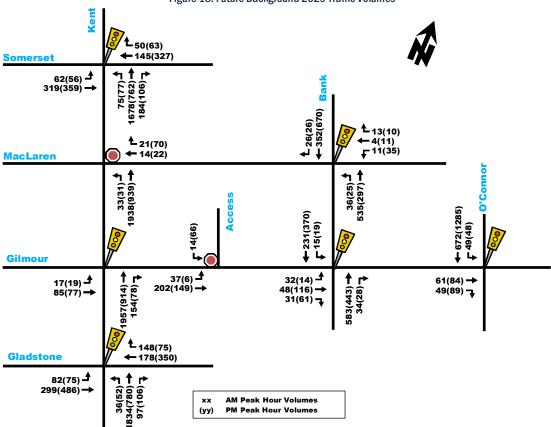


Figure 17: Future Background 2024 Traffic Volumes



Figure 18: Future Background 2029 Traffic Volumes



3.2.3. Other Developments

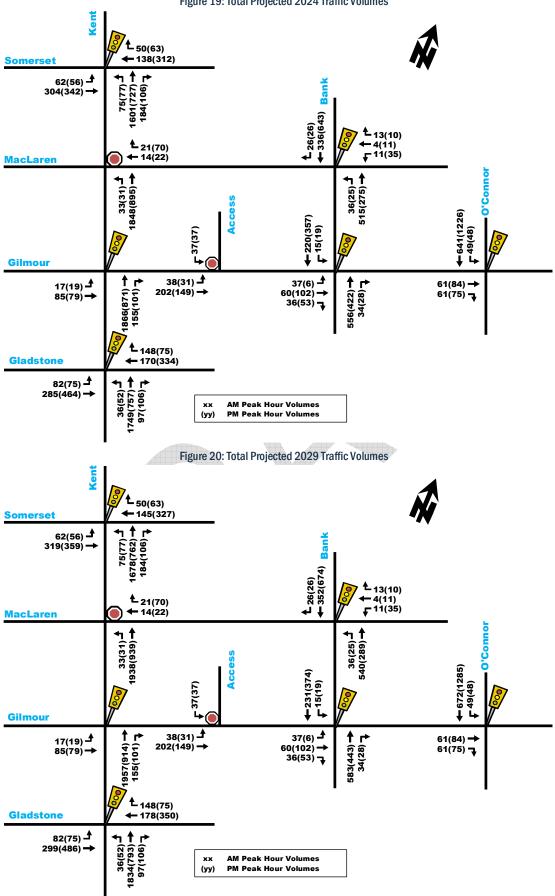
Description of other area developments taking place within the study area was provided in Section 2.1.3: Other Area Developments. Only one future development, located at 390 Bank St, was identified. Traffic volumes anticipated to be generated by this development at study area intersections are very minimal (5 vehicles or less). Therefore, these volumes have not been included and are assumed to be a part of the anticipated background growth percentage.

3.3. Demand Rationalization

The total projected future traffic volumes can be determined by superimposing the site-generated traffic volumes in Figure 14, onto the future background traffic volumes in Figure 17 and Figure 18, and providing a reduction due to the existing office building trips in Figure 15. The resulting total projected traffic volumes 2024 and 2029 illustrated in Figure 19 and Figure 20.



Figure 19: Total Projected 2024 Traffic Volumes





4. Analysis

4.1. Development Design

As this is a ZBLA, design related elements will be provided in more detail in the future Site Plan Application (SPA) submission of the proposed development. Vehicle and bicycle parking spaces are proposed to be provided in a five-level underground parking garage. Pedestrian, cyclist and transit amenities are expected to be maintained in the future. A loading bay and garbage pickup area will be provided at the east end of the site, immediately east of the underground parking garage entrance on Gilmour St. The City of Ottawa's TDM-supportive Development Design and Infrastructure checklist has been provided in Appendix D and discussed in more detail in Section 4.5.

4.2. Parking

Based on City of Ottawa Parking Provisions, Schedule 1A, the proposed development is located in "Area X", where no off-street resident or visitor parking is required for the first twelve units. For residential land uses in Area X, residents parking is required at a rate of 0.5 spaces per dwelling units, which equates to 200 parking spaces. Visitor parking is required at a rate of 0.1 spaces per unit and up to a maximum of 30 spaces, which means that 30 parking spaces must be provided for the 405 units. Bicycle parking is required at a rate of 0.5 per dwelling unit, which equates to 203 bicycle parking spaces.

The total number of parking spaces that will be provided by the proposed development is 332 vehicle parking spaces (289 resident and 43 visitor) and 191 bicycle parking spaces. As such, the proposed development meets the minimum required parking supply for vehicles and provides slightly less spaces than the minimum requirement for bicycle parking.

4.3. Boundary Street Design

The detailed Multi-Modal Level of Service (MMLOS) analysis for boundary streets and signalized intersections will be provided in the future Site Plan Application.

4.4. Access Intersection Design

Vehicle access to the underground parking garage of the proposed development will be provided along Gilmour St. near the east end of the site.

4.5. Transportation Demand Management

4.5.1. Context for TDM

Based on the type of development, it is assumed that most trips generated by the proposed site will be residents leaving the site in the AM peak to go to work and returning from work to the proposed site in the PM peak. Trips related to the office land use will exhibit the opposite travel pattern, where trips generated will travel to the site during the AM peak and leave the site during the PM peak. Sections 3.1.1 and 3.1.2 describe how many trips are anticipated per travel mode and anticipates the likely locations that they will travel to and from based on the OD-Survey 2011 for Ottawa. The site is located in the "Downtown Ottawa Urban Design Strategy" Design Priority Area (DPA) according to the Official Plan.

4.5.2. Need and Opportunity

The proposed development is located in a well-developed core area of the City of Ottawa, where transit and active transportation facilities, such as the LRT, the bike tracks and the sidewalks, are well-maintained and developed, which naturally results in increased transit and active transportation usage and decreased auto trips. In addition, the proposed development is expected to utilize measures to maintain sustainable transit and active mode shares, as described in more detail in Section 4.5.3 below.



4.5.3. TDM Program

The TDM Infrastructure and TDM Measures Checklists have been provided in Appendix D. Both the residential and non-residential checklists have been provided, given that both land uses will be present on site. The proposed measures for each respective checklist are provided below.

Proposed measures identified in the TDM Measures Checklist are:

- Display local area maps with walking/cycling access routes and key destinations at major entrances
- Display relevant transit schedules and route maps at entrances
- Provide online links to OC Transpo ad STO information
- Contract with provider to install on-site carshare vehicles and promote their use by residents (note that this will be investigated and confirmed as part of the future SPA)
- Unbundle parking cost from monthly rent and lease rates at multi-tenant sites
- Provide a multimodal travel option information package to new residents

Proposed measures identified in the TDM-supportive Development Design and Infrastructure Checklist are:

- Locate building close to the street, and do not locate parking areas between the street and building
- Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations
- Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort
- Provide convenient, direct access to stations or major stops along rapid transit routes within 600
 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly,
 weather protected (where possible) environment between rapid transit accesses and building
 entrances; ensure quality linkages from sidewalks through building entrances to integrated
 stops/stations
- Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances
 through such measures as: reducing distances between public sidewalks and major building
 entrances; providing walkways from public streets to major building entrances; within a site, providing
 walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas
 where people may congregate, such as courtyards and transit stops; and providing weather protection
 through canopies, colonnades, and other design elements wherever possible
- Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks
- Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps
- Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by
 active transportation. Provide links to the existing or planned network of public sidewalks, multi-use
 pathways and on- road cycle routes. Where public sidewalks and multi-use pathways intersect with
 roads, consider providing traffic control devices to give priority to cyclists and pedestrians
- Provide safe, direct and attractive walking routes from building entrances to nearby transit stops
- Ensure that walking routes to transit stops are secure and lighted wherever possible
- Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails
- Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible
- Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa: provide convenient access to main entrances or well- used areas
- Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored



- Where more than 50 bicycle parking spaces are provided for a single residential building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers
- Provide carshare parking spaces in permitted non-residential zones, occupying either required or provided parking spaces (note that this will be investigated and confirmed as part of the future SPA)
- Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a
 variance is being applied for (for residents, parking is provided at a rate of 0.7 and 0.1 per unit for
 residents and visitors, respectively)

4.6. Neighbourhood Traffic Management

This module compares the maximum one-way traffic of a local or collector road during morning and afternoon peak hours, to the respective threshold provided by the City of Ottawa TIA Guidelines. Site-generated traffic of the proposed development are expected to use local road Gilmour St as part of their access route to/from the proposed development. The thresholds provided in the TIA Guidelines indicate a maximum one-way traffic of 120 veh/h for local roads. Using the total projected 2029 traffic volumes in Figure 20, future traffic volumes along Gilmour St can be compared to its local road threshold as follows:

• The maximum one-way traffic is approximately 240 veh/h at the east leg of the Kent/Gilmour intersection during the morning peak hour. This volume is greater than the 120 veh/h threshold of a local road and is approaching a collector road threshold of 300 veh/h. Notably, these volumes are approximately the same in existing conditions, as the existing office building generated similar inbound traffic during the morning peak hour as the proposed residential building

Since Gilmour St is a one-way eastbound road with a relatively wide travel lane of approximately 6.0 m in some sections, it can be expected that the road can accommodate a greater amount of traffic than a regular local road. Therefore, there are no anticipated capacity issues for vehicles on this road.

4.7. Transit

Transit facilities are anticipated to continue operating in the future as mentioned in Section 2.1.2: Transit Network. The proposed development is anticipated to generate 45 and 32 transit trips during the morning and afternoon peak hours respectively. As such, the proposed development will have little impact to the surrounding transit network.

4.8. Review of Network Concept

Exempt - see Table 1.

4.9. Intersection Design

4.9.1. Intersection control

The site access to the existing underground parking garage is assumed to use Stop control for vehicles exiting, which will be sufficient given the low volumes.

4.9.2. Intersection design

Synchro 10 Trafficware was used to analyze intersection performance of intersections within the study area. Critical movements at each of the intersections were assessed based on either the movement with the highest volume-to-capacity ratio (for signalized intersections), or the movement experiencing the highest average delay (for unsignalized intersections). It should be noted that, as per the TIA Guidelines, the Peak Hour Factor (PHF) used for analysis was 0.9 in existing conditions and 1.0 in all future scenario conditions. All Synchro report outputs for existing and future conditions have been provided in Appendix E.

Existing Conditions

Table 17 below summarizes the intersection performance of study area intersections, based on the existing conditions traffic volumes illustrated in Figure 8.



Table 17: Existing Conditions Intersection Performance

Table 11. Existing conditions intersection (chominance									
	Weekday AM Peak (PM Peak)								
Intersection		Critical Mover	ment	Intersection 'As a Whole'					
mersection	LOS	max. v/c or avg. delay (s)	Movement	Delay (s)	LOS	v/c			
Kent St/Somerset St W (S)	B(A)	0.70(0.49)	NBT(WBT)	18.9(18.9)	B(A)	0.69(0.48)			
Kent St/Gilmour St (S)	B(A)	0.69(0.34)	NBT(NBT)	22.6(10.3)	B(A)	0.67(0.33)			
Kent St/Gladstone Ave (S)	C(A)	0.76(0.59)	NBT(EBT)	18.5(17.3)	C(A)	0.73(0.52)			
Bank St/MacLaren St (S)	A(B)	0.55(0.66)	NBT(SBT)	5.4(10.3)	A(A)	0.53(0.53)			
Bank St/Gilmour St (S)	A(A)	0.59(0.45)	NBT(NBT)	11.6(8.5)	A(A)	0.46(0.40)			
O'Connor St/Gilmour St (S)	A(B)	0.36(0.67)	SBT(SBT)	8.7(13.8)	A(B)	0.35(0.64)			
Kent St/MacLaren St (U)	D(B)	29.5(13.7)	WB(WB)	0.7(1.5)	A(A)	-			

Note: Analysis of signalized intersections assumes a PHF of 0.9 and a saturation flow rate of 1800 veh/h/lane.

As shown in Table 17, signalized intersections 'as a whole' operate at a LOS 'C' or better during both peak hours, with critical movement also operating at LOS 'C' or better. Critical movements of unsignalized intersections operate at a LOS 'D' or better during both peak hours.

Total Future Background 2024

Table 18 below summarizes the Synchro traffic operations at study area intersections, based on the future background 2024 conditions in Figure 17.

Table 18: Future Background 2024 Intersection Performance

	Weekday AM Peak (PM Peak)								
Intersection		Critical Movem	ent	Intersection 'As a Whole'					
intersection	LOS	max. v/c or avg. delay (s)	Movement	Delay (s)	LOS	v/c			
Kent St/Somerset St W (S)	B(A)	0.67(0.47)	NBT(WBT)	18.3(18.4)	B(A)	0.66(0.46)			
Kent St/Gilmour St (S)	B(A)	0.67(0.31)	NBT(NBT)	22.3(9.8)	B(A)	0.65(0.30)			
Kent St/Gladstone Ave (S)	C(A)	0.73(0.56)	NBT(EBT)	17.9(16.8)	B(A)	0.70(0.49)			
Bank St/MacLaren St (S)	A(B)	0.53(0.63)	NBT(SBT)	5.2(9.9)	A(A)	0.51(0.51)			
Bank St/Gilmour St (S)	A(A)	0.57(0.43)	NBT(EBT)	11.1(9.4)	A(A)	0.45(0.40)			
O'Connor St/Gilmour St (S)	A(B)	0.35(0.64)	SBT(SBT)	8.7(13.9)	A(B)	0.34(0.62)			
Kent St/MacLaren St (U)	D(B)	26.0(13.3)	WB(WB)	0.6(1.5)	A(A)	-			

Note: Analysis of signalized intersections assumes a PHF of 1.0 and a saturation flow rate of 1800 veh/h/lane.

As shown in Table 18, study area intersections are projected to operate similar or slightly better than existing conditions due to increasing the PHF to 1.0.

Total Future Background 2029

Table 19 below summarizes the Synchro traffic operations at study area intersections, based on future background 2029 traffic volumes in Figure 18.

⁽S) - Signalized intersection, critical movement based on max v/c

⁽U) - Unsignalized Intersection, critical movement based on highest average delay

⁽S) - Signalized intersection, critical movement based on max v/c

⁽U) - Unsignalized Intersection, critical movement based on highest average delay



Table 19: Total Future Background 2029 Intersection Performance

	Weekday AM Peak (PM Peak)								
Intersection		Critical Moven	nent	Intersec	Intersection 'As a Whole'				
intersection	LOS	max. v/c or avg. delay (s)	Movement	Delay (s)	LOS	v/c			
Kent St/Somerset St W (S)	B(A)	0.70(0.49)	NBT(WBT)	19.0(18.9)	B(A)	0.69(0.48)			
Kent St/Gilmour St (S)	B(A)	0.70(0.33)	NBT(NBT)	22.8(10.2)	B(A)	0.68(0.32)			
Kent St/Gladstone Ave (S)	C(A)	0.76(0.59)	NBT(EBT)	18.6(17.2)	C(A)	0.73(0.51)			
Bank St/MacLaren St (S)	A(B)	0.55(0.66)	NBT(SBT)	5.3(10.3)	A(A)	0.53(0.53)			
Bank St/Gilmour St (S)	A(A)	0.59(0.45)	NBT(NBT)	11.4(9.5)	A(A)	0.46(0.42)			
O'Connor St/Gilmour St (S)	A(B)	0.36(0.67)	SBT(SBT)	8.8(14.2)	A(B)	0.35(0.64)			
Kent St/MacLaren St (U)	D(B)	27.5(13.4)	WB(WB)	0.6(1.4)	A(A)	-			

Note: Analysis of signalized intersections assumes a PHF of 1.0 and a saturation flow rate of 1800 veh/h/lane.

As indicated by Table 19, traffic operations are anticipated to be similar to the future background 2024 traffic operations, with slightly higher delays and v/c ratios.

Total Projected 2024

Based on total projected 2024 traffic volumes in Figure 19, study area intersections were analyzed using Synchro, with results summarized in Table 20 below.

Table 20: Total Projected 2024 Intersection Performance

			Weekday AM	Peak (PM Pea	k)		
		Critical Move	ement	Intersection 'As a Whole'			
Intersection		max. v/c or	N4	D - I (-)	1.00		
	LOS	avg. delay (s)	Movement	Delay (s)	LOS	v/c	
Kent St/Somerset St W (S)	B(A)	0.67(0.47)	NBT(WBT)	18.3(18.4)	B(A)	0.66(0.46)	
Kent St/Gilmour St (S)	B(A)	0.67(0.32)	NBT(NBT)	22.3(9.9)	B(A)	0.65(0.31)	
Kent St/Gladstone Ave (S)	C(A)	0.73(0.56)	NBT(EBT)	17.9(16.9)	B(A)	0.70(0.49)	
Bank St/MacLaren St (S)	A(B)	0.54(0.63)	NBT(SBT)	5.3(10.0)	A(A)	0.52(0.51)	
Bank St/Gilmour St (S)	A(A)	0.57(0.43)	NBT(NBT)	11.5(8.9)	A(A)	0.45(0.40)	
O'Connor St/Gilmour St (S)	A(B)	0.35(0.64)	SBT(SBT)	8.5(13.6)	A(B)	0.34(0.62)	
Kent St/MacLaren St (U)	D(B)	25.9(13.2)	WB(WB)	0.6(1.4)	A(A)	-	
Gilmour St/Site Access (U)	B(A)	10.4(9.9)	SB(SB)	2.5(2.8)	A(A)	-	

Note: Analysis of signalized intersections assumes a PHF of 1.0 and a saturation flow rate of 1800 veh/h/lane.

As indicated by Table 20, traffic operations are anticipated to be similar to the future background 2024 traffic operations, with slightly higher delays and v/c ratios. The site access is expected to operate at a LOS 'B' or better during both peak hours.

Total Projected 2029

Based on total projected 2029 traffic volumes in Figure 20, study are intersections were analyzed using Synchro, with results summarized in Table 21 below.

⁽S) - Signalized intersection, critical movement based on max v/c

⁽U) - Unsignalized Intersection, critical movement based on highest average delay

⁽S) - Signalized intersection, critical movement based on max v/c

⁽U) - Unsignalized Intersection, critical movement based on highest average delay



Table 21: Total Projected 2029 Intersection Performance

Table 21: Tour 1 To cook 2020 Intersection 1 Chemicalou									
	Weekday AM Peak (PM Peak)								
		Critical Move	ement	Intersection 'As a Whole'					
Intersection		max. v/c or							
	LOS	avg. delay	Movement	Delay (s)	LOS	v/c			
		(s)							
Kent St/Somerset St W (S)	B(A)	0.70(0.49)	NBT(WBT)	19.0(18.9)	B(A)	0.69(0.48)			
Kent St/Gilmour St (S)	B(A)	0.70(0.34)	NBT(NBT)	22.8(10.4)	B(A)	0.68(0.33)			
Kent St/Gladstone Ave (S)	C(A)	0.76(0.59)	NBT(EBT)	18.6(17.3)	C(A)	0.73(0.52)			
Bank St/MacLaren St (S)	A(B)	0.56(0.66)	NBT(SBT)	5.5(10.4)	A(A)	0.54(0.53)			
Bank St/Gilmour St (S)	A(A)	0.59(0.45)	NBT(NBT)	11.8(9.0)	A(A)	0.46(0.41)			
O'Connor St/Gilmour St (S)	A(B)	0.36(0.67)	SBT(SBT)	8.5(14.0)	A(B)	0.35(0.64)			
Kent St/MacLaren St (U)	D(B)	27.5(13.4)	WB(WB)	0.6(1.4)	A(A)	-			
Gilmour St/Site Access (U)	B(A)	10.4(9.9)	SB(SB)	2.5(2.8)	A(A)	-			
				400000000000000000000000000000000000000					

Note: Analysis of signalized intersections assumes a PHF of 1.0 and a saturation flow rate of 1800 veh/h/lane.

As indicated by Table 21, traffic operations are anticipated to be similar to the future background 2029 traffic operations, with slightly higher delays and v/c ratios. The site access is expected to operate at a LOS 'B' or better during both peak hours.

5. Findings, Conclusions and Recommendations

Based on the results summarized herein, the following transportation related conclusions are offered:

Proposed Development

- The proposed development will be located at the municipal address of 359 Kent St, which is at the northwest corner of the intersection of Kent/Gilmour. The site is currently occupied by a 6-storey office building and a surface parking lot, which will be replaced by the proposed development.
- The development will consist of a 35-storey high-rise residential building with 405 apartment units, 21,388 ft² office space and 7,833 ft² commercial space and will be constructed in a single phase by 2024.
- Access will be provided via an underground parking garage ramp along Gilmour St. The proposed development will provide 332 vehicle parking and 191 bicycle parking spaces within a five-level underground parking garage.
- At full buildout in 2024, the development is anticipated to generate a total of approximately 245 person trips during both peak hours. Vehicle trips are anticipated to be 68 to 75 veh/h during both peak hours. Transit trips are anticipated to be 50 and 68 trips during the morning and afternoon peak hours respectively. Active transportation modes (bike and walk) are anticipated to generate the most trips given the site location in a core sector of the City of Ottawa, with 87 and 114 trips generated during the morning and afternoon peak hours respectively.
- Accounting for trips generated by the existing office building, the proposed development is anticipate to generate 147 to 168 'new' total person trips, 31 to 32 'new' vehicle trips, 28 to 40 'new' transit trips and 70 to 100 'new' active transportation trips during peak hours.

Existing and Background Conditions

• In existing conditions, all intersections 'as a whole' are anticipated to operate at LOS 'C' or better during both peak hours. Critical movements of signalized intersections are anticipated to operate at LOS 'C' or better during peak hours. Critical movements of unsignalized intersections are anticipated to operate at LOS 'D' or better during peak hours.

⁽S) - Signalized intersection, critical movement based on max v/c

⁽U) - Unsignalized Intersection, critical movement based on highest average delay



- A background growth rate of 1% per year was conservatively applied to through movements at major study area intersections during horizon years 2024 and 2029.
- As required by the TIA Guidelines, the PHF in future conditions is increased to 1.0, which results in similar traffic operations for total future background 2024 and 2029 compared to existing conditions.

Projected Conditions

- Total projected 2024 and 2029 traffic operations are similar to their respective future background operations, with slightly higher delays and v/c ratios.
- In both existing and future conditions, traffic along Gilmour St exceeds the 120 veh/h threshold of a local road, with 240 veh/h. There are no capacity concerns along the Gilmour St as traffic operations at study area intersections are acceptable and the road may accommodate more traffic due to eastbound only operations. As such, there are not recommended modifications or reclassification of Gilmour St.

In summary, the proposed development will have little impact on the surrounding road network and transit facilities and is recommended to proceed from a transportation perspective.



APPENDIX A SCREENING FORM



City of Ottawa 2017 TIA Guidelines

Date 28-Jun-21 **TIA Screening Form** Project 359 Kent Street TIA 908979-10065 Project Number

Results of Screening	Yes/No
Development Satisfies the Trip Generation Trigger	Yes
Development Satisfies the Location Trigger	Yes
Development Satisfies the Safety Trigger	Yes

Module 1.1 - Description of Proposed Development			
Municipal Address	359 Kent St, Ottawa, ON K2P 2M8		
Description of location	Northeast corner of Kent/Gilmour intersection		
Land Use	High-rise residential apartment building		
Development Size	30-storeys, 367 units		
Number of Accesses and Locations	Access to underground parking garage along Gilmour St		
Development Phasing	One phase		
Buildout Year	Assumed 2024		
Sketch Plan / Site Plan	See attached		

Module 1.2 - Trip Generation Trigger		
Land Use Type	Townhomes or Apartments	
Development Size	367 Units	
Trip Generation Trigger Met?	Yes	

Module 1.3 - Location Triggers		
Development Proposes a new driveway to a boundary street that is designated as part of the City's Transit Priority, Rapid Transit, or Spine Bicycle Networks (See Sheet 3)	No	
Development is in a Design Priority Area (DPA) or Transit- oriented Development (TOD) zone. (See Sheet 3)	Yes	Downtown Ottawa Urban Design Strategy DPA
Location Trigger Met?	Yes	

Module 1.4 - Safety Triggers		
Posted Speed Limit on any boundary road	<80	km/h
Horizontal / Vertical Curvature on a boundary street limits sight lines at a proposed driveway	No	
A proposed driveway is within the area of influence of an adjacent traffic signal or roundabout (i.e. within 300 m of intersection in rural conditions, or within 150 m of intersection in urban/ suburban conditions) or within auxiliary lanes of an intersection;	Yes	Signalized intersections along Gilmour St
A proposed driveway makes use of an existing median break that serves an existing site	No	
There is a documented history of traffic operations or safety concerns on the boundary streets within 500 m of the development	No	
The development includes a drive-thru facility	No	
Safety Trigger Met?	Yes	

APPENDIX B TRAFFIC COUNT DATA



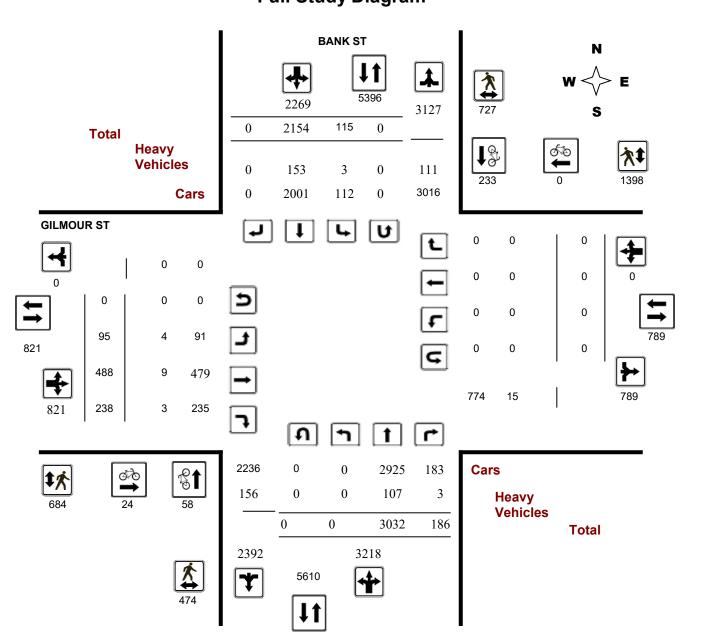
Turning Movement Count - Study Results

BANK ST @ GILMOUR ST

Survey Date: Tuesday, August 25, 2015 WO No: 35291

Start Time: 07:00 Device: Jamar Technologies, Inc

Full Study Diagram



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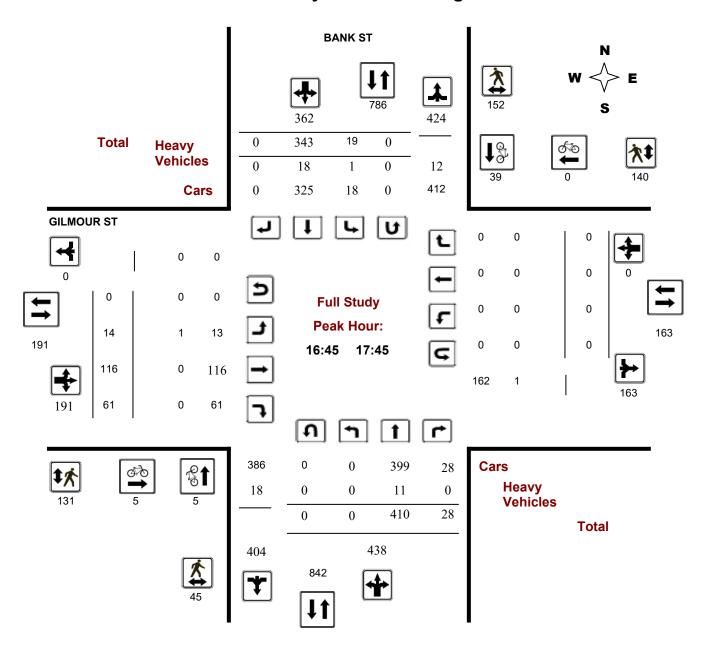
Turning Movement Count - Study Results

BANK ST @ GILMOUR ST

Survey Date: Tuesday, August 25, 2015 WO No: 35291

Start Time: 07:00 Device: Jamar Technologies, Inc

Full Study Peak Hour Diagram



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Turning Movement Count - Peak Hour Diagram

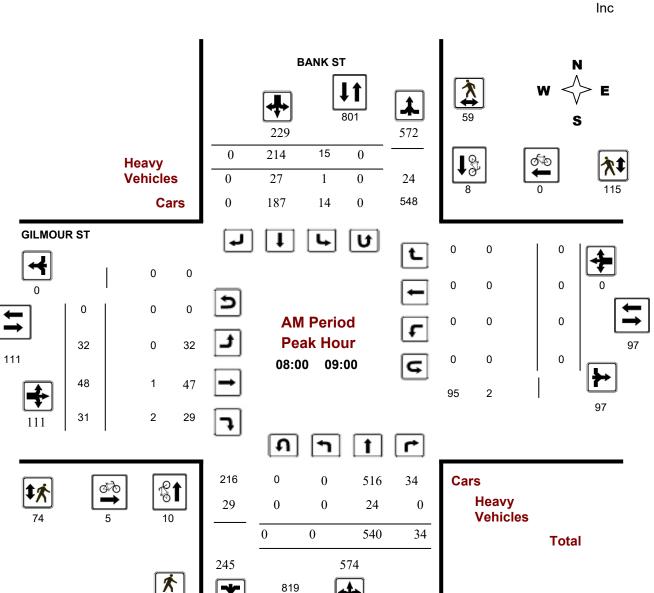
BANK ST @ GILMOUR ST

Survey Date: Tuesday, August 25, 2015

Start Time: 07:00

WO No: 35291

Device: Jamar Technologies,



Comments

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Turning Movement Count - Peak Hour Diagram

BANK ST @ GILMOUR ST

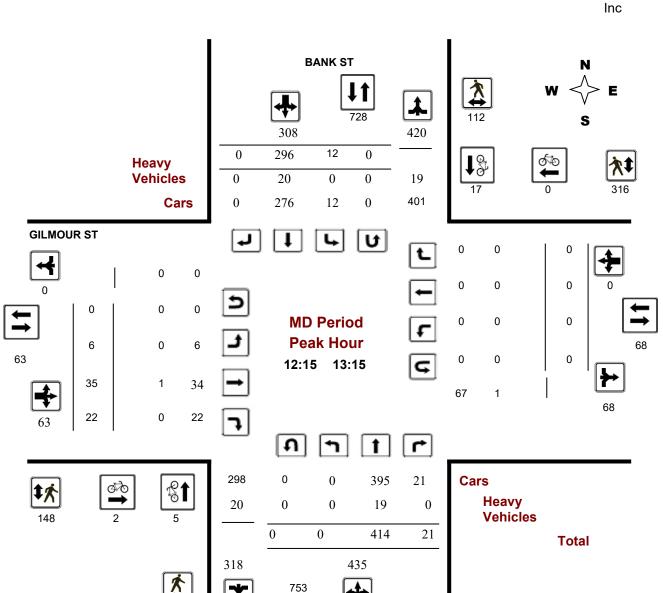
Survey Date: Tuesday, August 25, 2015

Start Time: 07:00

WO No: 35291

Device: Jamar

Jamar Technologies,



Comments

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Turning Movement Count - Peak Hour Diagram

BANK ST @ GILMOUR ST

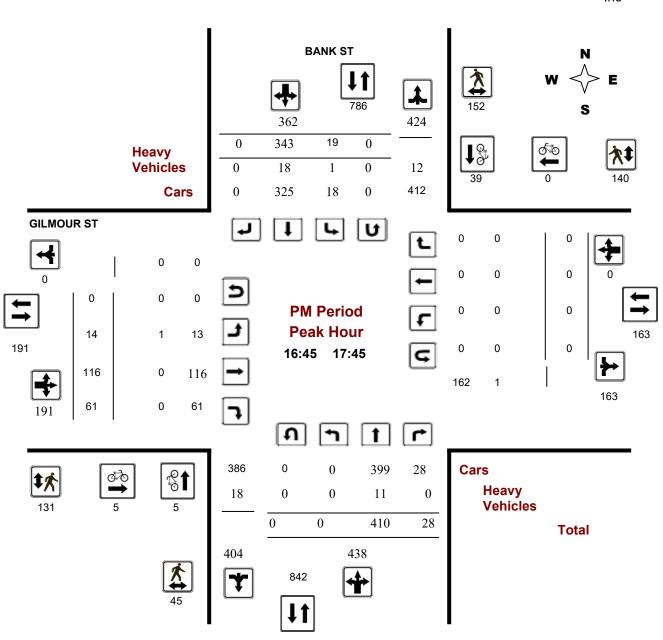
Survey Date: Tuesday, August 25, 2015

Start Time: 07:00

WO No: 35291

Device: Jamar Technologies,

Inc



Comments

2021-Jul-09 Page 3 of 3



Turning Movement Count - Study Results

BANK ST @ GILMOUR ST

Survey Date: Tuesday, August 25, 2015 WO No: 35291

Start Time: 07:00 Device: Jamar Technologies, Inc

Full Study Summary (8 HR Standard)

Survey Date: Tuesday, August 25, 2015 Total Observed U-Turns AADT Factor

Northbound: 0 Southbound: 0 .90

Eastbound: 0 Westbound: 0

			В	ANK S	Т							GIL	MOUF	RST					
	No	rthbou	nd		So	uthbou	nd			Е	astbou	ınd		W	estbou	ınd			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	0	277	22	299	12	174	0	186	485	8	51	13	72	0	0	0	0	72	557
08:00 09:00	0	540	34	574	15	214	0	229	803	32	48	31	111	0	0	0	0	111	914
09:00 10:00	0	362	21	383	12	209	0	221	604	11	31	17	59	0	0	0	0	59	663
11:30 12:30	0	381	6	387	8	271	0	279	666	5	40	23	68	0	0	0	0	68	734
12:30 13:30	0	342	24	366	10	280	0	290	656	6	26	22	54	0	0	0	0	54	710
15:00 16:00	0	376	29	405	23	352	0	375	780	7	62	36	105	0	0	0	0	105	885
16:00 17:00	0	389	26	415	23	326	0	349	764	10	103	29	142	0	0	0	0	142	906
17:00 18:00	0	365	24	389	12	328	0	340	729	16	127	67	210	0	0	0	0	210	939
Sub Total	0	3032	186	3218	115	2154	0	2269	5487	95	488	238	821	0	0	0	0	821	6308
U Turns	0			0	0			0	0	0			0	0			0	0	0
Total	0	3032	186	3218	115	2154	0	2269	5487	95	488	238	821	0	0	0	0	821	6308
EQ 12Hr	0	4214	259	4473	160	2994	0	3154	7627	132	678	331	1141	0	0	0	0	1141	8768
Note: These v	alues a	re calcu	lated by	/ multiply	ying the	totals b	y the a _l	ppropriat	e expans	ion fact	or.			1.39					
AVG 12Hr	0	3793	233	4026	144	2695	0	2839	6865	119	610	298	1027	0	0	0	0	1027	7892
Note: These v	olumes	are cal	culated	by multi	plying t	he Equiv	alent 1	2 hr. tota	ls by the	AADT f	factor.			.90					
AVG 24Hr	0	4969	305	5274	189	3530	0	3719	8993	156	799	390	1345	0	0	0	0	1345	10338
Note: These v	olumes	are cal	culated	by multi	plying t	he Avera	ige Dai	ly 12 hr.	totals by	12 to 24	4 expan	sion fac	tor.	1.31					

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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Turning Movement Count - Study Results

BANK ST @ GILMOUR ST

Survey Date: Tuesday, August 25, 2015 WO No: 35291

Start Time: 07:00 Device: Jamar Technologies, Inc

Full Study 15 Minute Increments

BANK ST GILMOUR ST

		N	orthbou	ınd		Sc	outhbou	nd			Е	astbour	nd		We	estbour	nd			
Time I	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	0	49	7	56	3	37	0	40	96	3	7	2	12	0	0	0	0	12	108
07:15	07:30	0	71	7	78	0	52	0	52	130	2	12	5	19	0	0	0	0	19	149
07:30	07:45	0	78	4	82	6	44	0	50	132	2	10	2	14	0	0	0	0	14	146
07:45	08:00	0	79	4	83	3	41	0	44	127	1	22	4	27	0	0	0	0	27	154
08:00	08:15	0	146	3	149	3	58	0	61	210	9	10	7	26	0	0	0	0	26	236
08:15	08:30	0	150	9	159	4	57	0	61	220	10	20	2	32	0	0	0	0	32	252
08:30	08:45	0	128	11	139	4	63	0	67	206	11	10	16	37	0	0	0	0	37	243
08:45	09:00	0	116	11	127	4	36	0	40	167	2	8	6	16	0	0	0	0	16	183
09:00	09:15	0	82	11	93	3	44	0	47	140	5	12	4	21	0	0	0	0	21	161
09:15	09:30	0	85	1	86	3	51	0	54	140	6	11	1	18	0	0	0	0	18	158
09:30	09:45	0	106	2	108	4	52	0	56	164	0	3	6	9	0	0	0	0	9	173
09:45	10:00	0	89	7	96	2	62	0	64	160	0	5	6	11	0	0	0	0	11	171
11:30	11:45	0	110	2	112	0	45	0	45	157	0	8	10	18	0	0	0	0	18	175
11:45	12:00	0	77	1	78	2	58	0	60	138	1	13	3	17	0	0	0	0	17	155
12:00	12:15	0	88	3	91	4	75	0	79	170	3	6	3	12	0	0	0	0	12	182
12:15	12:30	0	106	0	106	2	93	0	95	201	1	13	7	21	0	0	0	0	21	222
12:30	12:45	0	102	8	110	1	54	0	55	165	1	7	6	14	0	0	0	0	14	179
12:45	13:00	0	120	8	128	5	71	0	76	204	2	9	4	15	0	0	0	0	15	219
13:00	13:15	0	86	5	91	4	78	0	82	173	2	6	5	13	0	0	0	0	13	186
13:15	13:30	0	34	3	37	0	77	0	77	114	1	4	7	12	0	0	0	0	12	126
15:00	15:15	0	79	1	80	4	66	0	70	150	2	7	5	14	0	0	0	0	14	164
15:15	15:30	0	124	4	128	8	96	0	104	232	3	13	5	21	0	0	0	0	21	253
15:30	15:45	0	87	15	102	6	103	0	109	211	1	17	7	25	0	0	0	0	25	236
15:45	16:00	0	86	9	95	5	87	0	92	187	1	25	19	45	0	0	0	0	45	232
16:00	16:15	0	67	5	72	2	91	0	93	165	2	26	12	40	0	0	0	0	40	205
16:15	16:30	0	67	8	75	4	73	0	77	152	5	30	7	42	0	0	0	0	42	194
16:30	16:45	0	110	3	113	10	64	0	74	187	1	28	4	33	0	0	0	0	33	220
16:45	17:00	0	145	10	155	7	98	0	105	260	2	19	6	27	0	0	0	0	27	287
17:00	17:15	0	128	9	137	4	78	0	82	219	4	35	11	50	0	0	0	0	50	269
17:15	17:30	0	49	3	52	5	63	0	68	120	5	46	20	71	0	0	0	0	71	191
17:30	17:45	0	88	6	94	3	104	0	107	201	3	16	24	43	0	0	0	0	43	244
17:45	18:00	0	100	6	106	0	83	0	83	189	4	30	12	46	0	0	0	0	46	235
Total:		0	3032	186	3218	115	2154	0	2269	5487	95	488	238	821	0	0	0	0	5487	6,308

Note: U-Turns are included in Totals.

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Turning Movement Count - Study Results

BANK ST @ GILMOUR ST

Survey Date: Tuesday, August 25, 2015 WO No: 35291

Start Time: 07:00 Device: Jamar Technologies, Inc

Full Study Cyclist Volume

BANK ST GILMOUR ST

Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	_ Grand Total
07:00 07:15	3	0	3	0	0	0	3
07:15 07:30	3	1	4	0	0	0	4
07:30 07:45	1	3	4	1	0	1	5
07:45 08:00	4	0	4	1	0	1	5
08:00 08:15	1	3	4	0	0	0	4
08:15 08:30	1	1	2	3	0	3	5
08:30 08:45	3	0	3	2	0	2	5
08:45 09:00	5	4	9	0	0	0	9
09:00 09:15	1	1	2	1	0	1	3
09:15 09:30	2	5	7	0	0	0	7
09:30 09:45	5	7	12	0	0	0	12
09:45 10:00	0	3	3	0	0	0	3
11:30 11:45	0	6	6	0	0	0	6
11:45 12:00	0	9	9	0	0	0	9
12:00 12:15	3	13	16	4	0	4	20
12:15 12:30	0	4	4	1	0	1	5
12:30 12:45	0	2	2	0	0	0	2
12:45 13:00	0	4	4	0	0	0	4
13:00 13:15	5	7	12	1	0	1	13
13:15 13:30	0	0	0	0	0	0	0
15:00 15:15	0	7	7	1	0	1	8
15:15 15:30	2	6	8	2	0	2	10
15:30 15:45	1	21	22	0	0	0	22
15:45 16:00	2	8	10	1	0	1	11
16:00 16:15	1	21	22	0	0	0	22
16:15 16:30	1	15	16	1	0	1	17
16:30 16:45	2	20	22	0	0	0	22
16:45 17:00	2	13	15	0	0	0	15
17:00 17:15	1	10	11	4	0	4	15
17:15 17:30	0	6	6	0	0	0	6
17:30 17:45	2	10	12	1	0	1	13
17:45 18:00	7	23	30	0	0	0	30
Total	58	233	291	24	0	24	315

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Turning Movement Count - Study Results

BANK ST @ GILMOUR ST

Survey Date: Tuesday, August 25, 2015 WO No: 35291

Start Time: 07:00 Device: Jamar Technologies, Inc

Full Study Pedestrian Volume

BANK ST GILMOUR ST

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	7	9	16	6	5	11	27
07:15 07:30	6	5	11	16	13	29	40
07:30 07:45	7	3	10	9	16	25	35
07:45 08:00	13	12	25	29	28	57	82
08:00 08:15	4	13	17	6	7	13	30
08:15 08:30	7	20	27	50	29	79	106
08:30 08:45	3	12	15	10	42	52	67
08:45 09:00	13	14	27	8	37	45	72
09:00 09:15	11	21	32	9	21	30	62
09:15 09:30	4	17	21	6	32	38	59
09:30 09:45	5	8	13	21	15	36	49
09:45 10:00	23	21	44	8	43	51	95
11:30 11:45	15	9	24	23	34	57	81
11:45 12:00	21	24	45	11	64	75	120
12:00 12:15	38	45	83	29	143	172	255
12:15 12:30	21	46	67	31	115	146	213
12:30 12:45	18	15	33	42	79	121	154
12:45 13:00	23	29	52	51	91	142	194
13:00 13:15	19	22	41	24	31	55	96
13:15 13:30	15	32	47	22	45	67	114
15:00 15:15	18	17	35	7	32	39	74
15:15 15:30	35	24	59	21	33	54	113
15:30 15:45	10	14	24	27	11	38	62
15:45 16:00	38	18	56	10	49	59	115
16:00 16:15	20	32	52	14	72	86	138
16:15 16:30	11	38	49	13	75	88	137
16:30 16:45	9	22	31	1	45	46	77
16:45 17:00	9	39	48	11	48	59	107
17:00 17:15	16	54	70	51	58	109	179
17:15 17:30	5	27	32	27	10	37	69
17:30 17:45	15	32	47	42	24	66	113
17:45 18:00	15	33	48	49	51	100	148
Total	474	727	1201	684	1398	2082	3283

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Turning Movement Count - Study Results

BANK ST @ GILMOUR ST

Survey Date: Tuesday, August 25, 2015 WO No: 35291

Start Time: 07:00 Device: Jamar Technologies, Inc

Full Study Heavy Vehicles

BANK ST GILMOUR ST

		No	orthbou	ınd		Sc	uthbou	nd			Е	astbour	nd		We	estbour	nd			
Time Peri	riod	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07	7:15	0	3	2	5	0	5	0	5	10	0	0	0	0	0	0	0	0	0	10
07:15 07	7:30	0	4	0	4	0	4	0	4	8	0	1	0	1	0	0	0	0	1	9
07:30 07	7:45	0	2	0	2	0	6	0	6	8	0	0	0	0	0	0	0	0	0	8
07:45 08	3:00	0	1	0	1	0	3	0	3	4	0	3	0	3	0	0	0	0	3	7
08:00 08	3:15	0	8	0	8	0	4	0	4	12	0	0	1	1	0	0	0	0	1	13
08:15 08	3:30	0	4	0	4	0	4	0	4	8	0	0	0	0	0	0	0	0	0	8
08:30 08	3:45	0	6	0	6	1	7	0	8	14	0	0	0	0	0	0	0	0	0	14
08:45 09	9:00	0	6	0	6	0	12	0	12	18	0	1	1	2	0	0	0	0	2	20
09:00 09	9:15	0	3	0	3	0	3	0	3	6	0	0	0	0	0	0	0	0	0	6
09:15 09	9:30	0	3	0	3	0	7	0	7	10	1	1	0	2	0	0	0	0	2	12
09:30 09	9:45	0	4	0	4	0	6	0	6	10	0	0	0	0	0	0	0	0	0	10
09:45 10	0:00	0	4	0	4	0	5	0	5	9	0	0	0	0	0	0	0	0	0	9
11:30 11	1:45	0	3	0	3	0	5	0	5	8	0	0	1	1	0	0	0	0	1	9
11:45 12	2:00	0	2	0	2	0	4	0	4	6	0	0	0	0	0	0	0	0	0	6
	2:15	0	3	0	3	1	2	0	3	6	2	0	0	2	0	0	0	0	2	8
	2:30	0	5	0	5	0	5	0	5	10	0	1	0	1	0	0	0	0	1	11
	2:45	0	5	0	5	0	3	0	3	8	0	0	0	0	0	0	0	0	0	8
	3:00	0	2	0	2	0	5	0	5	7	0	0	0	0	0	0	0	0	0	7
13:00 13	3:15	0	7	0	7	0	7	0	7	14	0	0	0	0	0	0	0	0	0	14
	3:30	0	1	0	1	0	3	0	3	4	0	0	0	0	0	0	0	0	0	4
	5:15	0	4	0	4	0	4	0	4	8	0	0	0	0	0	0	0	0	0	8
	5:30	0	4	0	4	0	6	0	6	10	0	0	0	0	0	0	0	0	0	10
	5:45	0	1	0	1	0	6	0	6	7	0	1	0	1	0	0	0	0	1	8
	3:00	0	2	0	2	0	4	0	4	6	0	0	0	0	0	0	0	0	0	6
	3:15	0	3	0	3	0	3	0	3	6	0	1	0	1	0	0	0	0	1	7
	3:30	0	1	0	1	0	3	0	3	4	0	0	0	0	0	0	0	0	0	4
	3:45	0	2	0	2	0	4	0	4	6	0	0	0	0	0	0	0	0	0	6
	7:00	0	3	0	3	1	5	0	6	9	0	0	0	0	0	0	0	0	0	9
	7:15	0	2	0	2	0	4	0	4	6	1	0	0	1	0	0	0	0	1	7
	7:30	0	1	0	1	0	3	0	3	4	0	0	0	0	0	0	0	0	0	4
	7:45	0	5	0	5	0	6	0	6	11	0	0	0	0	0	0	0	0	0	11
	3:00	0	3	1	4	0	5	0	5	9	0	0	0	0	0	0	0	0	0	9
Total: No	one	0	107	3	110	3	153	0	156	266	4	9	3	16	0	0	0	0	16	282

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Turning Movement Count - Study Results

BANK ST @ GILMOUR ST

Survey Date: Tuesday, August 25, 2015 WO No: 35291

Start Time: 07:00 Device: Jamar Technologies, Inc

Full Study 15 Minute U-Turn Total BANK ST GILMOUR ST

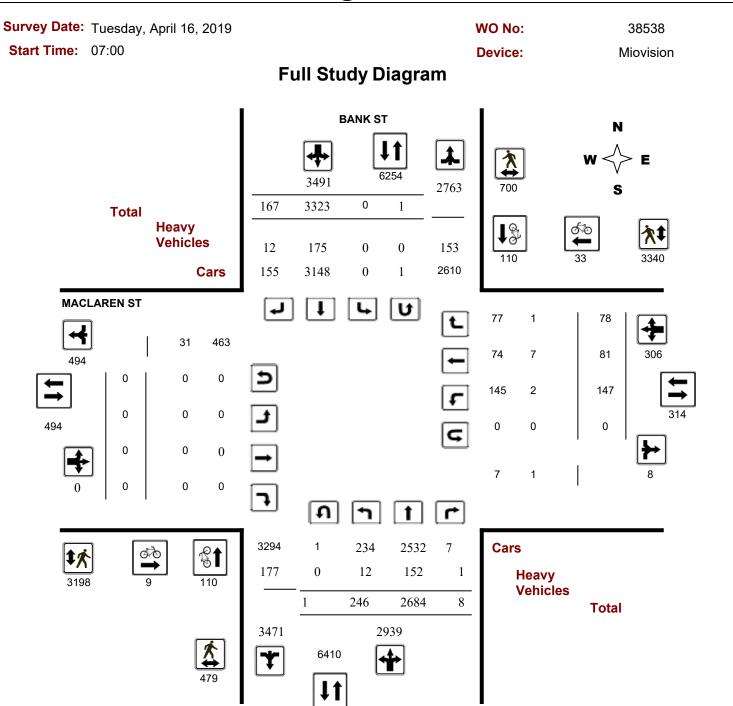
Time F	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
To	otal	0	0	0	0	0

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Turning Movement Count - Study Results

BANK ST @ MACLAREN ST



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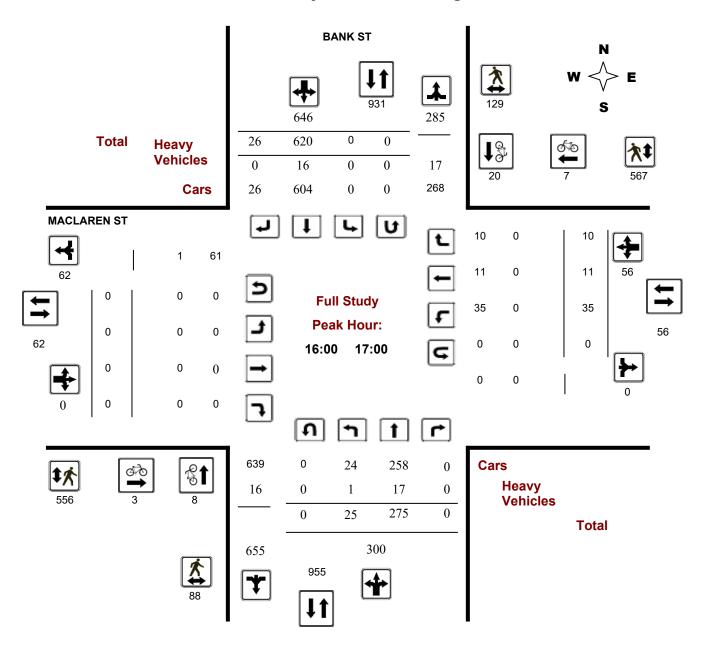
Turning Movement Count - Study Results

BANK ST @ MACLAREN ST

Survey Date: Tuesday, April 16, 2019 WO No: 38538

Start Time: 07:00 Device: Miovision

Full Study Peak Hour Diagram



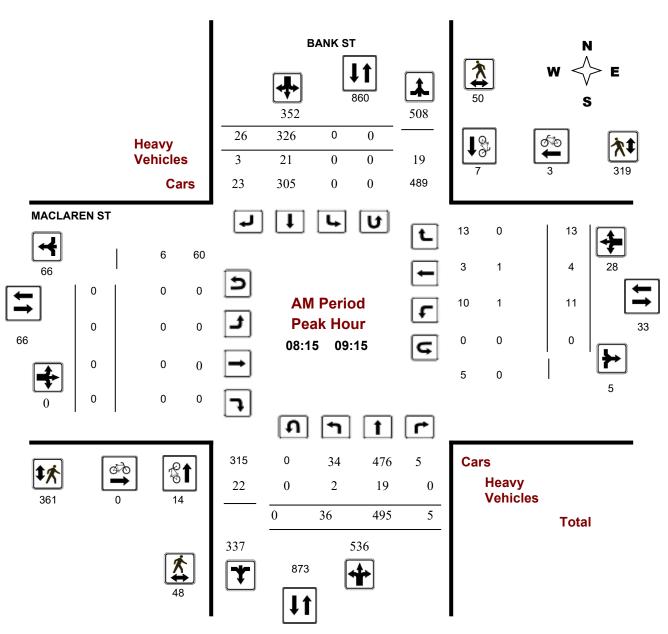
July 9, 2021 Page 2 of 8



Turning Movement Count - Peak Hour Diagram

BANK ST @ MACLAREN ST





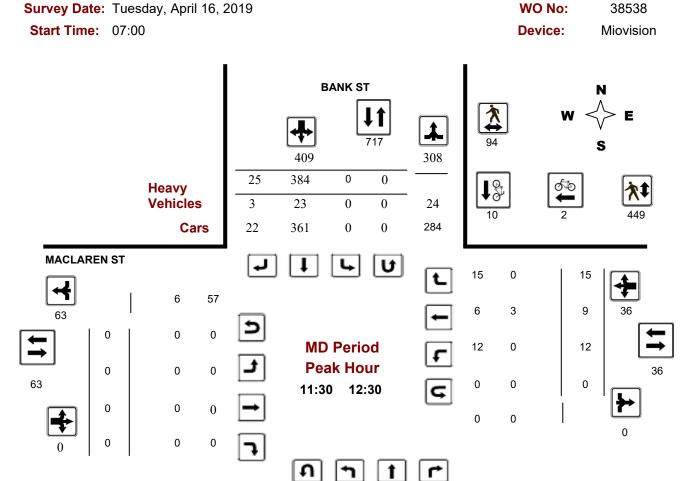
Comments

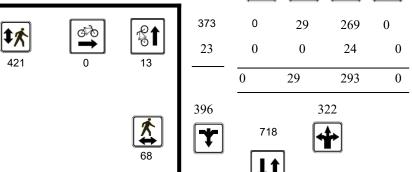
2021-Jul-09 Page 1 of 3



Turning Movement Count - Peak Hour Diagram

BANK ST @ MACLAREN ST





Cars
Heavy
Vehicles
Total

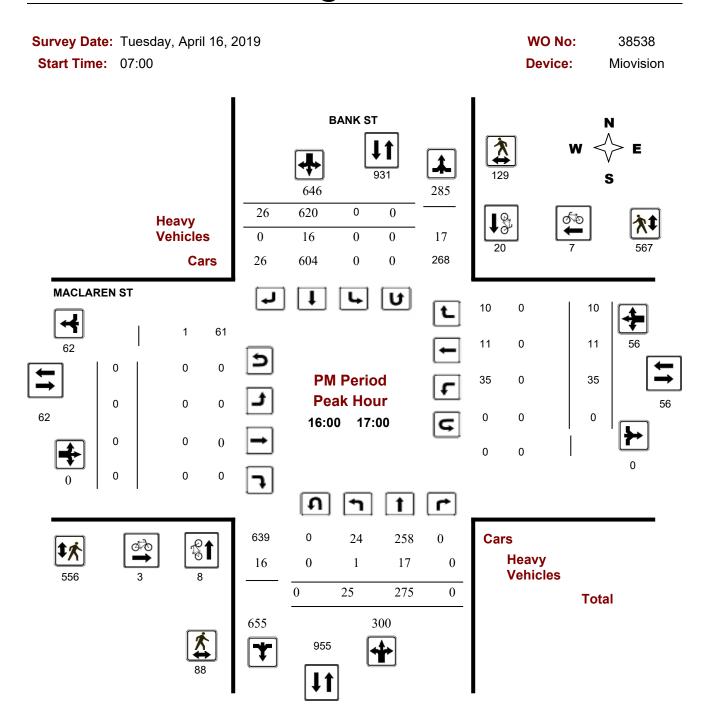
Comments

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Turning Movement Count - Peak Hour Diagram

BANK ST @ MACLAREN ST



Comments

2021-Jul-09 Page 3 of 3



Turning Movement Count - Study Results

BANK ST @ MACLAREN ST

Survey Date: Tuesday, April 16, 2019 WO No: 38538

Start Time: 07:00 Device: Miovision

Full Study Summary (8 HR Standard)

Survey Date: Tuesday, April 16, 2019 Total Observed U-Turns AADT Factor

Northbound: 1 Southbound: 1 .90

Eastbound: 0 Westbound: 0

			В	ANK S	Т							MAC	CLARE	N ST					
	No	rthbou	nd		So	uthbou	ınd			Ea	astbou	nd		W	estbou	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	29	386	0	415	0	237	12	249	664	0	0	0	0	12	6	8	26	26	690
08:00 09:00	34	488	2	524	0	304	21	325	849	0	0	0	0	17	7	11	35	35	884
09:00 10:00	42	448	6	496	0	355	20	375	871	0	0	0	0	3	0	4	7	7	878
11:30 12:30	29	293	0	322	0	384	25	409	731	0	0	0	0	12	9	15	36	36	767
12:30 13:30	29	282	0	311	0	362	25	387	698	0	0	0	0	12	15	13	40	40	738
15:00 16:00	35	245	0	280	0	524	16	540	820	0	0	0	0	24	16	8	48	48	868
16:00 17:00	25	275	0	300	0	620	26	646	946	0	0	0	0	35	11	10	56	56	1002
17:00 18:00	23	267	0	290	0	537	22	559	849	0	0	0	0	32	17	9	58	58	907
Sub Total	246	2684	8	2938	0	3323	167	3490	6428	0	0	0	0	147	81	78	306	306	6734
U Turns	1			1	1			1	2	0			0	0			0	0	2
Total	247	2684	8	2939	1	3323	167	3491	6430	0	0	0	0	147	81	78	306	306	6736
EQ 12Hr	343	3731	11	4085	1	4619	232	4852	8937	0	0	0	0	204	113	108	425	425	9362
Note: These \	alues a	re calcul	ated by	/ multiply	ing the	totals b	y the a _l	opropriat	e expans	ion facto	or.			1.39					
AVG 12Hr	309	3358	10	3677	1	4157	209	4367	8044	0	0	0	0	184	102	97	383	383	8427
Note: These v	olumes	are calc	ulated	by multip	olying th	ne Equiv	alent 1	2 hr. tota	als by the	AADT fa	actor.			.90					
AVG 24Hr	405	4399	13	4817	1	5446	274	5721	10538	0	0	0	0	241	134	127	502	502	11040
Note: These \	olumes	are calc	ulated	by multip	olying tl	ne Avera	ige Dai	ly 12 hr.	totals by	12 to 24	expans	sion fac	tor.	1.31					

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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Turning Movement Count - Study Results

BANK ST @ MACLAREN ST

Survey Date: Tuesday, April 16, 2019 WO No: 38538

Start Time: 07:00 Device: Miovision

Full Study 15 Minute Increments

BANK ST MACLAREN ST

		No	orthbou	ınd		Sc	outhbou	nd			Е	astbour	nd		We	estbour	nd			
Time I	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	5	84	0	89	0	52	3	55	144	0	0	0	0	2	1	1	4	4	148
07:15	07:30	8	94	0	102	0	62	2	64	166	0	0	0	0	3	1	0	4	4	170
07:30	07:45	6	91	0	97	0	67	4	71	168	0	0	0	0	2	4	2	8	8	176
07:45	08:00	10	117	0	127	0	56	3	59	186	0	0	0	0	5	0	5	10	10	196
08:00	08:15	10	122	0	132	0	69	3	72	204	0	0	0	0	6	3	1	10	10	214
08:15	08:30	8	121	0	129	0	84	7	91	220	0	0	0	0	5	1	7	13	13	233
08:30	08:45	6	115	0	121	0	74	6	80	201	0	0	0	0	4	3	1	8	8	209
08:45	09:00	10	130	2	142	0	77	5	82	224	0	0	0	0	2	0	2	4	4	228
09:00	09:15	12	129	3	144	0	91	8	99	243	0	0	0	0	0	0	3	3	3	246
09:15	09:30	11	124	2	137	0	82	3	85	222	0	0	0	0	1	0	0	1	1	223
09:30	09:45	9	113	1	123	0	84	5	89	212	0	0	0	0	1	0	0	1	1	213
09:45	10:00	10	82	0	92	0	98	4	102	194	0	0	0	0	1	0	1	2	2	196
11:30	11:45	10	82	0	92	0	80	6	86	178	0	0	0	0	3	1	2	6	6	184
11:45	12:00	5	71	0	76	0	92	5	97	173	0	0	0	0	1	2	3	6	6	179
12:00	12:15	11	78	0	89	0	111	6	117	206	0	0	0	0	3	4	6	13	13	219
12:15	12:30	3	62	0	65	0	101	8	109	174	0	0	0	0	5	2	4	11	11	185
12:30	12:45	8	64	0	72	0	79	3	82	154	0	0	0	0	5	4	4	13	13	167
12:45	13:00	6	75	0	81	0	90	10	100	181	0	0	0	0	1	2	3	6	6	187
13:00	13:15	11	69	0	80	0	85	3	88	168	0	0	0	0	3	4	1	8	8	176
13:15	13:30	5	74	0	79	1	108	9	118	197	0	0	0	0	3	5	5	13	13	210
15:00	15:15	8	65	0	73	0	124	5	129	202	0	0	0	0	9	5	3	17	17	219
15:15	15:30	11	71	0	82	0	134	7	141	223	0	0	0	0	6	4	2	12	12	235
15:30	15:45	8	60	0	68	0	132	2	134	202	0	0	0	0	0	4	3	7	7	209
15:45	16:00	8	49	0	57	0	134	2	136	193	0	0	0	0	9	3	0	12	12	205
16:00	16:15	4	80	0	84	0	156	2	158	242	0	0	0	0	10	5	1	16	16	258
16:15	16:30	4	55	0	59	0	152	11	163	222	0	0	0	0	8	0	5	13	13	235
16:30	16:45	8	64	0	72	0	151	5	156	228	0	0	0	0	5	4	1	10	10	238
16:45	17:00	9	76	0	85	0	161	8	169	254	0	0	0	0	12	2	3	17	17	271
17:00	17:15	6	58	0	64	0	168	6	174	238	0	0	0	0	12	3	3	18	18	256
17:15	17:30	4	65	0	69	0	124	6	130	199	0	0	0	0	8	7	1	16	16	215
17:30	17:45	7	67	0	74	0	127	6	133	207	0	0	0	0	4	3	3	10	10	217
17:45	18:00	6	77	0	83	0	118	4	122	205	0	0	0	0	8	4	2	14	14	219
Total:		247	2684	8	2939	1	3323	167	3491	6430	0	0	0	0	147	81	78	306	6430	6,736

Note: U-Turns are included in Totals.

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Turning Movement Count - Study Results

BANK ST @ MACLAREN ST

Survey Date: Tuesday, April 16, 2019 WO No: 38538

Start Time: 07:00 Device: Miovision

Full Study Cyclist Volume

BANK ST MACLAREN ST

		DAIN OI			MAGEAREN		_
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 07:15	2	0	2	0	0	0	2
07:15 07:30	3	0	3	0	0	0	3
07:30 07:45	5	1	6	0	1	1	7
07:45 08:00	4	0	4	0	1	1	5
08:00 08:15	7	0	7	0	1	1	8
08:15 08:30	1	2	3	0	1	1	4
08:30 08:45	8	0	8	0	1	1	9
08:45 09:00	4	2	6	0	0	0	6
09:00 09:15	1	3	4	0	1	1	5
09:15 09:30	6	1	7	0	1	1	8
09:30 09:45	1	3	4	0	1	1	5
09:45 10:00	3	0	3	0	0	0	3
11:30 11:45	3	5	8	0	1	1	9
11:45 12:00	3	2	5	0	0	0	5
12:00 12:15	5	2	7	0	0	0	7
12:15 12:30	2	1	3	0	1	1	4
12:30 12:45	4	3	7	0	0	0	7
12:45 13:00	4	5	9	0	0	0	9
13:00 13:15	3	5	8	1	1	2	10
13:15 13:30	4	5	9	0	1	1	10
15:00 15:15	4	2	6	1	1	2	8
15:15 15:30	4	2	6	0	0	0	6
15:30 15:45	1	5	6	1	2	3	9
15:45 16:00	3	5	8	1	2	3	11
16:00 16:15	4	3	7	0	2	2	9
16:15 16:30	3	7	10	1	2	3	13
16:30 16:45	1	7	8	0	2	2	10
16:45 17:00	0	3	3	2	1	3	6
17:00 17:15	6	8	14	1	2	3	17
17:15 17:30	2	15	17	0	5	5	22
17:30 17:45	2	9	11	0	1	1	12
17:45 18:00	7	4	11	1	1	2	13
Total	110	110	220	9	33	42	262

July 9, 2021 Page 5 of 8



Turning Movement Count - Study Results

BANK ST @ MACLAREN ST

Survey Date: Tuesday, April 16, 2019 WO No: 38538

Start Time: 07:00 Miovision Device:

Full Study Pedestrian Volume

BANK ST MACLAREN ST

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	3	4	7	26	35	61	68
07:15 07:30	5	5	10	35	34	69	79
07:30 07:45	6	7	13	56	49	105	118
07:45 08:00	14	8	22	65	68	133	155
08:00 08:15	12	9	21	78	72	150	171
08:15 08:30	13	10	23	89	87	176	199
08:30 08:45	12	14	26	87	77	164	190
08:45 09:00	15	11	26	106	99	205	231
09:00 09:15	8	15	23	79	56	135	158
09:15 09:30	12	18	30	61	48	109	139
09:30 09:45	6	9	15	60	45	105	120
09:45 10:00	7	13	20	61	34	95	115
11:30 11:45	8	25	33	76	78	154	187
11:45 12:00	21	25	46	104	99	203	249
12:00 12:15	20	22	42	131	121	252	294
12:15 12:30	19	22	41	110	151	261	302
12:30 12:45	9	22	31	96	138	234	265
12:45 13:00	20	27	47	97	142	239	286
13:00 13:15	24	21	45	92	110	202	247
13:15 13:30	10	41	51	88	113	201	252
15:00 15:15	15	15	30	105	84	189	219
15:15 15:30	8	23	31	87	106	193	224
15:30 15:45	17	36	53	90	142	232	285
15:45 16:00	16	23	39	96	124	220	259
16:00 16:15	16	17	33	138	112	250	283
16:15 16:30	17	43	60	120	169	289	349
16:30 16:45	36	37	73	157	122	279	352
16:45 17:00	19	32	51	141	164	305	356
17:00 17:15	28	37	65	184	155	339	404
17:15 17:30	20	46	66	170	214	384	450
17:30 17:45	16	30	46	144	159	303	349
17:45 18:00	27	33	60	169	133	302	362
Total	. 479	700	1179	3198	3340	6538	7717

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Turning Movement Count - Study Results

BANK ST @ MACLAREN ST

Survey Date: Tuesday, April 16, 2019 WO No: 38538

Start Time: 07:00 Device: Miovision

Full Study Heavy Vehicles

BANK ST MACLAREN ST

		Northbo	und		Sc	uthbou	ınd			Е	astbour	nd		We	estbour	nd			
Time Perio	od L1	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07:	:15 1	9	0	10	0	5	0	5	15	0	0	0	0	0	0	0	0	0	15
07:15 07:	:30 0	5	0	5	0	8	0	8	13	0	0	0	0	0	0	0	0	0	13
07:30 07:	:45 0	3	0	3	0	4	0	4	7	0	0	0	0	0	0	0	0	0	7
07:45 08:	:00 1	4	0	5	0	5	0	5	10	0	0	0	0	0	0	0	0	0	10
08:00 08:	:15 1	4	0	5	0	4	0	4	9	0	0	0	0	0	0	0	0	0	9
08:15 08:	:30 1	6	0	7	0	6	2	8	15	0	0	0	0	1	1	0	2	2	17
08:30 08:	:45 0	5	0	5	0	7	0	7	12	0	0	0	0	0	0	0	0	0	12
08:45 09:	:00 0	3	0	3	0	4	1	5	8	0	0	0	0	0	0	0	0	0	8
09:00 09:	:15 1	5	0	6	0	4	0	4	10	0	0	0	0	0	0	0	0	0	10
09:15 09:	:30 1	3	1	5	0	9	1	10	15	0	0	0	0	0	0	0	0	0	15
09:30 09:	:45 0	8	0	8	0	8	1	9	17	0	0	0	0	0	0	0	0	0	17
09:45 10:	:00 1	6	0	7	0	2	1	3	10	0	0	0	0	0	0	0	0	0	10
11:30 11:	:45 0	5	0	5	0	8	2	10	15	0	0	0	0	0	0	0	0	0	15
11:45 12:	:00 0	5	0	5	0	4	0	4	9	0	0	0	0	0	1	0	1	1	10
12:00 12:	:15 0	9	0	9	0	2	1	3	12	0	0	0	0	0	1	0	1	1	13
12:15 12:	:30 0	5	0	5	0	9	0	9	14	0	0	0	0	0	1	0	1	1	15
12:30 12:	:45 0	7	0	7	0	7	0	7	14	0	0	0	0	0	1	1	2	2	16
12:45 13:	:00 1	5	0	6	0	3	0	3	9	0	0	0	0	0	0	0	0	0	9
13:00 13:	:15 1	3	0	4	0	9	0	9	13	0	0	0	0	0	1	0	1	1	14
13:15 13:	:30 1	4	0	5	0	9	2	11	16	0	0	0	0	0	0	0	0	0	16
15:00 15:	:15 0	4	0	4	0	7	0	7	11	0	0	0	0	0	0	0	0	0	11
15:15 15:	:30 0	6	0	6	0	2	1	3	9	0	0	0	0	0	0	0	0	0	9
15:30 15:	:45 1	2	0	3	0	5	0	5	8	0	0	0	0	0	1	0	1	1	9
15:45 16:	:00 0	3	0	3	0	9	0	9	12	0	0	0	0	1	0	0	1	1	13
16:00 16:	:15 1	3	0	4	0	3	0	3	7	0	0	0	0	0	0	0	0	0	7
16:15 16:	:30 0	6	0	6	0	4	0	4	10	0	0	0	0	0	0	0	0	0	10
16:30 16:	:45 0	4	0	4	0	7	0	7	11	0	0	0	0	0	0	0	0	0	11
16:45 17:	:00 0	4	0	4	0	2	0	2	6	0	0	0	0	0	0	0	0	0	6
17:00 17:		3	0	3	0	3	0	3	6	0	0	0	0	0	0	0	0	0	6
17:15 17:		6	0	6	0	4	0	4	10	0	0	0	0	0	0	0	0	0	10
17:30 17:	:45 0	3	0	3	0	7	0	7	10	0	0	0	0	0	0	0	0	0	10
17:45 18:	:00 0	4	0	4	0	5	0	5	9	0	0	0	0	0	0	0	0	0	9
Total: No	ne 12	152	1	165	0	175	12	187	352	0	0	0	0	2	7	1	10	10	362

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Turning Movement Count - Study Results

BANK ST @ MACLAREN ST

Survey Date: Tuesday, April 16, 2019 WO No: 38538

Start Time: 07:00 Device: Miovision

Full Study 15 Minute U-Turn Total BANK ST MACLAREN ST

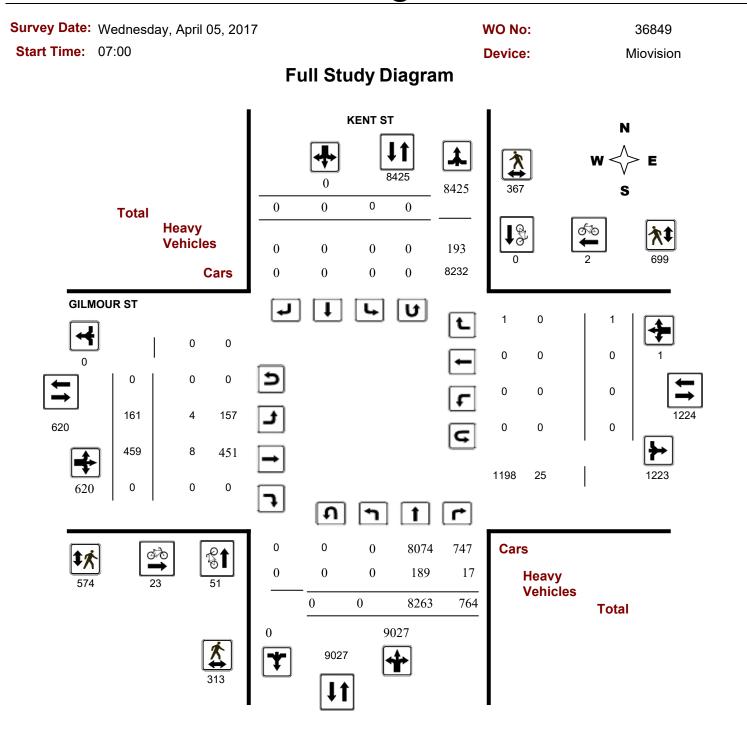
Time P	eriod	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	1	0	0	0	1
13:15	13:30	0	1	0	0	1
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
Tot	tal	1	1	0	0	2

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Turning Movement Count - Study Results

GILMOUR ST @ KENT ST



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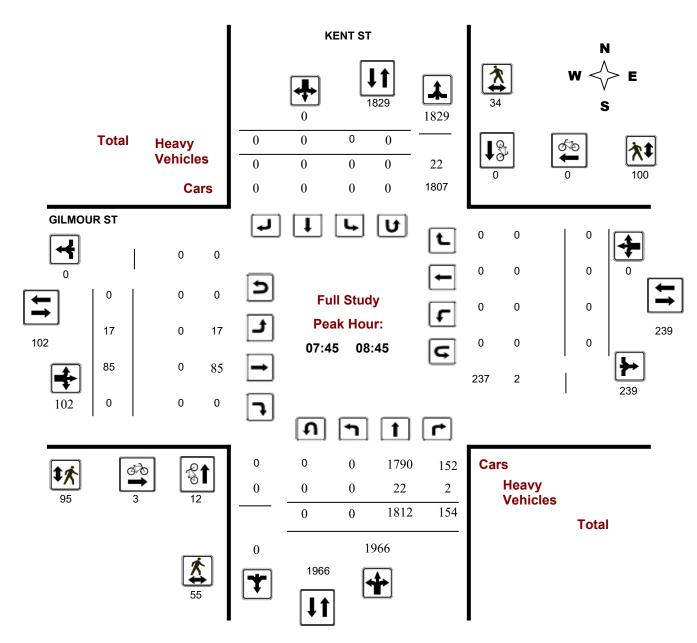
Turning Movement Count - Study Results

GILMOUR ST @ KENT ST

Survey Date: Wednesday, April 05, 2017 WO No: 36849

Start Time: 07:00 Device: Miovision

Full Study Peak Hour Diagram

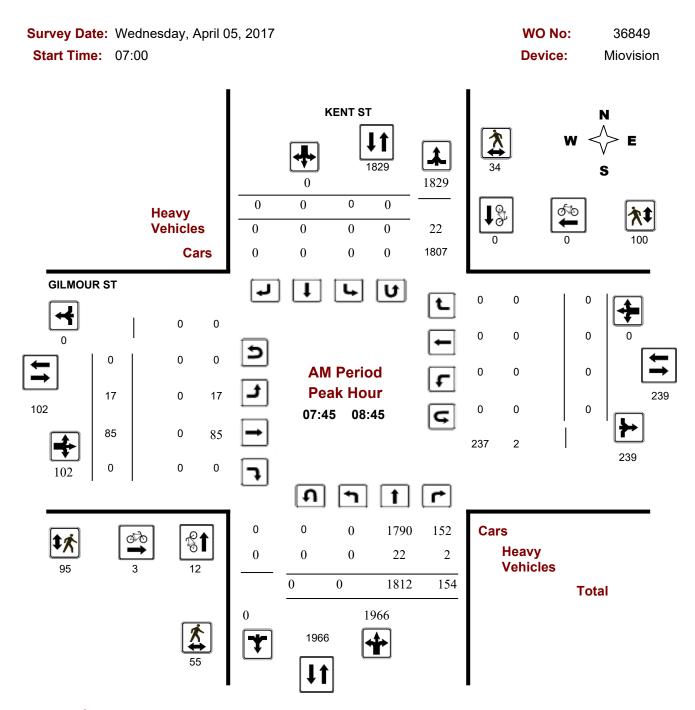


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Turning Movement Count - Peak Hour Diagram

GILMOUR ST @ KENT ST



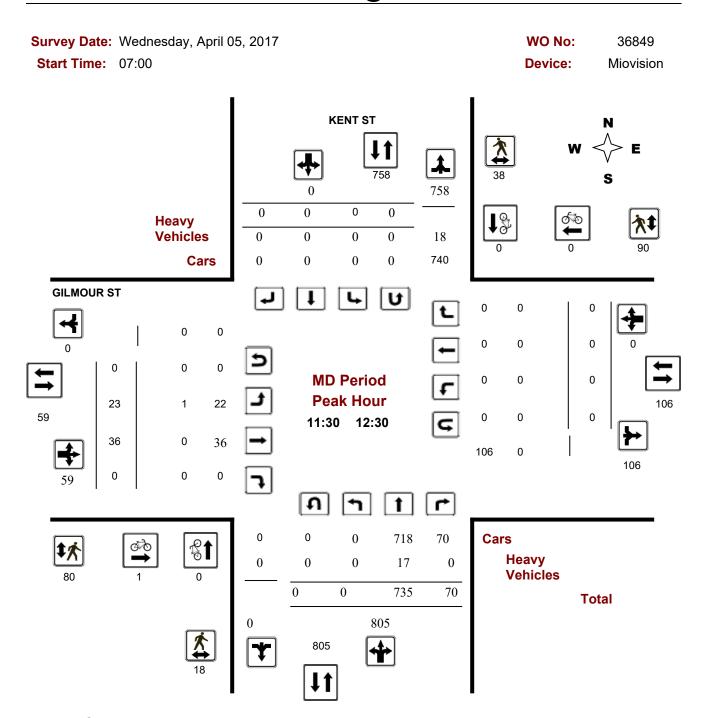
Comments

2021-Jul-09 Page 1 of 3



Turning Movement Count - Peak Hour Diagram

GILMOUR ST @ KENT ST



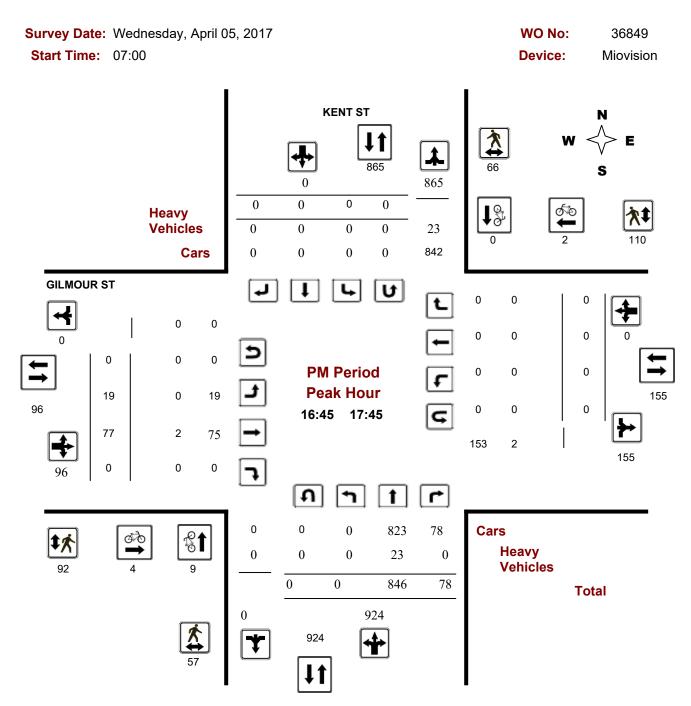
Comments

2021-Jul-09 Page 2 of 3



Turning Movement Count - Peak Hour Diagram

GILMOUR ST @ KENT ST



Comments

2021-Jul-09 Page 3 of 3



Turning Movement Count - Study Results

GILMOUR ST @ KENT ST

Survey Date: Wednesday, April 05, 2017 WO No: 36849

Start Time: 07:00 Device: Miovision

Full Study Summary (8 HR Standard)

Survey Date: Wednesday, April 05, 2017 Total Observed U-Turns AADT Factor

Northbound: 0 Southbound: 0

.90

Eastbound: 0 Westbound: 0

			K	ENT S	Т				GILMOUR ST										
	No	orthbou	ınd		Sou	ıthbou	nd			Е	astbou	ınd		W	estbou	ınd			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	0	1629	91	1720	0	0	0	0	1720	14	41	0	55	0	0	0	0	55	1775
08:00 09:00	0	1767	156	1923	0	0	0	0	1923	19	98	0	117	0	0	0	0	117	2040
09:00 10:00	0	1075	123	1198	0	0	0	0	1198	30	46	0	76	0	0	1	1	77	1275
11:30 12:30	0	735	70	805	0	0	0	0	805	23	36	0	59	0	0	0	0	59	864
12:30 13:30	0	638	86	724	0	0	0	0	724	18	60	0	78	0	0	0	0	78	802
15:00 16:00	0	777	83	860	0	0	0	0	860	24	45	0	69	0	0	0	0	69	929
16:00 17:00	0	792	86	878	0	0	0	0	878	15	60	0	75	0	0	0	0	75	953
17:00 18:00	0	850	69	919	0	0	0	0	919	18	73	0	91	0	0	0	0	91	1010
Sub Total	0	8263	764	9027	0	0	0	0	9027	161	459	0	620	0	0	1	1	621	9648
U Turns	0			0	0			0	0	0			0	0			0	0	0
Total	0	8263	764	9027	0	0	0	0	9027	161	459	0	620	0	0	1	1	621	9648
EQ 12Hr	0	11486	1062	12548	0	0	0	0	12548	224	638	0	862	0	0	1	1	863	13411
Note: These v	alues a	are calcu	ılated b	y multiply	ing the	totals b	y the ap	opropriat	e expans	ion fact	tor.			1.39					
AVG 12Hr	0	10337	956	11293	0	0	0	0	11293	202	574	0	776	0	0	1	1	777	12070
Note: These v	olumes	s are cal	culated	by multip	lying th	e Equiv	alent 1	2 hr. tota	als by the	AADT 1	factor.			.90					
AVG 24Hr	0	13541	1252	14793	0	0	0	0	14793	265	752	0	1017	0	0	1	1	1018	15811
Note: These v	olumes	s are cal	culated	by multip	olying th	e Avera	ige Dail	y 12 hr.	totals by	12 to 2	4 expan	sion fac	tor.	1.31					

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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Turning Movement Count - Study Results

GILMOUR ST @ KENT ST

Survey Date: Wednesday, April 05, 2017 WO No: 36849

Start Time: 07:00 Device: Miovision

Full Study 15 Minute Increments

KENT ST GILMOUR ST

		N	orthbou	ınd		So	uthbou	nd		Eastbound			Westbound							
Time Pe	eriod	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 0	07:15	0	361	16	377	0	0	0	0	377	2	8	0	10	0	0	0	0	10	387
07:15 0	07:30	0	394	21	415	0	0	0	0	415	3	10	0	13	0	0	0	0	13	428
07:30 0	07:45	0	409	23	432	0	0	0	0	432	3	10	0	13	0	0	0	0	13	445
07:45 0	08:00	0	465	31	496	0	0	0	0	496	6	13	0	19	0	0	0	0	19	515
08:00	08:15	0	430	43	473	0	0	0	0	473	7	30	0	37	0	0	0	0	37	510
08:15 0	08:30	0	480	43	523	0	0	0	0	523	3	19	0	22	0	0	0	0	22	545
08:30 0	08:45	0	437	37	474	0	0	0	0	474	1	23	0	24	0	0	0	0	24	498
08:45 0	09:00	0	420	33	453	0	0	0	0	453	8	26	0	34	0	0	0	0	34	487
09:00 0	09:15	0	351	26	377	0	0	0	0	377	7	17	0	24	0	0	0	0	24	401
09:15 0	09:30	0	276	30	306	0	0	0	0	306	8	12	0	20	0	0	0	0	20	326
09:30 0	09:45	0	248	30	278	0	0	0	0	278	8	12	0	20	0	0	1	1	21	299
09:45 1	10:00	0	200	37	237	0	0	0	0	237	7	5	0	12	0	0	0	0	12	249
11:30 1	11:45	0	179	23	202	0	0	0	0	202	4	5	0	9	0	0	0	0	9	211
11:45 1	12:00	0	220	21	241	0	0	0	0	241	6	14	0	20	0	0	0	0	20	261
12:00 1	12:15	0	165	16	181	0	0	0	0	181	5	9	0	14	0	0	0	0	14	195
12:15 1	12:30	0	171	10	181	0	0	0	0	181	8	8	0	16	0	0	0	0	16	197
12:30 1	12:45	0	169	12	181	0	0	0	0	181	4	13	0	17	0	0	0	0	17	198
12:45 1	13:00	0	154	26	180	0	0	0	0	180	4	12	0	16	0	0	0	0	16	196
13:00 1	13:15	0	161	25	186	0	0	0	0	186	6	14	0	20	0	0	0	0	20	206
13:15 1	13:30	0	154	23	177	0	0	0	0	177	4	21	0	25	0	0	0	0	25	202
15:00 1	15:15	0	178	17	195	0	0	0	0	195	7	13	0	20	0	0	0	0	20	215
15:15 1	15:30	0	205	23	228	0	0	0	0	228	5	10	0	15	0	0	0	0	15	243
15:30 1	15:45	0	206	22	228	0	0	0	0	228	4	13	0	17	0	0	0	0	17	245
15:45 1	16:00	0	188	21	209	0	0	0	0	209	8	9	0	17	0	0	0	0	17	226
16:00 1	16:15	0	179	24	203	0	0	0	0	203	5	13	0	18	0	0	0	0	18	221
16:15 1	16:30	0	191	18	209	0	0	0	0	209	4	17	0	21	0	0	0	0	21	230
16:30 1	16:45	0	215	19	234	0	0	0	0	234	2	14	0	16	0	0	0	0	16	250
16:45 1	17:00	0	207	25	232	0	0	0	0	232	4	16	0	20	0	0	0	0	20	252
17:00 1	17:15	0	200	13	213	0	0	0	0	213	6	21	0	27	0	0	0	0	27	240
17:15 1	17:30	0	226	26	252	0	0	0	0	252	7	18	0	25	0	0	0	0	25	277
17:30 1	17:45	0	213	14	227	0	0	0	0	227	2	22	0	24	0	0	0	0	24	251
17:45 1	18:00	0	211	16	227	0	0	0	0	227	3	12	0	15	0	0	0	0	15	242
Total:		0	8263	764	9027	0	0	0	0	9027	161	459	0	620	0	0	1	1	9027	9,648

Note: U-Turns are included in Totals.

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Turning Movement Count - Study Results

GILMOUR ST @ KENT ST

Survey Date: Wednesday, April 05, 2017 WO No: 36849

Start Time: 07:00 Device: Miovision

Full Study Cyclist Volume

KENT ST GILMOUR ST

07:00 07:15 07:15 07:30 07:30 07:45	Northbound 3 2 0	Southbound 0 0	Street Total	Eastbound 0	Westbound	Street Total	Grand Total
07:15 07:30 07:30 07:45	2		3	0			
07:30 07:45		Λ		l 0	0	0	3
	Λ	U	2	1	0	1	3
	U	0	0	1	0	1	1
07:45 08:00	2	0	2	1	0	1	3
08:00 08:15	2	0	2	1	0	1	3
08:15 08:30	3	0	3	1	0	1	4
08:30 08:45	5	0	5	0	0	0	5
08:45 09:00	3	0	3	0	0	0	3
09:00 09:15	1	0	1	2	0	2	3
09:15 09:30	0	0	0	1	0	1	1
09:30 09:45	0	0	0	1	0	1	1
09:45 10:00	2	0	2	1	0	1	3
11:30 11:45	0	0	0	0	0	0	0
11:45 12:00	0	0	0	0	0	0	0
12:00 12:15	0	0	0	1	0	1	1
12:15 12:30	0	0	0	0	0	0	0
12:30 12:45	0	0	0	0	0	0	0
12:45 13:00	2	0	2	1	0	1	3
13:00 13:15	0	0	0	1	0	1	1
13:15 13:30	1	0	1	1	0	1	2
15:00 15:15	1	0	1	0	0	0	1
15:15 15:30	3	0	3	2	0	2	5
15:30 15:45	1	0	1	1	0	1	2
15:45 16:00	0	0	0	1	0	1	1
16:00 16:15	5	0	5	0	0	0	5
16:15 16:30	0	0	0	0	0	0	0
16:30 16:45	3	0	3	0	0	0	3
16:45 17:00	1	0	1	1	0	1	2
17:00 17:15	1	0	1	1	0	1	2
17:15 17:30	5	0	5	0	1	1	6
17:30 17:45	2	0	2	2	1	3	5
17:45 18:00	3	0	3	1	0	1	4
Total	51	0	51	23	2	25	76

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Turning Movement Count - Study Results

GILMOUR ST @ KENT ST

Survey Date: Wednesday, April 05, 2017 WO No: 36849

Start Time: 07:00 Device: Miovision

Full Study Pedestrian Volume

KENT ST GILMOUR ST

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	3	9	12	3	13	16	28
07:15 07:30	5	7	12	9	6	15	27
07:30 07:45	1	5	6	5	18	23	29
07:45 08:00	12	8	20	14	22	36	56
08:00 08:15	10	8	18	21	22	43	61
08:15 08:30	15	7	22	32	24	56	78
08:30 08:45	18	11	29	28	32	60	89
08:45 09:00	18	9	27	17	21	38	65
09:00 09:15	11	6	17	19	17	36	53
09:15 09:30	15	10	25	14	11	25	50
09:30 09:45	9	7	16	17	19	36	52
09:45 10:00	8	13	21	14	20	34	55
11:30 11:45	3	9	12	12	17	29	41
11:45 12:00	4	10	14	22	30	52	66
12:00 12:15	4	9	13	19	23	42	55
12:15 12:30	7	10	17	27	20	47	64
12:30 12:45	11	14	25	27	30	57	82
12:45 13:00	8	13	21	16	23	39	60
13:00 13:15	9	15	24	12	16	28	52
13:15 13:30	9	12	21	8	17	25	46
15:00 15:15	8	9	17	16	20	36	53
15:15 15:30	13	20	33	18	18	36	69
15:30 15:45	13	13	26	25	24	49	75
15:45 16:00	9	17	26	13	31	44	70
16:00 16:15	6	11	17	14	22	36	53
16:15 16:30	8	16	24	17	28	45	69
16:30 16:45	11	17	28	28	31	59	87
16:45 17:00	15	21	36	23	45	68	104
17:00 17:15	15	17	32	18	29	47	79
17:15 17:30	20	11	31	28	19	47	78
17:30 17:45	7	17	24	23	17	40	64
17:45 18:00	8	6	14	15	14	29	43
Total	313	367	680	574	699	1273	1953

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Turning Movement Count - Study Results

GILMOUR ST @ KENT ST

Survey Date: Wednesday, April 05, 2017 WO No: 36849

Start Time: 07:00 Device: Miovision

Full Study Heavy Vehicles

KENT ST GILMOUR ST

		No	orthbou	ınd		Sc	uthbou	ınd		Eastbound				Westbound						
Time Pe	eriod	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 0	7:15	0	3	1	4	0	0	0	0	4	0	0	0	0	0	0	0	0	0	4
07:15 0	07:30	0	7	1	8	0	0	0	0	8	0	0	0	0	0	0	0	0	0	8
07:30 0	7:45	0	6	0	6	0	0	0	0	6	0	0	0	0	0	0	0	0	0	6
07:45 0	00:80	0	6	0	6	0	0	0	0	6	0	0	0	0	0	0	0	0	0	6
08:00	08:15	0	5	0	5	0	0	0	0	5	0	0	0	0	0	0	0	0	0	5
08:15 0	08:30	0	4	1	5	0	0	0	0	5	0	0	0	0	0	0	0	0	0	5
08:30 0	08:45	0	7	1	8	0	0	0	0	8	0	0	0	0	0	0	0	0	0	8
08:45 0	09:00	0	8	0	8	0	0	0	0	8	0	1	0	1	0	0	0	0	1	9
09:00 0	09:15	0	8	1	9	0	0	0	0	9	0	0	0	0	0	0	0	0	0	9
09:15 0	09:30	0	5	0	5	0	0	0	0	5	0	0	0	0	0	0	0	0	0	5
09:30 0	9:45	0	10	3	13	0	0	0	0	13	0	0	0	0	0	0	0	0	0	13
09:45 1	10:00	0	5	1	6	0	0	0	0	6	1	0	0	1	0	0	0	0	1	7
11:30 1	11:45	0	6	0	6	0	0	0	0	6	0	0	0	0	0	0	0	0	0	6
11:45 1	12:00	0	6	0	6	0	0	0	0	6	0	0	0	0	0	0	0	0	0	6
12:00 1	12:15	0	3	0	3	0	0	0	0	3	0	0	0	0	0	0	0	0	0	3
12:15 1	12:30	0	2	0	2	0	0	0	0	2	1	0	0	1	0	0	0	0	1	3
12:30 1	12:45	0	3	0	3	0	0	0	0	3	0	0	0	0	0	0	0	0	0	3
12:45 1	13:00	0	4	0	4	0	0	0	0	4	0	0	0	0	0	0	0	0	0	4
13:00 1	13:15	0	5	0	5	0	0	0	0	5	0	1	0	1	0	0	0	0	1	6
13:15 1	13:30	0	5	1	6	0	0	0	0	6	0	0	0	0	0	0	0	0	0	6
	15:15	0	6	2	8	0	0	0	0	8	1	1	0	2	0	0	0	0	2	10
15:15 1	15:30	0	12	0	12	0	0	0	0	12	0	1	0	1	0	0	0	0	1	13
15:30 1	15:45	0	12	0	12	0	0	0	0	12	0	0	0	0	0	0	0	0	0	12
15:45 1	16:00	0	3	2	5	0	0	0	0	5	1	1	0	2	0	0	0	0	2	7
	16:15	0	5	2	7	0	0	0	0	7	0	0	0	0	0	0	0	0	0	7
	16:30	0	11	0	11	0	0	0	0	11	0	1	0	1	0	0	0	0	1	12
16:30 1	16:45	0	6	1	7	0	0	0	0	7	0	0	0	0	0	0	0	0	0	7
16:45 1	17:00	0	8	0	8	0	0	0	0	8	0	0	0	0	0	0	0	0	0	8
17:00 1	17:15	0	5	0	5	0	0	0	0	5	0	1	0	1	0	0	0	0	1	6
	17:30	0	5	0	5	0	0	0	0	5	0	0	0	0	0	0	0	0	0	5
	17:45	0	5	0	5	0	0	0	0	5	0	1	0	1	0	0	0	0	1	6
17:45 1	18:00	0	3	0	3	0	0	0	0	3	0	0	0	0	0	0	0	0	0	3
Total: N	None	0	189	17	206	0	0	0	0	206	4	8	0	12	0	0	0	0	12	218

July 9, 2021 Page 7 of 8



Turning Movement Count - Study Results

GILMOUR ST @ KENT ST

Survey Date: Wednesday, April 05, 2017 WO No: 36849

Start Time: 07:00 Device: Miovision

Full Study 15 Minute U-Turn Total KENT ST GILMOUR ST

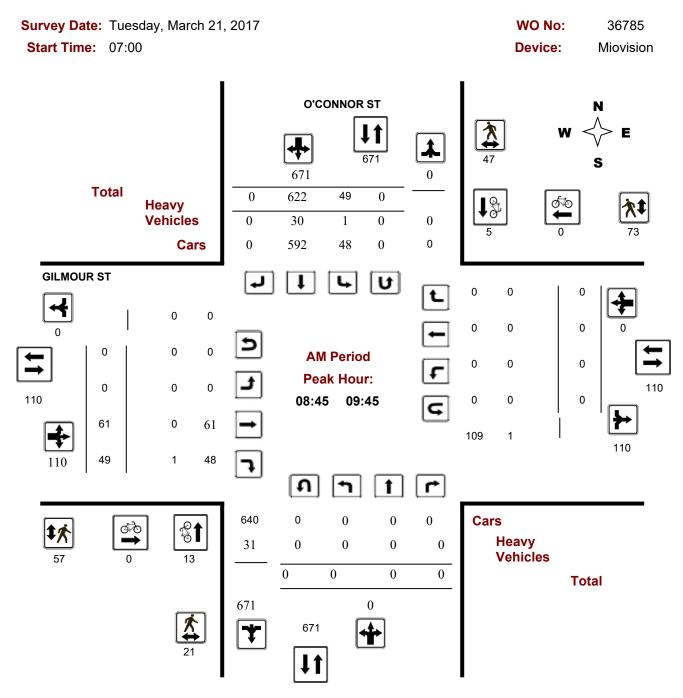
Time F	eriod	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
То	tal	0	0	0	0	0

July 9, 2021 Page 8 of 8



Turning Movement Count - Full Study Peak Hour Diagram

GILMOUR ST @ O'CONNOR ST



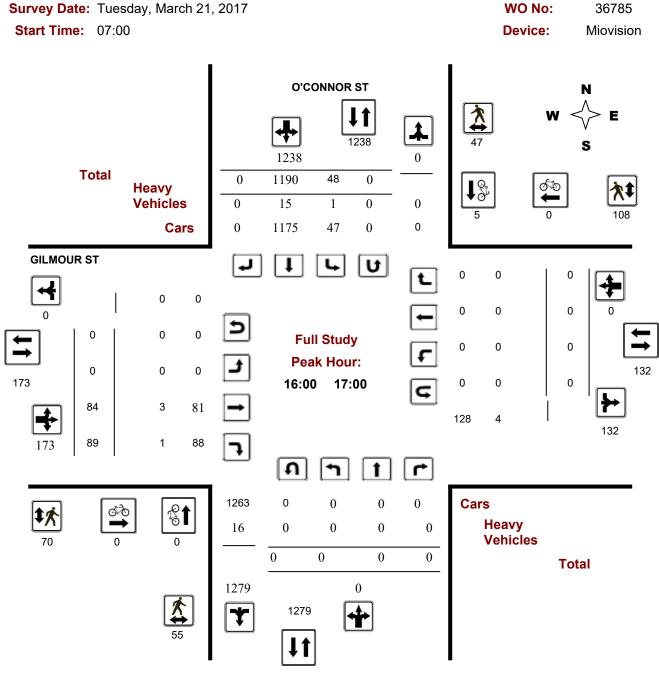
Comments

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Turning Movement Count - Full Study Peak Hour Diagram

GILMOUR ST @ O'CONNOR ST



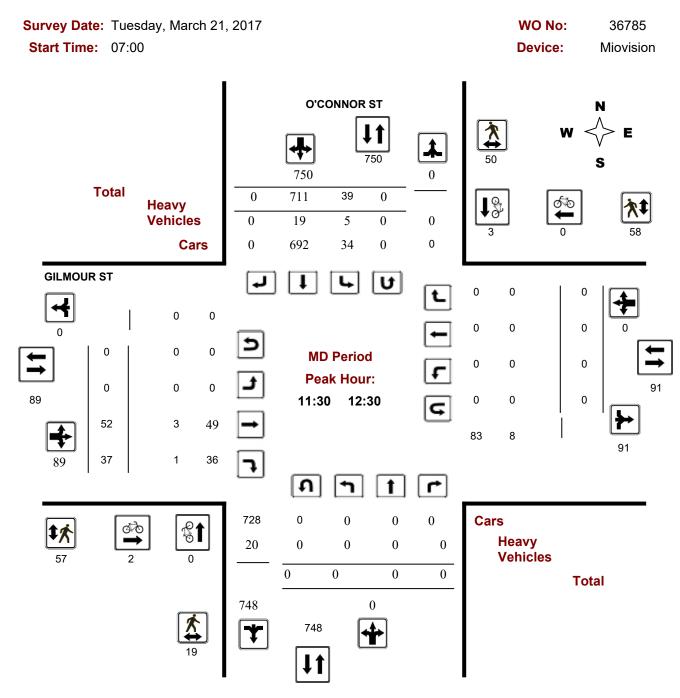
Comments

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Turning Movement Count - Full Study Peak Hour Diagram

GILMOUR ST @ O'CONNOR ST



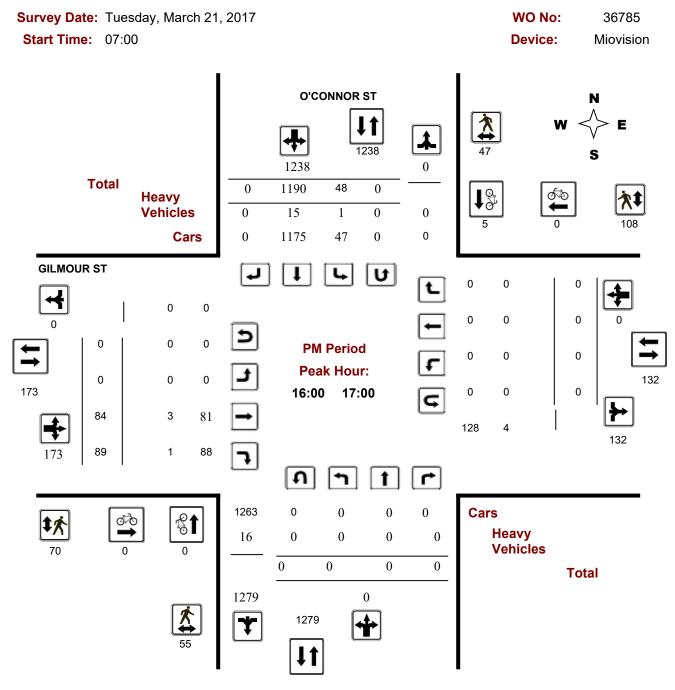
Comments

2019-Jun-13 Page 3 of 4



Turning Movement Count - Full Study Peak Hour Diagram

GILMOUR ST @ O'CONNOR ST



Comments

2019-Jun-13 Page 4 of 4

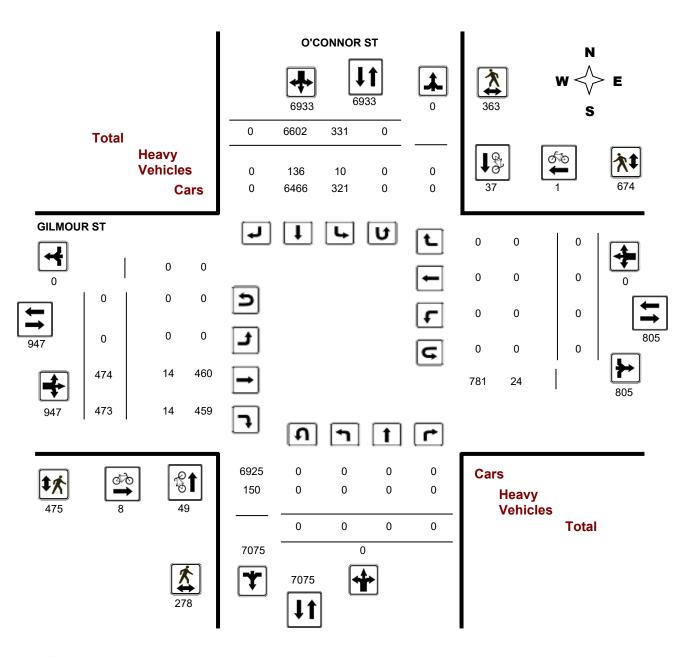


Turning Movement Count - Full Study Diagram

GILMOUR ST @ O'CONNOR ST

Survey Date: Tuesday, March 21, 2017 WO#: 36785

Device: Miovision



Comments

2019-Jun-13 Page 1 of 1



Work Order

36785

Turning Movement Count - Full Study Summary Report

GILMOUR ST @ O'CONNOR ST

Survey Date: Tuesday, March 21, 2017

Total Observed U-Turns

AADT Factor

Northbound: 0

Southbound: 0

0

1.00

Eastbound:

Westbound:

Full Study

			0'0	CONN	OR ST	Γ				•		G	ILMOL	JR ST					
	N	orthbo	ound		9	Southbo	ound		_		Eastbo	ound		٧	Vestbo	ound			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	0	0	0	0	39	555	0	594	594	0	39	26	65	0	0	0	0	65	659
08:00 09:00	0	0	0	0	45	601	0	646	646	0	79	43	122	0	0	0	0	122	768
09:00 10:00	0	0	0	0	51	619	0	670	670	0	52	41	93	0	0	0	0	93	763
11:30 12:30	0	0	0	0	39	711	0	750	750	0	52	37	89	0	0	0	0	89	839
12:30 13:30	0	0	0	0	25	615	0	640	640	0	41	50	91	0	0	0	0	91	731
15:00 16:00	0	0	0	0	38	1192	0	1230	1230	0	54	96	150	0	0	0	0	150	1380
16:00 17:00	0	0	0	0	48	1190	0	1238	1238	0	84	89	173	0	0	0	0	173	1411
17:00 18:00	0	0	0	0	46	1119	0	1165	1165	0	73	91	164	0	0	0	0	164	1329
Sub Total	0	0	0	0	331	6602	0	6933	6933	0	474	473	947	0	0	0	0	947	7880
U Turns				0				0	0				0				0	0	0
Total	0	0	0	0	331	6602	0	6933	6933	0	474	473	947	0	0	0	0	947	7880
EQ 12Hr	0	0	0	0	460	9177	0	9637	9637	0	659	657	1316	0	0	0	0	1316	10953
Note: These va	alues ar	e calcul	ated by	/ multiply	ing the	totals by	/ the ap	opropriat	te expansi	on fact	or.		1	.39					
AVG 12Hr	0	0	0	0	460	9177	0	9637	9637	0	659	657	1316	0	0	0	0	1316	10953
Note: These ve	olumes a	are calc	ulated	by multip	olying t	ne Equiva	alent 12	2 hr. tota	als by the	AADT f	actor.		1	.00					
AVG 24Hr	0	0	0	0	603	12022	0	12624	12624	0	863	861	1724	0	0	0	0	1724	14348
Note: These vo	olumes a	are calc	ulated	by multip	olying tl	ne Avera	ge Dail	y 12 hr.	totals by	12 to 24	l expan	sion fac	tor. 1	.31					

Comments:

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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36785

Turning Movement Count - 15 Minute Summary Report

GILMOUR ST @ O'CONNOR ST

Tuesday, March 21, 2017 **Survey Date:**

Total Observed U-Turns

Northbound: 0 Eastbound: 0

0 Westbound: 0

O'CONNOR ST

GILMOUR ST

Southbound:

					ININUI									IOUK .						
		No	orthbou	ınd		So	uthboun	d	_		Eas	stbound		_	Wes	stbound	d			
Time Per	riod	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 0	7:15	0	0	0	0	8	103	0	111	111	0	1	2	3	0	0	0	0	3	114
07:15 0	7:30	0	0	0	0	12	129	0	141	141	0	9	3	12	0	0	0	0	12	153
07:30 0	7:45	0	0	0	0	10	162	0	172	172	0	15	12	27	0	0	0	0	27	199
07:45 08	00:8	0	0	0	0	9	161	0	170	170	0	14	9	23	0	0	0	0	23	193
08:00 08	8:15	0	0	0	0	14	156	0	170	170	0	24	9	33	0	0	0	0	33	203
08:15 0	8:30	0	0	0	0	13	127	0	140	140	0	19	10	29	0	0	0	0	29	169
08:30 08	8:45	0	0	0	0	7	159	0	166	166	0	14	7	21	0	0	0	0	21	187
08:45 0	9:00	0	0	0	0	11	159	0	170	170	0	22	17	39	0	0	0	0	39	209
09:00 09	9:15	0	0	0	0	13	158	0	171	171	0	15	9	24	0	0	0	0	24	195
09:15 0	9:30	0	0	0	0	11	137	0	148	148	0	20	11	31	0	0	0	0	31	179
09:30 09	9:45	0	0	0	0	14	168	0	182	182	0	4	12	16	0	0	0	0	16	198
09:45 10	0:00	0	0	0	0	13	156	0	169	169	0	13	9	22	0	0	0	0	22	191
11:30 1	1:45	0	0	0	0	6	184	0	190	190	0	13	5	18	0	0	0	0	18	208
11:45 1:	2:00	0	0	0	0	17	178	0	195	195	0	9	11	20	0	0	0	0	20	215
12:00 12	2:15	0	0	0	0	8	179	0	187	187	0	19	9	28	0	0	0	0	28	215
12:15 1	2:30	0	0	0	0	8	170	0	178	178	0	11	12	23	0	0	0	0	23	201
12:30 12	2:45	0	0	0	0	7	152	0	159	159	0	11	15	26	0	0	0	0	26	185
12:45 1	3:00	0	0	0	0	10	160	0	170	170	0	11	15	26	0	0	0	0	26	196
13:00 1	3:15	0	0	0	0	3	154	0	157	157	0	7	9	16	0	0	0	0	16	173
13:15 1	3:30	0	0	0	0	5	149	0	154	154	0	12	11	23	0	0	0	0	23	177
15:00 1	5:15	0	0	0	0	5	335	0	340	340	0	19	30	49	0	0	0	0	49	389
15:15 1	5:30	0	0	0	0	14	288	0	302	302	0	14	20	34	0	0	0	0	34	336
15:30 1	5:45	0	0	0	0	7	277	0	284	284	0	10	23	33	0	0	0	0	33	317
15:45 10	6:00	0	0	0	0	12	292	0	304	304	0	11	23	34	0	0	0	0	34	338
16:00 10	6:15	0	0	0	0	16	298	0	314	314	0	22	29	51	0	0	0	0	51	365
16:15 10	6:30	0	0	0	0	13	305	0	318	318	0	18	22	40	0	0	0	0	40	358
16:30 10	6:45	0	0	0	0	7	291	0	298	298	0	19	14	33	0	0	0	0	33	331
16:45 1	7:00	0	0	0	0	12	296	0	308	308	0	25	24	49	0	0	0	0	49	357
17:00 1	7:15	0	0	0	0	16	303	0	319	319	0	21	24	45	0	0	0	0	45	364
17:15 1	7:30	0	0	0	0	11	288	0	299	299	0	28	23	51	0	0	0	0	51	350
17:30 1	7:45	0	0	0	0	13	285	0	298	298	0	13	23	36	0	0	0	0	36	334
17:45 18	8:00	0	0	0	0	6	243	0	249	249	0	11	21	32	0	0	0	0	32	281
TOTAL:		0	0	0	0	331	6602	0	6933	6933	0	474	473	947	0	0	0	0	947	7880

Note: U-Turns are included in Totals.

Comment:

2019-Jun-13 Page 1 of 1



Turning Movement Count - Cyclist Volume Report

Work Order 36785

GILMOUR ST @ O'CONNOR ST

Count Date: Tuesday, March 21, 2017 Start Time: 07:00

		O'CONNOR ST			GILMOUR ST		
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 08:00	5	0	5	1	0	1	6
08:00 09:00	26	1	27	2	0	2	29
09:00 10:00	8	5	13	0	0	0	13
11:30 12:30	0	3	3	2	0	2	5
12:30 13:30	3	0	3	1	1	2	5
15:00 16:00	5	2	7	0	0	0	7
16:00 17:00	0	5	5	0	0	0	5
17:00 18:00	2	21	23	2	0	2	25
T-4-1	40	27	0.0	0	4	•	0.5

Comment:

Note: These volumes consists of bicycles only (no mopeds or motorcycles) and ARE NOT included in the Turning Movement Count Summary.

2019-Jun-13 Page 1 of 1



Total

Transportation Services - Traffic Services

W.O. 36785

Turning Movement Count - Heavy Vehicle Report

GILMOUR ST @ O'CONNOR ST

Survey Date: Tuesday, March 21, 2017

0

10

136

			0'0	CONN	IOR S	Τ						G	ILMO	UR ST	Г					
	-	Northb	ound			Southb	ound	_			Eastb	ound		,	Westbo	ound				
Time I	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	08:00	0	0	0	0	0	18	0	18	18	0	0	0	0	0	0	0	0	0	18
08:00	09:00	0	0	0	0	1	22	0	23	23	0	2	1	3	0	0	0	0	3	26
09:00	10:00	0	0	0	0	0	29	0	29	29	0	0	1	1	0	0	0	0	1	30
11:30	12:30	0	0	0	0	5	19	0	24	24	0	3	1	4	0	0	0	0	4	28
12:30	13:30	0	0	0	0	2	16	0	18	18	0	4	3	7	0	0	0	0	7	25
15:00	16:00	0	0	0	0	0	8	0	8	8	0	1	3	4	0	0	0	0	4	12
16:00	17:00	0	0	0	0	1	15	0	16	16	0	3	1	4	0	0	0	0	4	20
17:00	18:00	0	0	0	0	1	9	0	10	10	0	1	4	5	0	0	0	0	5	15
Sub	Total	0	0	0	0	10	136	0	146	146	0	14	14	28	0	0	0	0	28	174
U-Turn	s (Heav	y Veh	icles)		0				0	0				0				0	0	0

14

14

28

0

0

28

174

Heavy Vehicles include Buses, Single-Unit Trucks and Articulated Trucks. Further, they ARE included in the Turning Movement Count Summary.

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Work Order

Turning Movement Count - Pedestrian Volume Report

GILMOUR ST @ O'CONNOR ST Count Date: Tuesday, March 21, 2017 **Start Time:** 07:00 NB Approach SB Approach EB Approach WB Approach Time Period **Grand Total** Total **Total** (E or W Crossing) (E or W Crossing) (N or S Crossing) (N or S Crossing) 07:00 07:15 07:15 07:30 07:30 07:45 07:45 08:00 07:00 08:00 08:00 08:15 08:15 08:30 08:30 08:45 08:45 09:00 08:00 09:00 09:00 09:15 09:15 09:30 09:30 09:45 09:45 10:00 09:00 10:00 11:30 11:45 11:45 12:00 12:00 12:15 12:15 12:30 11:30 12:30 12:30 12:45 12:45 13:00 13:00 13:15 13:15 13:30 12:30 13:30 15:00 15:15 15:15 15:30 15:30 15:45 15:45 16:00 15:00 16:00 16:00 16:15 16:15 16:30 16:30 16:45 16:45 17:00 16:00 17:00 17:00 17:15 17:15 17:30 17:30 17:45 17:45 18:00 17:00 18:00

Comment:

Total

2019-Jun-13 Page 1 of 1







Turning Movement Count - 15 Min U-Turn Total Report

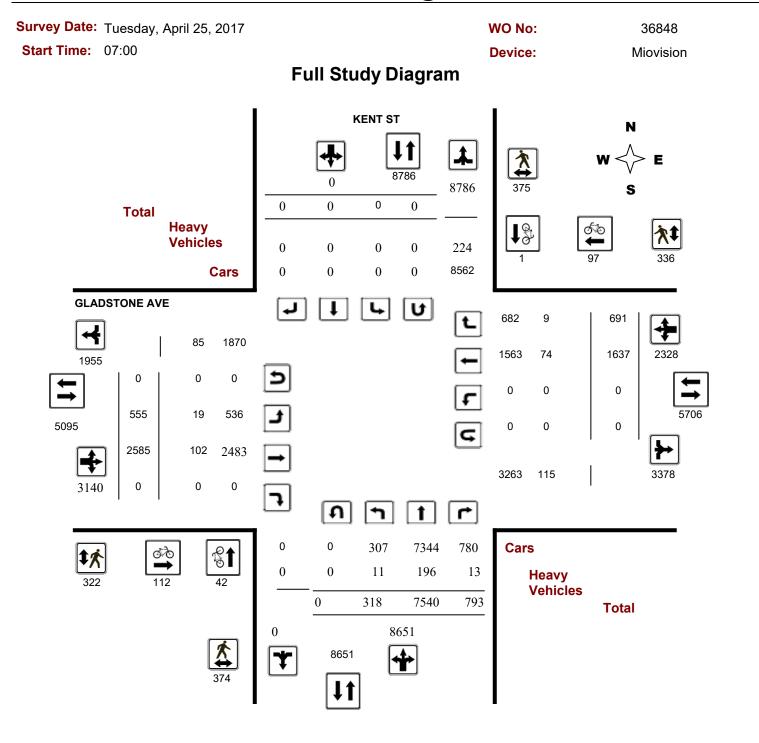
GILMOUR ST @ O'CONNOR ST

Survey Date:	Τι	uesday, March 21	, 2017			
Time Pe	riod	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
Tota	1	0	0	0	0	0



Turning Movement Count - Study Results

GLADSTONE AVE @ KENT ST



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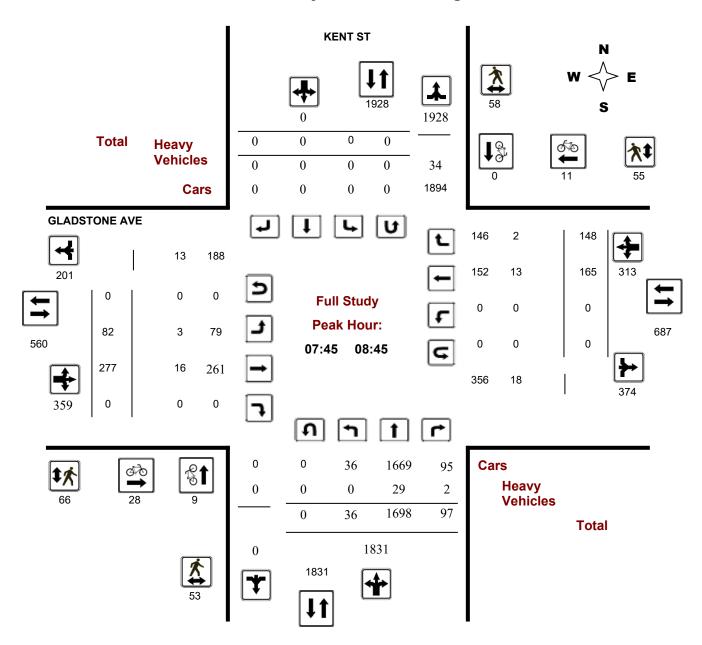
Turning Movement Count - Study Results

GLADSTONE AVE @ KENT ST

Survey Date: Tuesday, April 25, 2017 WO No: 36848

Start Time: 07:00 Device: Miovision

Full Study Peak Hour Diagram

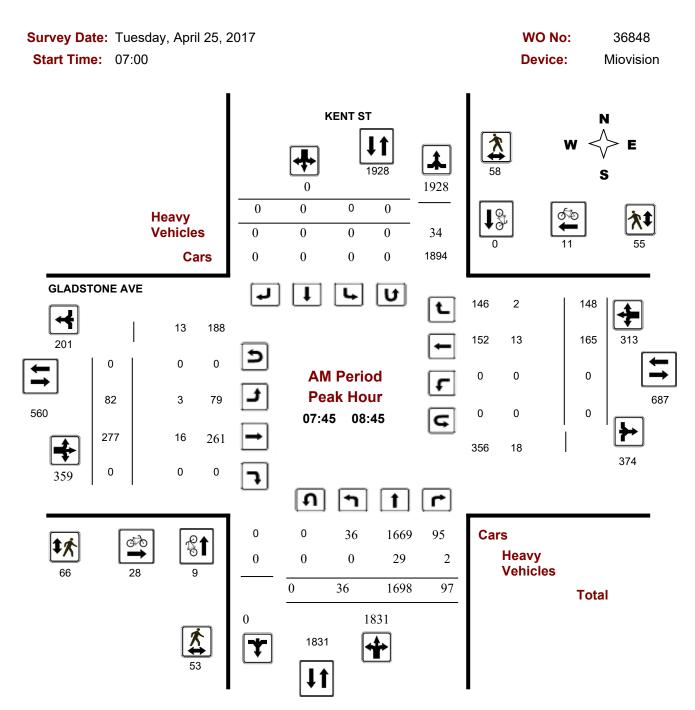


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Turning Movement Count - Peak Hour Diagram

GLADSTONE AVE @ KENT ST



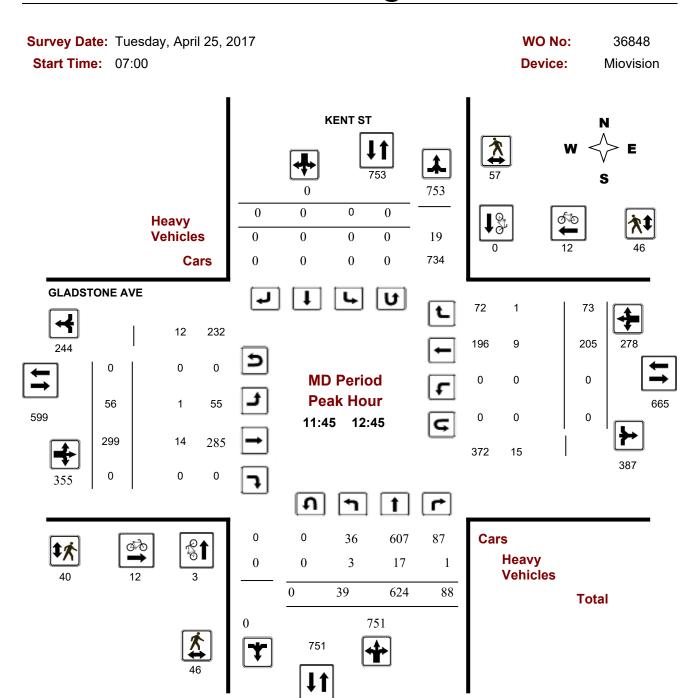
Comments

2021-Jul-09 Page 1 of 3



Turning Movement Count - Peak Hour Diagram

GLADSTONE AVE @ KENT ST



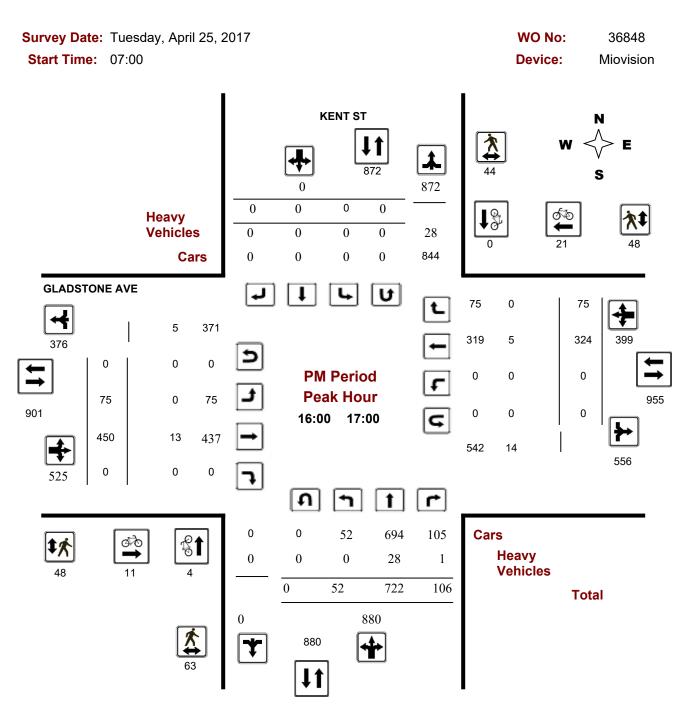
Comments

2021-Jul-09 Page 2 of 3



Turning Movement Count - Peak Hour Diagram

GLADSTONE AVE @ KENT ST



Comments

2021-Jul-09 Page 3 of 3



Turning Movement Count - Study Results

GLADSTONE AVE @ KENT ST

Survey Date: Tuesday, April 25, 2017 WO No: 36848

Start Time: 07:00 Device: Miovision

Full Study Summary (8 HR Standard)

Survey Date: Tuesday, April 25, 2017 Total Observed U-Turns AADT Factor

Northbound: 0 Southbound: 0 .90

Eastbound: 0 Westbound: 0

			K	ENT S	Т							GLAD	OTEC	NE AVI	Ξ				
	No	orthbou	ınd		Sou	ıthbou	nd			Е	astbou	ınd		٧	/estbo	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	14	1536	96	1646	0	0	0	0	1646	61	214	0	275	0	100	104	204	479	2125
08:00 09:00	42	1679	98	1819	0	0	0	0	1819	82	291	0	373	0	172	130	302	675	2494
09:00 10:00	34	943	107	1084	0	0	0	0	1084	76	256	0	332	0	173	76	249	581	1665
11:30 12:30	36	633	80	749	0	0	0	0	749	55	288	0	343	0	182	77	259	602	1351
12:30 13:30	33	569	97	699	0	0	0	0	699	54	321	0	375	0	201	74	275	650	1349
15:00 16:00	47	667	108	822	0	0	0	0	822	66	394	0	460	0	237	72	309	769	1591
16:00 17:00	52	722	106	880	0	0	0	0	880	75	450	0	525	0	324	75	399	924	1804
17:00 18:00	60	791	101	952	0	0	0	0	952	86	371	0	457	0	248	83	331	788	1740
Sub Total	318	7540	793	8651	0	0	0	0	8651	555	2585	0	3140	0	1637	691	2328	5468	14119
U Turns	0			0	0			0	0	0			0	0			0	0	0
Total	318	7540	793	8651	0	0	0	0	8651	555	2585	0	3140	0	1637	691	2328	5468	14119
EQ 12Hr	442	10481	1102	12025	0	0	0	0	12025	771	3593	0	4364	0	2275	960	3235	7599	19624
Note: These	values a	are calcu	ılated b	y multiply	ing the	totals b	y the a	opropriat	te expans	ion fac	tor.			1.39					
AVG 12Hr	398	9433	992	10823	0	0	0	0	10823	694	3234	0	3928	0	2048	864	2912	6840	17663
Note: These	volume	s are cal	culated	by multip	olying th	e Equiv	alent 1	2 hr. tota	als by the	AADT	factor.			.90					
AVG 24Hr	521	12357	1300	14178	0	0	0	0	14178	909	4237	0	5146	0	2683	1132	3815	8961	23139
Note: These	volume	s are cal	culated	by multip	olying th	e Avera	ige Dail	ly 12 hr.	totals by	12 to 2	4 expan	sion fac	ctor.	1.31					

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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Turning Movement Count - Study Results

GLADSTONE AVE @ KENT ST

Survey Date: Tuesday, April 25, 2017 WO No: 36848

Start Time: 07:00 Device: Miovision

KENT ST

Full Study 15 Minute Increments GLADSTONE AVE

		No	orthbou	ınd		Sc	uthbou	ınd			Е	astbour	nd		W	estbour	nd			
Time I	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	1	319	24	344	0	0	0	0	344	17	51	0	68	0	20	10	30	98	442
07:15	07:30	2	382	24	408	0	0	0	0	408	13	40	0	53	0	18	21	39	92	500
07:30	07:45	6	402	25	433	0	0	0	0	433	15	64	0	79	0	22	31	53	132	565
07:45	08:00	5	433	23	461	0	0	0	0	461	16	59	0	75	0	40	42	82	157	618
08:00	08:15	8	428	33	469	0	0	0	0	469	22	79	0	101	0	31	42	73	174	643
08:15	08:30	7	435	19	461	0	0	0	0	461	22	70	0	92	0	50	28	78	170	631
08:30	08:45	16	402	22	440	0	0	0	0	440	22	69	0	91	0	44	36	80	171	611
08:45	09:00	11	414	24	449	0	0	0	0	449	16	73	0	89	0	47	24	71	160	609
09:00	09:15	6	319	28	353	0	0	0	0	353	17	66	0	83	0	51	31	82	165	518
09:15	09:30	14	243	26	283	0	0	0	0	283	27	66	0	93	0	33	17	50	143	426
09:30	09:45	4	177	29	210	0	0	0	0	210	15	61	0	76	0	41	15	56	132	342
09:45	10:00	10	204	24	238	0	0	0	0	238	17	63	0	80	0	48	13	61	141	379
11:30	11:45	6	158	12	176	0	0	0	0	176	14	67	0	81	0	33	18	51	132	308
11:45	12:00	8	167	28	203	0	0	0	0	203	17	82	0	99	0	55	25	80	179	382
12:00	12:15	14	153	25	192	0	0	0	0	192	13	65	0	78	0	44	19	63	141	333
12:15	12:30	8	155	15	178	0	0	0	0	178	11	74	0	85	0	50	15	65	150	328
12:30	12:45	9	149	20	178	0	0	0	0	178	15	78	0	93	0	56	14	70	163	341
12:45	13:00	6	153	19	178	0	0	0	0	178	12	66	0	78	0	47	22	69	147	325
13:00	13:15	15	141	32	188	0	0	0	0	188	16	96	0	112	0	48	19	67	179	367
13:15	13:30	3	126	26	155	0	0	0	0	155	11	81	0	92	0	50	19	69	161	316
15:00	15:15	11	154	15	180	0	0	0	0	180	20	85	0	105	0	59	19	78	183	363
15:15	15:30	14	157	28	199	0	0	0	0	199	18	102	0	120	0	56	21	77	197	396
15:30	15:45	9	167	21	197	0	0	0	0	197	16	107	0	123	0	65	14	79	202	399
15:45	16:00	13	189	44	246	0	0	0	0	246	12	100	0	112	0	57	18	75	187	433
16:00	16:15	14	172	23	209	0	0	0	0	209	14	118	0	132	0	86	25	111	243	452
16:15	16:30	9	195	28	232	0	0	0	0	232	24	111	0	135	0	71	19	90	225	457
16:30	16:45	16	167	18	201	0	0	0	0	201	19	116	0	135	0	89	18	107	242	443
16:45	17:00	13	188	37	238	0	0	0	0	238	18	105	0	123	0	78	13	91	214	452
17:00	17:15	19	176	29	224	0	0	0	0	224	20	99	0	119	0	69	17	86	205	429
17:15	17:30	14	201	27	242	0	0	0	0	242	26	109	0	135	0	53	25	78	213	455
17:30	17:45	15	215	26	256	0	0	0	0	256	25	93	0	118	0	69	21	90	208	464
17:45	18:00	12	199	19	230	0	0	0	0	230	15	70	0	85	0	57	20	77	162	392
Total:		318	7540	793	8651	0	0	0	0	8651	555	2585	0	3140	0	1637	691	2328	8651	14,119

Note: U-Turns are included in Totals.

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Turning Movement Count - Study Results

GLADSTONE AVE @ KENT ST

Survey Date: Tuesday, April 25, 2017 WO No: 36848

Start Time: 07:00 Device: Miovision

Full Study Cyclist Volume

KENT ST GLADSTONE AVE

		IXEIVI OI			OLADO I ONL A	- - -	=
Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	Grand Total
07:00 07:15	1	0	1	2	3	5	6
07:15 07:30	1	0	1	3	2	5	6
07:30 07:45	0	0	0	4	5	9	9
07:45 08:00	3	0	3	6	2	8	11
08:00 08:15	2	0	2	9	3	12	14
08:15 08:30	4	0	4	6	1	7	11
08:30 08:45	0	0	0	7	5	12	12
08:45 09:00	7	0	7	7	4	11	18
09:00 09:15	0	0	0	7	1	8	8
09:15 09:30	7	0	7	1	2	3	10
09:30 09:45	1	0	1	2	1	3	4
09:45 10:00	0	0	0	3	2	5	5
11:30 11:45	0	0	0	3	3	6	6
11:45 12:00	1	0	1	1	4	5	6
12:00 12:15	0	0	0	6	2	8	8
12:15 12:30	1	0	1	2	2	4	5
12:30 12:45	1	0	1	3	4	7	8
12:45 13:00	3	0	3	2	7	9	12
13:00 13:15	1	0	1	3	1	4	5
13:15 13:30	3	0	3	0	1	1	4
15:00 15:15	0	0	0	4	1	5	5
15:15 15:30	0	1	1	1	0	1	2
15:30 15:45	0	0	0	3	2	5	5
15:45 16:00	0	0	0	0	2	2	2
16:00 16:15	0	0	0	4	2	6	6
16:15 16:30	3	0	3	1	7	8	11
16:30 16:45	0	0	0	2	7	9	9
16:45 17:00	1	0	1	4	5	9	10
17:00 17:15	0	0	0	6	5	11	11
17:15 17:30	1	0	1	4	7	11	12
17:30 17:45	0	0	0	3	3	6	6
17:45 18:00	1	0	1	3	1	4	5
Total	42	1	43	112	97	209	252

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Turning Movement Count - Study Results

GLADSTONE AVE @ KENT ST

Survey Date: Tuesday, April 25, 2017 WO No: 36848

Start Time: 07:00 Device: Miovision

Full Study Pedestrian Volume

KENT ST GLADSTONE AVE

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	2	4	6	3	4	7	13
07:15 07:30	3	9	12	9	5	14	26
07:30 07:45	8	5	13	16	5	21	34
07:45 08:00	9	8	17	13	8	21	38
08:00 08:15	10	11	21	18	12	30	51
08:15 08:30	17	24	41	21	17	38	79
08:30 08:45	17	15	32	14	18	32	64
08:45 09:00	12	10	22	13	10	23	45
09:00 09:15	6	11	17	7	14	21	38
09:15 09:30	6	6	12	8	6	14	26
09:30 09:45	6	13	19	4	4	8	27
09:45 10:00	6	15	21	8	7	15	36
11:30 11:45	9	6	15	11	5	16	31
11:45 12:00	5	13	18	4	8	12	30
12:00 12:15	13	16	29	8	12	20	49
12:15 12:30	12	13	25	14	17	31	56
12:30 12:45	16	15	31	14	9	23	54
12:45 13:00	14	10	24	3	9	12	36
13:00 13:15	15	16	31	4	13	17	48
13:15 13:30	12	16	28	6	8	14	42
15:00 15:15	10	14	24	5	12	17	41
15:15 15:30	12	6	18	17	15	32	50
15:30 15:45	13	11	24	2	16	18	42
15:45 16:00	12	9	21	5	11	16	37
16:00 16:15	12	11	23	9	10	19	42
16:15 16:30	17	12	29	17	14	31	60
16:30 16:45	18	10	28	9	12	21	49
16:45 17:00	16	11	27	13	12	25	52
17:00 17:15	23	13	36	13	13	26	62
17:15 17:30	8	17	25	13	14	27	52
17:30 17:45	20	12	32	9	10	19	51
17:45 18:00	15	13	28	12	6	18	46
Total	374	375	749	322	336	658	1407

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Turning Movement Count - Study Results

GLADSTONE AVE @ KENT ST

Survey Date: Tuesday, April 25, 2017 WO No: 36848

Start Time: 07:00 Device: Miovision

Full Study Heavy Vehicles

KENT ST GLADSTONE AVE

		No	orthbou	und		Sc	uthbou	ınd			Е	astbour	nd		We	estbour	nd			
Time Per	eriod	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 0	7:15	0	5	0	5	0	0	0	0	5	3	2	0	5	0	2	0	2	7	12
07:15 0	7:30	1	3	1	5	0	0	0	0	5	1	2	0	3	0	1	1	2	5	10
07:30 0	7:45	1	4	0	5	0	0	0	0	5	1	5	0	6	0	5	0	5	11	16
07:45 08	00:80	0	5	1	6	0	0	0	0	6	0	3	0	3	0	3	0	3	6	12
08:00	8:15	0	7	1	8	0	0	0	0	8	3	4	0	7	0	2	2	4	11	19
08:15 08	8:30	0	10	0	10	0	0	0	0	10	0	3	0	3	0	4	0	4	7	17
08:30 08	8:45	0	7	0	7	0	0	0	0	7	0	6	0	6	0	4	0	4	10	17
08:45 09	9:00	2	7	0	9	0	0	0	0	9	2	3	0	5	0	0	0	0	5	14
09:00	9:15	2	5	3	10	0	0	0	0	10	0	5	0	5	0	3	2	5	10	20
09:15 09	9:30	0	4	1	5	0	0	0	0	5	0	2	0	2	0	3	0	3	5	10
09:30 09	9:45	0	9	1	10	0	0	0	0	10	1	2	0	3	0	2	0	2	5	15
09:45 10	0:00	0	6	0	6	0	0	0	0	6	1	3	0	4	0	7	0	7	11	17
11:30 1	1:45	0	5	0	5	0	0	0	0	5	1	2	0	3	0	2	0	2	5	10
11:45 12	2:00	1	7	1	9	0	0	0	0	9	0	2	0	2	0	3	0	3	5	14
12:00 12	2:15	0	4	0	4	0	0	0	0	4	0	4	0	4	0	2	1	3	7	11
12:15 12	2:30	1	4	0	5	0	0	0	0	5	1	4	0	5	0	1	0	1	6	11
12:30 12	2:45	1	2	0	3	0	0	0	0	3	0	4	0	4	0	3	0	3	7	10
12:45 13	3:00	0	3	0	3	0	0	0	0	3	1	3	0	4	0	0	0	0	4	7
13:00 13	3:15	0	3	0	3	0	0	0	0	3	0	2	0	2	0	3	0	3	5	8
13:15 13	3:30	1	4	0	5	0	0	0	0	5	1	7	0	8	0	2	0	2	10	15
	5:15	0	13	1	14	0	0	0	0	14	1	4	0	5	0	5	1	6	11	25
15:15 1	5:30	0	11	2	13	0	0	0	0	13	0	1	0	1	0	1	1	2	3	16
15:30 1	5:45	0	12	0	12	0	0	0	0	12	0	6	0	6	0	3	0	3	9	21
15:45 10	6:00	0	9	0	9	0	0	0	0	9	1	2	0	3	0	3	1	4	7	16
	6:15	0	8	0	8	0	0	0	0	8	0	4	0	4	0	2	0	2	6	14
16:15 16	6:30	0	9	0	9	0	0	0	0	9	0	5	0	5	0	1	0	1	6	15
	6:45	0	7	1	8	0	0	0	0	8	0	1	0	1	0	1	0	1	2	10
16:45 1	7:00	0	4	0	4	0	0	0	0	4	0	3	0	3	0	1	0	1	4	8
17:00 1	7:15	1	6	0	7	0	0	0	0	7	1	4	0	5	0	1	0	1	6	13
	7:30	0	7	0	7	0	0	0	0	7	0	0	0	0	0	0	0	0	0	7
	7:45	0	4	0	4	0	0	0	0	4	0	4	0	4	0	1	0	1	5	9
17:45 18	8:00	0	2	0	2	0	0	0	0	2	0	0	0	0	0	3	0	3	3	5
Total: N	None	11	196	13	220	0	0	0	0	220	19	102	0	121	0	74	9	83	204	424

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Turning Movement Count - Study Results

GLADSTONE AVE @ KENT ST

Survey Date: Tuesday, April 25, 2017 WO No: 36848

Start Time: 07:00 Device: Miovision

Full Study 15 Minute U-Turn Total KENT ST GLADSTONE AVE

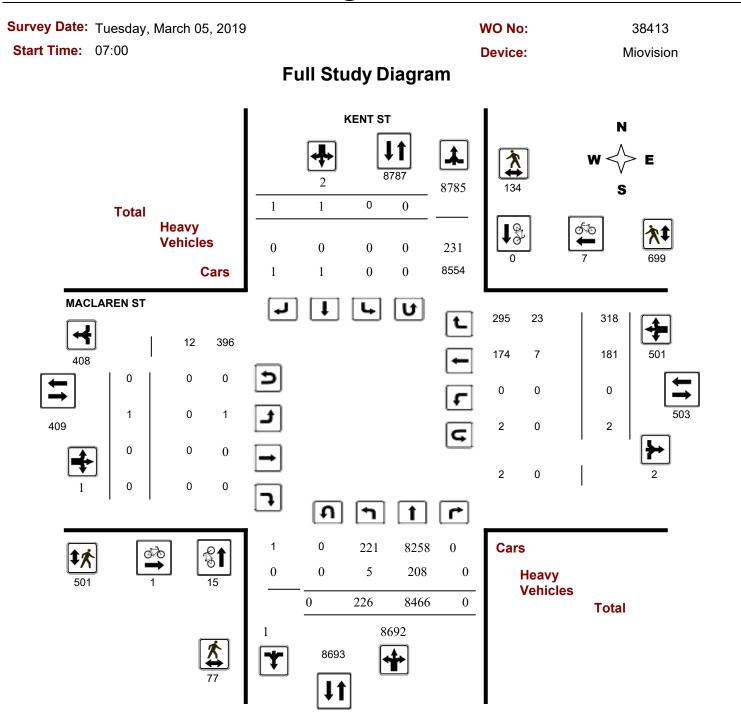
Time F	Period	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
To	otal	0	0	0	0	0

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Turning Movement Count - Study Results

KENT ST @ MACLAREN ST



August 11, 2020 Page 1 of 8



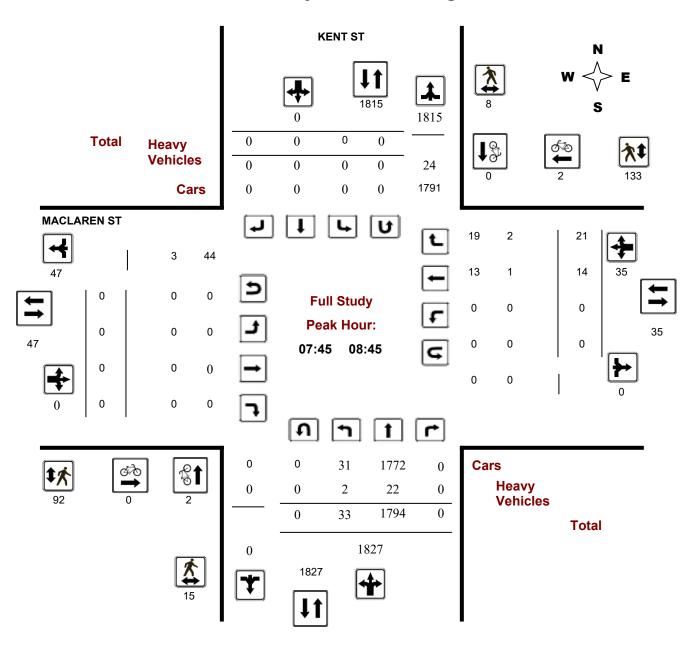
Turning Movement Count - Study Results

KENT ST @ MACLAREN ST

Survey Date: Tuesday, March 05, 2019 WO No: 38413 Start Time: 07:00 Device:

Full Study Peak Hour Diagram

Miovision

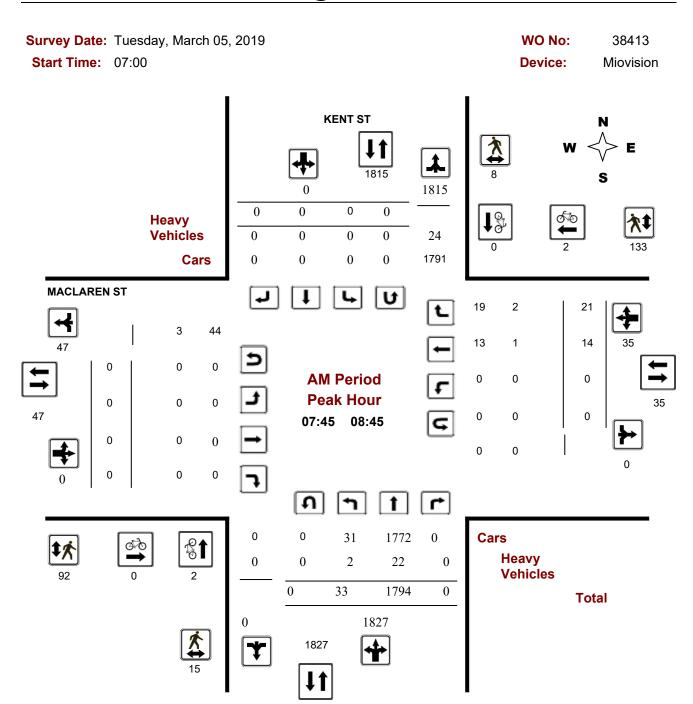


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Turning Movement Count - Peak Hour Diagram

KENT ST @ MACLAREN ST



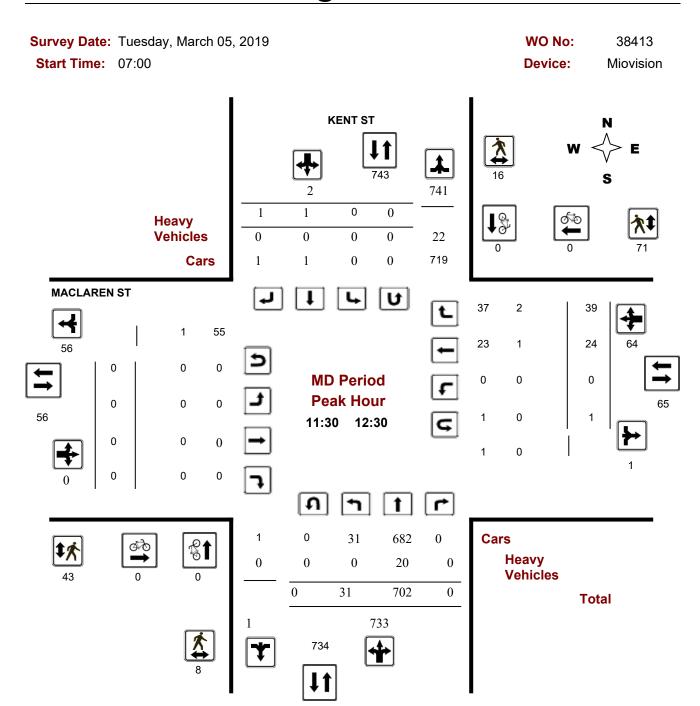
Comments

2020-Aug-11 Page 1 of 3



Turning Movement Count - Peak Hour Diagram

KENT ST @ MACLAREN ST



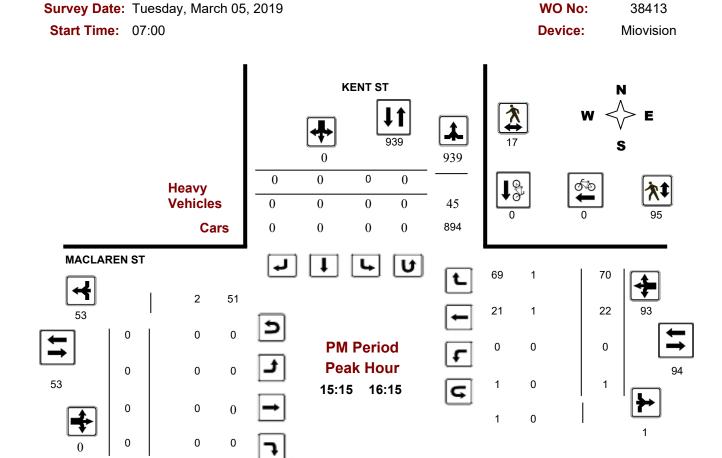
Comments

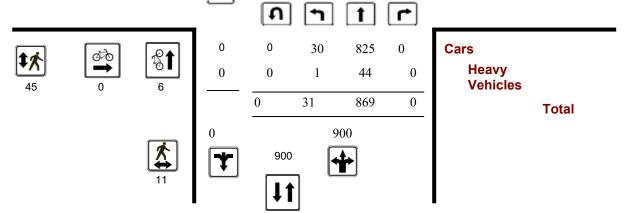
2020-Aug-11 Page 2 of 3



Turning Movement Count - Peak Hour Diagram

KENT ST @ MACLAREN ST





Comments

2020-Aug-11 Page 3 of 3



Turning Movement Count - Study Results

KENT ST @ MACLAREN ST

Survey Date: Tuesday, March 05, 2019 WO No: 38413

Start Time: 07:00 Device: Miovision

Full Study Summary (8 HR Standard)

Survey Date: Tuesday, March 05, 2019 Total Observed U-Turns AADT Factor

Northbound: 0 Southbound: 0 1.00

Eastbound: 0 Westbound: 2

			K	ENT S	Т							MAG	CLARE	N ST					
	No	rthbou	nd		Sou	uthbou	ınd			Ea	astbou	ınd		W	estbou	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	16	1672	0	1688	0	0	0	0	1688	0	0	0	0	0	9	23	32	32	1720
08:00 09:00	38	1780	0	1818	0	0	0	0	1818	0	0	0	0	0	16	22	38	38	1856
09:00 10:00	31	1103	0	1134	0	0	0	0	1134	0	0	0	0	0	16	29	45	45	1179
11:30 12:30	31	702	0	733	0	1	1	2	735	0	0	0	0	0	24	39	63	63	798
12:30 13:30	21	689	0	710	0	0	0	0	710	0	0	0	0	0	23	44	67	67	777
15:00 16:00	22	839	0	861	0	0	0	0	861	0	0	0	0	0	27	58	85	85	946
16:00 17:00	32	825	0	857	0	0	0	0	857	0	0	0	0	0	23	55	78	78	935
17:00 18:00	35	856	0	891	0	0	0	0	891	1	0	0	1	0	43	48	91	92	983
Sub Total	226	8466	0	8692	0	1	1	2	8694	1	0	0	1	0	181	318	499	500	9194
U Turns				0				0	0				0				2	2	2
Total	226	8466	0	8692	0	1	1	2	8694	1	0	0	1	0	181	318	501	502	9196
EQ 12Hr	314	11768	0	12082	0	1	1	3	12085	1	0	0	1	0	252	442	696	698	12782
Note: These	values a	are calcul	lated by	y multiply	ing the	totals b	y the ap	opropriat	e expansi	ion facto	or.			1.39					
AVG 12Hr	296	11090	0	11387	0	1	1	3	12085	1	0	0	1	0	237	417	656	698	12782
Note: These	volumes	are calc	culated	by multip	lying th	e Equiv	alent 1	2 hr. tota	als by the	AADT f	actor.			1					
AVG 24Hr	388	14529	0	14916	0	2	2	3	14919	2	0	0	2	0	311	546	860	862	15781
Note: These	volumes	s are calc	culated	by multip	olying th	e Avera	ige Dail	y 12 hr.	totals by	12 to 24	l expan	sion fac	ctor.	1.31					

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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Turning Movement Count - Study Results

KENT ST @ MACLAREN ST

Survey Date: Tuesday, March 05, 2019 WO No: 38413

Start Time: 07:00 Device: Miovision

Full Study 15 Minute Increments

KENT ST MACLAREN ST

		Northbound			Southbound					Eastbound			Westbound							
Time I	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	5	362	0	367	0	0	0	0	8	0	0	0	0	0	3	3	6	8	373
07:15	07:30	2	429	0	431	0	0	0	0	8	0	0	0	0	0	0	10	10	8	441
07:30	07:45	5	436	0	441	0	0	0	0	9	0	0	0	0	0	3	5	8	9	449
07:45	08:00	4	445	0	449	0	0	0	0	5	0	0	0	0	0	3	5	8	5	457
08:00	08:15	10	425	0	435	0	0	0	0	5	0	0	0	0	0	2	7	9	5	444
08:15	08:30	8	455	0	463	0	0	0	0	6	0	0	0	0	0	3	5	8	6	471
08:30	08:45	11	469	0	480	0	0	0	0	8	0	0	0	0	0	6	4	10	8	490
08:45	09:00	9	431	0	440	0	0	0	0	7	0	0	0	0	0	5	6	11	7	451
09:00	09:15	8	375	0	383	0	0	0	0	12	0	0	0	0	0	3	7	10	12	393
09:15	09:30	4	247	0	251	0	0	0	0	6	0	0	0	0	0	7	5	12	6	263
09:30	09:45	12	261	0	273	0	0	0	0	7	0	0	0	0	0	4	10	14	7	287
09:45	10:00	7	220	0	227	0	0	0	0	9	0	0	0	0	0	2	7	9	9	236
11:30	11:45	6	188	0	194	0	0	1	1	7	0	0	0	0	0	5	9	14	7	209
11:45	12:00	8	213	0	221	0	0	0	0	3	0	0	0	0	0	5	11	17	3	238
12:00	12:15	13	177	0	190	0	0	0	0	7	0	0	0	0	0	6	10	16	7	206
12:15	12:30	4	124	0	128	0	1	0	1	3	0	0	0	0	0	8	9	17	3	146
12:30	12:45	5	155	0	160	0	0	0	0	5	0	0	0	0	0	5	10	15	5	175
12:45	13:00	4	206	0	210	0	0	0	0	6	0	0	0	0	0	8	10	18	6	228
13:00	13:15	7	161	0	168	0	0	0	0	3	0	0	0	0	0	4	11	15	3	183
13:15	13:30	5	167	0	172	0	0	0	0	7	0	0	0	0	0	6	13	19	7	191
15:00	15:15	2	179	0	181	0	0	0	0	7	0	0	0	0	0	8	13	21	7	202
15:15	15:30	5	216	0	221	0	0	0	0	14	0	0	0	0	0	9	16	25	14	246
15:30	15:45	8	216	0	224	0	0	0	0	12	0	0	0	0	0	6	15	22	12	246
15:45	16:00	7	228	0	235	0	0	0	0	10	0	0	0	0	0	4	14	18	10	253
16:00	16:15	11	209	0	220	0	0	0	0	9	0	0	0	0	0	3	25	28	9	248
16:15	16:30	13	196	0	209	0	0	0	0	5	0	0	0	0	0	5	11	16	5	225
16:30	16:45	5	220	0	225	0	0	0	0	11	0	0	0	0	0	10	11	21	11	246
16:45	17:00	3	200	0	203	0	0	0	0	3	0	0	0	0	0	5	8	13	3	216
17:00	17:15	2	223	0	225	0	0	0	0	3	0	0	0	0	0	10	12	22	3	247
17:15	17:30	12	209	0	221	0	0	0	0	2	1	0	0	1	0	11	17	28	2	250
17:30	17:45	13	201	0	214	0	0	0	0	4	0	0	0	0	0	12	11	23	4	237
17:45	18:00	8	223	0	231	0	0	0	0	2	0	0	0	0	0	10	8	18	2	249
Total:		226	8466	0	8692	0	1	1	2	213	1	0	0	1	0	181	318	501	213	9,196

Note: U-Turns are included in Totals.

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Turning Movement Count - Study Results

KENT ST @ MACLAREN ST

Survey Date: Tuesday, March 05, 2019 WO No: 38413

Start Time: 07:00 Device: Miovision

Full Study Cyclist Volume

KENT ST MACLAREN ST

Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	 Grand Total
07:00 07:15	0	0	0	0	0	0	0
07:15 07:30	0	0	0	0	0	0	0
07:30 07:45	0	0	0	0	0	0	0
07:45 08:00	0	0	0	0	0	0	0
08:00 08:15	1	0	1	0	1	1	2
08:15 08:30	1	0	1	0	1	1	2
08:30 08:45	0	0	0	0	0	0	0
08:45 09:00	1	0	1	0	0	0	1
09:00 09:15	0	0	0	0	1	1	1
09:15 09:30	2	0	2	0	0	0	2
09:30 09:45	0	0	0	0	0	0	0
09:45 10:00	0	0	0	0	0	0	0
11:30 11:45	0	0	0	0	0	0	0
11:45 12:00	0	0	0	0	0	0	0
12:00 12:15	0	0	0	0	0	0	0
12:15 12:30	0	0	0	0	0	0	0
12:30 12:45	0	0	0	0	1	1	1
12:45 13:00	2	0	2	1	2	3	5
13:00 13:15	0	0	0	0	0	0	0
13:15 13:30	0	0	0	0	0	0	0
15:00 15:15	0	0	0	0	0	0	0
15:15 15:30	5	0	5	0	0	0	5
15:30 15:45	1	0	1	0	0	0	1
15:45 16:00	0	0	0	0	0	0	0
16:00 16:15	0	0	0	0	0	0	0
16:15 16:30	0	0	0	0	0	0	0
16:30 16:45	1	0	1	0	0	0	1
16:45 17:00	0	0	0	0	0	0	0
17:00 17:15	0	0	0	0	0	0	0
17:15 17:30	0	0	0	0	1	1	1
17:30 17:45	0	0	0	0	0	0	0
17:45 18:00	1	0	1	0	0	0	1
Total	15	0	15	1	7	8	23

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Turning Movement Count - Study Results

KENT ST @ MACLAREN ST

Survey Date: Tuesday, March 05, 2019 WO No: 38413

Start Time: 07:00 Device: Miovision

Full Study Pedestrian Volume

KENT ST MACLAREN ST

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	0	4	4	4	8	12	16
07:15 07:30	1	4	5	11	13	24	29
07:30 07:45	2	2	4	10	21	31	35
07:45 08:00	9	1	10	19	30	49	59
08:00 08:15	2	2	4	23	28	51	55
08:15 08:30	3	3	6	28	37	65	71
08:30 08:45	1	2	3	22	38	60	63
08:45 09:00	1	3	4	30	38	68	72
09:00 09:15	0	3	3	12	18	30	33
09:15 09:30	0	5	5	16	15	31	36
09:30 09:45	1	4	5	8	11	19	24
09:45 10:00	1	4	5	4	10	14	19
11:30 11:45	2	1	3	9	21	30	33
11:45 12:00	2	6	8	11	17	28	36
12:00 12:15	4	6	10	13	16	29	39
12:15 12:30	0	3	3	10	17	27	30
12:30 12:45	3	0	3	11	15	26	29
12:45 13:00	1	7	8	20	24	44	52
13:00 13:15	3	7	10	10	12	22	32
13:15 13:30	3	6	9	11	14	25	34
15:00 15:15	3	5	8	18	17	35	43
15:15 15:30	3	5	8	12	15	27	35
15:30 15:45	2	4	6	9	23	32	38
15:45 16:00	5	7	12	12	19	31	43
16:00 16:15	1	1	2	12	38	50	52
16:15 16:30	5	7	12	19	29	48	60
16:30 16:45	7	2	9	20	27	47	56
16:45 17:00	7	2	9	23	28	51	60
17:00 17:15	2	2	4	27	35	62	66
17:15 17:30	2	11	13	23	32	55	68
17:30 17:45	1	6	7	23	16	39	46
17:45 18:00	0	9	9	21	17	38	47
Total	77	134	211	501	699	1200	1411

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Turning Movement Count - Study Results

KENT ST @ MACLAREN ST

Survey Date: Tuesday, March 05, 2019 WO No: 38413

Start Time: 07:00 Device: Miovision

Full Study Heavy Vehicles

KENT ST MACLAREN ST

Northbound		Southbound						E	Eastbound			Westbound							
Time Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07:15	0	8	0	8	0	0	0	0	8	0	0	0	0	0	1	0	1	1	9
07:15 07:30	0	8	0	8	0	0	0	0	8	0	0	0	0	0	0	2	2	2	10
07:30 07:45	0	9	0	9	0	0	0	0	9	0	0	0	0	0	0	0	0	0	9
07:45 08:00	1	4	0	5	0	0	0	0	5	0	0	0	0	0	0	1	1	1	6
08:00 08:15	0	5	0	5	0	0	0	0	5	0	0	0	0	0	0	0	0	0	5
08:15 08:30	0	6	0	6	0	0	0	0	6	0	0	0	0	0	1	1	2	2	8
08:30 08:45	1	7	0	8	0	0	0	0	8	0	0	0	0	0	0	0	0	0	8
08:45 09:00	0	7	0	7	0	0	0	0	7	0	0	0	0	0	1	1	2	2	9
09:00 09:15	1	11	0	12	0	0	0	0	12	0	0	0	0	0	0	3	3	3	15
09:15 09:30	0	6	0	6	0	0	0	0	6	0	0	0	0	0	0	1	1	1	7
09:30 09:45	0	7	0	7	0	0	0	0	7	0	0	0	0	0	0	3	3	3	10
09:45 10:00	1	8	0	9	0	0	0	0	9	0	0	0	0	0	0	0	0	0	9
11:30 11:45	0	7	0	7	0	0	0	0	7	0	0	0	0	0	1	1	2	2	9
11:45 12:00	0	3	0	3	0	0	0	0	3	0	0	0	0	0	0	1	1	1	4
12:00 12:15		7	0	7	0	0	0	0	7	0	0	0	0	0	0	0	0	0	7
12:15 12:30	0	3	0	3	0	0	0	0	3	0	0	0	0	0	0	0	0	0	3
12:30 12:45	0	5	0	5	0	0	0	0	5	0	0	0	0	0	1	1	2	2	7
12:45 13:00	0	6	0	6	0	0	0	0	6	0	0	0	0	0	0	2	2	2	8
13:00 13:15	0	3	0	3	0	0	0	0	3	0	0	0	0	0	1	1	2	2	5
13:15 13:30	+	7	0	7	0	0	0	0	7	0	0	0	0	0	0	1	1	1	8
15:00 15:15		7	0	7	0	0	0	0	7	0	0	0	0	0	0	0	0	0	7
15:15 15:30	+	14	0	14	0	0	0	0	14	0	0	0	0	0	0	0	0	0	14
15:30 15:45	_	12	0	12	0	0	0	0	12	0	0	0	0	0	1	1	2	2	14
15:45 16:00	+	10	0	10	0	0	0	0	10	0	0	0	0	0	0	0	0	0	10
16:00 16:15	+	8	0	9	0	0	0	0	9	0	0	0	0	0	0	0	0	0	9
16:15 16:30	_	5	0	5	0	0	0	0	5	0	0	0	0	0	0	1	1	1	6
16:30 16:45	+	11	0	11	0	0	0	0	11	0	0	0	0	0	0	0	0	0	11
16:45 17:00	+	3	0	3	0	0	0	0	3	0	0	0	0	0	0	0	0	0	3
17:00 17:15	+	3	0	3	0	0	0	0	3	0	0	0	0	0	0	0	0	0	3
17:15 17:30	+	2	0	2	0	0	0	0	2	0	0	0	0	0	0	1	1	1	3
17:30 17:45	_	4	0	4	0	0	0	0	4	0	0	0	0	0	0	1	1	1	5
17:45 18:00	_	2	0	2	0	0	0	0	2	0	0	0	0	0	0	0	0	0	2
Total: None	5	208	0	213	0	0	0	0	213	0	0	0	0	0	7	23	30	30	243

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Turning Movement Count - Study Results

KENT ST @ MACLAREN ST

Survey Date: Tuesday, March 05, 2019 WO No: 38413

Start Time: 07:00 Device: Miovision

Full Study 15 Minute U-Turn Total KENT ST MACLAREN ST

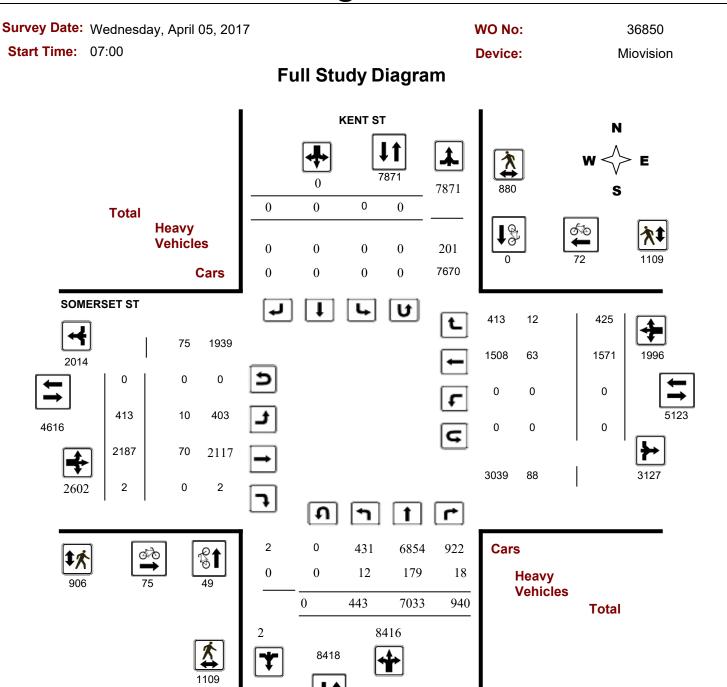
Time P	eriod	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	1	1
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	1	1
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
То	tal	0	0	0	2	2

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Turning Movement Count - Study Results

KENT ST @ SOMERSET ST



July 9, 2021 Page 1 of 8



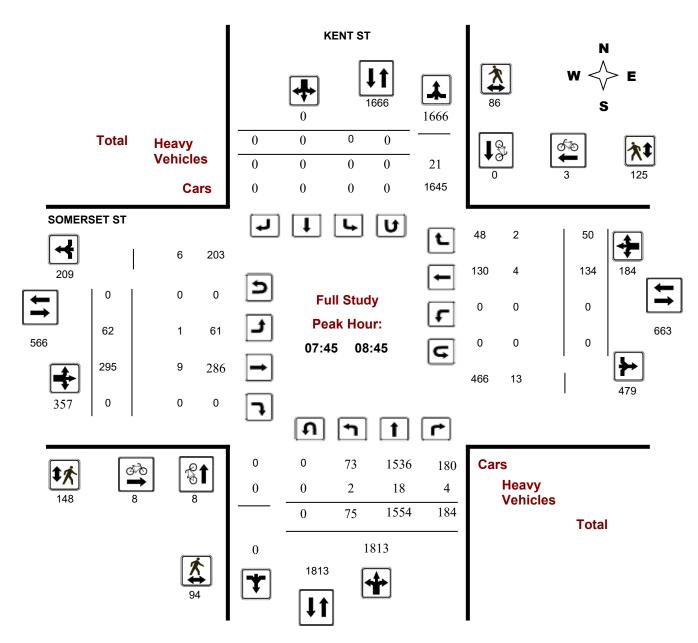
Turning Movement Count - Study Results

KENT ST @ SOMERSET ST

Survey Date: Wednesday, April 05, 2017 WO No: 36850

Start Time: 07:00 Device: Miovision

Full Study Peak Hour Diagram



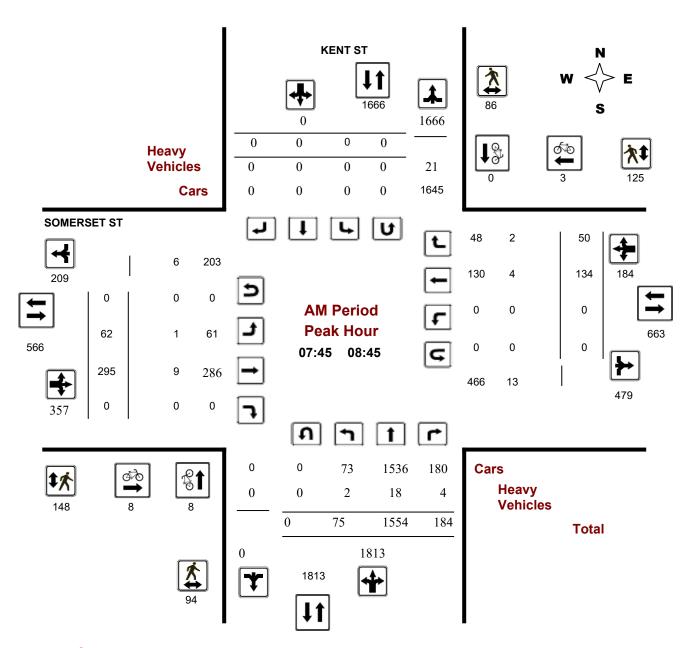
July 9, 2021 Page 2 of 8



Turning Movement Count - Peak Hour Diagram

KENT ST @ SOMERSET ST

Survey Date: Wednesday, April 05, 2017 WO No: 36850
Start Time: 07:00 Device: Miovision



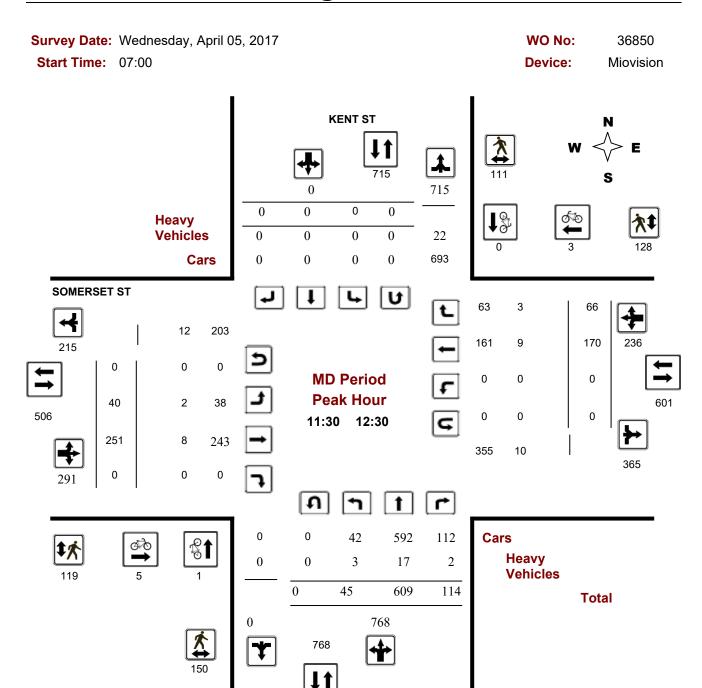
Comments

2021-Jul-09 Page 1 of 3



Turning Movement Count - Peak Hour Diagram

KENT ST @ SOMERSET ST



Comments

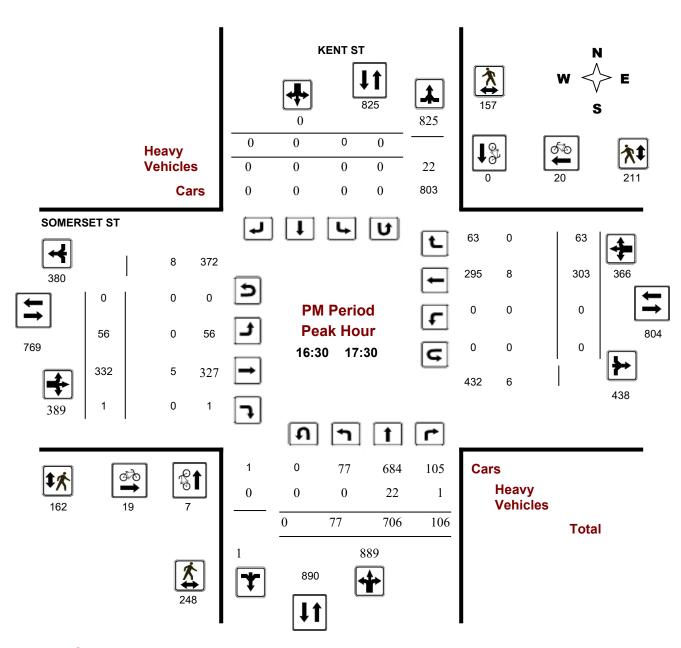
2021-Jul-09 Page 2 of 3



Turning Movement Count - Peak Hour Diagram

KENT ST @ SOMERSET ST





Comments

2021-Jul-09 Page 3 of 3



Turning Movement Count - Study Results

KENT ST @ SOMERSET ST

Survey Date: Wednesday, April 05, 2017 WO No: 36850

Start Time: 07:00 Device: Miovision

Full Study Summary (8 HR Standard)

Survey Date: Wednesday, April 05, 2017 Total Observed U-Turns AADT Factor

Northbound: 0 Southbound: 0

.90

Eastbound: 0 Westbound: 0

			K	ENT S	Т							SON	MERSI	ET ST					
	No	rthbou	ınd		Sou	uthbou	nd			Е	astbou	ınd		V	/estbo	und			
Period	LT	ST	RT	NB TOT	LT	ST	RT	SB TOT	STR TOT	LT	ST	RT	EB TOT	LT	ST	RT	WB TOT	STR TOT	Grand Total
07:00 08:00	53	1393	126	1572	0	0	0	0	1572	46	245	0	291	0	93	44	137	428	2000
08:00 09:00	69	1516	190	1775	0	0	0	0	1775	68	301	0	369	0	152	48	200	569	2344
09:00 10:00	48	944	106	1098	0	0	0	0	1098	56	233	0	289	0	166	40	206	495	1593
11:30 12:30	45	609	114	768	0	0	0	0	768	40	251	0	291	0	170	66	236	527	1295
12:30 13:30	42	530	90	662	0	0	0	0	662	45	242	1	288	0	161	41	202	490	1152
15:00 16:00	56	651	113	820	0	0	0	0	820	35	305	0	340	0	251	56	307	647	1467
16:00 17:00	64	682	96	842	0	0	0	0	842	61	330	1	392	0	318	66	384	776	1618
17:00 18:00	66	708	105	879	0	0	0	0	879	62	280	0	342	0	260	64	324	666	1545
Sub Total	443	7033	940	8416	0	0	0	0	8416	413	2187	2	2602	0	1571	425	1996	4598	13014
U Turns	0			0	0			0	0	0			0	0			0	0	0
Total	443	7033	940	8416	0	0	0	0	8416	413	2187	2	2602	0	1571	425	1996	4598	13014
EQ 12Hr	616	9776	1307	11699	0	0	0	0	11699	574	3040	3	3617	0	2184	591	2775	6392	18091
Note: These	values a	are calcu	ılated b	y multiply	ing the	totals b	y the a	opropriat	te expans	ion fac	tor.			1.39					
AVG 12Hr	554	8798	1176	10528	0	0	0	0	10528	517	2736	3	3256	0	1966	532	2498	5754	16282
Note: These	volumes	s are cal	culated	by multip	olying th	e Equiv	alent 1	2 hr. tota	als by the	AADT	factor.			.90					
AVG 24Hr	726	11525	1541	13792	0	0	0	0	13792	677	3584	4	4265	0	2575	697	3272	7537	21329
Note: These	volumes	s are cal	culated	by multip	olying th	ie Avera	ige Dai	ly 12 hr.	totals by	12 to 2	4 expan	sion fac	ctor.	1.31					

Note: U-Turns provided for approach totals. Refer to 'U-Turn' Report for specific breakdown.

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Turning Movement Count - Study Results

KENT ST @ SOMERSET ST

Survey Date: Wednesday, April 05, 2017 WO No: 36850

Start Time: 07:00 Device: Miovision

Full Study 15 Minute Increments

KENT ST SOMERSET ST

		No	orthbou	ınd		Sc	uthbou	ınd			Е	astbour	nd		W	estbour	nd			
Time F	Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00	07:15	8	312	26	346	0	0	0	0	346	6	61	0	67	0	14	12	26	93	439
07:15	07:30	8	335	33	376	0	0	0	0	376	7	46	0	53	0	22	9	31	84	460
07:30	07:45	12	348	30	390	0	0	0	0	390	16	65	0	81	0	35	11	46	127	517
07:45	08:00	25	398	37	460	0	0	0	0	460	17	73	0	90	0	22	12	34	124	584
08:00	08:15	17	373	39	429	0	0	0	0	429	14	68	0	82	0	29	12	41	123	552
08:15	08:30	18	416	46	480	0	0	0	0	480	12	67	0	79	0	41	15	56	135	615
08:30	08:45	15	367	62	444	0	0	0	0	444	19	87	0	106	0	42	11	53	159	603
08:45	09:00	19	360	43	422	0	0	0	0	422	23	79	0	102	0	40	10	50	152	574
09:00	09:15	9	306	34	349	0	0	0	0	349	14	59	0	73	0	42	11	53	126	475
09:15	09:30	16	250	20	286	0	0	0	0	286	13	60	0	73	0	42	9	51	124	410
09:30	09:45	10	202	27	239	0	0	0	0	239	13	55	0	68	0	38	13	51	119	358
09:45	10:00	13	186	25	224	0	0	0	0	224	16	59	0	75	0	44	7	51	126	350
11:30	11:45	12	145	31	188	0	0	0	0	188	10	61	0	71	0	40	22	62	133	321
11:45	12:00	9	197	28	234	0	0	0	0	234	12	63	0	75	0	49	12	61	136	370
12:00	12:15	15	125	27	167	0	0	0	0	167	9	66	0	75	0	47	17	64	139	306
12:15	12:30	9	142	28	179	0	0	0	0	179	9	61	0	70	0	34	15	49	119	298
12:30	12:45	9	139	23	171	0	0	0	0	171	13	56	0	69	0	48	9	57	126	297
12:45	13:00	14	121	24	159	0	0	0	0	159	8	64	0	72	0	37	9	46	118	277
13:00	13:15	8	141	21	170	0	0	0	0	170	8	68	0	76	0	44	19	63	139	309
13:15	13:30	11	129	22	162	0	0	0	0	162	16	54	1	71	0	32	4	36	107	269
15:00	15:15	11	144	30	185	0	0	0	0	185	3	80	0	83	0	53	20	73	156	341
15:15	15:30	16	183	26	225	0	0	0	0	225	13	78	0	91	0	60	8	68	159	384
15:30	15:45	15	175	17	207	0	0	0	0	207	9	72	0	81	0	70	13	83	164	371
15:45	16:00	14	149	40	203	0	0	0	0	203	10	75	0	85	0	68	15	83	168	371
16:00	16:15	18	158	21	197	0	0	0	0	197	18	88	0	106	0	73	16	89	195	392
16:15	16:30	13	172	17	202	0	0	0	0	202	15	64	0	79	0	85	22	107	186	388
16:30	16:45	14	181	26	221	0	0	0	0	221	12	85	0	97	0	82	11	93	190	411
16:45	17:00	19	171	32	222	0	0	0	0	222	16	93	1	110	0	78	17	95	205	427
17:00	17:15	24	167	29	220	0	0	0	0	220	9	76	0	85	0	76	12	88	173	393
17:15	17:30	20	187	19	226	0	0	0	0	226	19	78	0	97	0	67	23	90	187	413
17:30	17:45	7	193	24	224	0	0	0	0	224	16	69	0	85	0	68	18	86	171	395
17:45	18:00	15	161	33	209	0	0	0	0	209	18	57	0	75	0	49	11	60	135	344
Total:		443	7033	940	8416	0	0	0	0	8416	413	2187	2	2602	0	1571	425	1996	8416	13,014

Note: U-Turns are included in Totals.

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Turning Movement Count - Study Results

KENT ST @ SOMERSET ST

Survey Date: Wednesday, April 05, 2017 WO No: 36850

Start Time: 07:00 Device: Miovision

Full Study Cyclist Volume

KENT ST SOMERSET ST

Time Period	Northbound	Southbound	Street Total	Eastbound	Westbound	Street Total	 Grand Total
07:00 07:15	3	0	3	3	2	5	8
07:15 07:30	2	0	2	4	0	4	6
07:30 07:45	0	0	0	1	2	3	3
07:45 08:00	3	0	3	4	0	4	7
08:00 08:15	0	0	0	1	1	2	2
08:15 08:30	3	0	3	3	0	3	6
08:30 08:45	2	0	2	0	2	2	4
08:45 09:00	5	0	5	4	2	6	11
09:00 09:15	1	0	1	2	0	2	3
09:15 09:30	1	0	1	3	0	3	4
09:30 09:45	1	0	1	0	4	4	5
09:45 10:00	4	0	4	2	2	4	8
11:30 11:45	0	0	0	3	0	3	3
11:45 12:00	1	0	1	1	2	3	4
12:00 12:15	0	0	0	0	0	0	0
12:15 12:30	0	0	0	1	1	2	2
12:30 12:45	0	0	0	5	0	5	5
12:45 13:00	2	0	2	4	1	5	7
13:00 13:15	1	0	1	1	5	6	7
13:15 13:30	1	0	1	1	2	3	4
15:00 15:15	1	0	1	1	3	4	5
15:15 15:30	2	0	2	3	1	4	6
15:30 15:45	1	0	1	1	1	2	3
15:45 16:00	0	0	0	3	5	8	8
16:00 16:15	4	0	4	0	8	8	12
16:15 16:30	0	0	0	3	2	5	5
16:30 16:45	3	0	3	5	6	11	14
16:45 17:00	1	0	1	10	4	14	15
17:00 17:15	0	0	0	1	5	6	6
17:15 17:30	3	0	3	3	5	8	11
17:30 17:45	2	0	2	1	1	2	4
17:45 18:00	2	0	2	1	5	6	8
Total	49	0	49	75	72	147	196

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Turning Movement Count - Study Results

KENT ST @ SOMERSET ST

Survey Date: Wednesday, April 05, 2017 WO No: 36850

Start Time: 07:00 Device: Miovision

Full Study Pedestrian Volume

KENT ST SOMERSET ST

Time Period	NB Approach (E or W Crossing)	SB Approach (E or W Crossing)	Total	EB Approach (N or S Crossing)	WB Approach (N or S Crossing)	Total	Grand Total
07:00 07:15	2	7	9	3	12	15	24
07:15 07:30	11	13	24	14	14	28	52
07:30 07:45	5	15	20	10	24	34	54
07:45 08:00	23	21	44	36	27	63	107
08:00 08:15	22	18	40	27	27	54	94
08:15 08:30	17	22	39	32	38	70	109
08:30 08:45	32	25	57	53	33	86	143
08:45 09:00	26	22	48	35	29	64	112
09:00 09:15	30	17	47	33	22	55	102
09:15 09:30	22	16	38	30	30	60	98
09:30 09:45	20	16	36	19	14	33	69
09:45 10:00	23	16	39	18	25	43	82
11:30 11:45	36	17	53	12	22	34	87
11:45 12:00	42	24	66	36	32	68	134
12:00 12:15	35	33	68	43	39	82	150
12:15 12:30	37	37	74	28	35	63	137
12:30 12:45	27	38	65	28	35	63	128
12:45 13:00	40	33	73	33	36	69	142
13:00 13:15	32	29	61	29	37	66	127
13:15 13:30	23	29	52	12	32	44	96
15:00 15:15	28	32	60	30	31	61	121
15:15 15:30	19	30	49	29	27	56	105
15:30 15:45	55	27	82	32	45	77	159
15:45 16:00	30	40	70	18	39	57	127
16:00 16:15	49	29	78	19	56	75	153
16:15 16:30	60	38	98	38	53	91	189
16:30 16:45	66	45	111	51	70	121	232
16:45 17:00	55	32	87	46	54	100	187
17:00 17:15	71	40	111	33	45	78	189
17:15 17:30	56	40	96	32	42	74	170
17:30 17:45	50	27	77	16	44	60	137
17:45 18:00	65	52	117	31	40	71	188
Total	1109	880	1989	906	1109	2015	4004

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Turning Movement Count - Study Results

KENT ST @ SOMERSET ST

Survey Date: Wednesday, April 05, 2017 WO No: 36850

Start Time: 07:00 Device: Miovision

Full Study Heavy Vehicles

KENT ST SOMERSET ST

	N	orthbou	und		Sc	uthbou	ınd			Е	astbour	nd		We	estbour	nd			
Time Period	LT	ST	RT	N TOT	LT	ST	RT	S TOT	STR TOT	LT	ST	RT	E TOT	LT	ST	RT	W TOT	STR TOT	Grand Total
07:00 07:15	0	2	1	3	0	0	0	0	3	0	2	0	2	0	2	1	3	5	8
07:15 07:30	1	5	0	6	0	0	0	0	6	1	4	0	5	0	3	0	3	8	14
07:30 07:45	1	6	0	7	0	0	0	0	7	1	2	0	3	0	1	1	2	5	12
07:45 08:00	0	5	0	5	0	0	0	0	5	0	2	0	2	0	1	0	1	3	8
08:00 08:15	1	4	2	7	0	0	0	0	7	1	5	0	6	0	1	2	3	9	16
08:15 08:30	0	3	1	4	0	0	0	0	4	0	0	0	0	0	1	0	1	1	5
08:30 08:45	1	6	1	8	0	0	0	0	8	0	2	0	2	0	1	0	1	3	11
08:45 09:00	1	7	1	9	0	0	0	0	9	1	6	0	7	0	3	0	3	10	19
09:00 09:15	0	8	1	9	0	0	0	0	9	1	2	0	3	0	2	0	2	5	14
09:15 09:30	1	6	0	7	0	0	0	0	7	0	3	0	3	0	6	1	7	10	17
09:30 09:45	1	9	1	11	0	0	0	0	11	0	6	0	6	0	1	0	1	7	18
09:45 10:00	0	7	1	8	0	0	0	0	8	0	4	0	4	0	4	0	4	8	16
11:30 11:45	1	8	0	9	0	0	0	0	9	0	1	0	1	0	3	1	4	5	14
11:45 12:00	1	4	1	6	0	0	0	0	6	0	5	0	5	0	1	2	3	8	14
12:00 12:15	0	4	1	5	0	0	0	0	5	1	1	0	2	0	4	0	4	6	11
12:15 12:30	1	1	0	2	0	0	0	0	2	1	1	0	2	0	1	0	1	3	5
12:30 12:45	0	2	0	2	0	0	0	0	2	0	0	0	0	0	2	0	2	2	4
12:45 13:00	0	3	0	3	0	0	0	0	3	0	2	0	2	0	1	0	1	3	6
13:00 13:15	0	5	0	5	0	0	0	0	5	0	2	0	2	0	2	1	3	5	10
13:15 13:30	1	4	0	5	0	0	0	0	5	2	1	0	3	0	1	1	2	5	10
15:00 15:15	0	7	3	10	0	0	0	0	10	0	2	0	2	0	3	0	3	5	15
15:15 15:30	0	12	0	12	0	0	0	0	12	0	1	0	1	0	1	0	1	2	14
15:30 15:45	0	12	2	14	0	0	0	0	14	0	2	0	2	0	3	0	3	5	19
15:45 16:00	0	3	0	3	0	0	0	0	3	0	2	0	2	0	1	0	1	3	6
16:00 16:15	0	5	0	5	0	0	0	0	5	1	3	0	4	0	3	1	4	8	13
16:15 16:30	1	11	0	12	0	0	0	0	12	0	1	0	1	0	1	0	1	2	14
16:30 16:45	0	5	1	6	0	0	0	0	6	0	2	0	2	0	2	0	2	4	10
16:45 17:00	0	6	0	6	0	0	0	0	6	0	1	0	1	0	1	0	1	2	8
17:00 17:15	0	7	0	7	0	0	0	0	7	0	1	0	1	0	2	0	2	3	10
17:15 17:30	0	4	0	4	0	0	0	0	4	0	1	0	1	0	3	0	3	4	8
17:30 17:45	0	6	0	6	0	0	0	0	6	0	2	0	2	0	1	1	2	4	10
17:45 18:00	0	2	1	3	0	0	0	0	3	0	1	0	1	0	1	0	1	2	5
Total: None	12	179	18	209	0	0	0	0	209	10	70	0	80	0	63	12	75	155	364

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Turning Movement Count - Study Results

KENT ST @ SOMERSET ST

Survey Date: Wednesday, April 05, 2017 WO No: 36850

Start Time: 07:00 Device: Miovision

Full Study 15 Minute U-Turn Total KENT ST SOMERSET ST

Time P	eriod	Northbound U-Turn Total	Southbound U-Turn Total	Eastbound U-Turn Total	Westbound U-Turn Total	Total
07:00	07:15	0	0	0	0	0
07:15	07:30	0	0	0	0	0
07:30	07:45	0	0	0	0	0
07:45	08:00	0	0	0	0	0
08:00	08:15	0	0	0	0	0
08:15	08:30	0	0	0	0	0
08:30	08:45	0	0	0	0	0
08:45	09:00	0	0	0	0	0
09:00	09:15	0	0	0	0	0
09:15	09:30	0	0	0	0	0
09:30	09:45	0	0	0	0	0
09:45	10:00	0	0	0	0	0
11:30	11:45	0	0	0	0	0
11:45	12:00	0	0	0	0	0
12:00	12:15	0	0	0	0	0
12:15	12:30	0	0	0	0	0
12:30	12:45	0	0	0	0	0
12:45	13:00	0	0	0	0	0
13:00	13:15	0	0	0	0	0
13:15	13:30	0	0	0	0	0
15:00	15:15	0	0	0	0	0
15:15	15:30	0	0	0	0	0
15:30	15:45	0	0	0	0	0
15:45	16:00	0	0	0	0	0
16:00	16:15	0	0	0	0	0
16:15	16:30	0	0	0	0	0
16:30	16:45	0	0	0	0	0
16:45	17:00	0	0	0	0	0
17:00	17:15	0	0	0	0	0
17:15	17:30	0	0	0	0	0
17:30	17:45	0	0	0	0	0
17:45	18:00	0	0	0	0	0
То	tal	0	0	0	0	0

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APPENDIX C COLLISION DATA

Total Area

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	SMV Other	SMV Unattended Vehicle	Other	Total
P.D. only	15	12	24	17	1	1	17	3	90
Non-fatal injury	4	5	0	3	0	8	1	0	21
Non-reportable	0	0	0	0	0	0	1	0	1
Total	19	17	24	20	1	9	19	3	112
	#2 170/	#5 150/	#1 210/	#2 100/	#0 10/	#6 00/	#2 170/	#7 20/	

80% 19% 1% 100%

Bank St/Gilmour St

Dank Sty Gilli	ioui St			
Years	Total # Collisions	24 Hr AADT Veh Volume	Days	Collisions/MEV
2015-2019	11	10,338	1825	0.58

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total
P.D. only	4	0	3	0	0	0	0	1	8
Non-fatal injury	1	2	0	0	0	0	0	0	3
Non reportable	0	0	0	0	0	0	0	0	0
Total	5	2	3	0	0	0	0	1	11
	45%	18%	27%	0%	0%	0%	0%	9%	

73% 27% 0% 100%

Bank St/MacLaren St

Years	Total # Collisions	24 Hr AADT Veh Volume	Days	Collisions/MEV
2015-2019	1	11,040	1825	0.05

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total
P.D. only	0	0	0	0	0	0	0	0	0
Non-fatal injury	1	0	0	0	0	0	0	0	1
Non reportable	0	0	0	0	0	0	0	0	0
Total	1	0	0	0	0	0	0	0	1
	100%	0%	0%	0%	0%	0%	0%	0%	

0% 100% 0% 100%

Bank St, MacLaren St to Gilmour St

Years	Total # Collisions	24 Hr AADT Veh Volume	Days	Collisions/MEV
2015-2019	4	n/a	1825	n/a

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total
P.D. only	0	0	1	0	0	0	3	0	4
Non-fatal injury	0	0	0	0	0	0	0	0	0
Non reportable	0	0	0	0	0	0	0	0	0
Total	0	0	1	0	0	0	3	0	4
	0%	0%	25%	0%	0%	0%	75%	0%	<u> </u>

100% 0% 0% 100%

Gilmour St/Kent St

Years	Total #	24 Hr AADT	Days	Collisions/MEV
i ears	Collisions	Veh Volume	Days	Collisions/InLV
2015-2019	5	15 811	1825	0.17

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total	
P.D. only	0	1	2	0	0	0	0	1	4	Ī
Non-fatal injury	0	0	0	0	0	1	0	0	1	Ī
Non reportable	0	0	0	0	0	0	0	0	0	Ī
Total	0	1	2	0	0	1	0	1	5] :
	0%	20%	40%	0%	0%	20%	0%	20%		-

80% 20% 0% 100%

Gladstone Ave/Kent St

Years	Total # Collisions	24 Hr AADT Veh Volume	Days	Collisions/MEV
2015-2019	29	23,139	1825	0.69

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total	Ī
P.D. only	7	3	4	12	0	0	0	0	26	90
Non-fatal injury	0	0	0	2	0	1	0	0	3	10
Non reportable	0	0	0	0	0	0	0	0	0	0
Total	7	3	4	14	0	1	0	0	29	10
	24%	10%	14%	48%	0%	3%	0%	0%		-

90% 10% 0% 100%

Kent St/MacLaren st

Years	Total # Collisions	24 Hr AADT Veh Volume	Days	Collisions/MEV
2015-2019	11	15.781	1825	0.38

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total
P.D. only	1	4	4	1	0	0	0	0	10
Non-fatal injury	0	0	0	1	0	0	0	0	1
Non reportable	0	0	0	0	0	0	0	0	0
Total	1	4	4	2	0	0	0	0	11
	00/-	260/-	260/-	100/-	00/-	0%	00/-	094	

91% 9% 0% 100%

Kent St/Somerset St

Years	Total # Collisions			Collisions/MEV
2015-2019	23	21,329	1825	0.59

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total
P.D. only	1	4	4	3	0	0	1	0	13
Non-fatal injury	1	3	0	0	0	6	0	0	10
Non reportable	0	0	0	0	0	0	0	0	0
Total	2	7	4	3	0	6	1	0	23
	9%	30%	17%	13%	0%	26%	4%	0%	

57% 43% 0% 100%

Kent St, Florence St to Gladstone Ave

Years	Total # Collisions	24 Hr AADT Veh Volume	Days	Collisions/MEV
2015-2019	1	n/a	1825	n/a

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total
P.D. only	0	0	1	0	0	0	0	0	1
Non-fatal injury	0	0	0	0	0	0	0	0	0
Non reportable	0	0	0	0	0	0	0	0	0
Total	0	0	1	0	0	0	0	0	1
	0%	0%	100%	0%	0%	0%	0%	0%	

100% 0% 0% 100%

Kent St, Gilmour St to James St

Years	Total # Collisions	24 Hr AADT Veh Volume	Days	Collisions/MEV
2015-2019	3	n/a	1825	n/a

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total	
P.D. only	0	0	1	0	0	1	0	0	2	Ι (
Non-fatal injury	0	0	0	0	0	0	1	0	1	T :
Non reportable	0	0	0	0	0	0	0	0	0	Ī
Total	0	0	1	0	0	1	1	0	3	1
	0%	0%	33%	0%	0%	33%	33%	0%	•	_

67% 33% 0% 100%

Kent St, James St to Florence St

Years	Total #	24 Hr AADT	Davs	Collisions/MEV
rears	Collisions	Veh Volume	Days	COIIISIOIIS/I·ILV
201E 2010	5	n/a	1025	n/a

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total	
P.D. only	0	0	1	0	0	0	3	0	4	ĺ
Non-fatal injury	1	0	0	0	0	0	0	0	1	ĺ
Non reportable	0	0	0	0	0	0	0	0	0	ĺ
Total	1	0	1	0	0	0	3	0	5	ĺ
	20%	0%	20%	0%	0%	0%	60%	0%		

80% 20% 0% 100%

Kent St, MacLaren St to Gilmour St

Years	Total # Collisions	24 Hr AADT Veh Volume	Days	Collisions/MEV
2015-2019	5	n/a	1825	n/a

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total
P.D. only	2	0	1	0	1	0	1	0	5
Non-fatal injury	0	0	0	0	0	0	0	0	0
Non reportable	0	0	0	0	0	0	0	0	0
Total	2	0	1	0	1	0	1	0	5
	40%	0%	20%	0%	20%	0%	20%	0%	

100% 0% 0% 100%

Kent St, Somerset St to MacLaren St

Rent St, Son	CISCI SI IO II	acearch St		
Years	Total # Collisions	24 Hr AADT Veh Volume	Days	Collisions/MEV
2015-2019	3	n/a	1825	n/a

Classification of Accident Rear End Turning Movement Sideswipe Angle	Approaching Single Vehicle (other)	(Unattended Other vehicle)	Total
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•	0%	0%	67%	0%	0%	0%	33%	0%		
Total	0	0	2	0	0	0	1	0	3]
Non reportable	0	0	0	0	0	0	0	0	0]
Non-fatal injury	0	0	0	0	0	0	0	0	0]
P.D. only	0	0	2	0	0	0	1	0	3]

100% 0% 0% 100%

MacLaren St, Kent St to Bank St

Years	Total # Collisions	24 Hr AADT Veh Volume	Days	Collisions/MEV
2015-2019	10	n/a	1825	n/a

Classification of Accident	Rear End	Turning Movement	Sideswipe	Angle	Approaching	Single Vehicle (other)	Single vehicle (Unattended vehicle)	Other	Total
P.D. only	0	0	0	1	0	0	8	1	10
Non-fatal injury	0	0	0	0	0	0	0	0	0
Non reportable	0	0	0	0	0	0	0	0	0
Total	0	0	0	1	0	0	8	1	10
	0%	0%	0%	10%	0%	0%	80%	10%	

100% 0% 0% 100%



Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: BANK ST @ GILMOUR ST

Traffic Control: Traffic signal Total Collisions: 11

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Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2015-Jul-17, Fri,14:13	Clear	Rear end	Non-fatal injury	Dry	South	Slowing or stoppin	g Automobile, station wagon	Other motor vehicle	0
					South	Stopped	Pick-up truck	Other motor vehicle	
2015-Nov-27, Fri,00:50	Clear	Rear end	P.D. only	Dry	North	Going ahead	Pick-up truck	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2015-Dec-02, Wed,03:00	Clear	Sideswipe	P.D. only	Dry	South	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Jun-03, Fri,18:00	Clear	Turning movement	Non-fatal injury	Dry	North	Turning right	Unknown	Cyclist	0
					North	Stopped	Bicycle	Other motor vehicle	
2018-Jan-29, Mon,13:54	Clear	Sideswipe	P.D. only	Wet	East	Stopped	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Jun-08, Fri,15:16	Clear	Turning movement	Non-fatal injury	Dry	South	Overtaking	Motorcycle	Other motor vehicle	0
					South	Turning left	Automobile, station wagon	Other motor vehicle	
2018-Jul-13, Fri,20:47	Clear	Rear end	P.D. only	Dry	North	Slowing or stoppin	g Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Aug-06, Mon,23:18	Clear	Rear end	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Oct-10, Wed,20:37	Clear	Rear end	P.D. only	Dry	East	Slowing or stoppin	g Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Automobile, station wagon	Other motor vehicle	
					South	Stopped	Unknown	Other motor vehicle	
2019-Mar-24, Sun,20:24	Clear	Other	P.D. only	Dry	North	Reversing	Pick-up truck	Other motor vehicle	0
					South	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Jun-26, Wed,16:40	Clear	Sideswipe	P.D. only	Dry	East	Unknown	Unknown	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	

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Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: BANK ST @ MACLAREN ST

Traffic Control: Traffic signal Total Collisions: 2

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2017-Sep-25, Mon,11:10	Clear	SMV unattended vehicle	Non-reportable	Dry	North	Turning left	Truck - tractor	Unattended vehicle	0
2017-Oct-17, Tue,16:39	Clear	Rear end	Non-fatal injury	Dry	South	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Stopped	Automobile, station wagon	Other motor vehicle	

Location: BANK ST btwn MACLAREN ST & GILMOUR ST

Traffic Control: No control

Total Collisions: 4

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2015-May-16, Sat,08:10	Clear	SMV unattended vehicle	P.D. only	Dry	East	Reversing	Delivery van	Unattended vehicle	0
2016-Sep-12, Mon,00:35	Clear	SMV unattended vehicle	P.D. only	Dry	South	Going ahead	Pick-up truck	Unattended vehicle	0
2017-Mar-26, Sun,03:04	Clear	Sideswipe	P.D. only	Dry	South	Going ahead	Automobile, station wagon	Other motor vehicle	0
					South	Stopped	Automobile, station wagon	Other motor vehicle	
2017-May-05, Fri,00:00	Clear	SMV unattended vehicle	P.D. only	Dry	Unknown	Unknown	Unknown	Unattended vehicle	0

Location: GILMOUR ST @ KENT ST

Traffic Control: Traffic signal Total Collisions: 5

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2015-Jan-23, Fri,06:35	Clear	Turning movement	P.D. only	Wet	North	Turning right	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Pick-up truck	Other motor vehicle	
2015-Oct-04, Sun,16:27	Clear	Other	P.D. only	Dry	South	Reversing	Police vehicle	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2016-Aug-21, Sun,16:33	Rain	SMV other	Non-fatal injury	Wet	East	Going ahead	Unknown	Pedestrian	1
2018-Sep-08, Sat,16:03	Clear	Sideswipe	P.D. only	Dry	North	Unknown	Unknown	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	

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Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: GILMOUR ST @ KENT ST

Traffic Control: Traffic signal Total Collisions: 5

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2019-Nov-17, Sun,09:07	Clear	Sideswipe	P.D. only	Dry	North	Turning right	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	

Location: GLADSTONE AVE @ KENT ST

Traffic Control: Traffic signal Total Collisions: 29

	9								
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2015-Jan-31, Sat,17:33	Clear	Sideswipe	P.D. only	Dry	North	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Pick-up truck	Other motor vehicle	
2015-Feb-20, Fri,18:19	Clear	Angle	P.D. only	Wet	East	Going ahead	Pick-up truck	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2015-Mar-29, Sun,21:53	Clear	Angle	Non-fatal injury	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2015-Apr-11, Sat,15:41	Clear	Rear end	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Changing lanes	Automobile, station wagon	Other motor vehicle	
2015-Jul-20, Mon,21:47	Clear	Angle	Non-fatal injury	Dry	North	Going ahead	Pick-up truck	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2015-Aug-07, Fri,23:30	Clear	Angle	P.D. only	Dry	North	Turning right	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Delivery van	Other motor vehicle	
2016-Jan-28, Thu,11:45	Snow	Rear end	P.D. only	Wet	North	Going ahead	Pick-up truck	Other motor vehicle	0
					North	Turning left	Automobile, station wagon	Other motor vehicle	
2016-Feb-26, Fri,12:19	Clear	Rear end	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Pick-up truck	Other motor vehicle	
2016-Apr-08, Fri,12:30	Clear	Angle	P.D. only	Dry	North	Slowing or stoppin	g Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	

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Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: GLADSTONE AVE @ KENT ST

Traffic Control: Traffic signal Total Collisions: 29

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2017-Jan-23, Mon,19:34	Clear	Turning movement	P.D. only	Dry	North	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Feb-09, Thu,12:11	Clear	Angle	P.D. only	Wet	East	Going ahead	Pick-up truck	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Feb-20, Mon,16:40	Clear	Angle	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Apr-25, Tue,13:36	Clear	Rear end	P.D. only	Dry	North	Turning right	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Jul-04, Tue,10:35	Clear	Angle	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Turning left	Pick-up truck	Other motor vehicle	
2017-Jul-06, Thu,20:04	Clear	Rear end	P.D. only	Dry	North	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					North	Turning right	Automobile, station wagon	Other motor vehicle	
2017-Sep-09, Sat,09:42	Clear	Angle	P.D. only	Dry	East	Going ahead	Passenger van	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Sep-30, Sat,21:13	Clear	Turning movement	P.D. only	Dry	North	Turning right	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Nov-10, Fri,08:27	Clear	Angle	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Dec-24, Sun,01:24	Clear	Angle	P.D. only	Wet	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Passenger van	Other motor vehicle	
2018-Apr-04, Wed,11:31	Freezing Rain	Rear end	P.D. only	Ice	East	Going ahead	Automobile, station wagon	Other motor vehicle	0
					East	Stopped	Pick-up truck	Other motor vehicle	
2018-Dec-04, Tue,19:00	Clear	Angle	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	

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Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: GLADSTONE AVE @ KENT ST

Traffic Control: Traffic signal Total Collisions: 29

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2019-Jan-10, Thu,18:05	Clear	Angle	P.D. only	Wet	North	Unknown	Automobile, station wagon	Other motor vehicle	0
					East	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Feb-26, Tue,08:40	Clear	Turning movement	P.D. only	Dry	North	Turning right	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-May-23, Thu,16:35	Rain	Sideswipe	P.D. only	Wet	North	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Turning left	Automobile, station wagon	Other motor vehicle	
2019-Jun-27, Thu,17:07	Clear	Rear end	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Jul-19, Fri,17:45	Clear	Sideswipe	P.D. only	Dry	North	Unknown	Unknown	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Oct-09, Wed,07:19	Clear	Sideswipe	P.D. only	Dry	North	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Pick-up truck	Other motor vehicle	
2019-Oct-20, Sun,18:07	Clear	Angle	P.D. only	Dry	North	Going ahead	Passenger van	Other motor vehicle	0
					West	Going ahead	Passenger van	Other motor vehicle	
2019-Dec-09, Mon,20:27	Rain	SMV other	Non-fatal injury	Wet	North	Turning left	Automobile, station wagon	Pedestrian	1

Location: KENT ST @ MACLAREN ST

Traffic Control: Stop sign

Total Collisions: 11

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2015-Jun-19, Fri,18:50	Clear	Rear end	P.D. only	Dry	North	Slowing or stoppin	g Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
					North	Going ahead	Pick-up truck	Other motor vehicle	
2015-Jul-20, Mon,16:00	Clear	Sideswipe	P.D. only	Dry	North	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Municipal transit bus	Other motor vehicle	

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Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: KENT ST @ MACLAREN ST

Traffic Control: Stop sign

Total Collisions: 11

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2015-Oct-08, Thu,12:08	Clear	Turning movement	P.D. only	Dry	North	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2015-Oct-09, Fri,09:29	Rain	Turning movement	P.D. only	Wet	North	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Truck-other	Other motor vehicle	
2017-Apr-14, Fri,12:39	Clear	Sideswipe	P.D. only	Dry	North	Changing lanes	Passenger van	Other motor vehicle	0
					North	Going ahead	Pick-up truck	Other motor vehicle	
2017-May-03, Wed,14:32	Clear	Angle	Non-fatal injury	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Jun-27, Wed,13:45	Clear	Sideswipe	P.D. only	Dry	North	Pulling away from shoulder or curb	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Passenger van	Other motor vehicle	
2018-Jul-18, Wed,15:30	Clear	Sideswipe	P.D. only	Dry	North	Changing lanes	Passenger van	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Jan-23, Wed,10:05	Snow	Turning movement	P.D. only	Loose snow	North	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Delivery van	Other motor vehicle	
2019-May-14, Tue,09:52	Clear	Angle	P.D. only	Dry	West	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Dec-05, Thu,11:16	Clear	Turning movement	P.D. only	Wet	North	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	

Location: KENT ST @ SOMERSET ST

Traffic Control: Traffic signal Total Collisions: 23

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2015-Mar-24, Tue,13:54	Clear	Sideswipe	P.D. only	Dry	North	Changing lanes	Passenger van	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	

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Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: KENT ST @ SOMERSET ST

Traffic Control: Traffic signal Total Collisions: 23

Trainic Control. Tra	ilic Signal						Total Comstons	Total Collisions. 23		
Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped	
2015-Jun-25, Thu,10:33	Clear	Sideswipe	P.D. only	Dry	North	Changing lanes	Automobile, station wagon	Other motor vehicle	0	
					North	Going ahead	Automobile, station wagon	Other motor vehicle		
2015-Dec-10, Thu,16:30	Clear	SMV other	Non-fatal injury	Wet	North	Turning left	Pick-up truck	Pedestrian	1	
2016-Jan-09, Sat,15:55	Rain	SMV other	Non-fatal injury	Wet	West	Turning right	Automobile, station wagon	Pedestrian	2	
2016-Feb-03, Wed,17:37	Clear	SMV other	Non-fatal injury	Wet	North	Turning left	Automobile, station wagon	Pedestrian	1	
2016-Apr-27, Wed,14:11	Clear	Turning movement	Non-fatal injury	Dry	East	Turning left	Automobile, station wagon	Other motor vehicle	0	
					West	Going ahead	Pick-up truck	Other motor vehicle		
2016-Jul-10, Sun,18:52	Clear	Angle	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0	
					North	Going ahead	Pick-up truck	Other motor vehicle		
2017-Mar-28, Tue,12:05	Clear	Turning movement	P.D. only	Wet	North	Turning left	Automobile, station wagon	Other motor vehicle	0	
					North	Going ahead	Delivery van	Other motor vehicle		
2017-Jun-24, Sat,15:13	Clear	Angle	P.D. only	Dry	East	Going ahead	Automobile, station wagon	Other motor vehicle	0	
					North	Going ahead	Automobile, station wagon	Other motor vehicle		
2017-Jul-06, Thu,22:12	Clear	SMV other	Non-fatal injury	Dry	North	Turning left	Pick-up truck	Pedestrian	1	
2017-Oct-09, Mon,14:17	Clear	Turning movement	P.D. only	Wet	North	Turning left	Automobile, station wagon	Other motor vehicle	0	
					North	Going ahead	Automobile, station wagon	Other motor vehicle		
2017-Dec-05, Tue,12:04	Rain	Rear end	Non-fatal injury	Wet	West	Slowing or stoppin	g Automobile, station wagon	Other motor vehicle	0	
					West	Stopped	Automobile, station wagon	Other motor vehicle		
2017-Dec-21, Thu,17:15	Clear	SMV other	Non-fatal injury	Slush	North	Turning left	Automobile, station wagon	Pedestrian	1	
2018-Jan-19, Fri,21:06	Clear	Turning movement	P.D. only	Wet	North	Turning left	Automobile, station wagon	Other motor vehicle	0	
					North	Going ahead	Automobile, station wagon	Other motor vehicle		
2018-May-03, Thu,09:24	Rain	Angle	P.D. only	Wet	West	Going ahead	Automobile, station wagon	Other motor vehicle	0	
					North	Going ahead	Automobile, station wagon	Other motor vehicle		

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Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: KENT ST @ SOMERSET ST

Traffic Control: Traffic signal Total Collisions: 23

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2018-May-05, Sat,15:13	Clear	Turning movement	Non-fatal injury	Dry	North	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2018-Aug-12, Sun,21:31	Clear	Turning movement	Non-fatal injury	Dry	West	Turning right	Automobile, station wagon	Cyclist	0
					West	Going ahead	Bicycle	Other motor vehicle	
2018-Aug-24, Fri,18:18	Clear	SMV unattended vehicle	P.D. only	Dry	East	Going ahead	Municipal transit bus	Unattended vehicle	0
2018-Dec-12, Wed,17:50	Clear	SMV other	Non-fatal injury	Dry	North	Turning left	Automobile, station wagon	Pedestrian	1
2019-Mar-21, Thu,08:40	Clear	Sideswipe	P.D. only	Dry	North	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Jun-02, Sun,18:30	Clear	Turning movement	P.D. only	Dry	North	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Oct-13, Sun,12:53	Clear	Rear end	P.D. only	Dry	North	Slowing or stoppin	g Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Dec-19, Thu,20:59	Clear	Sideswipe	P.D. only	Dry	North	Turning left	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	

Location: KENT ST btwn FLORENCE ST & GLADSTONE AVE

Traffic Control: No control Total Collisions: 1

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2015-Sep-03, Thu,23:30	Clear	Sideswipe	P.D. only	Dry	North	Changing lanes	Passenger van	Other motor vehicle	0
					North	Unknown	Automobile, station wagon	Other motor vehicle	

Location: KENT ST btwn GILMOUR ST & JAMES ST

Traffic Control: No control

Total Collisions: 3

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuver Vehicle type	First Event	No. Ped
				Cond'n				

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Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: KENT ST btwn GILMOUR ST & JAMES ST

Traffic Control: No control

Total Collisions: 3

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2015-Mar-05, Thu,10:03	Clear	SMV unattended vehicle	Non-fatal injury	Dry	North	Changing lanes	Pick-up truck	Unattended vehicle	0
2017-Mar-24, Fri,16:55	Clear	SMV other	P.D. only	Loose snow	North	Overtaking	Automobile, station wagon	Pole (sign, parking meter	er) 0
2019-Dec-20, Fri,12:30	Clear	Sideswipe	P.D. only	Dry	North	Pulling away from shoulder or curb	Unknown	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	

Location: KENT ST btwn JAMES ST & FLORENCE ST

Traffic Control: No control

Total Collisions: 5

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	Vehicle type	First Event	No. Ped
2015-Nov-03, Tue,07:53	Clear	SMV unattended vehicle	P.D. only	Dry	South	Reversing	Truck - open	Unattended vehicle	0
2016-Jul-13, Wed,16:42	Clear	Rear end	Non-fatal injury	Dry	West	Going ahead	Pick-up truck	Other motor vehicle	0
					West	Slowing or stopping	Pick-up truck	Other motor vehicle	
					West	Slowing or stopping	Passenger van	Other motor vehicle	
2017-Jun-14, Wed,00:00	Clear	SMV unattended vehicle	P.D. only	Dry	North	Unknown	Unknown	Unattended vehicle	0
2017-Oct-28, Sat,05:03	Clear	Sideswipe	P.D. only	Dry	North	Pulling away from shoulder or curb	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Passenger van	Other motor vehicle	
2018-Jun-30, Sat,00:00	Clear	SMV unattended vehicle	P.D. only	Dry	Unknown	Unknown	Unknown	Unattended vehicle	0

Location: KENT ST btwn MACLAREN ST & GILMOUR ST

Traffic Control: No control

Total Collisions: 5

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2016-Jun-17, Fri,14:04	Clear	Rear end	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Slowing or stoppin	g Pick-up truck	Other motor vehicle	

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Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: KENT ST btwn MACLAREN ST & GILMOUR ST

Traffic Control: No control

Total Collisions: 5

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2017-May-31, Wed,21:12	Clear	Sideswipe	P.D. only	Dry	North	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2017-Dec-02, Sat,14:01	Clear	SMV unattended vehicle	P.D. only	Dry	North	Going ahead	Automobile, station wagon	Unattended vehicle	0
2019-Jul-26, Fri,16:17	Clear	Approaching	P.D. only	Dry	South	Unknown	Pick-up truck	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	
2019-Nov-01, Fri,06:55	Rain	Rear end	P.D. only	Wet	North	Going ahead	Automobile, station wagon	Other motor vehicle	0
					North	Stopped	Automobile, station wagon	Other motor vehicle	

Location: KENT ST btwn SOMERSET ST W & MACLAREN ST

Traffic Control: No control

Total Collisions: 3

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	Vehicle type	First Event	No. Ped
2018-Dec-15, Sat,13:45	Clear	Sideswipe	P.D. only	Dry	North	Changing lanes	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Sep-11, Wed,00:00	Clear	SMV unattended vehicle	P.D. only	Dry	Unknown	Unknown	Unknown	Unattended vehicle	0
2019-Nov-10, Sun,00:35	Clear	Sideswipe	P.D. only	Dry	North	Pulling away from shoulder or curb	Automobile, station wagon	Other motor vehicle	0
					North	Going ahead	Automobile, station wagon	Other motor vehicle	

Location: MACLAREN ST btwn KENT ST & BANK ST

Traffic Control: No control Total Collisions: 10

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	r Vehicle type	First Event	No. Ped
2015-May-14, Thu,11:47	Clear	SMV unattended vehicle	P.D. only	Dry	West	Pulling onto shoulder or toward curb	Pick-up truck I	Unattended vehicle	0
2015-Sep-09, Wed,00:00	Clear	SMV unattended vehicle	P.D. only	Dry	Unknown	Unknown	Unknown	Unattended vehicle	0

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Collision Details Report - Public Version

From: January 1, 2015 **To:** December 31, 2019

Location: MACLAREN ST btwn KENT ST & BANK ST

Traffic Control: No control

Total Collisions: 10

Date/Day/Time	Environment	Impact Type	Classification	Surface Cond'n	Veh. Dir	Vehicle Manoeuve	er Vehicle type	First Event	No. Ped
2016-Mar-08, Tue,00:00	Rain	SMV unattended vehicle	P.D. only	Wet	Unknown	Unknown	Unknown	Unattended vehicle	0
2016-Apr-22, Fri,13:50	Clear	SMV unattended vehicle	P.D. only	Dry	South	Reversing	Unknown	Unattended vehicle	0
2016-May-16, Mon,18:46	Clear	SMV unattended vehicle	P.D. only	Dry	East	Reversing	Automobile, station wagon	Unattended vehicle	0
2017-Aug-23, Wed,18:05	Clear	Other	P.D. only	Dry	East	Reversing	Automobile, station wagon	Other motor vehicle	0
					West	Stopped	Automobile, station wagon	Other motor vehicle	
2018-Feb-08, Thu,00:00	Clear	SMV unattended vehicle	P.D. only	Wet	Unknown	Unknown	Unknown	Unattended vehicle	0
2018-Apr-08, Sun,00:00	Clear	SMV unattended vehicle	P.D. only	Dry	East	Unknown	Unknown	Unattended vehicle	0
2019-Jan-23, Wed,14:30	Snow	Angle	P.D. only	Packed snow	South	Reversing	Automobile, station wagon	Other motor vehicle	0
					West	Going ahead	Automobile, station wagon	Other motor vehicle	
2019-Mar-18, Mon,00:00	Clear	SMV unattended vehicle	P.D. only	Dry	Unknown	Unknown	Unknown	Unattended vehicle	0

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APPENDIX D TDM CHECKLISTS

TDM-Supportive Development Design and Infrastructure Checklist:

Residential Developments (multi-family or condominium)

Legend The Official Plan or Zoning By-law provides related guidance that must be followed BASIC The measure is generally feasible and effective, and in most cases would benefit the development and its users The measure could maximize support for users of sustainable modes, and optimize development performance

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	1.	WALKING & CYCLING: ROUTES	
	1.1	Building location & access points	
BASIC	1.1.1	Locate building close to the street, and do not locate parking areas between the street and building entrances	
BASIC	1.1.2	Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	
BASIC	1.1.3	Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	
	1.2	Facilities for walking & cycling	
REQUIRED	1.2.1	Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see Official Plan policy 4.3.3)	
REQUIRED	1.2.2	Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (see Official Plan policy 4.3.12)	

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
REQUIRED	1.2.3	Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see Official Plan policy 4.3.10)	
REQUIRED	1.2.4	Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see Official Plan policy 4.3.10)	
REQUIRED	1.2.5	Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and onroad cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see Official Plan policy 4.3.11)	
BASIC	1.2.6	Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	
BASIC	1.2.7	Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible	
BASIC	1.2.8	Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility	
	1.3	Amenities for walking & cycling	
BASIC	1.3.1	Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails	
BASIC	1.3.2	Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)	

	TDM-s	supportive design & infrastructure measures: Residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	2.	WALKING & CYCLING: END-OF-TRIP FACILI	TIES
	2.1	Bicycle parking	<u> </u>
REQUIRED	2.1.1	Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see Official Plan policy 4.3.6)	
REQUIRED	2.1.2	Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well-used areas (see Zoning By-law Section 111)	
REQUIRED	2.1.3	Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored (see Zoning By-law Section 111)	
BASIC	2.1.4	Provide bicycle parking spaces equivalent to the expected number of resident-owned bicycles, plus the expected peak number of visitor cyclists	
	2.2	Secure bicycle parking	
REQUIRED	2.2.1	Where more than 50 bicycle parking spaces are provided for a single residential building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see Zoning By-law Section 111)	
BETTER	2.2.2	Provide secure bicycle parking spaces equivalent to at least the number of units at condominiums or multifamily residential developments	
	2.3	Bicycle repair station	
BETTER	2.3.1	Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	
	3.	TRANSIT	
	3.1	Customer amenities	
BASIC	3.1.1	Provide shelters, lighting and benches at any on-site transit stops	
BASIC	3.1.2	Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter	No stop near development.
BETTER	3.1.3	Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building	

	TDM-s	supportive design & infrastructure measures: Residential developments		Check if completed & descriptions, explanations plan/drawing references
	4.	RIDESHARING		
	4.1	Pick-up & drop-off facilities		
BASIC	4.1.1	Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones		
	5.	CARSHARING & BIKESHARING		
	5.1	Carshare parking spaces		
BETTER	5.1.1	Provide up to three carshare parking spaces in an R3, R4 or R5 Zone for specified residential uses (see Zoning By-law Section 94)		
	5.2	Bikeshare station location		
BETTER	5.2.1	Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection		
	6.	PARKING		
	6.1	Number of parking spaces		
REQUIRED	6.1.1	Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for		Currently providing a parking ratio of 0.7 residents and 0.1 visitor.
BASIC	6.1.2	Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking		
BASIC	6.1.3	Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (see Zoning By-law Section 104)		
BETTER	6.1.4	Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (see Zoning By-law Section 111)		
	6.2	Separate long-term & short-term parking areas	:	
BETTER	6.2.1	Provide separate areas for short-term and long-term parking (using signage or physical barriers) to permit access controls and simplify enforcement (i.e. to discourage residents from parking in visitor spaces, and vice versa)		

TDM-Supportive Development Design and Infrastructure Checklist:

Non-Residential Developments (office, institutional, retail or industrial)

Legend			
REQUIRED	The Official Plan or Zoning By-law provides related guidance that must be followed		
BASIC	The measure is generally feasible and effective, and in most cases would benefit the development and its users		
BETTER	The measure could maximize support for users of sustainable modes, and optimize development performance		

	TDM-supportive design & infrastructure measures: Non-residential developments		Check if completed & add descriptions, explanations or plan/drawing references
	1.	WALKING & CYCLING: ROUTES	
	1.1	Building location & access points	
BASIC	1.1.1	Locate building close to the street, and do not locate parking areas between the street and building entrances	
BASIC	1.1.2	Locate building entrances in order to minimize walking distances to sidewalks and transit stops/stations	
BASIC	1.1.3	Locate building doors and windows to ensure visibility of pedestrians from the building, for their security and comfort	
	1.2	Facilities for walking & cycling	
REQUIRED	1.2.1	Provide convenient, direct access to stations or major stops along rapid transit routes within 600 metres; minimize walking distances from buildings to rapid transit; provide pedestrian-friendly, weather-protected (where possible) environment between rapid transit accesses and building entrances; ensure quality linkages from sidewalks through building entrances to integrated stops/stations (see Official Plan policy 4.3.3)	
REQUIRED	1.2.2	Provide safe, direct and attractive pedestrian access from public sidewalks to building entrances through such measures as: reducing distances between public sidewalks and major building entrances; providing walkways from public streets to major building entrances; within a site, providing walkways along the front of adjoining buildings, between adjacent buildings, and connecting areas where people may congregate, such as courtyards and transit stops; and providing weather protection through canopies, colonnades, and other design elements wherever possible (see Official Plan policy 4.3.12)	

	TDM-supportive design & infrastructure measures: Non-residential developments			Check if completed & lescriptions, explanations plan/drawing references
REQUIRED	1.2.3	Provide sidewalks of smooth, well-drained walking surfaces of contrasting materials or treatments to differentiate pedestrian areas from vehicle areas, and provide marked pedestrian crosswalks at intersection sidewalks (see Official Plan policy 4.3.10)		
REQUIRED	1.2.4	Make sidewalks and open space areas easily accessible through features such as gradual grade transition, depressed curbs at street corners and convenient access to extra-wide parking spaces and ramps (see Official Plan policy 4.3.10)		
REQUIRED	1.2.5	Include adequately spaced inter-block/street cycling and pedestrian connections to facilitate travel by active transportation. Provide links to the existing or planned network of public sidewalks, multi-use pathways and onroad cycle routes. Where public sidewalks and multi-use pathways intersect with roads, consider providing traffic control devices to give priority to cyclists and pedestrians (see Official Plan policy 4.3.11)		
BASIC	1.2.6	Provide safe, direct and attractive walking routes from building entrances to nearby transit stops	V	
BASIC	1.2.7	Ensure that walking routes to transit stops are secure, visible, lighted, shaded and wind-protected wherever possible		Will provide secure and lighted within property limits.
BASIC	1.2.8	Design roads used for access or circulation by cyclists using a target operating speed of no more than 30 km/h, or provide a separated cycling facility		
	1.3	Amenities for walking & cycling		
BASIC	1.3.1	Provide lighting, landscaping and benches along walking and cycling routes between building entrances and streets, sidewalks and trails		
BASIC	1.3.2	Provide wayfinding signage for site access (where required, e.g. when multiple buildings or entrances exist) and egress (where warranted, such as when directions to reach transit stops/stations, trails or other common destinations are not obvious)		

	TDM-s	supportive design & infrastructure measures: Non-residential developments	Check if completed & add descriptions, explanations or plan/drawing references
	2.	WALKING & CYCLING: END-OF-TRIP FACILI	TIES
	2.1	Bicycle parking	<u> </u>
REQUIRED	2.1.1	Provide bicycle parking in highly visible and lighted areas, sheltered from the weather wherever possible (see Official Plan policy 4.3.6)	
REQUIRED	2.1.2	Provide the number of bicycle parking spaces specified for various land uses in different parts of Ottawa; provide convenient access to main entrances or well-used areas (see Zoning By-law Section 111)	
REQUIRED	2.1.3	Ensure that bicycle parking spaces and access aisles meet minimum dimensions; that no more than 50% of spaces are vertical spaces; and that parking racks are securely anchored (see Zoning By-law Section 111)	
BASIC	2.1.4	Provide bicycle parking spaces equivalent to the expected number of commuter cyclists (assuming the cycling mode share target is met), plus the expected peak number of customer/visitor cyclists	
BETTER	2.1.5	Provide bicycle parking spaces equivalent to the expected number of commuter and customer/visitor cyclists, plus an additional buffer (e.g. 25 percent extra) to encourage other cyclists and ensure adequate capacity in peak cycling season	
	2.2	Secure bicycle parking	
REQUIRED	2.2.1	Where more than 50 bicycle parking spaces are provided for a single office building, locate at least 25% of spaces within a building/structure, a secure area (e.g. supervised parking lot or enclosure) or bicycle lockers (see Zoning By-law Section 111)	
BETTER	2.2.2	Provide secure bicycle parking spaces equivalent to the expected number of commuter cyclists (assuming the cycling mode share target is met)	
	2.3	Shower & change facilities	
BASIC	2.3.1	Provide shower and change facilities for the use of active commuters	
BETTER	2.3.2	In addition to shower and change facilities, provide dedicated lockers, grooming stations, drying racks and laundry facilities for the use of active commuters	
	2.4	Bicycle repair station	
BETTER	2.4.1	Provide a permanent bike repair station, with commonly used tools and an air pump, adjacent to the main bicycle parking area (or secure bicycle parking area, if provided)	

	TDM-s	supportive design & infrastructure measures: Non-residential developments	add de	heck if completed & escriptions, explanations lan/drawing references
	3.	TRANSIT		
	3.1	Customer amenities		
BASIC	3.1.1	Provide shelters, lighting and benches at any on-site transit stops		Not applicable.
BASIC	3.1.2	Where the site abuts an off-site transit stop and insufficient space exists for a transit shelter in the public right-of-way, protect land for a shelter and/or install a shelter		Not applicable.
BETTER	3.1.3	Provide a secure and comfortable interior waiting area by integrating any on-site transit stops into the building		Not applicable.
	4.	RIDESHARING		
	4.1	Pick-up & drop-off facilities		
BASIC	4.1.1	Provide a designated area for carpool drivers (plus taxis and ride-hailing services) to drop off or pick up passengers without using fire lanes or other no-stopping zones		
	4.2	Carpool parking		
BASIC	4.2.1	Provide signed parking spaces for carpools in a priority location close to a major building entrance, sufficient in number to accommodate the mode share target for carpools		
BETTER	4.2.2	At large developments, provide spaces for carpools in a separate, access-controlled parking area to simplify enforcement		
	5.	CARSHARING & BIKESHARING		
	5.1	Carshare parking spaces		
BETTER	5.1.1	Provide carshare parking spaces in permitted non-residential zones, occupying either required or provided parking spaces (see Zoning By-law Section 94)		Will be considered in site plan phase.
	5.2	Bikeshare station location		
BETTER	5.2.1	Provide a designated bikeshare station area near a major building entrance, preferably lighted and sheltered with a direct walkway connection		

TDM-supportive design & infrastructure measures: Non-residential developments			Check if completed & add descriptions, explanations or plan/drawing references
	6.	PARKING	
	6.1	Number of parking spaces	
REQUIRED	6.1.1	Do not provide more parking than permitted by zoning, nor less than required by zoning, unless a variance is being applied for	
BASIC	6.1.2	Provide parking for long-term and short-term users that is consistent with mode share targets, considering the potential for visitors to use off-site public parking	
BASIC	6.1.3	Where a site features more than one use, provide shared parking and reduce the cumulative number of parking spaces accordingly (see Zoning By-law Section 104)	
BETTER	6.1.4	Reduce the minimum number of parking spaces required by zoning by one space for each 13 square metres of gross floor area provided as shower rooms, change rooms, locker rooms and other facilities for cyclists in conjunction with bicycle parking (see Zoning By-law Section 111)	
	6.2	Separate long-term & short-term parking areas	
BETTER	6.2.1	Separate short-term and long-term parking areas using signage or physical barriers, to permit access controls and simplify enforcement (i.e. to discourage employees from parking in visitor spaces, and vice versa)	
	7.	OTHER	
	7.1	On-site amenities to minimize off-site trips	
BETTER	7.1.1	Provide on-site amenities to minimize mid-day or mid-commute errands	

TDM Measures Checklist:

Residential Developments (multi-family, condominium or subdivision)

EASIC The measure is generally feasible and effective, and in most cases would benefit the development and its users The measure could maximize support for users of sustainable modes, and optimize development performance The measure is one of the most dependably effective tools to encourage the use of sustainable modes

	TDM	measures: Residential developments	Check if proposed & add descriptions
	1.	TDM PROGRAM MANAGEMENT	
	1.1	Program coordinator	
BASIC	★ 1.1.1	Designate an internal coordinator, or contract with an external coordinator	
	1.2	Travel surveys	
BETTER	1.2.1	Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress	
	2.	WALKING AND CYCLING	
	2.1	Information on walking/cycling routes & des	tinations
BASIC	2.1.1	Display local area maps with walking/cycling access routes and key destinations at major entrances (multi-family, condominium)	
	2.2	Bicycle skills training	
BETTER	2.2.1	Offer on-site cycling courses for residents, or subsidize off-site courses	

	TDM	measures: Residential developments	Check if proposed & add descriptions
	3.	TRANSIT	
	3.1	Transit information	
BASIC	3.1.1	Display relevant transit schedules and route maps at entrances (multi-family, condominium)	
BETTER	3.1.2	Provide real-time arrival information display at entrances (multi-family, condominium)	
	3.2	Transit fare incentives	
BASIC ★	3.2.1	Offer PRESTO cards preloaded with one monthly transit pass on residence purchase/move-in, to encourage residents to use transit	
BETTER	3.2.2	Offer at least one year of free monthly transit passes on residence purchase/move-in	
	3.3	Enhanced public transit service	
BETTER ★	3.3.1	Contract with OC Transpo to provide early transit services until regular services are warranted by occupancy levels (subdivision)	
	3.4	Private transit service	
BETTER	3.4.1	Provide shuttle service for seniors homes or lifestyle communities (e.g. scheduled mall or supermarket runs)	
	4.	CARSHARING & BIKESHARING	
	4.1	Bikeshare stations & memberships	
BETTER	4.1.1	Contract with provider to install on-site bikeshare station (multi-family)	
BETTER	4.1.2	Provide residents with bikeshare memberships, either free or subsidized (multi-family)	
	4.2	Carshare vehicles & memberships	:
BETTER	4.2.1	Contract with provider to install on-site carshare vehicles and promote their use by residents	Will investigate at site plan phase.
BETTER	4.2.2	Provide residents with carshare memberships, either free or subsidized	
	5.	PARKING	
	5.1	Priced parking	
BASIC *	5.1.1	Unbundle parking cost from purchase price (condominium)	
BASIC ★	5.1.2	Unbundle parking cost from monthly rent (multi-family)	

TDM	measures: Residential developments	Check if proposed & add descriptions
6.	TDM MARKETING & COMMUNICATION	IS
6.1	Multimodal travel information	•
BASIC ★ 6.1.1	Provide a multimodal travel option information package to new residents	
6.2	Personalized trip planning	
BETTER ★ 6.2.1	Offer personalized trip planning to new residents	

TDM Measures Checklist:

Non-Residential Developments (office, institutional, retail or industrial)

BASIC The measure is generally feasible and effective, and in most cases would benefit the development and its users The measure could maximize support for users of sustainable modes, and optimize development performance The measure is one of the most dependably effective tools to encourage the use of sustainable modes

	TDM	measures: Non-residential developments	Check if proposed & add descriptions
	1.	TDM PROGRAM MANAGEMENT	
	1.1	Program coordinator	
BASIC	★ 1.1.1	Designate an internal coordinator, or contract with an external coordinator	
	1.2	Travel surveys	
BETTER	1.2.1	Conduct periodic surveys to identify travel-related behaviours, attitudes, challenges and solutions, and to track progress	
	2.	WALKING AND CYCLING	
	2.1	Information on walking/cycling routes & destin	ations
BASIC	2.1.1	Display local area maps with walking/cycling access routes and key destinations at major entrances	
	2.2	Bicycle skills training	
		Commuter travel	
BETTER	★ 2.2.1	Offer on-site cycling courses for commuters, or subsidize off-site courses	
	2.3	Valet bike parking	
		Visitor travel	
BETTER	2.3.1	Offer secure valet bike parking during public events when demand exceeds fixed supply (e.g. for festivals, concerts, games)	

	TDM	measures: Non-residential developments	Check if proposed & add descriptions
	3.	TRANSIT	
	3.1	Transit information	
BASIC	3.1.1	Display relevant transit schedules and route maps at entrances	
BASIC	3.1.2	Provide online links to OC Transpo and STO information	
BETTER	3.1.3	Provide real-time arrival information display at entrances	
	3.2	Transit fare incentives	
		Commuter travel	
BETTER	3.2.1	Offer preloaded PRESTO cards to encourage commuters to use transit	
BETTER ★	3.2.2	Subsidize or reimburse monthly transit pass purchases by employees	
		Visitor travel	
BETTER	3.2.3	Arrange inclusion of same-day transit fare in price of tickets (e.g. for festivals, concerts, games)	
	3.3	Enhanced public transit service	
		Commuter travel	
BETTER	3.3.1	Contract with OC Transpo to provide enhanced transit services (e.g. for shift changes, weekends)	
		Visitor travel	
BETTER	3.3.2	Contract with OC Transpo to provide enhanced transit services (e.g. for festivals, concerts, games)	
	3.4	Private transit service	
		Commuter travel	
BETTER	3.4.1	Provide shuttle service when OC Transpo cannot offer sufficient quality or capacity to serve demand (e.g. for shift changes, weekends)	
		Visitor travel	
BETTER	3.4.2	Provide shuttle service when OC Transpo cannot offer sufficient quality or capacity to serve demand (e.g. for festivals, concerts, games)	

	TDM	measures: Non-residential developments	Check if proposed & add descriptions
	4.	RIDESHARING	
	4.1	Ridematching service	
BASIC *	4.1.1	Commuter travel Provide a dedicated ridematching portal at OttawaRideMatch.com	
	4.2	Carpool parking price incentives	•
		Commuter travel	
BETTER	4.2.1	Provide discounts on parking costs for registered carpools	
	4.3	Vanpool service	
		Commuter travel	
BETTER	4.3.1	Provide a vanpooling service for long-distance commuters	
	5.	CARSHARING & BIKESHARING	
	5.1	Bikeshare stations & memberships	
BETTER	5.1.1	Contract with provider to install on-site bikeshare station for use by commuters and visitors	
		Commuter travel	
BETTER	5.1.2	Provide employees with bikeshare memberships for local business travel	
	5.2	Carshare vehicles & memberships	
		Commuter travel	
BETTER	5.2.1	Contract with provider to install on-site carshare vehicles and promote their use by tenants	
BETTER	5.2.2	Provide employees with carshare memberships for local business travel	
	6.	PARKING	
	6.1	Priced parking	
		Commuter travel	
BASIC ★	6.1.1	Charge for long-term parking (daily, weekly, monthly)	
BASIC	6.1.2	Unbundle parking cost from lease rates at multi-tenant sites	
		Visitor travel	
BETTER	6.1.3	Charge for short-term parking (hourly)	

	TDM	measures: Non-residential developments	Check if proposed & add descriptions
	7.	TDM MARKETING & COMMUNICATIONS	
	7.1	Multimodal travel information	
		Commuter travel	
BASIC *	7.1.1	Provide a multimodal travel option information package to new/relocating employees and students	
		Visitor travel	:
BETTER ★	7.1.2	Include multimodal travel option information in invitations or advertising that attract visitors or customers (e.g. for festivals, concerts, games)	
	7.2	Personalized trip planning	
		Commuter travel	
BETTER ★	7.2.1	Offer personalized trip planning to new/relocating employees	
	7.3	Promotions	
		Commuter travel	
BETTER	7.3.1	Deliver promotions and incentives to maintain awareness, build understanding, and encourage trial of sustainable modes	
	8.	OTHER INCENTIVES & AMENITIES	
	8.1	Emergency ride home	
	8.1	Emergency ride home Commuter travel	
BETTER ★	8.1 8.1.1		
BETTER ★		Commuter travel Provide emergency ride home service to non-driving	
BETTER ★	8.1.1	Commuter travel Provide emergency ride home service to non-driving commuters	
	8.1.1	Commuter travel Provide emergency ride home service to non-driving commuters Alternative work arrangements Commuter travel	
	8.1.1 8.2 8.2.1	Commuter travel Provide emergency ride home service to non-driving commuters Alternative work arrangements Commuter travel	
BASIC ★ BETTER	8.1.1 8.2 8.2.1 8.2.2	Commuter travel Provide emergency ride home service to non-driving commuters Alternative work arrangements Commuter travel Encourage flexible work hours	
BASIC ★ BETTER	8.1.1 8.2 8.2.1 8.2.2	Commuter travel Provide emergency ride home service to non-driving commuters Alternative work arrangements Commuter travel Encourage flexible work hours Encourage compressed workweeks	
BASIC ★ BETTER	8.1.1 8.2 8.2.1 8.2.2 8.2.3	Commuter travel Provide emergency ride home service to non-driving commuters Alternative work arrangements Commuter travel Encourage flexible work hours Encourage compressed workweeks Encourage telework	
BASIC ★ BETTER BETTER ★	8.1.1 8.2 8.2.1 8.2.2 8.2.3	Provide emergency ride home service to non-driving commuters Alternative work arrangements Commuter travel Encourage flexible work hours Encourage compressed workweeks Encourage telework Local business travel options	
BASIC ★ BETTER BETTER ★	8.1.1 8.2 8.2.1 8.2.2 8.2.3 8.3	Commuter travel Provide emergency ride home service to non-driving commuters Alternative work arrangements Commuter travel Encourage flexible work hours Encourage compressed workweeks Encourage telework Local business travel options Commuter travel Provide local business travel options that minimize the	
BASIC ★ BETTER BETTER ★	8.1.1 8.2 8.2.1 8.2.2 8.2.3 8.3	Provide emergency ride home service to non-driving commuters Alternative work arrangements Commuter travel Encourage flexible work hours Encourage compressed workweeks Encourage telework Local business travel options Commuter travel Provide local business travel options that minimize the need for employees to bring a personal car to work	
BASIC ★ BETTER BETTER ★	8.1.1 8.2 8.2.1 8.2.2 8.2.3 8.3	Provide emergency ride home service to non-driving commuters Alternative work arrangements Commuter travel Encourage flexible work hours Encourage compressed workweeks Encourage telework Local business travel options Commuter travel Provide local business travel options that minimize the need for employees to bring a personal car to work Commuter incentives	
BASIC ★ BETTER ★ BETTER ★	8.1.1 8.2 8.2.1 8.2.2 8.2.3 8.3 8.3.1	Provide emergency ride home service to non-driving commuters Alternative work arrangements Commuter travel Encourage flexible work hours Encourage compressed workweeks Encourage telework Local business travel options Commuter travel Provide local business travel options that minimize the need for employees to bring a personal car to work Commuter travel Offer employees a taxable, mode-neutral commuting	
BASIC ★ BETTER ★ BETTER ★	8.1.1 8.2 8.2.1 8.2.2 8.2.3 8.3.1 8.4 8.4.1	Provide emergency ride home service to non-driving commuters Alternative work arrangements Commuter travel Encourage flexible work hours Encourage compressed workweeks Encourage telework Local business travel options Commuter travel Provide local business travel options that minimize the need for employees to bring a personal car to work Commuter incentives Commuter travel Offer employees a taxable, mode-neutral commuting allowance	





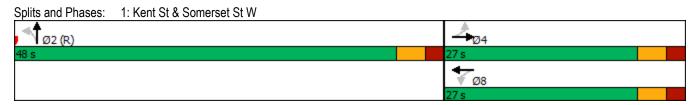
	•	-	←	1	†
Lane Group	EBL	EBT	WBT	NBL	NBT
Lane Configurations	ሻ	1 >	43-	ች	ተተ _ጉ
Traffic Volume (vph)	62	295	134	75	1554
Future Volume (vph)	62	295	134	75	1554
Lane Group Flow (vph)	69	328	205	83	1931
Turn Type	Perm	NA	NA	Perm	NA
Protected Phases		4	8		2
Permitted Phases	4			2	
Minimum Split (s)	23.5	23.5	23.5	23.4	23.4
Total Split (s)	27.0	27.0	27.0	48.0	48.0
Total Split (%)	36.0%	36.0%	36.0%	64.0%	64.0%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	2.1	2.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.5	5.5	5.5	5.4	5.4
Lead/Lag					
Lead-Lag Optimize?					
Act Effct Green (s)	21.5	21.5	21.5	42.6	42.6
Actuated g/C Ratio	0.29	0.29	0.29	0.57	0.57
v/c Ratio	0.26	0.64	0.43	0.13	0.73
Control Delay	23.9	30.2	25.3	13.7	17.3
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	23.9	30.2	25.3	13.7	17.3
LOS	С	С	С	В	В
Approach Delay		29.1	25.3		17.1
Approach LOS		С	С		В
Queue Length 50th (m)	7.5	40.4	23.5	6.9	57.9
Queue Length 95th (m)	17.7	66.1	41.5	m11.0	75.4
Internal Link Dist (m)		97.2	154.7		68.5
Turn Bay Length (m)	20.0			20.0	
Base Capacity (vph)	264	511	474	628	2652
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.26	0.64	0.43	0.13	0.73
Intersection Summary					
Cycle Length: 75					
Actuated Cycle Length: 75					
Offset: 66 (88%), Reference	ed to phase	e 2:NBTI	. Start of	Green	
Natural Cycle: 60	ou to prido		, June 31	J. 0011	

Natural Cycle: 60 Control Type: Pretimed Maximum v/c Ratio: 0.73

Intersection Signal Delay: 19.6 Intersection LOS: B Intersection Capacity Utilization 80.1% Analysis Period (min) 15 ICU Level of Service D

Synchro 10 Report Parsons

m Volume for 95th percentile queue is metered by upstream signal.



	←	†
Lane Group	WBT	NBT
Lane Configurations	ĵ.	414
Traffic Volume (vph)	14	1794
Future Volume (vph)	14	1794
Lane Group Flow (vph)	39	2030
Sign Control	Stop	Free
Intersection Summary		
Control Type: Unsignalize	ed	
Intersection Capacity Util	ization 51.9%	

Analysis Period (min) 15

	٠	→	•	•	←	•	4	†	<i>></i>	/	ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					f)			₽₽₽				
Traffic Volume (veh/h)	0	0	0	0	14	21	33	1794	0	0	0	0
Future Volume (Veh/h)	0	0	0	0	14	21	33	1794	0	0	0	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	0	0	16	23	37	1993	0	0	0	0
Pedestrians		92			133			15			8	
Lane Width (m)		0.0			3.7			3.7			0.0	
Walking Speed (m/s)		1.1			1.1			1.1			1.1	
Percent Blockage		0			12			1			0	
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								78			92	
pX, platoon unblocked	0.70	0.70		0.70	0.70	0.70				0.70		
vC, conflicting volume	869	2292	107	2215	2292	805	92			2126		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	0	1330	107	1219	1330	0	92			1091		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	100	82	97	98			100		
cM capacity (veh/h)	530	91	913	73	91	661	1501			387		
Direction, Lane #	WB 1	NB 1	NB 2	NB 3								
Volume Total	39	436	797	797								
Volume Left	0	37	0	0								
Volume Right	23	0	0	0								
cSH	186	1501	1700	1700								
Volume to Capacity	0.21	0.02	0.47	0.47								
Queue Length 95th (m)	5.8	0.6	0.0	0.0								
Control Delay (s)	29.5	0.9	0.0	0.0								
Lane LOS	D	Α										
Approach Delay (s)	29.5	0.2										
Approach LOS	D											
Intersection Summary												
Average Delay			0.7									
Intersection Capacity Utiliza	ation		51.9%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

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Lane Group	EBT	NBT
Lane Configurations	4	ተተኈ
Traffic Volume (vph)	85	1812
Future Volume (vph)	85	1812
Lane Group Flow (vph)	113	2184
Turn Type	NA	NA
Protected Phases	4	2
Permitted Phases		
Minimum Split (s)	21.5	21.1
Total Split (s)	22.0	53.0
Total Split (%)	29.3%	70.7%
Yellow Time (s)	3.3	3.3
All-Red Time (s)	2.2	1.8
Lost Time Adjust (s)	0.0	0.0
Total Lost Time (s)	5.5	5.1
Lead/Lag		
Lead-Lag Optimize?		
Act Effct Green (s)	16.5	47.9
Actuated g/C Ratio	0.22	0.64
v/c Ratio	0.28	0.72
Control Delay	20.1	23.1
Queue Delay	0.0	0.0
Total Delay	20.1	23.1
LOS	С	С
Approach Delay	20.1	23.1
Approach LOS	С	С
Queue Length 50th (m)	9.6	118.7
Queue Length 95th (m)	22.3	132.1
Internal Link Dist (m)	157.9	224.2
Turn Bay Length (m)		
Base Capacity (vph)	410	3025
Starvation Cap Reductn	0	0
Spillback Cap Reductn	0	0
Storage Cap Reductn	0	0
Reduced v/c Ratio	0.28	0.72
Intersection Summary		
Cycle Length: 75		
Actuated Cycle Length: 75	·	
Offset: 5 (7%), Referenced		NBT. Sta
Natural Cycle: 60	2 10 pha00 Z.	b i , ota
Control Type: Pretimed		
Maximum v/c Ratio: 0.72		
Intersection Signal Delay:	22.9	
Intersection Capacity Utiliz		
Analysis Period (min) 15	-5.1011 00.2 /0	
, ,		
Splits and Phases: 3: Ke	ent St & Gilm	our St
T ø2 (R)		
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Lane Group	EBL	EBT	WBT	NBL	NBT		
Lane Configurations	<u> </u>	<u></u>	1	ሻ	411		
Traffic Volume (vph)	82	277	165	36	1698		
Future Volume (vph)	82	277	165	36	1698		
Lane Group Flow (vph)	91	308	347	40	1995		
Turn Type	Perm	NA	NA	Perm	NA		
Protected Phases		4	8		2		
Permitted Phases	4			2			
Minimum Split (s)	23.4	23.4	23.4	23.4	23.4		
Total Split (s)	30.0	30.0	30.0	45.0	45.0		
Total Split (%)	40.0%	40.0%	40.0%	60.0%	60.0%		
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3		
All-Red Time (s)	2.1	2.1	2.1	2.1	2.1		
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		
Total Lost Time (s)	5.4	5.4	5.4	5.4	5.4		
Lead/Lag							
Lead-Lag Optimize?							
Act Effct Green (s)	24.6	24.6	24.6	39.6	39.6		
Actuated g/C Ratio	0.33	0.33	0.33	0.53	0.53		
v/c Ratio	0.42	0.53	0.66	0.05	0.79		
Control Delay	27.0	24.4	28.4	8.9	17.0		
Queue Delay	0.0	0.0	0.0	0.0	0.0		
Total Delay	27.0	24.4	28.4	8.9	17.0		
LOS	С	С	С	Α	В		
Approach Delay		25.0	28.4		16.8		
Approach LOS		С	С		В		
Queue Length 50th (m)	9.9	35.0	40.8	2.6	76.8		
Queue Length 95th (m)	23.2	57.7	68.0	6.7	95.9		
Internal Link Dist (m)		152.2	162.6		69.7		
Turn Bay Length (m)	25.0						
Base Capacity (vph)	217	585	526	747	2537		
Starvation Cap Reductn	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0		
Reduced v/c Ratio	0.42	0.53	0.66	0.05	0.79		
ntersection Summary							
Cycle Length: 75							
Actuated Cycle Length: 75							
Offset: 36 (48%), Reference	ed to phase	2·NRTI	Start of	Green			
Natural Cycle: 60	eu to priast	Z.NDIL	, Start Or	Oleen			
Control Type: Pretimed							
Maximum v/c Ratio: 0.79							
Intersection Signal Delay: 1	9.5			l.	ntersectio	I OS: B	
Intersection Signal Delay. I						of Service D	
Analysis Period (min) 15	auon 19.0%)		10	OO LEVEL	DI SEIVICE D	
miaiyələ i elibu (IIIII) 13							
Splits and Phases: 4: Kei	nt St & Gla	dstone A	ve				
, ↑ Ø2 (R)						40	4
45 s						30 s	
						←	8
						30 s	

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Lane Group	WBT	NBL	NBT	SBT			
Lane Configurations	4		4	7			
Traffic Volume (vph)	4	36	495	326			
Future Volume (vph)	4	36	495	326			
Lane Group Flow (vph)	30	0	590	391			
Turn Type	NA	Perm	NA	NA			
Protected Phases	8	. 0	2	6			
Permitted Phases	_	2	_	-			
Minimum Split (s)	24.2	23.0	23.0	23.0			
Fotal Split (s)	24.0	51.0	51.0	51.0			
Γotal Split (%)	32.0%	68.0%	68.0%	68.0%			
Yellow Time (s)	3.3	3.3	3.3	3.3			
All-Red Time (s)	1.9	1.7	1.7	1.7			
ost Time Adjust (s)	0.0		0.0	0.0			
Total Lost Time (s)	5.2		5.0	5.0			
Lead/Lag							
Lead-Lag Optimize?							
Act Effct Green (s)	18.8		46.0	46.0			
Actuated g/C Ratio	0.25		0.61	0.61			
v/c Ratio	0.08		0.59	0.38			
Control Delay	15.5		3.2	8.4			
Queue Delay	0.0		0.2	0.0			
Total Delay	15.5		3.3	8.4			
LOS	В		Α	Α			
Approach Delay	15.5		3.3	8.4			
Approach LOS	В		A	А			
Queue Length 50th (m)	1.7		4.9	24.0			
Queue Length 95th (m)	7.7		6.9	39.2			
Internal Link Dist (m)	126.3		56.4	50.2			
Turn Bay Length (m)	000		4004	4007			
Base Capacity (vph)	383		1004	1037			
Starvation Cap Reductn	0		51	0			
Spillback Cap Reductn	0		0	0			
Storage Cap Reductn	0		0 62	0 20			
Reduced v/c Ratio	0.08		0.62	0.38			
tersection Summary							
ycle Length: 75							
ctuated Cycle Length: 75							
Offset: 42 (56%), Referenced	to phase	e 2:NBTL	and 6:SE	BT, Start o	f Green		
Natural Cycle: 60							
Control Type: Pretimed							
Maximum v/c Ratio: 0.59							
ntersection Signal Delay: 5.6					tersection LOS: A		
ntersection Capacity Utilization	n 78.5%	Ď		IC	CU Level of Service D		
Analysis Period (min) 15							
Splits and Phases: 5: Bank	St & Ma	ıcLaren S	: f				
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Ø2 (R)							
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▼ Ø6 (R)						_L`	₹ ø

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Lane Group	EBT	NBT	SBL	SBT	
Lane Configurations	4	f)		4	
Traffic Volume (vph)	48	540	15	214	
Future Volume (vph)	48	540	15	214	
Lane Group Flow (vph)	123	638	0	255	
Turn Type	NA	NA	Perm	NA	
Protected Phases	4	2		6	
Permitted Phases			6		
Minimum Split (s)	23.2	26.1	26.1	26.1	
Total Split (s)	25.0	50.0	50.0	50.0	
Total Split (%)	33.3%	66.7%	66.7%	66.7%	
Yellow Time (s)	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.9	1.8	1.8	1.8	
Lost Time Adjust (s)	0.0	0.0		0.0	
Total Lost Time (s)	5.2	5.1		5.1	
Lead/Lag					
Lead-Lag Optimize?					
Act Effct Green (s)	19.8	44.9		44.9	
Actuated g/C Ratio	0.26	0.60		0.60	
v/c Ratio	0.28	0.61		0.25	
Control Delay	21.9	12.6		4.8	
Queue Delay	0.0	0.0		0.3	
Total Delay	21.9	12.6		5.1	
LOS	C	B		A	
Approach Delay	21.9	12.6		5.1	
Approach LOS	C	В		A	
Queue Length 50th (m)	11.4	50.6		7.2	
Queue Length 95th (m)	m21.1	80.5		11.4	
Internal Link Dist (m)	115.9	89.4		56.4	
Turn Bay Length (m)	400	4040		4040	
Base Capacity (vph)	439	1043		1012	
Starvation Cap Reductn	0	0		347	
Spillback Cap Reductn	0	3		0	
Storage Cap Reductn	0	0		0	
Reduced v/c Ratio	0.28	0.61		0.38	
Intersection Summary					
Cycle Length: 75					
Actuated Cycle Length: 75					
Offset: 37 (49%), Reference	ed to phase	e 2:NBT a	and 6:SBT	ΓL, Start o	of Green
Natural Cycle: 60					
Control Type: Pretimed					
Maximum v/c Ratio: 0.61					
Intersection Signal Delay: 1					ntersection LOS: B
Intersection Capacity Utiliza	ation 56.1%	ò		10	CU Level of Service B

Analysis Period (min) 15

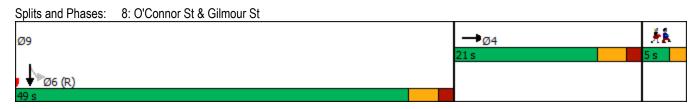
m Volume for 95th percentile queue is metered by upstream signal.



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Lane Group	EBT	SBT	Ø9	
Lane Configurations	4	414		
Traffic Volume (vph)	61	622		
Future Volume (vph)	61	622		
Lane Group Flow (vph)	122	745		
Turn Type	NA	NA		
Protected Phases	4	6	9	
Permitted Phases				
Minimum Split (s)	20.1	26.1	5.0	
Total Split (s)	21.0	49.0	5.0	
Total Split (%)	28.0%	65.3%	7%	
Yellow Time (s)	3.3	3.3	2.0	
All-Red Time (s)	1.8	1.8	0.0	
Lost Time Adjust (s)	0.0	0.0		
Total Lost Time (s)	5.1	5.1		
Lead/Lag				
Lead-Lag Optimize?				
Act Effct Green (s)	15.9	43.9		
Actuated g/C Ratio	0.21	0.59		
v/c Ratio	0.32	0.38		
Control Delay	14.5	8.2		
Queue Delay	0.0	0.0		
Total Delay	14.5	8.2		
LOS	B	A		
Approach Delay	14.5	8.2		
Approach LOS	В	Α		
Queue Length 50th (m)	6.8	24.1		
Queue Length 95th (m)	m15.6	34.2		
Internal Link Dist (m)	159.1	135.3		
Turn Bay Length (m)	200	1070		
Base Capacity (vph)	386	1976		
Starvation Cap Reductn	0	0		
Spillback Cap Reductn	0	0		
Storage Cap Reductn	0	0		
Reduced v/c Ratio	0.32	0.38		
Intersection Summary				
Cycle Length: 75				
Actuated Cycle Length: 75				
Offset: 46 (61%), Reference	ced to phase	e 6:SBTL,	Start of Green	
Natural Cycle: 55				
Control Type: Pretimed				
Maximum v/c Ratio: 0.38	0.4			
Intersection Signal Delay:				Intersection LOS: A
Intersection Capacity Utiliz	ation 44.0%)		ICU Level of Service A

m Volume for 95th percentile queue is metered by upstream signal.

Analysis Period (min) 15



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Lane Group	EBL	EBT	WBT	NBL	NBT	
Lane Configurations	ሻ	ĵ»	4	ሻ	ተተ _ጉ	
Traffic Volume (vph)	56	332	303	77	706	
Future Volume (vph)	56	332	303	77	706	
Lane Group Flow (vph)	62	369	407	86	902	
Turn Type	Perm	NA	NA	Perm	NA	
Protected Phases		4	8		2	
Permitted Phases	4			2		
Minimum Split (s)	23.5	23.5	23.5	23.4	23.4	
Total Split (s)	41.0	41.0	41.0	34.0	34.0	
Total Split (%)	54.7%	54.7%	54.7%	45.3%	45.3%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.2	2.2	2.2	2.1	2.1	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.5	5.5	5.5	5.4	5.4	
Lead/Lag	0.0	0.0	0.0	• • • • • • • • • • • • • • • • • • • •	.	
Lead-Lag Optimize?						
Act Effct Green (s)	35.5	35.5	35.5	28.6	28.6	
Actuated g/C Ratio	0.47	0.47	0.47	0.38	0.38	
v/c Ratio	0.20	0.44	0.51	0.26	0.51	
Control Delay	13.8	15.2	16.6	23.5	22.5	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	13.8	15.2	16.6	23.5	22.5	
LOS	В	В	В	C	C	
Approach Delay	_	15.0	16.6		22.6	
Approach LOS		В	В		C	
Queue Length 50th (m)	4.9	33.0	38.0	8.1	30.2	
Queue Length 95th (m)	12.3	53.2	61.6	21.4	60.2	
Internal Link Dist (m)		97.2	154.7		68.5	
Turn Bay Length (m)	20.0			20.0		
Base Capacity (vph)	305	844	791	326	1752	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.20	0.44	0.51	0.26	0.51	
ntersection Summary						
Cycle Length: 75						
Actuated Cycle Length: 75 Offset: 50 (67%), Reference	d to phoo	2.NDTI	Ctart of	Croon		
	eu to priase	Z.INDIL	, Start or	Green		
Natural Cycle: 50 Control Type: Pretimed						
Maximum v/c Ratio: 0.51						
ntersection Signal Delay: 1	0.5			l.	atoropatio	a LOC. D
ntersection Signal Delay: I					ntersection	of Service C
Analysis Period (min) 15	111011 72.170)		10	SO Level	oi Service C
· ,						
Splits and Phases: 1: Ker	nt St & Sor	nerset St	W		<u> </u>	
√ ¶ø2 (R)					² Ø4	
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Lane Group	WBT	NBT
Lane Configurations	ĵ.	414
Traffic Volume (vph)	22	869
Future Volume (vph)	22	869
Lane Group Flow (vph)	102	1000
Sign Control	Stop	Free
Intersection Summary		
Control Type: Unsignalized	d	
Intersection Capacity Utiliz	zation 37.0%	

Analysis Period (min) 15

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					f)			₽₽₽₽				
Traffic Volume (veh/h)	0	0	0	0	22	70	31	869	0	0	0	0
Future Volume (Veh/h)	0	0	0	0	22	70	31	869	0	0	0	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	0	0	24	78	34	966	0	0	0	0
Pedestrians		45			95			11			17	
Lane Width (m)		0.0			3.7			3.7			0.0	
Walking Speed (m/s)		1.1			1.1			1.1			1.1	
Percent Blockage		0			9			1			0	
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								78			92	
pX, platoon unblocked	0.92	0.92		0.92	0.92	0.92		. 0		0.92	<u> </u>	
vC, conflicting volume	542	1174	56	1140	1174	434	45			1061		
vC1, stage 1 conf vol	0.2			11.0						1001		
vC2, stage 2 conf vol												
vCu, unblocked vol	186	875	56	838	875	68	45			752		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)	7.0	0.0	0.0	7.0	0.0	0.0						
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	100	90	90	98			100		
cM capacity (veh/h)	532	234	988	197	234	820	1561			714		
, , ,					204	020	1001			, , , ,		
Direction, Lane #	WB 1	NB 1	NB 2	NB 3								
Volume Total	102	227	386	386								
Volume Left	0	34	0	0								
Volume Right	78	0	0	0								
cSH	516	1561	1700	1700								
Volume to Capacity	0.20	0.02	0.23	0.23								
Queue Length 95th (m)	5.5	0.5	0.0	0.0								
Control Delay (s)	13.7	1.3	0.0	0.0								
Lane LOS	В	Α										
Approach Delay (s)	13.7	0.3										
Approach LOS	В											
Intersection Summary												
Average Delay			1.5									
Intersection Capacity Utilization	ation		37.0%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

	→	†	
Lane Group	EBT	NBT	
Lane Configurations	4	ተ ተጉ	
Traffic Volume (vph)	77	846	
Future Volume (vph)	77	846	
Lane Group Flow (vph)	107	1027	
Turn Type	NA	NA	
Protected Phases	4	2	
Permitted Phases	•	_	
Minimum Split (s)	21.5	21.1	
Total Split (s)	22.0	53.0	
Total Split (%)	29.3%	70.7%	
Yellow Time (s)	3.3	3.3	
All-Red Time (s)	2.2	1.8	
Lost Time Adjust (s)	0.0	0.0	
Total Lost Time (s)	5.5	5.1	
Lead/Lag	0.0	0.1	
Lead-Lag Optimize?			
Act Effct Green (s)	16.5	47.9	
Actuated g/C Ratio	0.22	0.64	
v/c Ratio	0.26	0.34	
Control Delay	19.8	9.5	
Queue Delay	0.0	0.0	
Total Delay	19.8	9.5	
LOS	В	A	
Approach Delay	19.8	9.5	
Approach LOS	В	A	
Queue Length 50th (m)	8.8	23.6	
Queue Length 95th (m)	21.2	37.7	
Internal Link Dist (m)	157.9	224.2	
Turn Bay Length (m)	70110		
Base Capacity (vph)	405	3012	
Starvation Cap Reductn	0	0	
Spillback Cap Reductn	0	0	
Storage Cap Reductn	0	0	
Reduced v/c Ratio	0.26	0.34	
Intersection Summary			
Cycle Length: 75			
Actuated Cycle Length: 75			
Offset: 0 (0%), Referenced to	to phase 2	:NBT, Sta	art of Green
Natural Cycle: 45		, - 10	
Control Type: Pretimed			
Maximum v/c Ratio: 0.34			
Intersection Signal Delay: 10	0.5		Intersection LOS: B
Intersection Capacity Utiliza			ICU Level of Service A
Analysis Period (min) 15			
Splits and Phases: 3: Ker	nt St & Giln	nour St	
*	il ol a GIIN	ioui Si	
Ø2 (R)			→ Ø4

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Lane Group	EBL	EBT	WBT	NBL	NBT		
Lane Configurations	ች	†	1>	*	ተተኈ		
Traffic Volume (vph)	75	450	324	52	722		
Future Volume (vph)	75	450	324	52	722		
Lane Group Flow (vph)	83	500	443	58	920		
Turn Type	Perm	NA	NA	Perm	NA		
Protected Phases		4	8	. 0	2		
Permitted Phases	4	•		2	_		
Minimum Split (s)	23.4	23.4	23.4	23.4	23.4		
Total Split (s)	40.0	40.0	40.0	35.0	35.0		
Total Split (%)	53.3%	53.3%	53.3%	46.7%	46.7%		
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3		
All-Red Time (s)	2.1	2.1	2.1	2.1	2.1		
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		
Total Lost Time (s)	5.4	5.4	5.4	5.4	5.4		
Lead/Lag	O. r	- U. r	O. r	O. r	0.1		
Lead-Lag Optimize?							
Act Effct Green (s)	34.6	34.6	34.6	29.6	29.6		
Actuated g/C Ratio	0.46	0.46	0.46	0.39	0.39		
v/c Ratio	0.27	0.61	0.55	0.10	0.49		
Control Delay	15.4	19.1	17.1	15.0	17.3		
Queue Delay	0.0	0.0	0.0	0.0	0.0		
Total Delay	15.4	19.1	17.1	15.0	17.3		
LOS	В	В	В	В	В		
Approach Delay		18.5	17.1		17.1		
Approach LOS		В	В		В		
Queue Length 50th (m)	6.9	50.5	41.0	5.0	33.0		
Queue Length 95th (m)	16.3	79.9	67.0	11.9	43.8		
Internal Link Dist (m)	.0.0	152.2	162.6		69.7		
Turn Bay Length (m)	25.0	.02.2	.02.0		00.1		
Base Capacity (vph)	311	823	801	588	1877		
Starvation Cap Reductn	0	020	0	0	0		
Spillback Cap Reductn	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0		
Reduced v/c Ratio	0.27	0.61	0.55	0.10	0.49		
	0.21	0.01	0.55	0.10	0.43		
Intersection Summary							
Cycle Length: 75							
Actuated Cycle Length: 75							
Offset: 23 (31%), Referenced	d to phase	2:NBTL	, Start of	Green			
Natural Cycle: 50			,				
Control Type: Pretimed							
Maximum v/c Ratio: 0.61							
Intersection Signal Delay: 17	.5			lr	ntersection	n LOS: B	
Intersection Capacity Utilizat)				of Service B	
Analysis Period (min) 15							
Splits and Phases: 4: Kent	t St & Gla	dstone A	ve				
1 Ø2 (R)					<u></u>		
35 s				4	10 s		
					←		

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_ane Group	WBT	NBL	NBT	SBT	
_ane Configurations	4		ર્ન	f)	
Traffic Volume (vph)	11	25	275	620	
Future Volume (vph)	11	25	275	620	
Lane Group Flow (vph)	62	0	334	718	
Turn Type	NA	Perm	NA	NA	
Protected Phases	8		2	6	
Permitted Phases	•	2			
Minimum Split (s)	24.2	23.0	23.0	23.0	
Total Split (s)	24.0	51.0	51.0	51.0	
Total Split (%)	32.0%	68.0%	68.0%	68.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.9	1.7	1.7	1.7	
Lost Time Adjust (s)	0.0	1.7	0.0	0.0	
Total Lost Time (s)	5.2		5.0	5.0	
Lead/Lag	J.Z		5.0	5.0	
Lead-Lag Optimize?					
	18.8		46.0	46.0	
Act Effet Green (s)	0.25		0.61	0.61	
Actuated g/C Ratio				0.61	
v/c Ratio	0.17		0.34		
Control Delay	20.3		2.7	13.6	
Queue Delay	0.0		0.2	0.0	
Total Delay	20.3		2.9	13.6	
LOS	С		A	B	
Approach Delay	20.3		2.9	13.6	
Approach LOS	C		A	В	
Queue Length 50th (m)	5.6		4.3	59.4	
Queue Length 95th (m)	14.7		6.1	95.7	
Internal Link Dist (m)	126.3		56.4	50.2	
Turn Bay Length (m)					
Base Capacity (vph)	366		980	1058	
Starvation Cap Reductn	0		201	0	
Spillback Cap Reductn	0		0	0	
Storage Cap Reductn	0		0	0	
Reduced v/c Ratio	0.17		0.43	0.68	
Intersection Summary					
Cycle Length: 75					
Actuated Cycle Length: 75					
Offset: 7 (9%), Referenced	to phase 2	:NBTL ar	nd 6:SBT	Start of 0	Green
Natural Cycle: 60	paoo 2		0.001,	310	
Control Type: Pretimed					
Maximum v/c Ratio: 0.68					
Intersection Signal Delay: 1	0.8			lr	ntersection LOS: B
Intersection Capacity Utiliza					CU Level of Service B
Analysis Period (min) 15	111011 01.07	,		IV.	DO EGVELOR GELVICE B
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-4	nk St & Ma	CLAIEII S			
Ø2 (R) 51 s					
Ø6 (R)					₹ ø8

Ø6 (R)

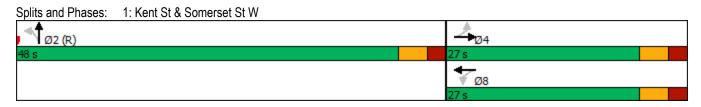
	→	†	>	ļ				
Lane Group	EBT	NBT	SBL	SBT				
Lane Configurations	4	1 >		4				
Traffic Volume (vph)	116	410	19	343				
Future Volume (vph)	116	410	19	343				
Lane Group Flow (vph)	213	487	0	402				
Turn Type	NA	NA	Perm	NA				
Protected Phases	4	2		6				
Permitted Phases	•	_	6					
Minimum Split (s)	23.2	26.1	26.1	26.1				
Total Split (s)	25.0	50.0	50.0	50.0				
Total Split (%)	33.3%	66.7%	66.7%	66.7%				
Yellow Time (s)	3.3	3.3	3.3	3.3				
All-Red Time (s)	1.9	1.8	1.8	1.8				
Lost Time Adjust (s)	0.0	0.0	1.0	0.0				
Total Lost Time (s)	5.2	5.1		5.1				
Lead/Lag	0.2	0.1		0.1				
Lead-Lag Optimize?								
Act Effct Green (s)	19.8	44.9		44.9				
Actuated g/C Ratio	0.26	0.60		0.60				
v/c Ratio	0.20	0.47		0.39				
Control Delay	19.6	10.1		4.3				
Queue Delay	0.0	0.0		0.6				
Total Delay	19.6	10.1		5.0				
LOS	19.0 B	10.1 B		3.0 A				
Approach Delay	19.6	10.1		5.0				
Approach LOS	19.0 B	10.1 B		3.0 A				
Queue Length 50th (m)	14.5	33.9		9.3				
	28.4	54.3		13.5				
Queue Length 95th (m)	115.9	89.4		56.4				
Internal Link Dist (m)	115.9	09.4		30.4				
Turn Bay Length (m)	444	1027		1000				
Base Capacity (vph)		1037		1026				
Starvation Cap Reductn	0	0		312				
Spillback Cap Reductn	0	0		0				
Storage Cap Reductn	0 10	0 47		0				
Reduced v/c Ratio	0.48	0.47		0.56				
Intersection Summary								
Cycle Length: 75								
Actuated Cycle Length: 75								
Offset: 6 (8%), Referenced to	phase 2	:NBT and	6:SBTL,	Start of	Green			
Natural Cycle: 50								
Control Type: Pretimed								
Maximum v/c Ratio: 0.48								
ntersection Signal Delay: 10.1 Intersection LOS: B								
ntersection Capacity Utilization 63.0% ICU Level of Service B								
Analysis Period (min) 15								
Splits and Phases: 6: Bank	k St & Gil	mour St						
*	0. 4 011				1.8			
Ø2 (R)					2→04			

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Lane Group	EBT	SBT	Ø9	
Lane Configurations	f)	414		
Traffic Volume (vph)	84	1190		
Future Volume (vph)	84	1190		
Lane Group Flow (vph)	192	1375		
Turn Type	NA	NA		
Protected Phases	4	6	9	
Permitted Phases		U	3	
Minimum Split (s)	20.1	26.1	5.0	
	21.0	49.0	5.0	
Total Split (s)				
Total Split (%)	28.0%	65.3%	7%	
Yellow Time (s)	3.3	3.3	2.0	
All-Red Time (s)	1.8	1.8	0.0	
Lost Time Adjust (s)	0.0	0.0		
Total Lost Time (s)	5.1	5.1		
Lead/Lag				
Lead-Lag Optimize?				
Act Effct Green (s)	15.9	43.9		
Actuated g/C Ratio	0.21	0.59		
v/c Ratio	0.50	0.69		
Control Delay	32.1	12.6		
Queue Delay	0.0	0.0		
Total Delay	32.1	12.6		
LOS	С	В		
Approach Delay	32.1	12.6		
Approach LOS	С	В		
Queue Length 50th (m)	19.8	61.7		
Queue Length 95th (m)	38.6	83.4		
Internal Link Dist (m)	159.1	135.3		
Turn Bay Length (m)	100.1	100.0		
Base Capacity (vph)	386	1985		
Starvation Cap Reductn	0	0		
Spillback Cap Reductn	0	0		
Storage Cap Reductn	0.50	0.60		
Reduced v/c Ratio	0.50	0.69		
Intersection Summary				
Cycle Length: 75				
Actuated Cycle Length: 75				
Offset: 71 (95%), Reference	ed to phase	6:SBTL,	Start of Gree	1
Natural Cycle: 60				
Control Type: Pretimed				
Maximum v/c Ratio: 0.69				
Intersection Signal Delay: 1	5.0			Intersection LOS: B
Intersection Capacity Utiliza				ICU Level of Service B
Analysis Period (min) 15	2000			
,				
Splits and Phases: 8: 0'0	Connor St 8	Gilmour	St	
Ø9				
Ø6 (R)				



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Lane Group	EBL	EBT	WBT	NBL	NBT	
Lane Configurations	ሻ	ĵ.	4	ħ	ተ ተኈ	
Traffic Volume (vph)	62	304	138	75	1601	
Future Volume (vph)	62	304	138	75	1601	
Lane Group Flow (vph)	62	304	188	75	1785	
Turn Type	Perm	NA	NA	Perm	NA	
Protected Phases		4	8		2	
Permitted Phases	4	02 E	02 E	2	22.4	
Minimum Split (s) Total Split (s)	23.5 27.0	23.5 27.0	23.5 27.0	23.4 48.0	23.4 48.0	
Total Split (%)	36.0%	36.0%	36.0%	64.0%	64.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.2	2.2	2.2	2.1	2.1	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.5	5.5	5.5	5.4	5.4	
Lead/Lag						
Lead-Lag Optimize?						
Act Effct Green (s)	21.5	21.5	21.5	42.6	42.6	
Actuated g/C Ratio	0.29	0.29	0.29	0.57	0.57	
v/c Ratio	0.22	0.59	0.40	0.12	0.67	
Control Delay	22.9	28.7	24.6	13.3	15.9	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	22.9	28.7	24.6	13.3	15.9	
LOS	С	С	С	В	В	
Approach Delay		27.7	24.6		15.8	
Approach LOS	0.0	C	C	5 0	B	
Queue Length 50th (m)	6.6	36.7	21.3	5.8	49.4	
Queue Length 95th (m)	15.9	61.0	38.3 154.7	m10.1	66.7 68.5	
Internal Link Dist (m) Turn Bay Length (m)	20.0	97.2	104.7	20.0	00.5	
Base Capacity (vph)	286	511	475	628	2656	
Starvation Cap Reductn	0	0	0	020	2030	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.22	0.59	0.40	0.12	0.67	
Intersection Summary						
Cycle Length: 75						
Actuated Cycle Length: 75						
Offset: 66 (88%), Reference	d to phase	2·NRTI	Start of	Green		
Natural Cycle: 55	a to priast	Z.INDIL	, Otali oi	Olcon		
Control Type: Pretimed						
Maximum v/c Ratio: 0.67						
Intersection Signal Delay: 18	3.3			lr	ntersectio	n LOS: B
Intersection Capacity Utilizat)				of Service D
Analysis Period (min) 15						
Analysis Periou (min) 15	منتمنية	:	ممسينيا ام			

m Volume for 95th percentile queue is metered by upstream signal.



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Lane Group	WBT	NBT
Lane Configurations	1.	414
Traffic Volume (vph)	14	1848
Future Volume (vph)	14	1848
Lane Group Flow (vph)	35	1881
Sign Control	Stop	Free
Intersection Summary		
Control Type: Unsignalized	d	
Intersection Capacity Utiliz	ration 53 0%	

Analysis Period (min) 15

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					ĵ»			414				
Traffic Volume (veh/h)	0	0	0	0	14	21	33	1848	0	0	0	0
Future Volume (Veh/h)	0	0	0	0	14	21	33	1848	0	0	0	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	0	0	0	0	14	21	33	1848	0	0	0	0
Pedestrians		92			133			15			8	
Lane Width (m)		0.0			3.7			3.7			0.0	
Walking Speed (m/s)		1.1			1.1			1.1			1.1	
Percent Blockage		0			12			1			0	
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								78			92	
pX, platoon unblocked	0.74	0.74		0.74	0.74	0.74				0.74		
vC, conflicting volume	810	2139	107	2062	2139	757	92			1981		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	0	1308	107	1204	1308	0	92			1095		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	100	86	97	98			100		
cM capacity (veh/h)	585	100	913	79	100	702	1501			410		
Direction, Lane #	WB 1	NB 1	NB 2	NB 3								
Volume Total	35	403	739	739								
Volume Left	0	33	0	0								
Volume Right	21	0	0	0								
cSH	206	1501	1700	1700								
Volume to Capacity	0.17	0.02	0.43	0.43								
Queue Length 95th (m)	4.5	0.5	0.0	0.0								
Control Delay (s)	26.0	0.8	0.0	0.0								
Lane LOS	D	Α										
Approach Delay (s)	26.0	0.2										
Approach LOS	D											
Intersection Summary												
Average Delay			0.6									
Intersection Capacity Utiliza	ation		53.0%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

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Lane Group	EBT	NBT
Lane Configurations	र्स	↑ ↑₽
Traffic Volume (vph)	85	1866
Future Volume (vph)	85	1866
ane Group Flow (vph)	102	2020
Turn Type	NA	NA
Protected Phases	4	2
Permitted Phases		
Minimum Split (s)	21.5	21.1
Total Split (s)	22.0	53.0
Total Split (%)	29.3%	70.7%
Yellow Time (s)	3.3	3.3
All-Red Time (s)	2.2	1.8
Lost Time Adjust (s)	0.0	0.0
Total Lost Time (s)	5.5	5.1
Lead/Lag		
Lead-Lag Optimize?		
Act Effct Green (s)	16.5	47.9
Actuated g/C Ratio	0.22	0.64
v/c Ratio	0.25	0.67
Control Delay	19.4	22.4
Queue Delay	0.0	0.0
Total Delay	19.4	22.4
LOS	В	С
Approach Delay	19.4	22.4
Approach LOS	В	C
Queue Length 50th (m)	8.2	107.5
Queue Length 95th (m)	20.1	122.0
Internal Link Dist (m)	157.9	224.2
Turn Bay Length (m)	440	2000
Base Capacity (vph)	410	3029
Starvation Cap Reductn	0	0
Spillback Cap Reductn	0	0
Storage Cap Reductn	0	0
Reduced v/c Ratio	0.25	0.67
Intersection Summary		
Cycle Length: 75		
Actuated Cycle Length: 75		
Offset: 5 (7%), Referenced	to phase 2:	NBT, Sta
Natural Cycle: 55		
Control Type: Pretimed		
Maximum v/c Ratio: 0.67		
Intersection Signal Delay: 2		
Intersection Capacity Utiliza	ation 64.3%	
Analysis Period (min) 15		
Splits and Phases: 3: Kei	nt St & Giln	nour St
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Lane Group	EBL	EBT	WBT	NBL	NBT			
Lane Configurations	*	†	ą.	7	ተተኈ			
Traffic Volume (vph)	82	285	170	36	1749			
Future Volume (vph)	82	285	170	36	1749			
Lane Group Flow (vph)	82	285	318	36	1846			
Turn Type	Perm	NA	NA	Perm	NA			
Protected Phases		4	8		2			
Permitted Phases	4	•		2	-			
Minimum Split (s)	23.4	23.4	23.4	23.4	23.4			
Total Split (s)	30.0	30.0	30.0	45.0	45.0			
Total Split (%)	40.0%	40.0%	40.0%	60.0%	60.0%			
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3			
All-Red Time (s)	2.1	2.1	2.1	2.1	2.1			
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0			
Total Lost Time (s)	5.4	5.4	5.4	5.4	5.4			
Lead/Lag	J. 4							
Lead-Lag Optimize?								
3 i	24.6	24.6	24.6	39.6	39.6			
Act Effct Green (s) Actuated g/C Ratio	0.33	0.33	0.33	0.53	0.53			
v/c Ratio	0.33	0.33	0.60	0.05	0.53			
	23.7	23.6	26.2	8.9	15.5			
Control Delay								
Queue Delay	0.0	0.0	0.0 26.2	0.0	0.0			
Total Delay	23.7	23.6		8.9	15.5			
LOS	С	C	C	Α	B			
Approach Delay		23.6	26.2		15.4			
Approach LOS	0.0	C	C	0.0	B			
Queue Length 50th (m)	8.6	31.8	36.0	2.3	67.5			
Queue Length 95th (m)	20.1	53.0	60.9	6.3	84.4			
Internal Link Dist (m)	05.0	152.2	162.6		69.7			
Turn Bay Length (m)	25.0	F0F	F00	7.47	0500			
Base Capacity (vph)	247	585	529	747	2538			
Starvation Cap Reductn	0	0	0	0	0			
Spillback Cap Reductn	0	0	0	0	0			
Storage Cap Reductn	0	0	0	0	0			
Reduced v/c Ratio	0.33	0.49	0.60	0.05	0.73			
Intersection Summary								
Cycle Length: 75								
Actuated Cycle Length: 75								
Offset: 36 (48%), Reference	ed to phase	2:NBTL	, Start of	Green				
Natural Cycle: 60								
Control Type: Pretimed								
Maximum v/c Ratio: 0.73								
Intersection Signal Delay: 1	17.9			lr	ntersection	LOS: I	В	
Intersection Capacity Utiliza))			CU Level o			
Analysis Period (min) 15								
Splits and Phases: 4: Ke	nt St & Gla	dstana A	VP					
+	iii oi a Gla	usione A	٧ ٥				A	
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Lane Group	WBT	NBL	NBT	SBT					
_ane Configurations	4		4	1>					
Fraffic Volume (vph)	4	36	510	336					
Future Volume (vph)	4	36	510	336					
ane Group Flow (vph)	28	0	546	362					
Turn Type	NA	Perm	NA	NA					
Protected Phases	8	1 01111	2	6					
Permitted Phases		2	_						
Minimum Split (s)	24.2	23.0	23.0	23.0					
Total Split (s)	24.0	51.0	51.0	51.0					
Fotal Split (%)	32.0%	68.0%	68.0%	68.0%					
Yellow Time (s)	3.3	3.3	3.3	3.3					
All-Red Time (s)	1.9	1.7	1.7	1.7					
ost Time Adjust (s)	0.0	1.7	0.0	0.0					
Fotal Lost Time (s)	5.2		5.0	5.0					
Lead/Lag	J.Z		5.0	5.0					
Lead-Lag Optimize?	18.8		46.0	46.0					
Act Effct Green (s)									
Actuated g/C Ratio	0.25		0.61	0.61					
//c Ratio	0.07		0.53	0.35					
Control Delay	15.5		2.7	8.1					
Queue Delay	0.0		0.1	0.0					
Total Delay	15.5		2.8	8.1					
_OS	В		A	A					
Approach Delay	15.5		2.8	8.1					
Approach LOS	В		A	A					
Queue Length 50th (m)	1.6		4.3	21.6					
Queue Length 95th (m)	7.3		6.1	35.6					
Internal Link Dist (m)	126.3		56.4	50.2					
Turn Bay Length (m)									
Base Capacity (vph)	383		1027	1039					
Starvation Cap Reductn	0		66	0					
Spillback Cap Reductn	0		0	0					
Storage Cap Reductn	0		0	0					
Reduced v/c Ratio	0.07		0.57	0.35					
ntersection Summary									
Cycle Length: 75									
Actuated Cycle Length: 75									
Offset: 42 (56%), Reference	ed to phase	2:NBTL	and 6:SE	BT, Start	of Green				
Natural Cycle: 55									
Control Type: Pretimed									
Maximum v/c Ratio: 0.53									
ntersection Signal Delay: 5.	.2			lr	ntersection LOS: A				
Intersection Capacity Utilization 79.9% ICU Level of Service D									
Analysis Period (min) 15									
Splits and Phases: 5: Bar	nk St & Ma	ol aron S	·+						
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Ø2 (R)									
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▼ Ø6 (R)						¥ Ø8			

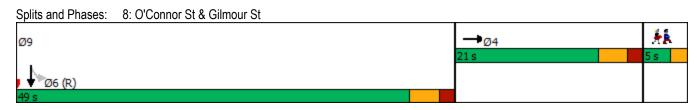
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Lane Group	EBT	NBT	SBL	SBT	
Lane Configurations	4	1		स	
Traffic Volume (vph)	48	556	15	220	
Future Volume (vph)	48	556	15	220	
Lane Group Flow (vph)	111	590	0	235	
Turn Type	NA	NA	Perm	NA	
Protected Phases	4	2		6	
Permitted Phases			6		
Minimum Split (s)	23.2	26.1	26.1	26.1	
Total Split (s)	25.0	50.0	50.0	50.0	
Total Split (%)	33.3%	66.7%	66.7%	66.7%	
Yellow Time (s)	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.9	1.8	1.8	1.8	
Lost Time Adjust (s)	0.0 5.2	0.0		0.0 5.1	
Total Lost Time (s)	5.2	5.1		5.1	
Lead/Lag					
Lead-Lag Optimize?	19.8	44.9		44.9	
Act Effct Green (s) Actuated g/C Ratio	0.26	0.60		0.60	
v/c Ratio	0.26	0.60		0.00	
Control Delay	21.2	11.7		4.8	
Queue Delay	0.0	0.0		0.3	
Total Delay	21.2	11.7		5.1	
LOS	C C	В		Α	
Approach Delay	21.2	11.7		5.1	
Approach LOS	C	В		Α	
Queue Length 50th (m)	9.7	44.8		6.7	
Queue Length 95th (m)	m19.4	71.3		10.7	
Internal Link Dist (m)	115.9	89.4		56.4	
Turn Bay Length (m)					
Base Capacity (vph)	439	1043		1020	
Starvation Cap Reductn	0	0		379	
Spillback Cap Reductn	0	0		0	
Storage Cap Reductn	0	0		0	
Reduced v/c Ratio	0.25	0.57		0.37	
Intersection Summary					
Cycle Length: 75					
Actuated Cycle Length: 75					
Offset: 37 (49%), Reference	ed to nhase	2·NRT :	and 6:SBT	ΓI Start (of Green
Natural Cycle: 55	ou to pridat	, Z.14D1 (a 0.0D	i L, Olait (OI OIOOII
Control Type: Pretimed					
Maximum v/c Ratio: 0.57					
Intersection Signal Delay: 1	1.1			lı	ntersection LOS: B
Intersection Capacity Utiliza		,)			CU Level of Service B
Analysis Period (min) 15					
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m Volume for 95th percentile queue is metered by upstream signal.



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Lane Group	EBT	SBT	Ø9	
Lane Configurations	4	414		
Traffic Volume (vph)	61	641		
Future Volume (vph)	61	641		
Lane Group Flow (vph)	110	690		
Turn Type	NA	NA		
Protected Phases	4	6	9	
Permitted Phases				
Minimum Split (s)	20.1	26.1	5.0	
Total Split (s)	21.0	49.0	5.0	
Total Split (%)	28.0%	65.3%	7%	
Yellow Time (s)	3.3	3.3	2.0	
All-Red Time (s)	1.8	1.8	0.0	
Lost Time Adjust (s)	0.0	0.0	0.0	
Total Lost Time (s)	5.1	5.1		
Lead/Lag	J. 1	J. I		
Lead-Lag Optimize?				
Act Effct Green (s)	15.9	43.9		
Actuated g/C Ratio	0.21	0.59		
v/c Ratio	0.21	0.35		
Control Delay	13.9	7.9		
Queue Delay	0.0	0.0		
Total Delay	13.9	7.9		
LOS	13.9 B	7.9 A		
Approach Delay	13.9	7.9		
	13.9 B	7.9 A		
Approach LOS	5.7	21.6		
Queue Length 50th (m)				
Queue Length 95th (m)	m14.2	31.0		
Internal Link Dist (m)	159.1	135.3		
Turn Bay Length (m)	007	4077		
Base Capacity (vph)	387	1977		
Starvation Cap Reductn	0	0		
Spillback Cap Reductn	0	0		
Storage Cap Reductn	0	0		
Reduced v/c Ratio	0.28	0.35		
Intersection Summary				
Cycle Length: 75				
Actuated Cycle Length: 75				
Offset: 46 (61%), Reference	ed to phase	6·SRTI	Start of Green	
Natural Cycle: 55	od to pridac	, J.OD I E,	otalt of Oldell	
Control Type: Pretimed				
Maximum v/c Ratio: 0.35				
Intersection Signal Delay: 8	7			
Intersection Capacity Utiliza				
Analysis Period (min) 15	ation 44.570	<u> </u>		ı
m Volume for OFth person	حديمين مائل	ia mada::	d h	_:

m Volume for 95th percentile queue is metered by upstream signal.



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EBL	EBT	WBT	NBL	NBT	
4	•		2	_	
	23.5	23.5		23.4	
0.0	0.0	0.0	U.7	0.4	
35.5	35.5	35.5	28.6	28.6	
U			U		
13			6.8		
10.5			10.5		
20.0	31.2	104.1	20.0	00.5	
	8//	702		1755	
0.10	0.41	0.47	0.24	0.47	
	0.1155	01 1 1	_		
d to phase	2:NBTL	, Start of	Green		
tion 73.5%)		I	CU Level o	of Service D
nt St & Son	<u>nerse</u> t St	<u>W</u>			
nt St & Sor	nerset St	W	1.	24	
nt St & Sor	nerset St	W	41		
8	8.4	56 342 56 342 56 342 Perm NA 4 23.5 23.5 41.0 41.0 54.7% 54.7% 3.3 3.3 2.2 2.2 0.0 0.0 5.5 5.5 35.5 35.5 0.47 0.47 0.16 0.41 12.8 14.7 0.0 0.0 12.8 14.7 B B 14.4 B 4.3 29.9 10.9 48.9 97.2 20.0 348 844 0 0 0 0 0 0 0.16 0.41 add to phase 2:NBTL	56 342 312 56 342 375 Perm NA NA 4 8 4 23.5 23.5 23.5 41.0 41.0 41.0 54.7% 54.7% 54.7% 3.3 3.3 3.3 2.2 2.2 2.2 0.0 0.0 0.0 0.0 5.5 5.5 5.5 35.5 35.5 35.5 0.47 0.47 0.47 0.16 0.41 0.47 12.8 14.7 15.9 0.0 0.0 0.0 12.8 14.7 15.9 B B B 4.3 29.9 34.2 10.9 48.9 55.7 97.2 154.7 20.0 348 844 792 0 16 0.41 0.47	56 342 312 77 56 342 312 77 56 342 375 77 Perm NA NA Perm 4 8 4 2 23.5 23.5 23.5 23.4 41.0 41.0 41.0 34.0 54.7% 54.7% 54.7% 45.3% 3.3 3.3 3.3 3.3 2.2 2.2 2.2 2.1 0.0 0.0 0.0 0.0 5.5 5.5 5.5 5.5 35.5 35.5 35.5 28.6 0.47 0.47 0.47 0.38 0.16 0.41 0.47 0.24 12.8 14.7 15.9 22.4 0.0 0.0 0.0 0.0 0.0 12.8 14.7 15.9 22.4 B B B C 14.4 15.9 B B B 4.3 29.9 34.2 6.8 10.9 48.9 55.7 16.9 97.2 154.7 20.0 20.0 348 844 792 326 0	\$ 1

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Lane Group	WBT	NBT	
Lane Configurations	- 1}	441	
Traffic Volume (vph)	22	895	
Future Volume (vph)	22	895	
Lane Group Flow (vph)	92	926	
Sign Control	Stop	Free	
Intersection Summary			
Control Type: Unsignalize			
Intersection Capacity Utili	ization 37.6%		ICU Level of Service A

Analysis Period (min) 15

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	•	→	•	•	←	•	•	†	/	>	↓	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					f)			414				
Traffic Volume (veh/h)	0	0	0	0	22	70	31	895	0	0	0	0
Future Volume (Veh/h)	0	0	0	0	22	70	31	895	0	0	0	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	0	0	0	0	22	70	31	895	0	0	0	0
Pedestrians		45			95			11			17	
Lane Width (m)		0.0			3.7			3.7			0.0	
Walking Speed (m/s)		1.1			1.1			1.1			1.1	
Percent Blockage		0			9			1			0	
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								78			92	
pX, platoon unblocked	0.93	0.93		0.93	0.93	0.93				0.93		
vC, conflicting volume	503	1097	56	1063	1097	410	45			990		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	191	831	56	794	831	91	45			716		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	100	91	91	98			100		
cM capacity (veh/h)	546	252	988	214	252	802	1561			744		
Direction, Lane #	WB 1	NB 1	NB 2	NB 3								
Volume Total	92	210	358	358								
Volume Left	0	31	0	0								
Volume Right	70	0	0	0								
cSH	527	1561	1700	1700								
Volume to Capacity	0.17	0.02	0.21	0.21								
Queue Length 95th (m)	4.8	0.5	0.0	0.0								
Control Delay (s)	13.3	1.2	0.0	0.0								
Lane LOS	В	Α										
Approach Delay (s)	13.3	0.3										
Approach LOS	В	0.0										
Intersection Summary												
Average Delay			1.5									
Intersection Capacity Utiliza	ation		37.6%	IC	CU Level	of Service			Α			
Analysis Period (min)			15									
,												

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Lane Group	EBT	NBT
Lane Configurations	ની	^
Traffic Volume (vph)	77	871
Future Volume (vph)	77	871
Lane Group Flow (vph)	96	949
Turn Type	NA	NA
Protected Phases	4	2
Permitted Phases		
Minimum Split (s)	21.5	21.1
Total Split (s)	22.0	53.0
Total Split (%)	29.3%	70.7%
Yellow Time (s)	3.3	3.3
All-Red Time (s)	2.2	1.8
Lost Time Adjust (s)	0.0	0.0
Total Lost Time (s)	5.5	5.1
Lead/Lag		
Lead-Lag Optimize?		
Act Effct Green (s)	16.5	47.9
Actuated g/C Ratio	0.22	0.64
v/c Ratio	0.24	0.31
Control Delay	19.0	8.8
Queue Delay	0.0	0.0
Total Delay	19.0	8.8
LOS	В	Α
Approach Delay	19.0	8.8
Approach LOS	В	Α
Queue Length 50th (m)	7.5	20.1
Queue Length 95th (m)	19.0	33.2
Internal Link Dist (m)	157.9	224.2
Turn Bay Length (m)		
Base Capacity (vph)	405	3017
Starvation Cap Reductn	0	0
Spillback Cap Reductn	0	0
Storage Cap Reductn	0	0
Reduced v/c Ratio	0.24	0.31
Intersection Summary		
Cycle Length: 75		
Actuated Cycle Length: 75	,	
Offset: 0 (0%), Referenced		NRT Sta
Natural Cycle: 45	u to priase 2.	NDT, SI
Control Type: Pretimed		
Maximum v/c Ratio: 0.31		
Intersection Signal Delay:	0.0	
Intersection Capacity Utiliz Analysis Period (min) 15	Laliuii 42.3%	
milalysis Fellou (IIIIII) 13		
Splits and Phases: 3: K	ent St & Gilm	nour St
•	J. N. D. W. O.III	.541 01
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Lane Group	EBL	EBT	WBT	NBL	NBT	
Lane Configurations	ች	†	4î	ሻ	ተተ _ጉ	
Traffic Volume (vph)	75	464	334	52	744	
Future Volume (vph)	75	464	334	52	744	
Lane Group Flow (vph)	75	464	409	52	850	
Turn Type	Perm	NA	NA	Perm	NA	
Protected Phases	. 0	4	8	. 0	2	
Permitted Phases	4	•		2	_	
Minimum Split (s)	23.4	23.4	23.4	23.4	23.4	
Total Split (s)	40.0	40.0	40.0	35.0	35.0	
	53.3%	53.3%	53.3%	46.7%	46.7%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.1	2.1	2.1	2.1	2.1	
_ost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.4	5.4	5.4	5.4	
Lead/Lag	J.4	0.4	0.4	5.4	5.4	
Lead/Lag Optimize?						
	34.6	34.6	34.6	29.6	29.6	
Act Effct Green (s) Actuated g/C Ratio	0.46	0.46	0.46	0.39	0.39	
	0.40	0.46	0.40	0.09	0.39	
//c Ratio						
Control Delay	14.4	18.0	16.3	14.9	16.8	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	14.4	18.0	16.3	14.9	16.8	
LOS	В	47.5	B	В	B	
Approach Delay		17.5	16.3		16.7	
Approach LOS	0.4	B	В	4.5	В	
Queue Length 50th (m)	6.1	45.6	36.8	4.5	29.8	
Queue Length 95th (m)	14.5	72.1	60.3	11.0	40.0	
Internal Link Dist (m)	05.0	152.2	162.6		69.7	
Turn Bay Length (m)	25.0	000	004	500	4077	
Base Capacity (vph)	339	823	801	588	1877	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.22	0.56	0.51	0.09	0.45	
ntersection Summary						
Cycle Length: 75						
Actuated Cycle Length: 75						
Offset: 23 (31%), Referenced t	to nhasi	2·NRTI	Start of	Green		
Natural Cycle: 50	o priusi	, L.1101L	, otali ol	010011		
Control Type: Pretimed						
Maximum v/c Ratio: 0.56						
ntersection Signal Delay: 16.8	}			l.	ntersection	LOS: B
Intersection Signal Delay. 10.6 Intersection Capacity Utilization		<u>'</u>				f Service C
Analysis Period (min) 15	11 04.07	U		I	OO LEVEI O	I DELVICE C
miarysis Feriou (IIIIII) 13						
Splits and Phases: 4: Kent S	St & Gla	dstone A	ve			
Splits and Phases: 4: Kent S	St & Gla	dstone A	ve		<u></u> ₩ _{Ø4}	

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Lane Group	WBT	NBL	NBT	SBT	
_ane Configurations	4		ર્ન	f	
Traffic Volume (vph)	11	25	283	639	
-uture Volume (vph)	11	25	283	639	
_ane Group Flow (vph)	56	0	308	665	
Turn Type	NA	Perm	NA	NA	
Protected Phases	8	1 01111	2	6	
Permitted Phases		2	_		
Minimum Split (s)	24.2	23.0	23.0	23.0	
Total Split (s)	24.0	51.0	51.0	51.0	
Fotal Split (%)	32.0%	68.0%	68.0%	68.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.9	1.7	1.7	1.7	
ost Time Adjust (s)	0.0	1.1	0.0	0.0	
Total Lost Time (s)	5.2		5.0	5.0	
Lead/Lag	5.2		5.0	3.0	
_ead/Lag _ead-Lag Optimize?					
• .	18.8		46.0	46.0	
Act Effct Green (s)	0.25		0.61	0.61	
Actuated g/C Ratio					
v/c Ratio	0.15		0.31	0.63	
Control Delay	20.1		2.5	12.4	
Queue Delay	0.0		0.3	0.0	
Total Delay	20.1		2.8	12.4	
LOS	C		A	B	
Approach Delay	20.1		2.8	12.4	
Approach LOS	C		A	B	
Queue Length 50th (m)	5.0		3.9	52.1	
Queue Length 95th (m)	13.5		5.7	83.4	
Internal Link Dist (m)	126.3		56.4	50.2	
Turn Bay Length (m)	000		4000	4050	
Base Capacity (vph)	366		1009	1059	
Starvation Cap Reductn	0		257	0	
Spillback Cap Reductn	0		0	0	
Storage Cap Reductn	0		0	0	
Reduced v/c Ratio	0.15		0.41	0.63	
ntersection Summary					
Cycle Length: 75					
Actuated Cycle Length: 75					
Offset: 7 (9%), Referenced	to phase 2	:NBTL ar	nd 6:SBT,	Start of	Green
Natural Cycle: 60					
Control Type: Pretimed					
Maximum v/c Ratio: 0.63					
ntersection Signal Delay: 9).9			lı	ntersection LOS: A
ntersection Capacity Utiliza		,)			CU Level of Service B
Analysis Period (min) 15					
Splits and Phases: 5: Ba	nk St & Ma	cLaren S	it		
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▼ Ø6 (R)					√ Ø8

Ø6 (R)

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Lane Group	EBT	NBT	SBL	SBT			
ane Configurations	4	ĵ.		4			
raffic Volume (vph)	116	422	19	353			
uture Volume (vph)	116	422	19	353			
ne Group Flow (vph)	191	450	0	372			
ırn Type	NA	NA	Perm	NA			
otected Phases	4	2	. 0	6			
ermitted Phases	•	_	6				
linimum Split (s)	23.2	26.1	26.1	26.1			
otal Split (s)	25.0	50.0	50.0	50.0			
otal Split (%)	33.3%	66.7%	66.7%	66.7%			
'ellow Time (s)	3.3	3.3	3.3	3.3			
II-Red Time (s)	1.9	1.8	1.8	1.8			
ost Time Adjust (s)	0.0	0.0		0.0			
otal Lost Time (s)	5.2	5.1		5.1			
ead/Lag							
ead-Lag Optimize?							
ct Effct Green (s)	19.8	44.9		44.9			
ctuated g/C Ratio	0.26	0.60		0.60			
c Ratio	0.43	0.43		0.36			
ontrol Delay	18.2	9.6		4.2			
ueue Delay	0.0	0.0		0.5			
otal Delay	18.2	9.6		4.8			
os ,	В	Α		Α			
pproach Delay	18.2	9.6		4.8			
proach LOS	В	Α		Α			
ueue Length 50th (m)	12.4	30.2		8.6			
ueue Length 95th (m)	24.5	48.9		12.5			
ternal Link Dist (m)	115.9	89.4		56.4			
urn Bay Length (m)							
ase Capacity (vph)	444	1038		1029			
tarvation Cap Reductn	0	0		313			
pillback Cap Reductn	0	0		0			
Storage Cap Reductn	0	0		0			
educed v/c Ratio	0.43	0.43		0.52			
tersection Summary							
cle Length: 75							
ctuated Cycle Length: 75							
Offset: 6 (8%), Referenced	I to phase 2	:NBT and	6:SBTL,	Start of	Green		
latural Cycle: 50							
ontrol Type: Pretimed							
aximum v/c Ratio: 0.43							
tersection Signal Delay:	9.4				ntersection LOS: A		
tersection Capacity Utiliz	ation 63.6%)			CU Level of Service B		
nalysis Period (min) 15							
Splits and Phases: 6: Ba	ank St & Gili	mour St					
*	and of a one	niour Ot				<u> </u>	
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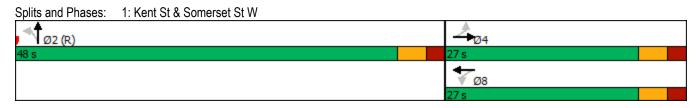
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Lane Group	EBL	EBT	WBT	NBL	NBT	
Lane Configurations	ሻ	1>	4	ሻ	ተተኈ	
Traffic Volume (vph)	62	319	145	75	1678	
Future Volume (vph)	62	319	145	75	1678	
Lane Group Flow (vph)	62	319	195	75	1862	
Turn Type	Perm	NA	NA	Perm	NA	
Protected Phases		4	8		2	
Permitted Phases	4			2		
Minimum Split (s)	23.5	23.5	23.5	23.4	23.4	
Total Split (s)	27.0	27.0	27.0	48.0	48.0	
Total Split (%)	36.0%	36.0%	36.0%	64.0%	64.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.2	2.2	2.2	2.1	2.1	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.5	5.5	5.5	5.4	5.4	
Lead/Lag						
Lead-Lag Optimize?						
Act Effct Green (s)	21.5	21.5	21.5	42.6	42.6	
Actuated g/C Ratio	0.29	0.29	0.29	0.57	0.57	
v/c Ratio	0.22	0.62	0.41	0.12	0.70	
Control Delay	23.0	29.6	24.8	13.4	16.7	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	23.0	29.6	24.8	13.4	16.7	
LOS	С	С	С	В	В	
Approach Delay		28.5	24.8		16.6	
Approach LOS		С	С		В	
Queue Length 50th (m)	6.6	39.0	22.2	6.0	54.0	
Queue Length 95th (m)	16.0	64.2	39.6	m10.1	71.5	
Internal Link Dist (m)		97.2	154.7		68.5	
Turn Bay Length (m)	20.0			20.0		
Base Capacity (vph)	281	511	476	628	2659	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.22	0.62	0.41	0.12	0.70	
Intersection Summary						
Cycle Length: 75						
Actuated Cycle Length: 75						
Offset: 66 (88%), Referenced	d to phase	2:NBTL	Start of	Green		
Natural Cycle: 60						
Control Type: Pretimed						
Maximum v/c Ratio: 0.70						
Intersection Signal Delay: 19	.0				ntersection	
Intersection Capacity Utilizat	ion 83.9%	,)		10	CU Level	of Service E

m Volume for 95th percentile queue is metered by upstream signal.

Analysis Period (min) 15



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Lane Group	WBT	NBT		
Lane Configurations	1. 1.	414		
Traffic Volume (vph)	14	1938		
Future Volume (vph)	14	1938		
Lane Group Flow (vph)	35	1971		
Sign Control	Stop	Free		
Intersection Summary				
Control Type: Unsignalized	d			
Intersection Capacity Utiliz	ffic Volume (vph) ure Volume (vph) ue Group Flow (vph) n Control tree Tree Tree			

Analysis Period (min) 15

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					ĵ.			414				
Traffic Volume (veh/h)	0	0	0	0	14	21	33	1938	0	0	0	0
Future Volume (Veh/h)	0	0	0	0	14	21	33	1938	0	0	0	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	0	0	0	0	14	21	33	1938	0	0	0	0
Pedestrians		92			133			15			8	
Lane Width (m)		0.0			3.7			3.7			0.0	
Walking Speed (m/s)		1.1			1.1			1.1			1.1	
Percent Blockage		0			12			1			0	
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								78			92	
pX, platoon unblocked	0.72	0.72		0.72	0.72	0.72				0.72		
vC, conflicting volume	840	2229	107	2152	2229	787	92			2071		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	0	1331	107	1223	1331	0	92			1110		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	100	85	97	98			100		
cM capacity (veh/h)	562	94	913	74	94	680	1501			392		
Direction, Lane #	WB 1	NB 1	NB 2	NB 3								
Volume Total	35	421	775	775								
Volume Left	0	33	0	0								
Volume Right	21	0	0	0								
cSH	195	1501	1700	1700								
Volume to Capacity	0.18	0.02	0.46	0.46								
Queue Length 95th (m)	4.8	0.5	0.0	0.0								
Control Delay (s)	27.5	0.8	0.0	0.0								
Lane LOS	D	Α										
Approach Delay (s)	27.5	0.2										
Approach LOS	D											
Intersection Summary												
Average Delay			0.6									
Intersection Capacity Utiliza	ation		54.8%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

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Lane Group	EBT	NBT
Lane Configurations	ની	ተተኈ
Traffic Volume (vph)	85	1957
Future Volume (vph)	85	1957
Lane Group Flow (vph)	102	2111
Turn Type	NA	NA
Protected Phases	4	2
Permitted Phases		
Minimum Split (s)	21.5	21.1
Total Split (s)	22.0	53.0
Total Split (%)	29.3%	70.7%
Yellow Time (s)	3.3	3.3
All-Red Time (s)	2.2	1.8
Lost Time Adjust (s)	0.0	0.0
Total Lost Time (s)	5.5	5.1
Lead/Lag		
Lead-Lag Optimize?		
Act Effct Green (s)	16.5	47.9
Actuated g/C Ratio	0.22	0.64
v/c Ratio	0.25	0.70
Control Delay	19.4	23.0
Queue Delay	0.0	0.0
Total Delay	19.4	23.0
LOS	В	С
Approach Delay	19.4	23.0
Approach LOS	В	С
Queue Length 50th (m)	8.2	114.7
Queue Length 95th (m)	20.1	128.7
Internal Link Dist (m)	157.9	224.2
Turn Bay Length (m)		
Base Capacity (vph)	410	3031
Starvation Cap Reductn	0	0
Spillback Cap Reductn	0	0
Storage Cap Reductn	0	0
Reduced v/c Ratio	0.25	0.70
Intersection Summary		
Cycle Length: 75		
Actuated Cycle Length: 75		
Offset: 5 (7%), Referenced	to phase 2:	NBT, Sta
Natural Cycle: 60	•	,
Control Type: Pretimed		
Maximum v/c Ratio: 0.70		
Intersection Signal Delay: 2	2.8	
Intersection Capacity Utiliza		
Analysis Period (min) 15	531170	
Splits and Phases: 3: Kei	nt St & Gilm	nour St
1 ø₂ (R)		
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Lane Group		•	→	•	4	†				
Lane Configurations 1	_ane Group	EBL	EBT	WBT	NBL	NBT				
Traffic Volume (vph) 82 299 178 36 1834 Lane Group Flow (vph) 82 299 326 36 1834 Turn Type Perm NA NA Perm NA Protected Phases 4 8 2 Permitted Phases 4 8 2 Minimum Split (s) 23.4 23.4 23.4 23.4 23.4 23.4 23.4 23.4	ane Configurations	ሻ		f.	ሻ	ተ ተኈ				
Future Volume (vph) 82 299 178 36 1834 Lane Group Flow (vph) 82 299 326 36 1931 Turn Type Perm NA NA Perm NA Protected Phases 4 2 Permitted Phases 4 2 Minimum Spitt (s) 23.4 23.4 23.4 23.4 23.4 23.4 3.4 Total Spitt (s) 30.0 30.0 30.0 45.0 45.0 Total Spitt (%) 40.0% 40.0% 40.0% 60.0% 60.0% 60.0% Vellow Time (s) 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 A.3 All-Red Time (s) 2.1 2.1 2.1 2.1 2.1 2.1 1.1 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.4 5.4 5.4 5.4 5.4 5.4 5.4 1.4 Lead/Lag Lead/Lag Optimize? Act Effect Green (s) 24.6 24.6 24.6 39.6 39.6 Actuated g/C Ratio 0.33 0.33 0.33 0.53 0.53 0.53 0.53 0.53										
Lane Group Flow (viph) 82 299 328 36 1931 Turn Type Perm NA NA Perm NA NA Perm		82								
Turn Type										
Protected Phases	,									
Permitted Phases 4 23.4 23.4 23.4 23.4 23.4 23.4 7.5 23.4 23.4 23.4 23.4 23.4 23.4 23.4 23.4										
Minimum Split (s)		4		_	2					
Total Split (s)			23.4	23.4		23.4				
Total Split (%)	,									
Vellow Time (s) 3.3										
NI-Red Time (s)										
Cost Time Adjust (s) 0.0										
Total Lost Time (s)										
Lead/Lag Optimize? Act Effct Green (s)										
Act Effic Green (s)			∪r	0.7	U.7	0.4				
Act Effct Green (s)										
Actuated g/C Ratio 0.33 0.33 0.33 0.53 0.53 0.53 0.53 0.56 (/c Ratio 0.34 0.51 0.62 0.05 0.76 Control Delay 24.0 24.1 26.8 8.9 16.3 Course Delay 0.0 0.0 0.0 0.0 0.0 Cotal Delay 24.0 24.1 26.8 8.9 16.3 Control Delay 24.1 26.8 8.9 16.3 Control Type: Pretimed		24.6	24.6	24.6	39.6	39.6				
C Ratio										
Control Delay 24.0 24.1 26.8 8.9 16.3 Dueue Delay 0.0 0.0 0.0 0.0 0.0 Doubley 24.0 24.1 26.8 8.9 16.3 Doubley 24.0 24.1 26.8 8.9 16.3 Doubley 24.1 26.8 8.9 16.3 Doubley 24.1 26.8 8.9 16.2 Doubley 24.1 26.8 16.2 Double Length Soth (m) 8.7 33.7 37.5 2.3 72.8 Dueue Length 95th (m) 20.3 55.7 63.1 6.3 90.8 Double Length 95th (m) 152.2 162.6 69.7 Double Length (m) 25.0 Double Starvation Cap Reductn 0 0 0 0 0 0 Double Cap Reductn 0 0 0 0	9									
Dicease Delay Dicease Diceas										
Total Delay	•									
Co C C C C B Reproach Delay 24.1 26.8 16.2										
Approach Delay 24.1 26.8 16.2 Approach LOS C C B Dueue Length 50th (m) 8.7 33.7 37.5 2.3 72.8 Dueue Length 95th (m) 20.3 55.7 63.1 6.3 90.8 Dueue Length (m) 25.0 69.7 Dueue Length (m) 25.0 Date Link Dist (m) 25.0 Date Capacity (vph) 241 585 529 747 2538 Date Capacity (vph) 241 585 529 747 2538 Date Capacity (vph) 241 585 529 747 2538 Date Capacity (vph										
Approach LOS		U			A					
Aueue Length 50th (m) 8.7 33.7 37.5 2.3 72.8 Aueue Length 95th (m) 20.3 55.7 63.1 6.3 90.8 Anternal Link Dist (m) 152.2 162.6 69.7 Aurum Bay Length (m) 25.0 Asse Capacity (vph) 241 585 529 747 2538 Catravation Cap Reductn 0 0 0 0 0 Assillaback Cap Reductn 0 0 0 0 0 Assillaback Cap Reductn 0 0 0 0 0 Assillaback Cap Reductn 0 0 0 0 0 Actorage Cap Reductn 0 0 0 0 0 0 Actorage Cap Reductn 0 0 0 0 0 0 Actorage Cap Reductn 0 0 0 0 0 0 Actorage Cap Reductn 0 0 0 0 0 0 Actorage Cap Reductn 0 0 0 0 0 0 Actorage Cap Reductn 0 0 0 0 0 0 Actorage Cap Reductn 0 0 0 0 0 0 Actorage Cap Reductn 0 0 0 0 0 0 Actorage Cap Reductn 0 0 0 0 Actorage Cap Reductn 0 0 0 0 Actorage Cap Reductn 0 0 0 0 0 Actorage Cap Reductn 0 0 0 0 Act										
Queue Length 95th (m) 20.3 55.7 63.1 6.3 90.8 Internal Link Dist (m) 152.2 162.6 69.7 Furn Bay Length (m) 25.0 Base Capacity (vph) 241 585 529 747 2538 Starvation Cap Reducth 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		0 7			2.2					
Internal Link Dist (m) 152.2 162.6 69.7 Furn Bay Length (m) 25.0 Base Capacity (vph) 241 585 529 747 2538 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Reduced v/c Ratio 0.34 0.51 0.62 0.05 0.76 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 36 (48%), Referenced to phase 2:NBTL, Start of Green Valural Cycle: 60 Control Type: Pretimed Maximum v/c Ratio: 0.76 Intersection Signal Delay: 18.6 Intersection LOS: B Intersection Capacity Utilization 83.2% ICU Level of Service E Analysis Period (min) 15 Splits and Phases: 4: Kent St & Gladstone Ave										
Furn Bay Length (m) 25.0 Base Capacity (vph) 241 585 529 747 2538 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 Reduced v/c Ratio 0.34 0.51 0.62 0.05 0.76 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 36 (48%), Referenced to phase 2:NBTL, Start of Green Valuated Cycle Ength: 75 Ocntrol Type: Pretimed Maximum v/c Ratio: 0.76 Intersection Signal Delay: 18.6 Intersection Capacity Utilization 83.2% Analysis Period (min) 15 Splits and Phases: 4: Kent St & Gladstone Ave		20.3			0.5					
Base Capacity (vph) 241 585 529 747 2538 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0	` ,	25.0	132.2	102.0		09.7				
Starvation Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			E0E	E20	7/7	2520				
Spillback Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0										
Storage Cap Reductn 0 0 0 0 0 0 Reduced v/c Ratio 0.34 0.51 0.62 0.05 0.76 Intersection Summary Cycle Length: 75 Intersection Summary Intersection										
Reduced v/c Ratio 0.34 0.51 0.62 0.05 0.76 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 36 (48%), Referenced to phase 2:NBTL, Start of Green Natural Cycle: 60 Control Type: Pretimed Maximum v/c Ratio: 0.76 Intersection Signal Delay: 18.6 Intersection Capacity Utilization 83.2% ICU Level of Service E Analysis Period (min) 15 Splits and Phases: 4: Kent St & Gladstone Ave	•									
Cycle Length: 75 Actuated Cycle Length: 75 Offset: 36 (48%), Referenced to phase 2:NBTL, Start of Green Matural Cycle: 60 Control Type: Pretimed Maximum v/c Ratio: 0.76 Intersection Signal Delay: 18.6 Intersection Capacity Utilization 83.2% Analysis Period (min) 15 Splits and Phases: 4: Kent St & Gladstone Ave										
Cycle Length: 75 Actuated Cycle Length: 75 Diffset: 36 (48%), Referenced to phase 2:NBTL, Start of Green Diatural Cycle: 60 Control Type: Pretimed Maximum v/c Ratio: 0.76 Intersection Signal Delay: 18.6 Intersection Capacity Utilization 83.2% Intersection Capacity Utilization 83.2% Explits and Phases: 4: Kent St & Gladstone Ave	Reduced V/C Ratio	0.34	0.51	0.62	0.05	0.76				
Actuated Cycle Length: 75 Offset: 36 (48%), Referenced to phase 2:NBTL, Start of Green Idutural Cycle: 60 Control Type: Pretimed Maximum v/c Ratio: 0.76 Intersection Signal Delay: 18.6 Intersection Capacity Utilization 83.2% ICU Level of Service E Intersection (min) 15 Splits and Phases: 4: Kent St & Gladstone Ave										
Offset: 36 (48%), Referenced to phase 2:NBTL, Start of Green Natural Cycle: 60 Control Type: Pretimed Maximum v/c Ratio: 0.76 Intersection Signal Delay: 18.6 Intersection Capacity Utilization 83.2% ICU Level of Service E Analysis Period (min) 15 Splits and Phases: 4: Kent St & Gladstone Ave	, ,									
Natural Cycle: 60 Control Type: Pretimed Maximum v/c Ratio: 0.76 Intersection Signal Delay: 18.6 Intersection Capacity Utilization 83.2% Intersection Capacity Utilization 83.2% ICU Level of Service E Analysis Period (min) 15 Splits and Phases: 4: Kent St & Gladstone Ave										
Control Type: Pretimed Maximum v/c Ratio: 0.76 Intersection Signal Delay: 18.6 Intersection Capacity Utilization 83.2% Intersection Capacity Utilization 83.2% Intersection Capacity Utilization 83.2% Intersection LOS: B ICU Level of Service E Analysis Period (min) 15 Splits and Phases: 4: Kent St & Gladstone Ave		ed to phase	2:NBTL	, Start of	Green					
Maximum v/c Ratio: 0.76 Intersection Signal Delay: 18.6 Intersection Capacity Utilization 83.2% Intersection Capacity Utilization 83.2% ICU Level of Service E I										
Intersection Signal Delay: 18.6 Intersection LOS: B Intersection Capacity Utilization 83.2% ICU Level of Service E Intersection Capacity Utilization 83.2% ICU Level of Service E ICU L										
ntersection Capacity Utilization 83.2% ICU Level of Service E Analysis Period (min) 15 Splits and Phases: 4: Kent St & Gladstone Ave										
Analysis Period (min) 15 Splits and Phases: 4: Kent St & Gladstone Ave	ntersection Signal Delay: 1	8.6								
Splits and Phases: 4: Kent St & Gladstone Ave		ation 83.2%)		10	CU Level o	of Service	ce E		
4	Analysis Period (min) 15									
4	Splits and Phases: 4: Ke	nt St & Gla	dstone A	ve						
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_ane Group	WBT	NBL	NBT	SBT	
ane Configurations	4		4	f _a	
Traffic Volume (vph)	4	36	535	352	
-uture Volume (vph)	4	36	535	352	
ane Group Flow (vph)	28	0	571	378	
Turn Type	NA	Perm	NA	NA	
Protected Phases	8		2	6	
Permitted Phases	•	2	_		
Minimum Split (s)	24.2	23.0	23.0	23.0	
Total Split (s)	24.0	51.0	51.0	51.0	
Total Split (%)	32.0%	68.0%	68.0%	68.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.9	1.7	1.7	1.7	
ost Time Adjust (s)	0.0	1.7	0.0	0.0	
Fotal Lost Time (s)	5.2		5.0	5.0	
Lead/Lag	J.Z		3.0	3.0	
Lead-Lag Optimize?					
Act Effct Green (s)	18.8		46.0	46.0	
Actuated g/C Ratio	0.25		0.61	0.61	
v/c Ratio	0.23		0.55	0.36	
Control Delay	15.5		2.7	8.2	
Queue Delay	0.0		0.2	0.2	
Total Delay	15.5		2.9	8.2	
LOS	13.5 B		2.9 A	0.Z A	
	15.5		2.9	8.2	
Approach Delay	13.5 B		2.9 A	0.Z A	
Approach LOS Queue Length 50th (m)	1.6		4.3	23.0	
	7.3		6.1	37.7	
Queue Length 95th (m)	126.3		56.4	50.2	
Internal Link Dist (m)	120.3		30.4	50.2	
Turn Bay Length (m)	383		1020	1041	
Base Capacity (vph)			1030		
Starvation Cap Reductn	0		66	0	
Spillback Cap Reductn	0		0	0	
Storage Cap Reductn	0		0	0	
Reduced v/c Ratio	0.07		0.59	0.36	
ntersection Summary					
Cycle Length: 75					
Actuated Cycle Length: 75					
Offset: 42 (56%), Reference	ed to phase	2:NBTL	and 6:SE	BT, Start o	of Green
Natural Cycle: 60					
Control Type: Pretimed					
Maximum v/c Ratio: 0.55					
ntersection Signal Delay: 5	.3			lr	ntersection LOS: A
ntersection Capacity Utiliza))			CU Level of Service E
Analysis Period (min) 15					
, ,	oli Ct O Ma	al ara:- 0	4		
Splits and Phases: 5: Ba	nk St & Ma	cLaren S	τ		T
√ T ø2 (R)					
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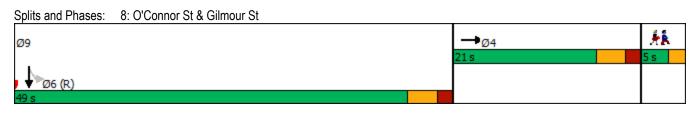
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Lane Group	EBT	NBT	SBL	SBT	
Lane Configurations	4	1		स	
Traffic Volume (vph)	48	583	15	231	
Future Volume (vph)	48	583	15	231	
Lane Group Flow (vph)	111	617	0	246	
Turn Type	NA	NA	Perm	NA	
Protected Phases	4	2		6	
Permitted Phases			6		
Minimum Split (s)	23.2	26.1	26.1	26.1	
Total Split (s)	25.0	50.0	50.0	50.0	
Total Split (%)	33.3%	66.7%	66.7%	66.7%	
Yellow Time (s)	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.9	1.8	1.8	1.8	
Lost Time Adjust (s)	0.0	0.0		0.0	
Total Lost Time (s)	5.2	5.1		5.1	
Lead/Lag					
Lead-Lag Optimize?	10.0	44.9		44.9	
Act Effct Green (s) Actuated g/C Ratio	19.8 0.26	0.60		0.60	
v/c Ratio	0.26	0.60		0.60	
Control Delay	21.3	12.1		4.8	
Queue Delay	0.0	0.0		0.3	
Total Delay	21.3	12.1		5.1	
LOS	Z1.5	12.1 B		J.1	
Approach Delay	21.3	12.1		5.1	
Approach LOS	C C	В		Α	
Queue Length 50th (m)	9.8	48.1		6.9	
Queue Length 95th (m)	m19.1	76.3		11.0	
Internal Link Dist (m)	115.9	89.4		56.4	
Turn Bay Length (m)					
Base Capacity (vph)	439	1045		1020	
Starvation Cap Reductn	0	0		365	
Spillback Cap Reductn	0	0		0	
Storage Cap Reductn	0	0		0	
Reduced v/c Ratio	0.25	0.59		0.38	
Intersection Summary					
Cycle Length: 75					
Actuated Cycle Length: 75					
Offset: 37 (49%), Reference	ed to phase	2·NRT a	and 6:SBT	ΓΙ Start (of Green
Natural Cycle: 60	ou to pridat	, Z.14D1 (a 0.0D	i L, Olait (OI OIOOII
Control Type: Pretimed					
Maximum v/c Ratio: 0.59					
Intersection Signal Delay: 1	1.4			lı	ntersection LOS: B
Intersection Capacity Utiliza)			CU Level of Service B
Analysis Period (min) 15				·	22.2. 2. 2011100 D
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m Volume for 95th percentile queue is metered by upstream signal.



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Lane Group	EBT	SBT	Ø9
Lane Configurations	f)	41∱	
Traffic Volume (vph)	61	672	
Future Volume (vph)	61	672	
Lane Group Flow (vph)	110	721	
Turn Type	NA	NA	
Protected Phases	4	6	9
Permitted Phases			
Minimum Split (s)	20.1	26.1	5.0
Total Split (s)	21.0	49.0	5.0
Total Split (%)	28.0%	65.3%	7%
Yellow Time (s)	3.3	3.3	2.0
All-Red Time (s)	1.8	1.8	0.0
Lost Time Adjust (s)	0.0	0.0	-0.0
Total Lost Time (s)	5.1	5.1	
Lead/Lag		J. 1	
Lead-Lag Optimize?			
Act Effct Green (s)	15.9	43.9	
Actuated g/C Ratio	0.21	0.59	
v/c Ratio	0.28	0.36	
Control Delay	13.7	8.1	
Queue Delay	0.0	0.0	
Total Delay	13.7	8.1	
LOS	13.7 B	Α	
Approach Delay	13.7	8.1	
Approach LOS	13.7 B	Α	
Queue Length 50th (m)	5.6	23.0	
	m14.0	32.7	
Queue Length 95th (m)	159.1	135.3	
Internal Link Dist (m)	159.1	133.3	
Turn Bay Length (m)	207	1000	
Base Capacity (vph)	387	1980	
Starvation Cap Reductn	0	0	
Spillback Cap Reductn	0	0	
Storage Cap Reductn	0	0	
Reduced v/c Ratio	0.28	0.36	
Intersection Summary			
Cycle Length: 75			
Actuated Cycle Length: 75			
Offset: 46 (61%), Reference	ed to phase	6:SBTI	Start of Green
Natural Cycle: 55	ou to pridoc	, 0.0D i L,	otali oi oi oiii
Control Type: Pretimed			
Maximum v/c Ratio: 0.36			
Intersection Signal Delay: 8	8.8		
Intersection Capacity Utiliza			
Analysis Period (min) 15	/ T.J.T / (, 	
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m Volume for 95th percentile queue is metered by upstream signal.



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Lane Group	EBL	EBT	WBT	NBL	NBT	
Lane Configurations	ች	1	4	*	ተ ተጉ	
Traffic Volume (vph)	56	359	327	77	762	
Future Volume (vph)	56	359	327	77	762	
Lane Group Flow (vph)	56	359	390	77	868	
Turn Type	Perm	NA	NA	Perm	NA	
Protected Phases	1 01111	4	8	1 01111	2	
Permitted Phases	4		U	2	2	
Minimum Split (s)	23.5	23.5	23.5	23.4	23.4	
Total Split (s)	41.0	41.0	41.0	34.0	34.0	
Total Split (%)	54.7%	54.7%	54.7%	45.3%	45.3%	
. , ,	3.3	3.3	3.3	3.3	3.3	
Yellow Time (s)	2.2	2.2	2.2	2.1	3.3 2.1	
All-Red Time (s)						
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.5	5.5	5.5	5.4	5.4	
Lead/Lag						
Lead-Lag Optimize?	A	0 = =	0		00.0	
Act Effct Green (s)	35.5	35.5	35.5	28.6	28.6	
Actuated g/C Ratio	0.47	0.47	0.47	0.38	0.38	
v/c Ratio	0.17	0.43	0.49	0.24	0.49	
Control Delay	12.9	15.0	16.2	22.7	21.8	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	12.9	15.0	16.2	22.7	21.8	
LOS	В	В	В	С	С	
Approach Delay		14.7	16.2		21.9	
Approach LOS		В	В		С	
Queue Length 50th (m)	4.3	31.8	35.9	7.0	28.3	
Queue Length 95th (m)	10.9	51.7	58.5	17.4	58.6	
Internal Link Dist (m)		97.2	154.7		68.5	
Turn Bay Length (m)	20.0			20.0		
Base Capacity (vph)	337	844	794	326	1759	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.17	0.43	0.49	0.24	0.49	
ntersection Summary						
Cycle Length: 75						
Actuated Cycle Length: 75						
Offset: 50 (67%), Referenced	d to phase	e 2:NBTL	, Start of	Green		
Natural Cycle: 50						
Control Type: Pretimed						
Maximum v/c Ratio: 0.49						
Intersection Signal Delay: 18	3.9				ntersection	
Intersection Capacity Utilizat	tion 75.9%)		10	CU Level o	of Service D
Analysis Period (min) 15						
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	t St & Sor	nerset St	W		A ₀₄	

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Lane Group	WBT	NBT
Lane Configurations	1 >	ተተቡ
Traffic Volume (vph)	22	939
Future Volume (vph)	22	939
Lane Group Flow (vph)	92	970
Sign Control	Stop	Free
Intersection Summary		
Control Type: Unsignalized	d	
Intersection Canacity Utiliz		

Intersection Capacity Utilization 38.5%

Analysis Period (min) 15

Synchro 10 Report Parsons

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					f)			414				
Traffic Volume (veh/h)	0	0	0	0	22	70	31	939	0	0	0	0
Future Volume (Veh/h)	0	0	0	0	22	70	31	939	0	0	0	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	0	0	0	0	22	70	31	939	0	0	0	0
Pedestrians		45			95			11			17	
Lane Width (m)		0.0			3.7			3.7			0.0	
Walking Speed (m/s)		1.1			1.1			1.1			1.1	
Percent Blockage		0			9			1			0	
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								78			92	
pX, platoon unblocked	0.92	0.92		0.92	0.92	0.92				0.92		
vC, conflicting volume	518	1141	56	1107	1141	425	45			1034		
vC1, stage 1 conf vol	<u> </u>											
vC2, stage 2 conf vol												
vCu, unblocked vol	179	855	56	818	855	78	45			739		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)		0.0	0.0		0.0	0.0						
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	100	91	91	98			100		
cM capacity (veh/h)	552	242	988	204	242	812	1561			725		
Direction, Lane #	WB 1	NB 1	NB 2	NB 3								
Volume Total	92	219	376	376								
		31										
Volume Left	0		0	0								
Volume Right	70	1561	1700	1700								
cSH	519	1561	1700	1700								
Volume to Capacity	0.18	0.02	0.22	0.22								
Queue Length 95th (m)	4.9	0.5	0.0	0.0								
Control Delay (s)	13.4	1.2	0.0	0.0								
Lane LOS	В	A										
Approach Delay (s)	13.4	0.3										
Approach LOS	В											
Intersection Summary												
Average Delay			1.4									
Intersection Capacity Utiliz	ation		38.5%	IC	CU Level	of Service			Α			
Analysis Period (min)			15									

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Lane Group	EBT	NBT
Lane Configurations	ની	ተተኈ
Traffic Volume (vph)	77	914
Future Volume (vph)	77	914
Lane Group Flow (vph)	96	992
Turn Type	NA	NA
Protected Phases	4	2
Permitted Phases		
Minimum Split (s)	21.5	21.1
Total Split (s)	22.0	53.0
Total Split (%)	29.3%	70.7%
Yellow Time (s)	3.3	3.3
All-Red Time (s)	2.2	1.8
Lost Time Adjust (s)	0.0	0.0
Total Lost Time (s)	5.5	5.1
Lead/Lag		
Lead-Lag Optimize?		
Act Effct Green (s)	16.5	47.9
Actuated g/C Ratio	0.22	0.64
v/c Ratio	0.24	0.33
Control Delay	19.0	9.4
Queue Delay	0.0	0.0
Total Delay	19.0	9.4
LOS	В	Α
Approach Delay	19.0	9.4
Approach LOS	В	Α
Queue Length 50th (m)	7.5	22.2
Queue Length 95th (m)	19.0	36.1
Internal Link Dist (m)	157.9	224.2
Turn Bay Length (m)		
Base Capacity (vph)	405	3019
Starvation Cap Reductn	0	0
Spillback Cap Reductn	0	0
Storage Cap Reductn	0	0
Reduced v/c Ratio	0.24	0.33
Intersection Summary		
Cycle Length: 75		
Actuated Cycle Length: 75		
Offset: 0 (0%), Referenced		NIDT C+~
	i to phase 2.	IND I, SIA
Natural Cycle: 45 Control Type: Pretimed		
Maximum v/c Ratio: 0.33		
	10.0	
Intersection Signal Delay:		
Intersection Capacity Utiliz	ation 43.1%	
Analysis Period (min) 15		
Splits and Phases: 3: Ke	ent St & Gilm	our St
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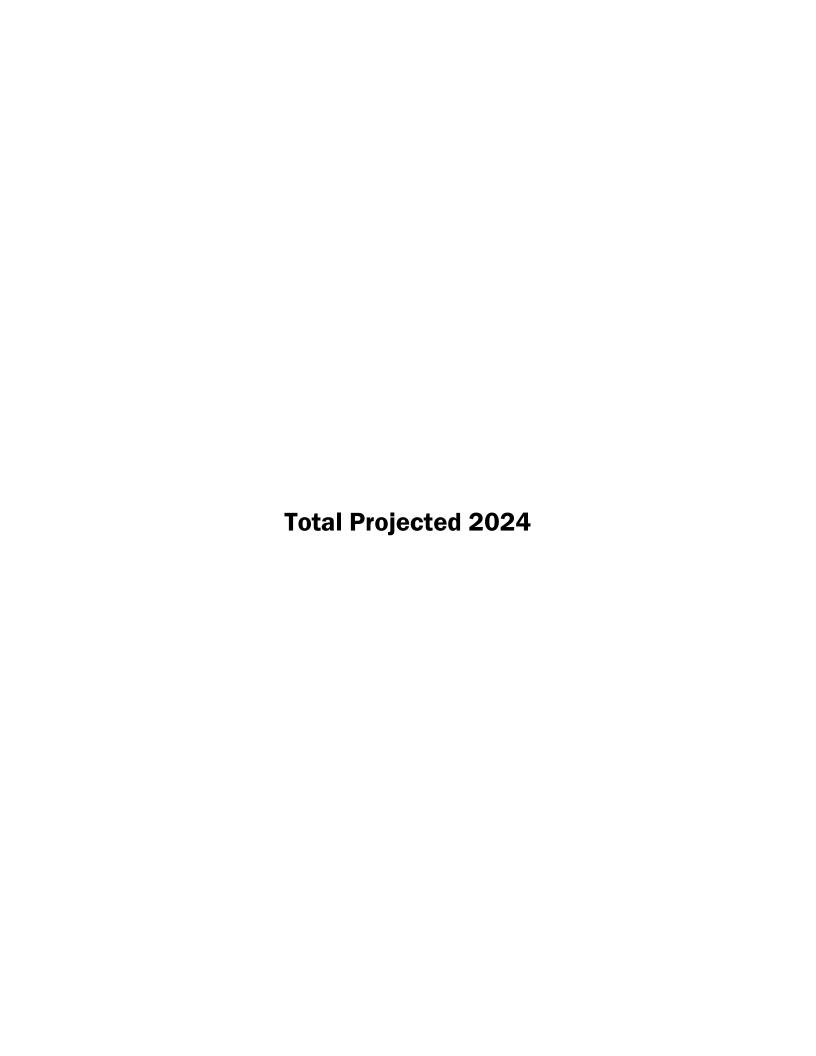
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Lane Group	EBL	EBT	WBT	NBL	NBT	
Lane Configurations	ሻ	†	ą.	ሻ	ተተኈ	
Traffic Volume (vph)	75	486	350	52	780	
Future Volume (vph)	75	486	350	52	780	
Lane Group Flow (vph)	75	486	425	52	886	
Turn Type	Perm	NA	NA	Perm	NA	
Protected Phases		4	8		2	
Permitted Phases	4		_	2	_	
Minimum Split (s)	23.4	23.4	23.4	23.4	23.4	
Total Split (s)	40.0	40.0	40.0	35.0	35.0	
Total Split (%)	53.3%	53.3%	53.3%	46.7%	46.7%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.1	2.1	2.1	2.1	2.1	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.4	5.4	5.4	5.4	
Lead/Lag	0.1	0.1	0.1	0.1	0.1	
Lead-Lag Optimize?						
Act Effct Green (s)	34.6	34.6	34.6	29.6	29.6	
Actuated g/C Ratio	0.46	0.46	0.46	0.39	0.39	
v/c Ratio	0.23	0.59	0.53	0.09	0.47	
Control Delay	14.6	18.6	16.7	14.9	17.1	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	14.6	18.6	16.7	14.9	17.1	
LOS	В	В	В	В	В	
Approach Delay		18.1	16.7		17.0	
Approach LOS		В	В		В	
Queue Length 50th (m)	6.1	48.6	38.9	4.5	31.5	
Queue Length 95th (m)	14.6	76.6	63.7	11.0	42.1	
Internal Link Dist (m)	17.0	152.2	162.6	11.0	69.7	
Turn Bay Length (m)	25.0	102.2	102.0		00.1	
Base Capacity (vph)	327	823	801	588	1879	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.23	0.59	0.53	0.09	0.47	
	0.20	0.00	0.00	0.00	0.47	
Intersection Summary						
Cycle Length: 75						
Actuated Cycle Length: 75	14	O NIDTI	01 1 1	^		
Offset: 23 (31%), Reference	ed to phase	e 2:NBTL	, Start of	Green		
Natural Cycle: 50						
Control Type: Pretimed						
Maximum v/c Ratio: 0.59						
Intersection Signal Delay: 1					ntersection	
Intersection Capacity Utiliza	tion 65.6%	ò		IC	CU Level	of Service C
Analysis Period (min) 15						
Splits and Phases: 4: Kei	nt St & Gla	dstone A	ve			
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35 s				4	10 s	
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Lane Group	WBT	NBL	NBT	SBT	
Lane Configurations	4		ર્ન	ą.	
Traffic Volume (vph)	11	25	297	670	
Future Volume (vph)	11	25	297	670	
Lane Group Flow (vph)	56	0	322	696	
Turn Type	NA	Perm	NA	NA	
Protected Phases	8		2	6	
Permitted Phases		2		_	
Minimum Split (s)	24.2	23.0	23.0	23.0	
Total Split (s)	24.0	51.0	51.0	51.0	
Total Split (%)	32.0%	68.0%	68.0%	68.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.9	1.7	1.7	1.7	
Lost Time Adjust (s)	0.0		0.0	0.0	
Total Lost Time (s)	5.2		5.0	5.0	
Lead/Lag	0.2		0.0	0.0	
Lead-Lag Optimize?					
Act Effct Green (s)	18.8		46.0	46.0	
Actuated g/C Ratio	0.25		0.61	0.61	
v/c Ratio	0.25		0.01	0.66	
Control Delay	20.1		2.6	13.0	
Queue Delay	0.0		0.3	0.0	
Total Delay	20.1		2.8	13.0	
LOS	Z0.1		2.0 A	13.0 B	
Approach Delay	20.1		2.8	13.0	
Approach LOS	Z0.1		2.0 A	13.0 B	
Queue Length 50th (m)	5.0		4.0	56.2	
Queue Length 95th (m)	13.5		5.8	90.1	
Internal Link Dist (m)	126.3		56.4	50.2	
Turn Bay Length (m)	120.5		30.4	30.2	
Base Capacity (vph)	366		1010	1061	
Starvation Cap Reductn	0		239	0	
Spillback Cap Reductn	0		239	0	
Storage Cap Reductn	0		0	0	
Reduced v/c Ratio	0.15		0.42	0.66	
Intersection Summary					
Cycle Length: 75					
Actuated Cycle Length: 75					
Offset: 7 (9%), Referenced	to phase 2	:NBTL ar	nd 6:SBT,	Start of (Green
Natural Cycle: 60					
Control Type: Pretimed					
Maximum v/c Ratio: 0.66					
Intersection Signal Delay: 1					ntersection LOS: B
Intersection Capacity Utiliza	ation 63.6%)		10	CU Level of Service B
Analysis Period (min) 15					
Splits and Phases: 5: Bai	nk St & Ma	cLaren S	it		
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Ø6 (R)

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Lane Group	EBT	NBT	SBL	SBT	
Lane Configurations	4	7>	- 555	4	
Traffic Volume (vph)	116	443	19	370	
Future Volume (vph)	116	443	19	370	
Lane Group Flow (vph)	191	471	0	389	
Turn Type	NA	NA	Perm	NA	
Protected Phases	4	2		6	
Permitted Phases			6		
Minimum Split (s)	23.2	26.1	26.1	26.1	
Total Split (s)	25.0	50.0	50.0	50.0	
Total Split (%)	33.3%	66.7%	66.7%	66.7%	
Yellow Time (s)	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.9	1.8	1.8	1.8	
Lost Time Adjust (s)	0.0	0.0		0.0	
Total Lost Time (s)	5.2	5.1		5.1	
Lead/Lag					
Lead-Lag Optimize?					
Act Effct Green (s)	19.8	44.9		44.9	
Actuated g/C Ratio	0.26	0.60		0.60	
v/c Ratio	0.43	0.45		0.38	
Control Delay	18.3	9.9		4.2	
Queue Delay	0.0	0.0		0.6	
Total Delay	18.3	9.9		4.8	
LOS	В	Α		Α	
Approach Delay	18.3	9.9		4.8	
Approach LOS	В	Α		Α	
Queue Length 50th (m)	12.6	32.3		8.8	
Queue Length 95th (m)	24.6	51.7		12.8	
Internal Link Dist (m)	115.9	89.4		56.4	
Turn Bay Length (m)					
Base Capacity (vph)	444	1040		1030	
Starvation Cap Reductn	0	0		314	
Spillback Cap Reductn	0	0		0	
Storage Cap Reductn	0	0		0	
Reduced v/c Ratio	0.43	0.45		0.54	
Intersection Summary					
Cycle Length: 75					
Actuated Cycle Length: 75	4 0	NDT	I C.ODTI	0111	D
Offset: 6 (8%), Referenced	to phase 2	::NB1 and	16:SBTL,	Start of 0	reen
Natural Cycle: 50					
Control Type: Pretimed					
Maximum v/c Ratio: 0.45	\ -				
Intersection Signal Delay: 9					itersection LOS: A
Intersection Capacity Utiliza	ation 64.5%	0		I	CU Level of Service C
Analysis Period (min) 15					
Splits and Phases: 6: Ba	ınk St & Gil	mour St			
A					
Ø2 (R)					

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Lane Group	EBT	SBT	Ø9		
Lane Configurations	4î	41∱			
Traffic Volume (vph)	84	1285			
Future Volume (vph)	84	1285			
ane Group Flow (vph)	173	1333			
Turn Type	NA	NA			
Protected Phases	4	6	9		
Permitted Phases	7	U	3		
Minimum Split (s)	20.1	26.1	5.0		
Fotal Split (s)	21.0	49.0	5.0		
Fotal Split (%)	28.0%	65.3%	7%		
fellow Time (s)	3.3	3.3	2.0		
All-Red Time (s)	1.8	1.8	0.0		
	0.0	0.0	0.0		
Lost Time Adjust (s)					
Total Lost Time (s)	5.1	5.1			
_ead/Lag					
Lead-Lag Optimize?	45.0	40.0			
Act Effct Green (s)	15.9	43.9			
Actuated g/C Ratio	0.21	0.59			
v/c Ratio	0.45	0.67			
Control Delay	30.3	12.2			
Queue Delay	0.0	0.0			
Total Delay	30.3	12.2			
_OS	С	В			
Approach Delay	30.3	12.2			
Approach LOS	С	В			
Queue Length 50th (m)	17.0	58.3			
Queue Length 95th (m)	34.7	79.0			
nternal Link Dist (m)	159.1	135.3			
Turn Bay Length (m)					
Base Capacity (vph)	386	1987			
Starvation Cap Reductn	0	0			
Spillback Cap Reductn	0	0			
Storage Cap Reductn	0	0			
Reduced v/c Ratio	0.45	0.67			
ntersection Summary					
Cycle Length: 75 Actuated Cycle Length: 75					
	d to phose	S G-CDTI	Ctart of Crass		
Offset: 71 (95%), Reference	u to priase	O.OBIL,	Start of Green		
Natural Cycle: 60					
Control Type: Pretimed					
Maximum v/c Ratio: 0.67	1.0			Internación I CO D	
ntersection Signal Delay: 14				Intersection LOS: B	
Intersection Capacity Utilizat	tion 63.4%)		ICU Level of Service B	
Analysis Period (min) 15					
Splits and Phases: 8: O'C	onnor St 8	& Gilmour	St		
'					I
Ø9					→ Ø4
					21 s
Ø6 (R)					1
49 s					ı



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Lane Group	EBL	EBT	WBT	NBL	NBT
Lane Configurations	ኝ	ĵ.	4	*	ተተኈ
Traffic Volume (vph)	62	304	138	75	1601
Future Volume (vph)	62	304	138	75	1601
Lane Group Flow (vph)	62	304	188	75	1785
Turn Type	Perm	NA	NA	Perm	NA
Protected Phases		4	8		2
Permitted Phases	4			2	
Minimum Split (s)	23.5	23.5	23.5	23.4	23.4
Total Split (s)	27.0	27.0	27.0	48.0	48.0
Total Split (%)	36.0%	36.0%	36.0%	64.0%	64.0%
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3
All-Red Time (s)	2.2	2.2	2.2	2.1	2.1
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.5	5.5	5.5	5.4	5.4
Lead/Lag					
Lead-Lag Optimize?					
Act Effct Green (s)	21.5	21.5	21.5	42.6	42.6
Actuated g/C Ratio	0.29	0.29	0.29	0.57	0.57
v/c Ratio	0.22	0.59	0.40	0.12	0.67
Control Delay	22.9	28.7	24.6	13.3	15.9
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	22.9	28.7	24.6	13.3	15.9
LOS	С	С	С	В	В
Approach Delay		27.7	24.6		15.8
Approach LOS		С	С		В
Queue Length 50th (m)	6.6	36.7	21.3	5.8	49.5
Queue Length 95th (m)	15.9	61.0	38.3	m10.1	66.8
Internal Link Dist (m)		97.2	154.7		68.5
Turn Bay Length (m)	20.0			20.0	
Base Capacity (vph)	286	511	475	628	2656
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.22	0.59	0.40	0.12	0.67
Intersection Summary					
Cycle Length: 75					
Actuated Cycle Length: 75					
Offset: 66 (88%), Reference	ed to phase	2·NRTI	Start of	Green	
Natural Cycle: 55	od to priast	, Z.IYD I L	, Start UI	Olecii	
Control Type: Pretimed					
Maximum v/c Ratio: 0.67					
Intersection Signal Delay: 1	8 3			lr	ntersectio
Intersection Signal Delay. I				11	

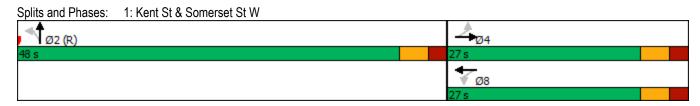
m Volume for 95th percentile queue is metered by upstream signal.

Intersection Capacity Utilization 81.5%

Analysis Period (min) 15

Parsons Synchro 10 Report

ICU Level of Service D



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Lane Group	WBT	NBT	
Lane Configurations	^	414	
Traffic Volume (vph)	14	1848	
Future Volume (vph)	14	1848	
Lane Group Flow (vph)	35	1881	
Sign Control	Stop	Free	
Intersection Summary			
Control Type: Unsignalize			
Intersection Capacity Util	ization 53.0%		ICU Level of Service A

Analysis Period (min) 15

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					f)			₽₽₽₽				
Traffic Volume (veh/h)	0	0	0	0	14	21	33	1848	0	0	0	0
Future Volume (Veh/h)	0	0	0	0	14	21	33	1848	0	0	0	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	0	0	0	0	14	21	33	1848	0	0	0	0
Pedestrians		92			133			15			8	
Lane Width (m)		0.0			3.7			3.7			0.0	
Walking Speed (m/s)		1.1			1.1			1.1			1.1	
Percent Blockage		0			12			1			0	
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								78			92	
pX, platoon unblocked	0.74	0.74		0.74	0.74	0.74		. 0		0.74	<u> </u>	
vC, conflicting volume	810	2139	107	2062	2139	757	92			1981		
vC1, stage 1 conf vol	0.0			2002	00	, 0,	V-			1001		
vC2, stage 2 conf vol												
vCu, unblocked vol	0	1306	107	1201	1306	0	92			1092		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)	7.0	0.0	0.0	7.0	0.0	0.0						
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	100	86	97	98			100		
cM capacity (veh/h)	584	101	913	80	101	702	1501			411		
, , ,					101	702	1001			711		
Direction, Lane #	WB 1	NB 1	NB 2	NB 3								
Volume Total	35	403	739	739								
Volume Left	0	33	0	0								
Volume Right	21	0	0	0								
cSH	207	1501	1700	1700								
Volume to Capacity	0.17	0.02	0.43	0.43								
Queue Length 95th (m)	4.5	0.5	0.0	0.0								
Control Delay (s)	25.9	0.8	0.0	0.0								
Lane LOS	D	Α										
Approach Delay (s)	25.9	0.2										
Approach LOS	D											
Intersection Summary												
Average Delay			0.6									
Intersection Capacity Utilization	ation		53.0%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

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Lane Group	EBT	NBT
Lane Configurations	<u></u> €1	^^
Traffic Volume (vph)	85	1866
Future Volume (vph)	85	1866
Lane Group Flow (vph)	102	2021
Turn Type	NA	NA
Protected Phases	4	2
Permitted Phases		
Minimum Split (s)	21.5	21.1
Total Split (s)	22.0	53.0
Total Split (%)	29.3%	70.7%
Yellow Time (s)	3.3	3.3
All-Red Time (s)	2.2	1.8
Lost Time Adjust (s)	0.0	0.0
Total Lost Time (s)	5.5	5.1
Lead/Lag		
Lead-Lag Optimize?		
Act Effct Green (s)	16.5	47.9
Actuated g/C Ratio	0.22	0.64
v/c Ratio	0.25	0.67
Control Delay	19.4	22.5
Queue Delay	0.0	0.0
Total Delay	19.4	22.5
LOS	В	С
Approach Delay	19.4	22.5
Approach LOS	В	С
Queue Length 50th (m)	8.2	107.6
Queue Length 95th (m)	20.1	122.1
Internal Link Dist (m)	157.9	224.2
Turn Bay Length (m)		
Base Capacity (vph)	410	3026
Starvation Cap Reductn	0	0
Spillback Cap Reductn	0	0
Storage Cap Reductn	0	0
Reduced v/c Ratio	0.25	0.67
Intersection Summary		
Cycle Length: 75		
Actuated Cycle Length: 75		
Offset: 5 (7%), Referenced	to phase 2	NBT Sta
Natural Cycle: 55	to pridoc 2.	IND I, Old
Control Type: Pretimed		
Maximum v/c Ratio: 0.67		
Intersection Signal Delay: 2	22.3	
Intersection Capacity Utiliza		
Analysis Period (min) 15	GGOTT 07.0 /0	
Splits and Phases: 3: Ke	ent St & Gilm	our St
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53 S		

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Lane Group	EBL	EBT	WBT	NBL	NBT	
Lane Configurations	¥	†	f)	ň	ተተ _ጮ	
Traffic Volume (vph)	82	285	170	36	1749	
Future Volume (vph)	82	285	170	36	1749	
Lane Group Flow (vph)	82	285	318	36	1846	
Turn Type	Perm	NA	NA	Perm	NA	
Protected Phases		4	8		2	
Permitted Phases	4			2		
Minimum Split (s)	23.4	23.4	23.4	23.4	23.4	
Total Split (s)	30.0	30.0	30.0	45.0	45.0	
Total Split (%)	40.0%	40.0%	40.0%	60.0%	60.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.1	2.1	2.1	2.1	2.1	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.4	5.4	5.4	5.4	
Lead/Lag	•	•	• • • • • • • • • • • • • • • • • • • •	•	U	
Lead-Lag Optimize?						
Act Effct Green (s)	24.6	24.6	24.6	39.6	39.6	
Actuated g/C Ratio	0.33	0.33	0.33	0.53	0.53	
v/c Ratio	0.33	0.49	0.60	0.05	0.73	
Control Delay	23.7	23.6	26.2	8.9	15.5	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	23.7	23.6	26.2	8.9	15.5	
LOS	C	C	C	A	В	
Approach Delay		23.6	26.2	, ,	15.4	
Approach LOS		С	C		В	
Queue Length 50th (m)	8.6	31.8	36.0	2.3	67.5	
Queue Length 95th (m)	20.1	53.0	60.9	6.3	84.4	
Internal Link Dist (m)		152.2	162.6	0.0	69.7	
Turn Bay Length (m)	25.0	. •	.02.0		•••	
Base Capacity (vph)	247	585	529	747	2538	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.33	0.49	0.60	0.05	0.73	
	0.00	0.10	0.00	0.00	0.70	
Intersection Summary						
Cycle Length: 75						
Actuated Cycle Length: 75						
Offset: 36 (48%), Reference	ed to phase	2:NBTL	, Start of	Green		
Natural Cycle: 60						
Control Type: Pretimed						
Maximum v/c Ratio: 0.73						
Intersection Signal Delay: 1					ntersectio	
Intersection Capacity Utiliza	tion 81.0%)		IC	CU Level	of Service D
Analysis Period (min) 15						
Splits and Phases: 4: Ker	nt St & Gla	dstone A	ve			
Ø2 (R)						♣ 04

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Lane Group	WBT	NBL	NBT	SBT		
Lane Configurations	4		4	1		
Traffic Volume (vph)	4	36	515	336		
Future Volume (vph)	4	36	515	336		
Lane Group Flow (vph)	28	0	551	362		
Turn Type	NA	Perm	NA	NA		
Protected Phases	8	. 0	2	6		
Permitted Phases		2	_	· ·		
Minimum Split (s)	24.2	23.0	23.0	23.0		
Total Split (s)	24.0	51.0	51.0	51.0		
Total Split (%)	32.0%	68.0%	68.0%	68.0%		
Yellow Time (s)	3.3	3.3	3.3	3.3		
All-Red Time (s)	1.9	1.7	1.7	1.7		
Lost Time Adjust (s)	0.0		0.0	0.0		
Total Lost Time (s)	5.2		5.0	5.0		
Lead/Lag						
Lead-Lag Optimize?						
Act Effct Green (s)	18.8		46.0	46.0		
Actuated g/C Ratio	0.25		0.61	0.61		
v/c Ratio	0.07		0.54	0.35		
Control Delay	15.5		2.9	8.1		
Queue Delay	0.0		0.1	0.0		
Total Delay	15.5		3.0	8.1		
LOS	В		Α	Α		
Approach Delay	15.5		3.0	8.1		
Approach LOS	В		Α	Α		
Queue Length 50th (m)	1.6		5.1	21.6		
Queue Length 95th (m)	7.3		7.1	35.6		
Internal Link Dist (m)	126.3		56.4	50.2		
Turn Bay Length (m)			1000	1655		
Base Capacity (vph)	383		1029	1039		
Starvation Cap Reductn	0		66	0		
Spillback Cap Reductn	0		0	0		
Storage Cap Reductn	0		0	0		
Reduced v/c Ratio	0.07		0.57	0.35		
ntersection Summary						
Cycle Length: 75						
Actuated Cycle Length: 75						
Offset: 42 (56%), Referenced	to phase	2:NBTL	and 6:SE	T, Start o	f Green	
Natural Cycle: 55						
Control Type: Pretimed						
Maximum v/c Ratio: 0.54						
Intersection Signal Delay: 5.3					tersection LOS: A	
ntersection Capacity Utilizati	on 80.2%)		IC	CU Level of Service D	
Analysis Period (min) 15						
Splits and Phases: 5: Bank	CSt & Ma	cLaren S	: f			
4	COLOX IVID	ocaleli S				
Ø2 (R)						
518						
▼ Ø6 (R)						₹ Ø8

Cane Configurations		-	†	>	ļ	
Anne Configurations raffic Volume (vph) 60 556 15 220	Lane Group	EBT	NBT	SBL	SBT	
Traffic Volume (vph) 60 556 15 220 Tuture Volume (vph) 60 556 15 220 Tuture Volume (vph) 60 556 15 220 Tuture Volume (vph) 133 590 0 235 Tuture Type NA NA Perm NA Protected Phases 4 2 6 Permitted Phases 6 Alinimum Split (s) 23.2 26.1 26.1 26.1 Total Split (s) 33.3 66.7% 66.7% 66.7% Fellow Time (s) 3.3 3.3 3.3 3.3 3.3 All-Red Time (s) 1.9 1.8 1.8 1.8 1.8 Lost Time Adjust (s) 0.0 0.0 0.0 Total Lost Time (s) 5.2 5.1 5.1 Lead/Lag Optimize? Act Effet Green (s) 19.8 44.9 44.9 Actuated g/C Ratio 0.26 0.60 0.60 Total Control Delay 22.4 11.7 4.8 Dueue Delay 0.0 0.0 0.3 Total Delay 22.4 11.7 5.1 COS C B A Approach Delay 22.4 11.7 5.1 OS C B A Approach LOS C B A Approach LOS C B A Approach LOS C B A Approach Lost (m) 12.4 44.8 6.7 Dueue Length 95th (m) 12.4 44.8 6.7 Dueue Length 95th (m) 12.4 44.8 6.7 Dueue Length 95th (m) 15.9 89.4 56.4 Tuture Bay Length (m) Base Capacity (vph) 440 1043 1020 Starvation Cap Reductn 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0						
Future Volume (vph) 60 556 15 220 Jane Group Flow (vph) 133 590 0 235 Furn Type NA NA Perm NA Protected Phases 4 2 6 Permitted Phases 7 Permi	Traffic Volume (vph)			15		
Start Star	Future Volume (vph)			15		
Protected Phases 6 Permitted Phases 6 Permitted Phases 6 Alinimum Split (s) 23.2 26.1 26.1 26.1 Total Split (s) 25.0 50.0 50.0 50.0 Total Split (%) 33.3% 66.7% 66.7% 66.7% All-Red Time (s) 3.3 3.3 3.3 3.3 All-Red Time (s) 1.9 1.8 1.8 1.8 All-Red Time (s) 5.2 5.1 5.1 Cost Time Adjust (s) 0.0 0.0 0.0 Total Lost Time (s) 5.2 5.1 5.1 Lost Time Adjust (s) 0.0 0.0 0.0 Total Lost Time (s) 5.2 5.1 5.1 Lost Time Adjust (s) 0.0 0.0 0.0 Total Lost Time (s) 5.2 5.1 5.1 Lost Time Adjust (s) 0.0 0.0 0.0 Total Lost Time (s) 5.2 5.1 5.1 Lost Time Adjust (s) 0.0 0.0 0.0 Lost Time Adjust (s) 0.0 0.0 0.60 Lost Time Adjust (s) 0.0 0.0 0.3 Lost Time Adjust (s) 0.0 0.0 Los	ane Group Flow (vph)	133	590	0	235	
Permitted Phases 6 23.2 26.1 26.1 26.1 26.1	Turn Type	NA	NA	Perm	NA	
Animimum Split (s) 23.2 26.1 26.1 26.1 26.1 26.1 26.1 26.1 26	Protected Phases	4	2		6	
Total Split (s) 25.0 50.0 50.0 50.0 50.0 Total Split (%) 33.3% 66.7% 66.	Permitted Phases			6		
Total Split (%) 33.3% 66.7% 66.7% 66.7% /ellow Time (s) 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.	Minimum Split (s)	23.2	26.1	26.1	26.1	
All-Red Time (s) 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3	Total Split (s)	25.0	50.0		50.0	
All-Red Time (s) 1.9 1.8 1.8 1.8 1.8 1.8 1.8 1.8	Total Split (%)					
Cost Time Adjust (s) 0.0	Yellow Time (s)					
Total Lost Time (s) 5.2 5.1 5.1 Lead/Lag Lead-Lag Optimize? Loct Effct Green (s) 19.8 44.9 44.9 Loctuated g/C Ratio 0.26 0.60 0.60 Loc Ratio 0.30 0.57 0.23 Control Delay 22.4 11.7 4.8 Queue Delay 0.0 0.0 0.3 Cotal Delay 22.4 11.7 5.1 LOS C B A Approach Delay 22.4 11.7 5.1 Approach LOS C B A Queue Length 50th (m) 12.4 44.8 6.7 Queue Length 95th (m) m24.0 71.3 10.7 Internal Link Dist (m) 115.9 89.4 56.4 Turn Bay Length (m) Base Capacity (vph) 440 1043 1020 Starvation Cap Reductn 0 0 379 Storage Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.30 0.57 0.37 Intersection Summary Cycle Length: 75 Loctuated Cycle Length: 75 Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B	All-Red Time (s)			1.8		
Lead/Lag Lead-Lag Optimize? Lock Effect Green (s) 19.8 44.9 44.9 Lock Late of CRatio 0.26 0.60 0.60 Lock Ratio 0.30 0.57 0.23 Lochtrol Delay 22.4 11.7 4.8 Locueue Delay 0.0 0.0 0.3 Locotal Delay 22.4 11.7 5.1 Lock CRATION CRATICON CRATIC	Lost Time Adjust (s)					
Lead-Lag Optimize? Lead-Lag Optimize?	Total Lost Time (s)	5.2	5.1		5.1	
Act Effet Green (s) 19.8 44.9 44.9 Actuated g/C Ratio 0.26 0.60 0.60 Actuated g/C Ratio 0.30 0.57 0.23 Control Delay 22.4 11.7 4.8 Queue Delay 0.0 0.0 0.3 Total Delay 22.4 11.7 5.1 Approach Delay 22.4 11.7 5.1 Approach LOS C B A Approach LOS C B A Queue Length 50th (m) 12.4 44.8 6.7 Queue Length 95th (m) m24.0 71.3 10.7 Anternal Link Dist (m) 115.9 89.4 56.4 Turn Bay Length (m) Asse Capacity (vph) 440 1043 1020 Starvation Cap Reductn 0 0 379 Storage Cap Reductn 0 0 0 Actuated v/c Ratio 0.30 0.57 0.37 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 37 (49%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B	Lead/Lag					
Actuated g/C Ratio 0.26 0.60 0.60 I/c Ratio 0.30 0.57 0.23 Control Delay 22.4 11.7 4.8 Queue Delay 0.0 0.0 0.3 Total Delay 22.4 11.7 5.1 OS C B A Approach Delay 22.4 11.7 5.1 Approach LOS C B A Queue Length 50th (m) 12.4 44.8 6.7 Queue Length 95th (m) m24.0 71.3 10.7 Internal Link Dist (m) 115.9 89.4 56.4 Turn Bay Length (m) Base Capacity (vph) 440 1043 1020 Starvation Cap Reductn 0 0 379 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.30 0.57 0.37 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B	Lead-Lag Optimize?					
### Control Delay	Act Effct Green (s)					
Control Delay	Actuated g/C Ratio					
Queue Delay 0.0 0.0 0.3 Total Delay 22.4 11.7 5.1 OS C B A Approach Delay 22.4 11.7 5.1 Approach LOS C B A Queue Length 50th (m) 12.4 44.8 6.7 Queue Length 95th (m) m24.0 71.3 10.7 Internal Link Dist (m) 115.9 89.4 56.4 Turn Bay Length (m) Base Capacity (vph) 440 1043 1020 Starvation Cap Reductn 0 0 379 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.30 0.57 0.37 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B	v/c Ratio	0.30	0.57		0.23	
Total Delay	Control Delay	22.4	11.7		4.8	
Approach Delay 22.4 11.7 5.1 Approach LOS C B A Approach LOS C B A Queue Length 50th (m) 12.4 44.8 6.7 Queue Length 95th (m) m24.0 71.3 10.7 Internal Link Dist (m) 115.9 89.4 56.4 Turn Bay Length (m) Base Capacity (vph) 440 1043 1020 Starvation Cap Reductn 0 0 379 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.30 0.57 0.37 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Diffset: 37 (49%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 55 Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B	Queue Delay	0.0	0.0		0.3	
Approach Delay 22.4 11.7 5.1 Approach LOS C B A Queue Length 50th (m) 12.4 44.8 6.7 Queue Length 95th (m) m24.0 71.3 10.7 Internal Link Dist (m) 115.9 89.4 56.4 Furn Bay Length (m) Base Capacity (vph) 440 1043 1020 Starvation Cap Reductn 0 0 379 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.30 0.57 0.37 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 37 (49%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 55 Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B	Total Delay	22.4	11.7		5.1	
Approach LOS C B A Queue Length 50th (m) 12.4 44.8 6.7 Queue Length 95th (m) m24.0 71.3 10.7 Internal Link Dist (m) 115.9 89.4 56.4 Turn Bay Length (m) Base Capacity (vph) 440 1043 1020 Starvation Cap Reductn 0 0 379 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.30 0.57 0.37 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 37 (49%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 55 Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B	LOS					
Queue Length 50th (m) 12.4 44.8 6.7 Queue Length 95th (m) m24.0 71.3 10.7 Internal Link Dist (m) 115.9 89.4 56.4 Furn Bay Length (m) Base Capacity (vph) 440 1043 1020 Starvation Cap Reductn 0 0 379 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.30 0.57 0.37 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 37 (49%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 55 Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B	Approach Delay	22.4	11.7		5.1	
Queue Length 95th (m) m24.0 71.3 10.7 Internal Link Dist (m) 115.9 89.4 56.4 Furn Bay Length (m) Base Capacity (vph) 440 1043 1020 Starvation Cap Reductn 0 0 379 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.30 0.57 0.37 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 37 (49%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 55 Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B	Approach LOS					
Internal Link Dist (m) Internal Link Dist (m) If any Length (m) Base Capacity (vph) Base	Queue Length 50th (m)	12.4	44.8		6.7	
Furn Bay Length (m) Base Capacity (vph) 440 1043 1020 Base Capacity (vph) 440 1043 1020 Barvation Cap Reductn 0 0 0 379 Spillback Cap Reductn 0 0 0 Broad Cap Reductn 0 0 0 0 0 Broad Cap Reductn 0 0 0 0 Broad Cap Reductn 0 0 0 0 0 Broad Cap Reductn 0 0 0 0 Broad Cap Reductn 0 0 0 0 0 Broad Cap Reductn 0 0 0 0 0 Broad Cap Reductn 0 0 0 0 0 0 Broad Cap Reductn 0 0 0 0 0 0 Broad Cap Reductn 0 0 0 0 0 0 Broad Cap Reductn 0 0 0 0 0 0 0 Broa	Queue Length 95th (m)	m24.0				
Base Capacity (vph) 440 1043 1020 Starvation Cap Reductn 0 0 0 379 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.30 0.57 0.37 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 37 (49%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 55 Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B	Internal Link Dist (m)	115.9	89.4		56.4	
Starvation Cap Reductn 0 0 0 379 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.30 0.57 0.37 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 37 (49%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 55 Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B	Turn Bay Length (m)					
Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.30 0.57 0.37 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 37 (49%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 55 Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B	Base Capacity (vph)	440	1043			
Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.30 0.57 0.37 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 37 (49%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 55 Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B	Starvation Cap Reductn	0	0		379	
Reduced v/c Ratio 0.30 0.57 0.37 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 37 (49%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 55 Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B	Spillback Cap Reductn	0	0		0	
Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 37 (49%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 55 Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B	Storage Cap Reductn	0	0		0	
Cycle Length: 75 Actuated Cycle Length: 75 Offset: 37 (49%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 55 Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B	Reduced v/c Ratio	0.30	0.57		0.37	
Cycle Length: 75 Actuated Cycle Length: 75 Offset: 37 (49%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 55 Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B	Intersection Summary					
Actuated Cycle Length: 75 Offset: 37 (49%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 55 Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B	Cycle Length: 75					
Offset: 37 (49%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 55 Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B	, .					
Natural Cycle: 55 Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B		ed to phase	e 2:NBT a	and 6:SBT	L. Start	of Green
Control Type: Pretimed Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B		- 15 pilaot				
Maximum v/c Ratio: 0.57 Intersection Signal Delay: 11.5 Intersection LOS: B						
ntersection Signal Delay: 11.5 Intersection LOS: B						
		1.5			lr	ntersection LOS: B
ntersection Capacity Utilization 57.2% ICU Level of Service B			'n			CU Level of Service B

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.



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Lane Group	EBT	SBL
Lane Configurations	ર્ન	ሻ
Traffic Volume (vph)	202	37
Future Volume (vph)	202	37
Lane Group Flow (vph)	240	37
Sign Control	Free	Stop
Intersection Summary		
Control Type: Unsignalized		
Intersection Capacity Utilizati	ion 23.4%	

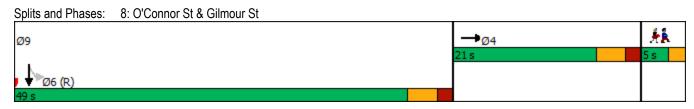
Analysis Period (min) 15

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Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations		ર્ન			ሻ			
Traffic Volume (veh/h)	38	202	0	0	37	0		
Future Volume (Veh/h)	38	202	0	0	37	0		
Sign Control		Free	Free		Stop			
Grade		0%	0%		0%			
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Hourly flow rate (vph)	38	202	0	0	37	0		
Pedestrians								
Lane Width (m)								
Walking Speed (m/s)								
Percent Blockage								
Right turn flare (veh)								
Median type		None	None					
Median storage veh)								
Upstream signal (m)		67	140					
pX, platoon unblocked					0.97			
vC, conflicting volume	0				278	0		
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	0				245	0		
tC, single (s)	4.1				6.4	6.2		
tC, 2 stage (s)								
tF (s)	2.2				3.5	3.3		
p0 queue free %	98				95	100		
cM capacity (veh/h)	1623				707	1085		
Direction, Lane #	EB 1	SB 1						
Volume Total	240	37						
Volume Left	38	37						
Volume Right	0	0						
cSH	1623	707						
Volume to Capacity	0.02	0.05						
Queue Length 95th (m)	0.5	1.3						
Control Delay (s)	1.3	10.4						
Lane LOS	A	В						
Approach Delay (s)	1.3	10.4						
Approach LOS		В						
Intersection Summary								
Average Delay			2.5					
Intersection Capacity Utiliza	ation		23.4%	IC	U Level	of Service	Α	
Analysis Period (min)			15					

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Lane Group	EBT	SBT	Ø9	
Lane Configurations	1 >	41₽		
Traffic Volume (vph)	61	641		
Future Volume (vph)	61	641		
Lane Group Flow (vph)	122	690		
Turn Type	NA	NA		
Protected Phases	4	6	9	
Permitted Phases				
Minimum Split (s)	20.1	26.1	5.0	
Total Split (s)	21.0	49.0	5.0	
Total Split (%)	28.0%	65.3%	7%	
Yellow Time (s)	3.3	3.3	2.0	
All-Red Time (s)	1.8	1.8	0.0	
Lost Time Adjust (s)	0.0	0.0	0.0	
Total Lost Time (s)	5.1	5.1		
Lead/Lag	0.1	5.1		
Lead-Lag Optimize?				
	15.9	43.9		
Act Effct Green (s)	0.21	0.59		
Actuated g/C Ratio v/c Ratio	0.21			
		0.35		
Control Delay	11.6	7.9		
Queue Delay	0.0	0.0		
Total Delay	11.6	7.9		
LOS	B	A		
Approach Delay	11.6	7.9		
Approach LOS	В	A		
Queue Length 50th (m)	4.6	21.6		
Queue Length 95th (m)	m13.0	31.0		
Internal Link Dist (m)	159.1	135.3		
Turn Bay Length (m)		1.5		
Base Capacity (vph)	392	1977		
Starvation Cap Reductn	0	0		
Spillback Cap Reductn	0	0		
Storage Cap Reductn	0	0		
Reduced v/c Ratio	0.31	0.35		
Intersection Summary				
Cycle Length: 75				
Actuated Cycle Length: 75				
Offset: 46 (61%), Reference	ed to phase	e 6:SBTL.	Start of Gree	າ
Natural Cycle: 55				
Control Type: Pretimed				
Maximum v/c Ratio: 0.35				
Intersection Signal Delay: 8	3.5			Intersection LOS: A
Intersection Capacity Utilization				ICU Level of Service A
Analysis Daried (min) 15	ulion 14 .3/0	J		IOO FEARI OI ORI AICE W

m Volume for 95th percentile queue is metered by upstream signal.

Analysis Period (min) 15



Lane Group Lane Configurations Traffic Volume (vph) Future Volume (vph) Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Minimum Split (s) Total Split (s) Total Split (%) Yellow Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m) Queue Length 95th (m)	EBL 56 56 56 Perm 4 23.5 41.0 54.7% 3.3 2.2 0.0 5.5	23.5 41.0 54.7% 3.3 2.2 0.0 5.5	WBT 312 312 375 NA 8 23.5 41.0 54.7% 3.3 2.2 0.0 5.5	NBL 77 77 77 77 Perm 2 23.4 34.0 45.3% 3.3 2.1	NBT 727 727 833 NA 2 23.4 34.0 45.3% 3.3 2.1	
Lane Configurations Traffic Volume (vph) Future Volume (vph) Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Minimum Split (s) Total Split (s) Total Split (%) Yellow Time (s) All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)	56 56 56 Perm 4 23.5 41.0 54.7% 3.3 2.2 0.0 5.5	342 342 342 NA 4 23.5 41.0 54.7% 3.3 2.2 0.0	312 312 375 NA 8 23.5 41.0 54.7% 3.3 2.2 0.0	77 77 77 77 Perm 2 23.4 34.0 45.3% 3.3 2.1	727 727 833 NA 2 23.4 34.0 45.3% 3.3	
Traffic Volume (vph) Future Volume (vph) Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Minimum Split (s) Total Split (s) Total Split (%) Yellow Time (s) All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)	56 56 56 Perm 4 23.5 41.0 54.7% 3.3 2.2 0.0 5.5	342 342 NA 4 23.5 41.0 54.7% 3.3 2.2 0.0	312 312 375 NA 8 23.5 41.0 54.7% 3.3 2.2 0.0	77 77 77 Perm 2 23.4 34.0 45.3% 3.3 2.1	727 727 833 NA 2 23.4 34.0 45.3% 3.3	
Future Volume (vph) Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Minimum Split (s) Total Split (s) Total Split (%) Yellow Time (s) All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)	56 56 Perm 4 23.5 41.0 54.7% 3.3 2.2 0.0 5.5	342 342 NA 4 23.5 41.0 54.7% 3.3 2.2 0.0	312 375 NA 8 23.5 41.0 54.7% 3.3 2.2 0.0	77 77 Perm 2 23.4 34.0 45.3% 3.3 2.1	727 833 NA 2 23.4 34.0 45.3% 3.3	
Lane Group Flow (vph) Turn Type Protected Phases Permitted Phases Minimum Split (s) Total Split (s) Total Split (%) Yellow Time (s) All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)	56 Perm 4 23.5 41.0 54.7% 3.3 2.2 0.0 5.5	342 NA 4 23.5 41.0 54.7% 3.3 2.2 0.0	375 NA 8 23.5 41.0 54.7% 3.3 2.2 0.0	77 Perm 2 23.4 34.0 45.3% 3.3 2.1	833 NA 2 23.4 34.0 45.3% 3.3	
Turn Type Protected Phases Permitted Phases Minimum Split (s) Total Split (s) Total Split (%) Yellow Time (s) All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)	Perm 4 23.5 41.0 54.7% 3.3 2.2 0.0 5.5	NA 4 23.5 41.0 54.7% 3.3 2.2 0.0	NA 8 23.5 41.0 54.7% 3.3 2.2 0.0	Perm 2 23.4 34.0 45.3% 3.3 2.1	NA 2 23.4 34.0 45.3% 3.3	
Protected Phases Permitted Phases Minimum Split (s) Total Split (s) Total Split (%) Yellow Time (s) All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)	4 23.5 41.0 54.7% 3.3 2.2 0.0 5.5	23.5 41.0 54.7% 3.3 2.2 0.0	8 23.5 41.0 54.7% 3.3 2.2 0.0	2 23.4 34.0 45.3% 3.3 2.1	2 23.4 34.0 45.3% 3.3	
Permitted Phases Minimum Split (s) Total Split (s) Total Split (%) Yellow Time (s) All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)	23.5 41.0 54.7% 3.3 2.2 0.0 5.5	23.5 41.0 54.7% 3.3 2.2 0.0	23.5 41.0 54.7% 3.3 2.2 0.0	23.4 34.0 45.3% 3.3 2.1	23.4 34.0 45.3% 3.3	
Minimum Split (s) Total Split (s) Total Split (%) Yellow Time (s) All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)	23.5 41.0 54.7% 3.3 2.2 0.0 5.5	41.0 54.7% 3.3 2.2 0.0	41.0 54.7% 3.3 2.2 0.0	23.4 34.0 45.3% 3.3 2.1	34.0 45.3% 3.3	
Total Split (s) Total Split (s) Yellow Time (s) All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)	41.0 54.7% 3.3 2.2 0.0 5.5 35.5 0.47	41.0 54.7% 3.3 2.2 0.0	41.0 54.7% 3.3 2.2 0.0	34.0 45.3% 3.3 2.1	34.0 45.3% 3.3	
Total Split (%) Yellow Time (s) All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Queue Length 50th (m)	54.7% 3.3 2.2 0.0 5.5 35.5 0.47	54.7% 3.3 2.2 0.0	54.7% 3.3 2.2 0.0	45.3% 3.3 2.1	45.3% 3.3	
Yellow Time (s) All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)	3.3 2.2 0.0 5.5 35.5 0.47	3.3 2.2 0.0	3.3 2.2 0.0	3.3 2.1	3.3	
All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)	2.2 0.0 5.5 35.5 0.47	2.2 0.0	2.2 0.0	2.1		
Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)	0.0 5.5 35.5 0.47	0.0	0.0			
Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)	5.5 35.5 0.47			0.0	0.0	
Lead/Lag Lead-Lag Optimize? Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)	35.5 0.47	0.0	א ל	5.4	5.4	
Lead-Lag Optimize? Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)	0.47		0.0	J. T	U.T	
Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)	0.47					
Actuated g/C Ratio v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)	0.47	35.5	35.5	28.6	28.6	
v/c Ratio Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)		0.47	0.47	0.38	0.38	
Control Delay Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)		0.41	0.47	0.36	0.36	
Queue Delay Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)	12.8	14.7	15.9	22.4	21.1	
Total Delay LOS Approach Delay Approach LOS Queue Length 50th (m)	0.0	0.0	0.0	0.0	0.0	
LOS Approach Delay Approach LOS Queue Length 50th (m)	12.8	14.7	15.9	22.4	21.1	
Approach Delay Approach LOS Queue Length 50th (m)	12.0 B	14.7 B	13.9 B	22.4 C	Z 1.1	
Approach LOS Queue Length 50th (m)	D	14.4	15.9	U	21.2	
Queue Length 50th (m)		14.4 B	13.9 B		Z1.Z	
	4.3	29.9	34.2	6.8	26.5	
	10.9	48.9	55.7	17.2	54.5	
Internal Link Dist (m)	10.9	97.2	154.7	17.2	68.5	
Turn Bay Length (m)	20.0	91.2	154.7	20.0	00.5	
	348	844	792	326	1755	
Base Capacity (vph)						
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0		0	
Storage Cap Reductn	0 16	0 44	0 47	0 24	0 47	
Reduced v/c Ratio	0.16	0.41	0.47	0.24	0.47	
ntersection Summary						
Cycle Length: 75						
Actuated Cycle Length: 75						
Offset: 50 (67%), Reference	ed to phase	2:NBTL	, Start of	Green		
Natural Cycle: 50						
Control Type: Pretimed						
Maximum v/c Ratio: 0.47						
Intersection Signal Delay: 1					ntersection	
Intersection Capacity Utiliza	ation 73.5%)		IC	CU Level	of Service D
Analysis Period (min) 15						
Splits and Phases: 1: Ke	nt St & Sor	nerset St	W			
- 4	5. 4 501		**		A	
Ø2 (R)					1 Ø4	
34 s				41	S	

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Lane Group	WBT	NBT	
Lane Configurations	^	414	
Traffic Volume (vph)	22	895	
Future Volume (vph)	22	895	
Lane Group Flow (vph)	92	926	
Sign Control	Stop	Free	
Intersection Summary			
Control Type: Unsignalize	ed		
Intersection Capacity Util	ization 37.6%		ICU Level of Service A

Analysis Period (min) 15

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					ĵ.			414				
Traffic Volume (veh/h)	0	0	0	0	22	70	31	895	0	0	0	0
Future Volume (Veh/h)	0	0	0	0	22	70	31	895	0	0	0	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	0	0	0	0	22	70	31	895	0	0	0	0
Pedestrians		45			95			11			17	
Lane Width (m)		0.0			3.7			3.7			0.0	
Walking Speed (m/s)		1.1			1.1			1.1			1.1	
Percent Blockage		0			9			1			0	
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								78			92	
pX, platoon unblocked	0.93	0.93		0.93	0.93	0.93				0.93		
vC, conflicting volume	503	1097	56	1063	1097	410	45			990		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	187	827	56	791	827	86	45			712		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	100	91	91	98			100		
cM capacity (veh/h)	550	253	988	215	253	807	1561			746		
Direction, Lane #	WB 1	NB 1	NB 2	NB 3								
Volume Total	92	210	358	358								
Volume Left	0	31	0	0								
Volume Right	70	0	0	0								
cSH	529	1561	1700	1700								
Volume to Capacity	0.17	0.02	0.21	0.21								
Queue Length 95th (m)	4.7	0.5	0.0	0.0								
Control Delay (s)	13.2	1.2	0.0	0.0								
Lane LOS	В	Α	4.4									
Approach Delay (s)	13.2	0.3										
Approach LOS	В	U.U										
Intersection Summary												
Average Delay			1.4									
Intersection Capacity Utiliza	ation		37.6%	IC	U Level	of Service			Α			
Analysis Period (min)			15			2000						
			10									

	-	†	
Lane Group	EBT	NBT	
Lane Configurations	ર્ન	ተተ _ጉ	
Traffic Volume (vph)	79	871	
Future Volume (vph)	79	871	
Lane Group Flow (vph)	98	972	
Turn Type	NA	NA	
Protected Phases	4	2	
Permitted Phases		_	
Minimum Split (s)	21.5	21.1	
Total Split (s)	22.0	53.0	
Total Split (%)	29.3%	70.7%	
Yellow Time (s)	3.3	3.3	
All-Red Time (s)	2.2	1.8	
ost Time Adjust (s)	0.0	0.0	
Fotal Lost Time (s)	5.5	5.1	
Lead/Lag	3.0	J. I	
Lead/Lag Optimize?			
Act Effct Green (s)	16.5	47.9	
	0.22	0.64	
Actuated g/C Ratio	0.22	0.64	
//c Ratio	19.2		
Control Delay		8.9	
Queue Delay	0.0	0.0	
Total Delay	19.2	8.9	
LOS	B	A	
Approach Delay	19.2	8.9	
Approach LOS	В	Α	
Queue Length 50th (m)	7.7	20.7	
Queue Length 95th (m)	19.4	34.1	
Internal Link Dist (m)	157.9	224.2	
Turn Bay Length (m)			
Base Capacity (vph)	405	2992	
Starvation Cap Reductn	0	0	
Spillback Cap Reductn	0	0	
Storage Cap Reductn	0	0	
Reduced v/c Ratio	0.24	0.32	
ntersection Summary			
Cycle Length: 75			
Actuated Cycle Length: 75			
Offset: 0 (0%), Referenced	to phase 2:	NRT Cto	t of Green
, ,,	to priase 2	indi, ola	t of Gleen
Natural Cycle: 45			
Control Type: Pretimed Maximum v/c Ratio: 0.32			
	0		Intercontion I OC: A
ntersection Signal Delay: 9			Intersection LOS: A
ntersection Capacity Utiliza	1110N 43.U%) 	ICU Level of Service A
Analysis Period (min) 15			
Splits and Phases: 3: Ker	nt St & Giln	nour St	
↑ ø2 (R)			→ 04

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Lane Group	EBL	EBT	WBT	NBL	NBT	
Lane Configurations	ች	↑	f.	ሻ	ተተኈ	
Traffic Volume (vph)	75	464	334	52	757	
Future Volume (vph)	75	464	334	52	757	
Lane Group Flow (vph)	75	464	409	52	863	
Turn Type	Perm	NA	NA	Perm	NA	
Protected Phases		4	8	. 0	2	
Permitted Phases	4	•		2	_	
Minimum Split (s)	23.4	23.4	23.4	23.4	23.4	
Total Split (s)	40.0	40.0	40.0	35.0	35.0	
Fotal Split (%)	53.3%	53.3%	53.3%	46.7%	46.7%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.1	2.1	2.1	2.1	2.1	
_ost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.4	5.4	5.4	5.4	
Lead/Lag	3.4	J. 4	J. 4	J. 4	J. 4	
_ead-Lag Optimize?						
0 .	34.6	34.6	34.6	29.6	29.6	
Act Effct Green (s)	0.46	0.46	0.46	0.39	0.39	
Actuated g/C Ratio	0.46		0.46	0.39	0.39	
/c Ratio		0.56				
Control Delay	14.4	18.0	16.3	14.9	16.9	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
otal Delay	14.4	18.0	16.3	14.9	16.9	
_OS	В	B	В	В	В	
Approach Delay		17.5	16.3		16.8	
Approach LOS	0.1	B	В	, -	В	
Queue Length 50th (m)	6.1	45.6	36.8	4.5	30.5	
Queue Length 95th (m)	14.5	72.1	60.3	11.0	40.8	
nternal Link Dist (m)		152.2	162.6		69.7	
urn Bay Length (m)	25.0					
Base Capacity (vph)	339	823	801	588	1879	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.22	0.56	0.51	0.09	0.46	
ntersection Summary						
Cycle Length: 75						
Actuated Cycle Length: 75						
Offset: 23 (31%), Reference	ed to phase	2:NBTL	, Start of	Green		
Natural Cycle: 50						
Control Type: Pretimed						
Maximum v/c Ratio: 0.56						
ntersection Signal Delay: 1	6.9			lr	ntersectio	n LOS: B
ntersection Capacity Utiliza)				of Service C
Analysis Period (min) 15						
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Splits and Phases: 4: Ke	nt St & Gla	dstone A	ve	-		
√¶ø2 (R)				l		
35 s				4	40 s	
					4	

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Lane Group	WBT	NBL	NBT	SBT	
Lane Configurations	4		ર્ન	₽	
Traffic Volume (vph)	11	25	275	643	
Future Volume (vph)	11	25	275	643	
Lane Group Flow (vph)	56	0	300	669	
Turn Type	NA	Perm	NA	NA	
Protected Phases	8		2	6	
Permitted Phases		2			
Minimum Split (s)	24.2	23.0	23.0	23.0	
Total Split (s)	24.0	51.0	51.0	51.0	
Total Split (%)	32.0%	68.0%	68.0%	68.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.9	1.7	1.7	1.7	
_ost Time Adjust (s)	0.0	• • • • • • • • • • • • • • • • • • • •	0.0	0.0	
Total Lost Time (s)	5.2		5.0	5.0	
_ead/Lag			0.0	0.0	
Lead-Lag Optimize?					
Act Effct Green (s)	18.8		46.0	46.0	
Actuated g/C Ratio	0.25		0.61	0.61	
v/c Ratio	0.15		0.30	0.63	
Control Delay	20.1		2.3	12.5	
Queue Delay	0.0		0.3	0.0	
Total Delay	20.1		2.6	12.5	
_OS	Z0.1		2.0 A	12.3 B	
Approach Delay	20.1		2.6	12.5	
Approach LOS	Z0.1		Α.	12.3 B	
Queue Length 50th (m)	5.0		3.1	52.6	
Queue Length 95th (m)	13.5		4.5	84.4	
nternal Link Dist (m)	126.3		56.4	50.2	
Turn Bay Length (m)	120.5		50.4	JU.Z	
Base Capacity (vph)	366		1007	1059	
Starvation Cap Reductn	0		259	0	
Spillback Cap Reductn	0		239	0	
Storage Cap Reductin	0		0	0	
Reduced v/c Ratio	0.15		0.40	0.63	
ntersection Summary					
Cycle Length: 75					
Actuated Cycle Length: 75		NDT	1000=	01 1 5	2
Offset: 7 (9%), Referenced to	to phase 2	:NB1L ar	nd 6:SBT,	Start of	Green
Natural Cycle: 60					
Control Type: Pretimed					
Maximum v/c Ratio: 0.63					
Intersection Signal Delay: 10					ntersection LOS: A
ntersection Capacity Utiliza	tion 62.1%)		10	CU Level of Service B
Analysis Period (min) 15					
Splits and Phases: 5: Bar	ok C+ 0 Ma	ol aran C	+		
*	nk St & Ma	cLaten 5	· L		
ÿ2 (R)					
51 s					
					I 4

Cycle Length: 75 Actuated Cycle Length: 75 Offset: 6 (8%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 50 Control Type: Pretimed Maximum v/c Ratio: 0.43 Intersection Signal Delay: 8.9 Intersection Capacity Utilization 60.4% ICU Level of Service B		→	†	/	↓		
Traffic Volume (vph) 102 422 19 357 "untru Volume (vph) 102 422 19 357 "ane Group Flow (vph) 161 450 0 376 "urn Type NA NA Perm NA "rotected Phases 4 2 2 6 Permitted Phases 7 Permitted Phases 8 Permitted Phases 8 Permitted Phases 9 Permi	Lane Group	EBT	NBT	SBL	SBT		
rireffic Volume (vph) 102 422 19 357 izune Volume (vph) 102 422 19 357 izune Group Flow (vph) 161 450 0 376 izun Type NA NA Perm NA rorected Phases 4 2 6 Permitted Phases 6 filinimum Split (s) 23.2 26.1 26.1 26.1 izotal Split (s) 25.0 50.0 50.0 fotal Split (s) 25.0 50.0 50.0 fotal Split (s) 25.0 50.0 50.0 fotal Split (s) 33.3 66.7% 66.7% 66.7% fellow Time (s) 3.3 3.3 3.3 3.3 Ill-Red Time (s) 1.9 1.8 1.8 1.8 Ill-Red Time (s) 5.2 5.1 5.1 ead/Lag ead-Lag Optimize? ket Effet Green (s) 19.8 44.9 44.9 ket effet Green (s) 19.8 44.9 44.9 ket effet Green (s) 19.8 44.9 44.9 control Delay 16.3 9.6 4.2 couleue Delay 0.0 0.0 0.5 cotal Delay 16.3 9.6 4.2 couleue Length 95th (m) 9.4 30.2 8.6 couleue Length 95th (m) 19.4 48.9 12.6 formal Link 1st (m) 115.9 89.4 56.4 izum Bay Length (m) abase Capacity (vph) 450 1038 1030 calcurate Cycle Length: 75 otherse Capacity Utilization 60.4% intersection Capacity Utilizati							
Future Volume (Vph) 102 422 19 357 ane Group Flow (vph) 161 450 0 376 urum Type NA NA Perm NA Permitted Phases 6 Protected Phases 4 2 6 Permitted Phases 6 Initiation Split (s) 23.2 26.1 26.1 26.1 Total Split (s) 25.0 50.0 50.0 50.0 Total Split (%) 33.3% 66.7% 66.7% 66.7% (fellow Time (s) 3.3 3.3 3.3 3.3 3.3 3.3 INI-Red Time (s) 1.9 1.8 1.8 1.8 1.8 Ost Time Adjust (s) 0.0 0.0 0.0 Total Lost Time (s) 5.2 5.1 5.1 ead/Lag ead-Lag Optimize? Votal Effict Green (s) 19.8 44.9 44.9 Votatated g/C Ratio 0.26 0.60 0.60 Voration 0.36 0.43 0.37 Zontrol Delay 16.3 9.6 4.2 Zueue Delay 0.0 0.0 0.5 Total Colledy 16.3 9.6 4.8 Approach Delay 16.3 9.6 4.8 Approach LoS B A A A A A A A Approach LoS B A A A A A A A A A A A A A A A A A A A				10			
ane Group Flow (vph) 161 450 0 376							
Turn Type							
Protected Phases Permitted Phase Permitted Phase Permitte							
### Permitted Phases 6 ### Africal Split (s) 23.2 26.1 26.1 26.1 ### Cotal Split (s) 23.2 26.1 26.1 26.1 ### Cotal Split (s) 23.3 3.3 3.3 3.3 ### Stellow Time (s) 3.3 3.3 3.3 3.3 ### Stellow Time (s) 3.3 3.3 3.3 3.3 ### Stellow Time (s) 3.4 3.3 3.3 3.3 ### Stellow Time (s) 3.5 3.5 ### Stellow Time (s) 5.2 5.1 5.1 ### Stellow Time (s) 5.2 5.1 ### Stellow Time (sold Time (s				I GIIII			
### Additional Split (s)		4		6	U		
Total Split (s)		22.2	26.1		26.1		
Total Split (%) 33.3% 66.7% 66.7% 66.7% fellow Time (s) 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.	,						
Vellow Time (s) 3.3 3.3 3.3 3.3 3.3 3.3 3.3 Vellow Time (s) 1.9 1.8 1.9 1.8 1.9 1.8 1.8 1.8 1.8 1.8 1.8 1.8 1.8 1.8 1.8 1							
Name							
tost Time Adjust (s) 0.0 0.0 0.0 0.0 color of tal Lost Time (s) 5.2 5.1 5.1 5.1 cead/Lag cead-Lag Optimize? Ixet Effet Green (s) 19.8 44.9 44.9 cktuated g/C Ratio 0.26 0.60 0.60 0.60 /c Ratio 0.36 0.43 0.37 control Delay 16.3 9.6 4.2 countrol Delay 16.3 9.6 4.2 countrol Delay 16.3 9.6 4.8 countrol Delay 16.3 9.0 countrol Type Pretimed 16.3 9.0 countrol Type Pretimed 16.3 9.0 countrol Type Pretimed 16.3 countrol Type							
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Lead/Lag Optimize? Act Effct Green (s) 19.8 44.9 44.9 Actuated g/C Ratio 0.26 0.60 0.60 Actuated g/C Ratio 0.36 0.43 0.37 Control Delay 16.3 9.6 4.2 Queue Delay 0.0 0.0 0.5 Approach Delay 16.3 9.6 4.8 Approach Delay 16.3 9.6 4.8 Approach LOS B A A Approach Soft (m) 9.4 30.2 8.6 Queue Length 95th (m) 19.4 48.9 12.6 Internal Link Dist (m) 115.9 89.4 56.4 Furn Bay Length (m) Base Capacity (vph) 450 1038 1030 Starvation Cap Reductn 0 0 313 Spillback Cap Reductn 0 0 0 Reduced V/c Ratio 0.36 0.43 0.52 Intersection Summary Dycle Length: 75 Control Type: Pretimed Alaximum V/c Ratio: 0.43 Intersection Capacity Utilization 60.4%							
Lead-Lag Optimize? Act Effc Green (s) 19.8 44.9 44.9 Act Lated g/C Ratio 0.26 0.60 0.60 Act Lated g/C Ratio 0.36 0.43 0.37 Control Delay 16.3 9.6 4.2 Queue Delay 0.0 0.0 0.5 Total Delay 16.3 9.6 4.8 Approach Delay 16.3 9.6 4.8 Approach Delay 16.3 9.6 4.8 Approach LOS B A A A Approach LOS B A A A Queue Length 50th (m) 9.4 30.2 8.6 Queue Length 95th (m) 19.4 48.9 12.6 Internal Link Dist (m) 115.9 89.4 56.4 Turn Bay Length (m) Base Capacity (vph) 450 1038 1030 Starvation Cap Reductn 0 0 313 Spillback Cap Reductn 0 0 0 313 Spillback Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.36 0.43 0.52 Intersection Summary Device Length: 75 Actuated Cycle Length: 75 Actuated Cycle Length: 75 Actuated Cycle So Control Type: Pretimed Analysis Period (min) 15 Spilts and Phases: 6: Bank St & Gilmour St ■ A4.9 A5.0 A5.0 A6.0 A6.0 A7.0 A7	` /	5.2	5.1		5.1		
Act Effct Green (s) 19.8							
Actuated g/C Ratio		40.0	44.0		440		
Control Delay							
Control Delay 16.3 9.6 4.2 Queue Delay 0.0 0.0 0.5 Total Delay 16.3 9.6 4.8 QUeue Delay 16.3 9.6 4.8 QUeue Delay 16.3 9.6 4.8 QUeue Delay 16.3 9.6 4.8 Approach Delay 16.3 9.6 4.8 Approach LOS B A A A QUeue Length 50th (m) 9.4 30.2 8.6 QUeue Length 95th (m) 19.4 48.9 12.6 Internal Link Dist (m) 115.9 89.4 56.4 Turn Bay Length (m) Starvation Cap Reductn 0 0 313 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.36 0.43 0.52 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Control Type: Pretimed Jaximum v/c Ratio: 0.43 Intersection Signal Delay: 8.9 Intersection Capacity Utilization 60.4% Analysis Period (min) 15 Splits and Phases: 6: Bank St & Gilmour St							
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Approach Delay 16.3 9.6 4.8 Approach LOS B A A A A Approach LOS B A A A Dueue Length 50th (m) 9.4 30.2 8.6 Dueue Length 95th (m) 19.4 48.9 12.6 Internal Link Dist (m) 115.9 89.4 56.4 Turn Bay Length (m) Base Capacity (vph) 450 1038 1030 Starvation Cap Reductn 0 0 313 Spillback Cap Reductn 0 0 0 Reduced v/c Ratio 0.36 0.43 0.52 Intersection Summary Dycle Length: 75 Actuated Cycle Length: 75 Diffset: 6 (8%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Statural Cycle: 50 Control Type: Pretimed Maximum v/c Ratio: 0.43 Intersection Signal Delay: 8.9 Intersection LOS: A Intersection Capacity Utilization 60.4% ICU Level of Service B Analysis Period (min) 15 Splits and Phases: 6: Bank St & Gilmour St							
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Queue Length 50th (m) 9.4 30.2 8.6 Queue Length 95th (m) 19.4 48.9 12.6 Internal Link Dist (m) 115.9 89.4 56.4 Furn Bay Length (m) Base Capacity (vph) 450 1038 1030 Starvation Cap Reductn 0 0 313 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.36 0.43 0.52 Intersection Summary Cycle Length: 75 Offset: 6 (8%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 50 Control Type: Pretimed Maximum v/c Ratio: 0.43 Intersection Signal Delay: 8.9 Intersection Capacity Utilization 60.4% Analysis Period (min) 15 Splits and Phases: 6: Bank St & Gilmour St							
Queue Length 95th (m) 19.4 48.9 12.6 Internal Link Dist (m) 115.9 89.4 56.4 Furn Bay Length (m) Base Capacity (vph) 450 1038 1030 Starvation Cap Reductn 0 0 313 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.36 0.43 0.52 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 6 (8%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 50 Control Type: Pretimed Maximum v/c Ratio: 0.43 Intersection Signal Delay: 8.9 Intersection Capacity Utilization 60.4% Analysis Period (min) 15 Splits and Phases: 6: Bank St & Gilmour St							
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Furn Bay Length (m) Base Capacity (vph) 450 1038 1030 Starvation Cap Reductn 0 0 0 313 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.36 0.43 0.52 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 6 (8%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 50 Control Type: Pretimed Maximum v/c Ratio: 0.43 Intersection Signal Delay: 8.9 Intersection LOS: A Intersection Capacity Utilization 60.4% Intersection Capacity Utilization 60.4% Splits and Phases: 6: Bank St & Gilmour St							
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Starvation Cap Reductn 0 0 0 313 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0 Reduced v/c Ratio 0.36 0.43 0.52 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 6 (8%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 50 Control Type: Pretimed Maximum v/c Ratio: 0.43 Intersection Signal Delay: 8.9 Intersection LOS: A Intersection Capacity Utilization 60.4% Analysis Period (min) 15 Splits and Phases: 6: Bank St & Gilmour St			4000		1005		
Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.36 0.43 0.52 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 6 (8%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 50 Control Type: Pretimed Maximum v/c Ratio: 0.43 Intersection Signal Delay: 8.9 Intersection LOS: A Intersection Capacity Utilization 60.4% Analysis Period (min) 15 Splits and Phases: 6: Bank St & Gilmour St							
Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.36 0.43 0.52 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 6 (8%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 50 Control Type: Pretimed Maximum v/c Ratio: 0.43 Intersection Signal Delay: 8.9 Intersection LOS: A Intersection Capacity Utilization 60.4% ICU Level of Service B Analysis Period (min) 15 Splits and Phases: 6: Bank St & Gilmour St							
Reduced v/c Ratio 0.36 0.43 0.52 Intersection Summary Cycle Length: 75 Actuated Cycle Length: 75 Offset: 6 (8%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 50 Control Type: Pretimed Maximum v/c Ratio: 0.43 Intersection Signal Delay: 8.9 Intersection Capacity Utilization 60.4% Analysis Period (min) 15 Splits and Phases: 6: Bank St & Gilmour St	•						
Actuated Cycle Length: 75 Actuated Cycle Length: 75 Diffset: 6 (8%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 50 Control Type: Pretimed Maximum v/c Ratio: 0.43 Intersection Signal Delay: 8.9 Intersection Capacity Utilization 60.4% Analysis Period (min) 15 Splits and Phases: 6: Bank St & Gilmour St							
Cycle Length: 75 Actuated Cycle Length: 75 Offset: 6 (8%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 50 Control Type: Pretimed Maximum v/c Ratio: 0.43 Intersection Signal Delay: 8.9 Intersection Capacity Utilization 60.4% Analysis Period (min) 15 Splits and Phases: 6: Bank St & Gilmour St	Reduced v/c Ratio	0.36	0.43		0.52		
Actuated Cycle Length: 75 Offset: 6 (8%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 50 Control Type: Pretimed Maximum v/c Ratio: 0.43 Intersection Signal Delay: 8.9 Intersection Capacity Utilization 60.4% Analysis Period (min) 15 Splits and Phases: 6: Bank St & Gilmour St	Intersection Summary						
Offset: 6 (8%), Referenced to phase 2:NBT and 6:SBTL, Start of Green Natural Cycle: 50 Control Type: Pretimed Maximum v/c Ratio: 0.43 Intersection Signal Delay: 8.9 Intersection Capacity Utilization 60.4% ICU Level of Service B Analysis Period (min) 15 Splits and Phases: 6: Bank St & Gilmour St	Cycle Length: 75						
Natural Cycle: 50 Control Type: Pretimed Maximum v/c Ratio: 0.43 Intersection Signal Delay: 8.9 Intersection Capacity Utilization 60.4% ICU Level of Service B Analysis Period (min) 15 Splits and Phases: 6: Bank St & Gilmour St	Actuated Cycle Length: 75						
Control Type: Pretimed Maximum v/c Ratio: 0.43 Intersection Signal Delay: 8.9 Intersection Capacity Utilization 60.4% ICU Level of Service B Analysis Period (min) 15 Splits and Phases: 6: Bank St & Gilmour St	\ /'	to phase 2	NBT and	6:SBTL,	Start of	Green	
Maximum v/c Ratio: 0.43 Intersection Signal Delay: 8.9 Intersection Capacity Utilization 60.4% ICU Level of Service B Analysis Period (min) 15 Splits and Phases: 6: Bank St & Gilmour St	Natural Cycle: 50						
Maximum v/c Ratio: 0.43 Intersection Signal Delay: 8.9 Intersection Capacity Utilization 60.4% ICU Level of Service B Analysis Period (min) 15 Splits and Phases: 6: Bank St & Gilmour St	Control Type: Pretimed						
ntersection Capacity Utilization 60.4% Analysis Period (min) 15 Splits and Phases: 6: Bank St & Gilmour St	Maximum v/c Ratio: 0.43						
ntersection Capacity Utilization 60.4% ICU Level of Service B Analysis Period (min) 15 Splits and Phases: 6: Bank St & Gilmour St	Intersection Signal Delay: 8	8.9			lı	ntersection LOS: A	
Analysis Period (min) 15 Splits and Phases: 6: Bank St & Gilmour St					I	CU Level of Service B	
1 Ø2 (R) ♣Ø4	Analysis Period (min) 15						
1 Ø2 (R) ♣Ø4	Splits and Phases: 6: Ba	ank St & Gili	mour St				
	•	and of a Oill	11001 01				A

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Lane Group	EBT	SBL	
Lane Configurations	4	ň	
Traffic Volume (vph)	149	37	
Future Volume (vph)	149	37	
Lane Group Flow (vph)	180	37	
Sign Control	Free	Stop	
Intersection Summary			
Control Type: Unsignalize	d		
Intersection Capacity Utiliz	zation 20.1%		ICU Level of Service A

Analysis Period (min) 15

	•	→	+	•	/	4	
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		ર્ન			*		
Traffic Volume (veh/h)	31	149	0	0	37	0	
Future Volume (Veh/h)	31	149	0	0	37	0	
Sign Control		Free	Free		Stop		
Grade		0%	0%		0%		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly flow rate (vph)	31	149	0	0	37	0	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type		None	None				
Median storage veh)							
Upstream signal (m)		67	140				
pX, platoon unblocked					0.98		
vC, conflicting volume	0				211	0	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	0				183	0	
tC, single (s)	4.1				6.4	6.2	
tC, 2 stage (s)							
tF (s)	2.2				3.5	3.3	
p0 queue free %	98				95	100	
cM capacity (veh/h)	1623				774	1085	
Direction, Lane #	EB 1	SB 1					
Volume Total	180	37					
Volume Left	31	37					
Volume Right	0	0					
cSH	1623	774					
Volume to Capacity	0.02	0.05					
Queue Length 95th (m)	0.4	1.1					
Control Delay (s)	1.4	9.9					
Lane LOS	Α	Α					
Approach Delay (s)	1.4	9.9					
Approach LOS		Α					
Intersection Summary							
Average Delay			2.8				
Intersection Capacity Utiliza	ation		20.1%	IC	U Level	of Service	
Analysis Period (min)			15				
marysis i enou (illiii)			IJ				

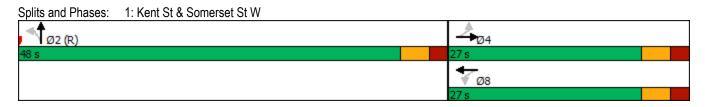
pproach Delay 29.4 11.6 pproach LOS C B Queue Length 50th (m) 15.8 54.0 Queue Length 95th (m) 33.2 73.1 pternal Link Dist (m) 159.1 135.3 queue Length (m) queu		-	↓		
ane Configurations raffic Volume (vph) 84 1226 uture Volume Volume (vph) 84 1226 uture Volume (vph) 84	Lane Group	EBT	SBT	Ø9	
raffic Volume (vph) 84 1226 uture Volume (vph) 84 1226 unr Type NA NA rotected Phases liminum Split (s) 20.1 26.1 5.0 otal Split (s) 21.0 49.0 5.0 otal Split (s) 28.0% 65.3% 7% ellow Time (s) 3.3 3.3 2.0 Ill-Red Time (s) 1.8 1.8 0.0 ost Time Adjust (s) 0.0 0.0 otal Lost Time (s) 5.1 5.1 ead/Lag ead-Lag Optimize? ct Effct Green (s) 15.9 43.9 ctuated g/C Ratio 0.21 0.59 /c Ratio 0.42 0.64 bueue Delay 29.4 11.6 obs C B proproach LOS C B proproach LOS C B proproach LOS C B pueue Length 50th (m) 15.8 54.0 bueue Length 95th (m) 33.2 73.1 terenal Link Dist (m) urn Bay Length (m) lase Capacity (vph) 381 1986 tarvation Cap Reductn 0 0 leduced Vic Ratio 0.42 0.64 theresection Summary cycle Length: 75 fiffset: 71 (95%), Referenced to phase 6:SBTL, Start of Green latural Cycle: 60 control Type: Pretimed laximum v/c Ratio: 0.64 terrsection Sand Phases: 8: O'Connor St & Gilmour St plits and Phases: 8: O'Connor St & Gilmour St				~~	
uture Volume (vph) 159 1274 ane Group Flow (vph) 159 1274 urm Type NA NA rotected Phases flinimum Split (s) 20.1 26.1 5.0 otal Split (s) 21.0 49.0 5.0 otal Split (s) 22.0% 65.3% 7% fellow Time (s) 3.3 3.3 2.0 ull-Red Time (s) 1.8 1.8 0.0 ost Time Adjust (s) 0.0 0.0 otal Lost Time (s) 5.1 5.1 ead/Lag ead-Lag Optimize? ct Effct Green (s) 15.9 43.9 ctuated g/C Ratio 0.21 0.59 /c Ratio 0.42 0.64 control Delay 29.4 11.6 OS C B upproach Delay 29.4 11.6 OS C B upproach LoS C B upproach Delay 29.4 11.6 OS C B upproach LoS C B upproach Delay 29.4 11.6 pyproach LoS C C B upproach Delay 29.4 11.6 pyproach Delay 29					
ane Group Flow (vph) In Type NA NA NA NA NA NA NA NA NA N					
rum Type					
remetted Phases fermitted Phases fermitted Phases flinimum Split (s)	,				
### Parameter Phases ### Param				9	
Inimum Split (s) 20.1 26.1 5.0 5.0 5.0		•		•	
otal Split (s)		20.1	26 1	5.0	
otal Split (%) 28.0% 65.3% 7% ellow Time (s) 3.3 3.3 2.0 III-Red Time (s) 1.8 1.8 0.0 ost Time Adjust (s) 0.0 0.0 0.0 otal Lost Time (s) 5.1 5.1 ead/Lag ead-Lag Optimize? ct Effct Green (s) 15.9 43.9 ctuated g/C Ratio 0.21 0.59 /c Ratio 0.42 0.64 control Delay 29.4 11.6 Ostal Delay 29.4 11.6 OS C B pproach Delay 29.4 11.6 OS C B pproach LOS C B pproach LOS C B pueue Length 50th (m) 15.8 54.0 pueue Length 95th (m) 33.2 73.1 ptemal Link Dist (m) 159.1 135.3 pueue Capacity (vph) 381 1986 ptarvation Cap Reductn 0 0 0 pillback Cap Reductn 0 0 0 pillback Cap Reductn 0 0 0 pillback Cap Reductn 0 0 0 ptorage Cap Reductn					
Sellow Time (s) 3.3 3.3 2.0					
1.8					
ost Time Adjust (s)					
total Lost Time (s) 5.1 5.1 ead/Lag ead-Lag Optimize? cct Effct Green (s) 15.9 43.9 cct tated g/C Ratio 0.21 0.59 /c Ratio 0.42 0.64 control Delay 29.4 11.6 control Delay 29.4 11.6 OS C B opproach Delay 29.4 11.6 opproach LOS C B opproach Los C				0.0	
ead/Lag ead-Lag Optimize? ct Effct Green (s) 15.9 43.9 ctuated g/C Ratio 0.21 0.59 /c Ratio 0.42 0.64 chortrol Delay 29.4 11.6 chotrol Delay 0.0 0.0 ctuated Delay 0.0 0.0 ctuated Delay 29.4 11.6 cos C B approach Delay 29.4 11.6 chocae Length 50th (m) 15.8 54.0 chocae Length 95th (m) 33.2 73.1 chocae Length Soth (m) 159.1 135.3 curn Bay Length (m) chase Capacity (vph) 381 1986 ctuated Cap Reductn 0 0 chorage Cap Reductn 0 0 chocae Capacity Capacity 0.0 chocae Cap					
ead-Lag Optimize? ct Effct Green (s) 15.9 43.9 ctuated g/C Ratio 0.21 0.59 /c Ratio 0.42 0.64 control Delay 29.4 11.6 control Delay 29.4 11.6 OS C B opproach Delay 29.4 11.6 operated Lorenth	\ <i>,</i>	0.1	0.1		
15.9					
actuated g/C Ratio 0.21 0.59 1/c Ratio 0.42 0.64 1.66 1.60 1		15.0	43 Q		
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OS C B Approach Delay 29.4 11.6 Approach LOS C B Approach LOS C					
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Auteueu Length 95th (m) Auternal Link Dist (m) Auter					
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turn Bay Length (m) tase Capacity (vph) asse Capacity (vph) tarvation Cap Reductn 0 object to a R					
Asse Capacity (vph) Starvation Cap Reductn Spillback Cap Reductn Storage Cap Reductn O Steduced v/c Ratio O Storage Cap Reductn O Steduced v/c Ratio Steduced Cycle Length: 75 Steductated Cycle Length: 75 Steductated Cycle Length: 75 Steductated Cycle: 60 Steductor Cycle: 60 Steductor Cycle: 60 Steductor Cycle: 60 Steductor Cycle: 10 Steductor Cycle: 1	· ,	100.1	100.0		
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pillback Cap Reductn 0 0 torage Cap Reductn 0 0 0 teduced v/c Ratio 0.42 0.64 Intersection Summary Eycle Length: 75 Intersection Cycle Length: 75 Intersection Symmary Eycle Length: 75 Intersection Symmary Eycle Length: 75 Intersection Cycle Length: 75 Intersection Cycle: 60 Intersection Type: Pretimed Intersection Signal Delay: 13.6 Intersection LOS: B Intersection Capacity Utilization 61.6% Intersection Cycle: B Intersection Cycle:					
Attersection Summary Eycle Length: 75 Inctuated Cycle Length: 75 Inctuated Cycle Length: 75 Inctuated Cycle: 60 Intersection Type: Pretimed Intersection Signal Delay: 13.6 Intersection Capacity Utilization 61.6% Intersection Capacity Utilization 61.6% Intersection Signal Phases: 8: O'Connor St & Gilmour St					
Attersection Summary Eycle Length: 75 Inctuated Cycle Length: 75 Inctuated Cycle Length: 75 Inctuated Cycle Length: 75 Inctuated Cycle Length: 75 Inffrset: 71 (95%), Referenced to phase 6:SBTL, Start of Green Idatural Cycle: 60 Intersection Type: Pretimed Intersection Signal Delay: 13.6 Intersection LOS: B Intersection Capacity Utilization 61.6% Intersection Capacity Utilization 61.6% Intersection Signal Delay: 13.6 Intersection Capacity Utilization 61.6% Intersection Capacity Utilization 61.6% Intersection Signal Delay: 13.6 Intersection Capacity Utilization 61.6% Intersection Signal Delay: 13.6 Intersection Signal Delay: 13.6 Intersection Capacity Utilization 61.6% Intersection Signal Delay: 13.6 Intersection Signal Delay					
Antersection Summary Cycle Length: 75 C					
cycle Length: 75 ctuated Cycle Length: 75 cffset: 71 (95%), Referenced to phase 6:SBTL, Start of Green latural Cycle: 60 control Type: Pretimed faximum v/c Ratio: 0.64 intersection Signal Delay: 13.6 intersection Capacity Utilization 61.6% intersection Capacity Utilization 61.6% inalysis Period (min) 15 splits and Phases: 8: O'Connor St & Gilmour St		0.42	0.04		
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Offset: 71 (95%), Referenced to phase 6:SBTL, Start of Green latural Cycle: 60 Control Type: Pretimed Maximum v/c Ratio: 0.64 Intersection Signal Delay: 13.6 Intersection Capacity Utilization 61.6% Intersection Capacity Utilization 61.6% Intersection Signal Delay: 8: O'Connor St & Gilmour St	Cycle Length: 75				
latural Cycle: 60 Control Type: Pretimed faximum v/c Ratio: 0.64 Intersection Signal Delay: 13.6 Intersection Capacity Utilization 61.6% ICU Level of Service B Inalysis Period (min) 15 Iplits and Phases: 8: O'Connor St & Gilmour St					
Control Type: Pretimed Maximum v/c Ratio: 0.64 Intersection Signal Delay: 13.6 Intersection Capacity Utilization 61.6% Intersection Capacity Utilization 61.6% Intersection Capacity Utilization 61.6% Intersection Capacity Utilization 61.6% Intersection LOS: B ICU Level of Service B Inalysis Period (min) 15 Intersection LOS: B ICU Level of Service B ICU Level of Service B Intersection LOS: B ICU Level of Service B ICU Level o		ed to phase	6:SBTL,	Start of Gre	n
Maximum v/c Ratio: 0.64 Intersection Signal Delay: 13.6 Intersection Capacity Utilization 61.6% ICU Level of Service B Intersection Capacity Utilization 61.6% Intersection LOS: B ICU Level of Service B ICU	Natural Cycle: 60				
Intersection Signal Delay: 13.6 Intersection LOS: B Intersection Capacity Utilization 61.6% ICU Level of Service B Inalysis Period (min) 15 Intersection LOS: B ICU Level of Service B	Control Type: Pretimed				
ntersection Capacity Utilization 61.6% ICU Level of Service B splits and Phases: 8: O'Connor St & Gilmour St	Maximum v/c Ratio: 0.64				
plits and Phases: 8: O'Connor St & Gilmour St	Intersection Signal Delay: 1	3.6			Intersection LOS: B
splits and Phases: 8: O'Connor St & Gilmour St	Intersection Capacity Utiliza	ation 61.6%)		ICU Level of Service B
1	Analysis Period (min) 15				
1	Splits and Phases: 8: 0'0	Connor St 8	k Gilmour	St	
	Ø9				



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Lane Group	EBL	EBT	WBT	NBL	NBT	
Lane Configurations	ሻ	(Î	4	7	ተተ _ጉ	
Traffic Volume (vph)	62	319	145	75	1678	
Future Volume (vph)	62	319	145	75	1678	
Lane Group Flow (vph)	62	319	195	75	1862	
Turn Type	Perm	NA	NA	Perm	NA	
Protected Phases		4	8		2	
Permitted Phases	4			2		
Minimum Split (s)	23.5	23.5	23.5	23.4	23.4	
Total Split (s)	27.0	27.0	27.0	48.0	48.0	
Total Split (%)	36.0%	36.0%	36.0%	64.0%	64.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.2	2.2	2.2	2.1	2.1	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.5	5.5	5.5	5.4	5.4	
Lead/Lag						
Lead-Lag Optimize?						
Act Effct Green (s)	21.5	21.5	21.5	42.6	42.6	
Actuated g/C Ratio	0.29	0.29	0.29	0.57	0.57	
v/c Ratio	0.22	0.62	0.41	0.12	0.70	
Control Delay	23.0	29.6	24.8	13.4	16.7	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	23.0	29.6	24.8	13.4	16.7	
LOS	С	С	С	В	В	
Approach Delay		28.5	24.8		16.6	
Approach LOS		С	С		В	
Queue Length 50th (m)	6.6	39.0	22.2	6.0	54.0	
Queue Length 95th (m)	16.0	64.2	39.6	m10.1	71.5	
Internal Link Dist (m)		97.2	154.7		68.5	
Turn Bay Length (m)	20.0			20.0		
Base Capacity (vph)	282	511	476	628	2659	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.22	0.62	0.41	0.12	0.70	
Intersection Summary						
Cycle Length: 75						
Actuated Cycle Length: 75						
Offset: 66 (88%), Reference	d to phase	2:NBTL	, Start of	Green		
Natural Cycle: 60						
Control Type: Pretimed						
Maximum v/c Ratio: 0.70						
Intersection Signal Delay: 19	0.0			Ir	ntersectio	n LOS: B
Intersection Capacity Utilizat)				of Service E
Analysis Period (min) 15		-				

m Volume for 95th percentile queue is metered by upstream signal.

Analysis Period (min) 15



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Lane Group	WBT	NBT
Lane Configurations	f)	₽₽₽₽
Traffic Volume (vph)	14	1938
Future Volume (vph)	14	1938
Lane Group Flow (vph)	35	1971
Sign Control	Stop	Free
Intersection Summary		
Control Type: Unsignalized		
Internaction Conscitutition		

Intersection Capacity Utilization 54.8% ICU Level of Service A

Analysis Period (min) 15

Synchro 10 Report Parsons

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BL EB	T EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
			£			₽₽₽₽				
0	0 0	0	14	21	33	1938	0	0	0	0
0	0 0	0	14	21	33	1938	0	0	0	0
Sto	р		Stop			Free			Free	
0%	6		0%			0%			0%	
0 1.0	0 1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
0	0 0	0	14	21	33	1938	0	0	0	0
9	2		133			15			8	
0.	0		3.7			3.7			0.0	
1.	1		1.1			1.1			1.1	
	0		12			1			0	
						None			None	
						78			92	
2 0.7	2	0.72	0.72	0.72				0.72		
0 222	9 107	2152	2229	787	92			2071		
0 133	0 107	1222	1330	0	92			1109		
.5 6.	5 6.9	7.5	6.5	6.9	4.1			4.1		
.5 4.	0 3.3	3.5	4.0	3.3	2.2			2.2		
0 10	0 100	100	85	97	98			100		
61 9	4 913	75	94	680	1501			392		
1 NB	1 NB 2	NB 3								
35 42	1 775	775								
0 3	3 0	0								
		0								
5 150	1 1700	1700								
	0.6									
		IC	CU Level	of Service			Α			
	15									_
	3L EB 0 0 0 Stop 00 1.00 0 99 0.1 1.00 1.00 1.00 1.00 1.00 1.00 1.0	3L EBT EBR 0 0 0 0 0 0 0 0 0 0 Stop 0% 00 1.00 1.00 0 0 92 0.0 1.1 0 0 1.1 0 0 72 0.72 40 2229 107 0 1330 107 .5 6.5 6.9 .5 4.0 3.3 00 100 100 61 94 913 1 NB 1 NB 2 0 33 0 0 100 100 61 94 913 0 0 33 0 0 100 100 61 94 913 0 0 33 0 0 100 100 61 94 913 0 0 0 65 1501 1700 18 0.02 0.46 0.8 0.5 0.0 0.5 0.8 0.0 0.5 0.8 0.0 0.5 0.8 0.0 0.6 54.8%	BL EBT EBR WBL 0	BL EBT EBR WBL WBT 0 0 0 0 0 14 0 0 0 0 14 Stop Stop 0% 0% 00 1.00 1.00 1.00 1.00 0 0 0 14 92 133 0.0 3.7 1.1 11 0 12 72 0.72 0.72 0.72 1.1 1.1 0 12 73 0.75 6.5 6.5 6.9 7.5 6.5 6.5 6.9 7.5 6.5 6.5 6.9 7.5 6.5 6.5 6.9 7.5 6.5 6.1 94 913 75 94 1 NB 1 NB 2 NB 3 6.1 94 913 75 94 1 NB 1 NB 2 NB 3 6.2 1 0 0 0 6.3 1501 1700 1700 6.4 0.46 6.5 0.5 0.0 0.0 6.5 0.8 0.0 0.	BL EBT EBR WBL WBT WBR 0	3L EBT EBR WBL WBT WBR NBL 1	SL EBT EBR WBL WBT WBR NBL NBT	BL EBT EBR WBL WBT WBR NBL NBT NBR 0 0 0 0 0 14 21 33 1938 0 0 0 0 14 21 33 1938 0 0 0 0 0 14 21 33 1938 0 0 0 0 0 1.00 1.00 1.00 1.00 1.00 1.00	State EBT EBR WBL WBT WBR NBL NBT NBR SBL	SI

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Lane Group	EBT	NBT		
Lane Configurations	र्स	ተተኈ		
Traffic Volume (vph)	85	1957		
Future Volume (vph)	85	1957		
Lane Group Flow (vph)	102	2112		
Turn Type	NA	NA		
Protected Phases	4	2		
Permitted Phases	•	_		
Minimum Split (s)	21.5	21.1		
Total Split (s)	22.0	53.0		
Total Split (%)	29.3%	70.7%		
Yellow Time (s)	3.3	3.3		
All-Red Time (s)	2.2	1.8		
Lost Time Adjust (s)	0.0	0.0		
Total Lost Time (s)	5.5	5.1		
Lead/Lag	0.0	J. I		
Lead-Lag Optimize?				
Act Effct Green (s)	16.5	47.9		
Actuated g/C Ratio	0.22	0.64		
v/c Ratio	0.22	0.70		
Control Delay	19.4	23.0		
Queue Delay	0.0	0.0		
Total Delay	19.4	23.0		
LOS	19.4 B	23.0 C		
Approach Delay	19.4	23.0		
Approach LOS	19.4 B	23.0 C		
Queue Length 50th (m)	8.2	114.7		
Queue Length 95th (m)	20.1	128.8		
Internal Link Dist (m)	157.9	224.2		
Turn Bay Length (m)	131.3	ZZ+.Z		
Base Capacity (vph)	410	3030		
Starvation Cap Reductn	0	0		
Spillback Cap Reductin	0	0		
Storage Cap Reductn	0	0		
Reduced v/c Ratio	0.25	0.70		
	0.23	0.70		
Intersection Summary				
Cycle Length: 75				
Actuated Cycle Length: 75				
Offset: 5 (7%), Referenced	to phase 2:	NBT, Sta	t of Green	
Natural Cycle: 60				
Control Type: Pretimed				
Maximum v/c Ratio: 0.70				
Intersection Signal Delay: 2			Intersection LOS: C	
Intersection Capacity Utiliza	ation 66.2%		ICU Level of Service	C
Analysis Period (min) 15				
Splits and Phases: 3: Ke	ent St & Giln	nour St		
A	01 4 01111	.Jui 01		A.
Ø2 (R)				→ Ø4
53 s				22 s

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Lane Group	EBL	EBT	WBT	NBL	NBT		
Lane Configurations	ሻ	<u></u>	7>	ሻ	^		
Traffic Volume (vph)	82	299	178	36	1834		
Future Volume (vph)	82	299	178	36	1834		
Lane Group Flow (vph)	82	299	326	36	1931		
Turn Type	Perm	NA	NA	Perm	NA		
Protected Phases	. 0	4	8	. 0	2		
Permitted Phases	4	•		2	_		
Minimum Split (s)	23.4	23.4	23.4	23.4	23.4		
Total Split (s)	30.0	30.0	30.0	45.0	45.0		
Total Split (%)	40.0%	40.0%	40.0%	60.0%	60.0%		
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3		
All-Red Time (s)	2.1	2.1	2.1	2.1	2.1		
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		
Total Lost Time (s)	5.4	5.4	5.4	5.4	5.4		
Lead/Lag							
Lead-Lag Optimize?							
Act Effct Green (s)	24.6	24.6	24.6	39.6	39.6		
Actuated g/C Ratio	0.33	0.33	0.33	0.53	0.53		
v/c Ratio	0.34	0.51	0.62	0.05	0.76		
Control Delay	24.0	24.1	26.8	8.9	16.3		
Queue Delay	0.0	0.0	0.0	0.0	0.0		
Total Delay	24.0	24.1	26.8	8.9	16.3		
LOS	C	C	C	A	В		
Approach Delay		24.1	26.8	, , , , , , , , , , , , , , , , , , ,	16.2		
Approach LOS		C	C		В		
Queue Length 50th (m)	8.7	33.7	37.5	2.3	72.8		
Queue Length 95th (m)	20.3	55.7	63.1	6.3	90.8		
Internal Link Dist (m)		152.2	162.6		69.7		
Turn Bay Length (m)	25.0		. 02.0				
Base Capacity (vph)	242	585	529	747	2538		
Starvation Cap Reductn	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0		
Reduced v/c Ratio	0.34	0.51	0.62	0.05	0.76		
	0.07	0.01	0.02	0.00	0.70		
Intersection Summary							
Cycle Length: 75							
Actuated Cycle Length: 75							
Offset: 36 (48%), Referenced	d to phase	e 2:NBTL	, Start of	Green			
Natural Cycle: 60							
Control Type: Pretimed							
Maximum v/c Ratio: 0.76							
Intersection Signal Delay: 18					ntersectio		
Intersection Capacity Utilizat	ion 83.2%	,)		IC	CU Level	of Service E	
Analysis Period (min) 15							
Splits and Phases: 4: Ken	t St & Gla	dstone A	ve				
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						30 s	

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Lane Group	WBT	NBL	NBT	SBT		
Lane Configurations	4		4	7		
Traffic Volume (vph)	4	36	540	352		
Future Volume (vph)	4	36	540	352		
Lane Group Flow (vph)	28	0	576	378		
Turn Type	NA	Perm	NA	NA		
Protected Phases	8	. 0/111	2	6		
Permitted Phases		2	_			
Minimum Split (s)	24.2	23.0	23.0	23.0		
Total Split (s)	24.0	51.0	51.0	51.0		
Total Split (%)	32.0%	68.0%	68.0%	68.0%		
Yellow Time (s)	3.3	3.3	3.3	3.3		
All-Red Time (s)	1.9	1.7	1.7	1.7		
Lost Time Adjust (s)	0.0		0.0	0.0		
Total Lost Time (s)	5.2		5.0	5.0		
Lead/Lag						
Lead-Lag Optimize?						
Act Effct Green (s)	18.8		46.0	46.0		
Actuated g/C Ratio	0.25		0.61	0.61		
v/c Ratio	0.07		0.56	0.36		
Control Delay	15.5		3.0	8.2		
Queue Delay	0.0		0.2	0.0		
Total Delay	15.5		3.1	8.2		
LOS	В		Α	Α		
Approach Delay	15.5		3.1	8.2		
Approach LOS	В		Α	Α		
Queue Length 50th (m)	1.6		5.1	23.0		
Queue Length 95th (m)	7.3		7.1	37.7		
Internal Link Dist (m)	126.3		56.4	50.2		
Turn Bay Length (m)			100	1611		
Base Capacity (vph)	383		1031	1041		
Starvation Cap Reductn	0		66	0		
Spillback Cap Reductn	0		0	0		
Storage Cap Reductn	0		0	0		
Reduced v/c Ratio	0.07		0.60	0.36		
ntersection Summary						
Cycle Length: 75						
Actuated Cycle Length: 75						
Offset: 42 (56%), Referenced	d to phase	2:NBTL	and 6:SE	T, Start o	f Green	
Natural Cycle: 60						
Control Type: Pretimed						
Maximum v/c Ratio: 0.56						
ntersection Signal Delay: 5.5					tersection LOS: A	
ntersection Capacity Utilizati	ion 82.4%	ò		IC	CU Level of Service E	
Analysis Period (min) 15						
Splits and Phases: 5: Banl	k St & Ma	ıcl aren S	it			
4	K OL & IVID	ocaron o				1
02 (R)						
						■
▼ Ø6 (R)						₹ Ø8

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Lane Group	EBT	NBT	SBL	SBT	
Lane Configurations	4	1		सी	
Traffic Volume (vph)	60	583	15	231	
Future Volume (vph)	60	583	15	231	
Lane Group Flow (vph)	133	617	0	246	
Turn Type	NA	NA	Perm	NA	
Protected Phases	4	2		6	
Permitted Phases			6		
Minimum Split (s)	23.2	26.1	26.1	26.1	
Total Split (s)	25.0	50.0	50.0	50.0	
Total Split (%)	33.3%	66.7%	66.7%	66.7%	
Yellow Time (s)	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.9	1.8	1.8	1.8	
Lost Time Adjust (s)	0.0	0.0		0.0	
Total Lost Time (s)	5.2	5.1		5.1	
Lead/Lag					
Lead-Lag Optimize?					
Act Effct Green (s)	19.8	44.9		44.9	
Actuated g/C Ratio	0.26	0.60		0.60	
v/c Ratio	0.30	0.59		0.24	
Control Delay	22.5	12.1		4.8	
Queue Delay	0.0	0.0		0.3	
Total Delay	22.5	12.1		5.1	
LOS	С	В		Α	
Approach Delay	22.5	12.1		5.1	
Approach LOS	С	В		Α	
Queue Length 50th (m)	12.5	48.1		6.9	
Queue Length 95th (m)	m23.4	76.3		11.0	
Internal Link Dist (m)	115.9	89.4		56.4	
Turn Bay Length (m)					
Base Capacity (vph)	440	1045		1020	
Starvation Cap Reductn	0	0		365	
Spillback Cap Reductn	0	1		0	
Storage Cap Reductn	0	0		0	
Reduced v/c Ratio	0.30	0.59		0.38	
Intersection Summary					
Cycle Length: 75					
Actuated Cycle Length: 75					
Offset: 37 (49%), Reference	d to phase	e 2:NBT a	and 6:SBT	L, Start	of Green
Natural Cycle: 60					
Control Type: Pretimed					
Maximum v/c Ratio: 0.59					
Intersection Signal Delay: 11	1.8			lı	ntersection LOS: B
Intersection Capacity Utilizat	tion 58.6%	,)		[(CU Level of Service B

Analysis Period (min) 15
m Volume for 95th percentile queue is metered by upstream signal.



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Lane Group	EBT	SBL
Lane Configurations	4) T
Traffic Volume (vph)	202	37
Future Volume (vph)	202	37
Lane Group Flow (vph)	240	37
Sign Control	Free	Stop
Intersection Summary		
Control Type: Unsignalized		
Intersection Capacity Utilizat	tion 23.4%	

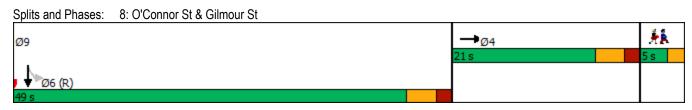
Analysis Period (min) 15

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Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		4			ሻ		
Traffic Volume (veh/h)	38	202	0	0	37	0	
Future Volume (Veh/h)	38	202	0	0	37	0	
Sign Control		Free	Free		Stop		
Grade		0%	0%		0%		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly flow rate (vph)	38	202	0	0	37	0	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type		None	None				
Median storage veh)							
Upstream signal (m)		67	140				
pX, platoon unblocked					0.97		
vC, conflicting volume	0				278	0	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	0				245	0	
tC, single (s)	4.1				6.4	6.2	
tC, 2 stage (s)							
tF (s)	2.2				3.5	3.3	
p0 queue free %	98				95	100	
cM capacity (veh/h)	1623				707	1085	
Direction, Lane #	EB 1	SB 1					
Volume Total	240	37					
Volume Left	38	37					
Volume Right	0	0					
cSH	1623	707					
Volume to Capacity	0.02	0.05					
Queue Length 95th (m)	0.02	1.3					
	1.3	10.4					
Control Delay (s)							
Lane LOS	A	B					
Approach Delay (s)	1.3	10.4					
Approach LOS		В					
Intersection Summary							
Average Delay			2.5				
Intersection Capacity Utiliza	ation		23.4%	IC	U Level o	of Service	
Analysis Period (min)			15				

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Lane Group	EBT	SBT	Ø9	
Lane Configurations	4	414		
Traffic Volume (vph)	61	672		
Future Volume (vph)	61	672		
Lane Group Flow (vph)	122	721		
Turn Type	NA	NA		
Protected Phases	4	6	9	
Permitted Phases				
Minimum Split (s)	20.1	26.1	5.0	
Total Split (s)	21.0	49.0	5.0	
Total Split (%)	28.0%	65.3%	7%	
Yellow Time (s)	3.3	3.3	2.0	
All-Red Time (s)	1.8	1.8	0.0	
Lost Time Adjust (s)	0.0	0.0		
Total Lost Time (s)	5.1	5.1		
Lead/Lag				
Lead-Lag Optimize?				
Act Effct Green (s)	15.9	43.9		
Actuated g/C Ratio	0.21	0.59		
v/c Ratio	0.31	0.36		
Control Delay	11.4	8.1		
Queue Delay	0.0	0.0		
Total Delay	11.4	8.1		
LOS	В	A		
Approach Delay	11.4	8.1		
Approach LOS	B	Α		
Queue Length 50th (m)	4.5	23.0		
Queue Length 95th (m)	m12.6	32.7		
Internal Link Dist (m)	159.1	135.3		
Turn Bay Length (m)	392	1980		
Base Capacity (vph) Starvation Cap Reductn		1960		
Spillback Cap Reductn	0	0		
Storage Cap Reductin	0	0		
Reduced v/c Ratio	0.31	0.36		
	0.01	0.00		
Intersection Summary				
Cycle Length: 75				
Actuated Cycle Length: 75				
Offset: 46 (61%), Reference	ed to phase	e 6:SBTL,	Start of Green	
Natural Cycle: 55				
Control Type: Pretimed				
Maximum v/c Ratio: 0.36	_			
Intersection Signal Delay: 8.				
Intersection Capacity Utiliza	tion 45.4%)		

m Volume for 95th percentile queue is metered by upstream signal.

Analysis Period (min) 15



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Lane Group	EBL	EBT	WBT	NBL	NBT		
Lane Configurations	ች	ĵ.	4	*	ተተኈ		
Traffic Volume (vph)	56	359	327	77	762		
Future Volume (vph)	56	359	327	77	762		
Lane Group Flow (vph)	56	359	390	77	868		
Turn Type	Perm	NA	NA	Perm	NA		
Protected Phases		4	8		2		
Permitted Phases	4			2			
Minimum Split (s)	23.5	23.5	23.5	23.4	23.4		
Total Split (s)	41.0	41.0	41.0	34.0	34.0		
Total Split (%)	54.7%	54.7%	54.7%	45.3%	45.3%		
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3		
All-Red Time (s)	2.2	2.2	2.2	2.1	2.1		
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		
Total Lost Time (s)	5.5	5.5	5.5	5.4	5.4		
Lead/Lag							
Lead-Lag Optimize?							
Act Effct Green (s)	35.5	35.5	35.5	28.6	28.6		
Actuated g/C Ratio	0.47	0.47	0.47	0.38	0.38		
v/c Ratio	0.17	0.43	0.49	0.24	0.49		
Control Delay	12.9	15.0	16.2	22.7	21.8		
Queue Delay	0.0	0.0	0.0	0.0	0.0		
Total Delay	12.9	15.0	16.2	22.7	21.8		
LOS	В	В	В	С	С		
Approach Delay		14.7	16.2		21.9		
Approach LOS		В	В		С		
Queue Length 50th (m)	4.3	31.8	35.9	7.0	28.5		
Queue Length 95th (m)	10.9	51.7	58.5	17.3	58.4		
Internal Link Dist (m)		97.2	154.7		68.5		
Turn Bay Length (m)	20.0			20.0			
Base Capacity (vph)	339	844	794	326	1759		
Starvation Cap Reductn	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0		
Reduced v/c Ratio	0.17	0.43	0.49	0.24	0.49		
Intersection Summary							
Cycle Length: 75							
Actuated Cycle Length: 75							
Offset: 50 (67%), Referenced	to nhace	2·NRTI	Start of	Green			
Natural Cycle: 50	i to priase	Z.NDIL	, otali or	Olecii			
Control Type: Pretimed							
Maximum v/c Ratio: 0.49							
Intersection Signal Delay: 18	Q			l.	ntersection	n I OS· B	
Intersection Signal Delay. To Intersection Capacity Utilizati						of Service D	
Analysis Period (min) 15	011 13.370) 		I	JO LEVEL	DI OCIVICE D	
Alialysis i chou (Illill) 13							
Splits and Phases: 1: Kent	St & Sor	nerset St	W				
↑ Ø2 (R)	· · · · · ·			T-	Ø4		
34 s				41	s		
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Lane Group	WBT	NBT	
Lane Configurations	^	₽₽₽₽	
Traffic Volume (vph)	22	939	
Future Volume (vph)	22	939	
Lane Group Flow (vph)	92	970	
Sign Control	Stop	Free	
Intersection Summary			
Control Type: Unsignalized			
Interception Canacity Litilize			ICILI aval of Carrios A

Intersection Capacity Utilization 38.5% ICU Level of Service A

Analysis Period (min) 15

Synchro 10 Report Parsons

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					ĵ»			414				
Traffic Volume (veh/h)	0	0	0	0	22	70	31	939	0	0	0	0
Future Volume (Veh/h)	0	0	0	0	22	70	31	939	0	0	0	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	0	0	0	0	22	70	31	939	0	0	0	0
Pedestrians		45			95			11			17	
Lane Width (m)		0.0			3.7			3.7			0.0	
Walking Speed (m/s)		1.1			1.1			1.1			1.1	
Percent Blockage		0			9			1			0	
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)								78			92	
pX, platoon unblocked	0.92	0.92		0.92	0.92	0.92				0.92		
vC, conflicting volume	518	1141	56	1107	1141	425	45			1034		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	174	851	56	814	851	73	45			734		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	100	91	91	98			100		
cM capacity (veh/h)	556	243	988	206	243	817	1561			727		
Direction, Lane #	WB 1	NB 1	NB 2	NB 3								
Volume Total	92	219	376	376								
Volume Left	0	31	0	0								
Volume Right	70	0	0	0								
cSH	522	1561	1700	1700								
Volume to Capacity	0.18	0.02	0.22	0.22								
Queue Length 95th (m)	4.8	0.5	0.0	0.0								
Control Delay (s)	13.4	1.2	0.0	0.0								
Lane LOS	В	Α										
Approach Delay (s)	13.4	0.3										
Approach LOS	В											
Intersection Summary												
Average Delay			1.4									
Intersection Capacity Utiliza	ation		38.5%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

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Lane Group	EBT	NBT	
Lane Configurations	4	ተ ተጉ	
Traffic Volume (vph)	79	914	
Future Volume (vph)	79	914	
Lane Group Flow (vph)	98	1015	
Turn Type	NA	NA	
Protected Phases	4	2	
Permitted Phases	•	_	
Minimum Split (s)	21.5	21.1	
Total Split (s)	22.0	53.0	
Total Split (%)	29.3%	70.7%	
Yellow Time (s)	3.3	3.3	
All-Red Time (s)	2.2	1.8	
Lost Time Adjust (s)	0.0	0.0	
Total Lost Time (s)	5.5	5.1	
Lead/Lag	0.0	0.1	
Lead-Lag Optimize?			
Act Effct Green (s)	16.5	47.9	
Actuated g/C Ratio	0.22	0.64	
v/c Ratio	0.24	0.34	
Control Delay	19.2	9.5	
Queue Delay	0.0	0.0	
Total Delay	19.2	9.5	
LOS	В	A	
Approach Delay	19.2	9.5	
Approach LOS	В	A	
Queue Length 50th (m)	7.7	22.9	
Queue Length 95th (m)	19.4	37.1	
Internal Link Dist (m)	157.9	224.2	
Turn Bay Length (m)	101.0		
Base Capacity (vph)	405	2998	
Starvation Cap Reductn	0	0	
Spillback Cap Reductn	0	0	
Storage Cap Reductn	0	0	
Reduced v/c Ratio	0.24	0.34	
Intersection Summary			
Cycle Length: 75			
Actuated Cycle Length: 75	a nhc 0	NDT O	# of O
Offset: 0 (0%), Referenced t	o phase 2	.เทษ เ , Sta	I OI GIEEN
Natural Cycle: 45			
Control Type: Pretimed			
Maximum v/c Ratio: 0.34	1		Intersection LOS: B
Intersection Signal Delay: 10			Intersection LOS: B ICU Level of Service A
Intersection Capacity Utilizat	uon 43.8%)	IOO Fenel of Selvice V
Analysis Period (min) 15			
Splits and Phases: 3: Ken	nt St & Giln	nour St	
↑ Ø2 (R)			<u></u> ♣ 04

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Lane Group	EBL	EBT	WBT	NBL	NBT	
Lane Configurations	ች	†	1	*	ተተኈ	
Traffic Volume (vph)	75	486	350	52	793	
Future Volume (vph)	75	486	350	52	793	
Lane Group Flow (vph)	75	486	425	52	899	
Turn Type	Perm	NA	NA	Perm	NA	
Protected Phases	1 01111	4	8	1 01111	2	
Permitted Phases	4	•		2	_	
Minimum Split (s)	23.4	23.4	23.4	23.4	23.4	
Total Split (s)	40.0	40.0	40.0	35.0	35.0	
Total Split (%)	53.3%	53.3%	53.3%	46.7%	46.7%	
Yellow Time (s)	3.3	3.3	3.3	3.3	3.3	
All-Red Time (s)	2.1	2.1	2.1	2.1	2.1	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.4	5.4	5.4	5.4	5.4	
Lead/Lag	J. 4	J. 4	J. 4	J. 4	J. 11	
Lead-Lag Optimize?						
Act Effct Green (s)	34.6	34.6	34.6	29.6	29.6	
Actuated g/C Ratio	0.46	0.46	0.46	0.39	0.39	
v/c Ratio	0.46	0.46	0.46	0.09	0.39	
	14.6	18.6	16.7	14.9	17.2	
Control Delay	0.0	0.0	0.0	0.0	0.0	
Queue Delay	14.6	18.6	16.7	14.9	17.2	
Total Delay LOS	14.0 B	10.0 B	16.7 B	14.9 B	17.2 B	
	Б	18.1	16.7	Б	17.1	
Approach Delay			16.7 B		17.1 B	
Approach LOS Queue Length 50th (m)	6.1	B 48.6	38.9	4.5	32.1	
	14.6	76.6	63.7	11.0	42.9	
Queue Length 95th (m)	14.0	152.2	162.6	11.0	69.7	
Internal Link Dist (m)	25.0	152.2	102.0		09.7	
Turn Bay Length (m)	25.0	000	004	Ε00	4070	
Base Capacity (vph)	327	823	801	588	1879	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0 10	
Reduced v/c Ratio	0.23	0.59	0.53	0.09	0.48	
Intersection Summary						
Cycle Length: 75						
Actuated Cycle Length: 75						
Offset: 23 (31%), Reference	ed to phase	2:NBTL	, Start of	Green		
Natural Cycle: 50			,			
Control Type: Pretimed						
Maximum v/c Ratio: 0.59						
Intersection Signal Delay: 1	7.3			lı	ntersectio	n LOS: B
Intersection Capacity Utiliza)				of Service C
Analysis Period (min) 15						
	nt St & Gla	dstone A	VA			
	in or a ola	asione A	¥0		<u> </u>	
Ø2 (R)					Ø4	
35 s					40 s	
				- 1	Ø8	

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Lane Group	WBT	NBL	NBT	SBT	
Lane Configurations	4		ર્ન	f)	
Traffic Volume (vph)	11	25	289	674	
Future Volume (vph)	11	25	289	674	
Lane Group Flow (vph)	56	0	314	700	
Turn Type	NA	Perm	NA	NA	
Protected Phases	8		2	6	
Permitted Phases	•	2		_	
Minimum Split (s)	24.2	23.0	23.0	23.0	
Total Split (s)	24.0	51.0	51.0	51.0	
Total Split (%)	32.0%	68.0%	68.0%	68.0%	
Yellow Time (s)	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.9	1.7	1.7	1.7	
Lost Time Adjust (s)	0.0	1.7	0.0	0.0	
Total Lost Time (s)	5.2		5.0	5.0	
Lead/Lag	0.2		0.0	0.0	
Lead-Lag Optimize?					
Act Effct Green (s)	18.8		46.0	46.0	
Actuated g/C Ratio	0.25		0.61	0.61	
v/c Ratio	0.25		0.01	0.66	
Control Delay	20.1		2.3	13.1	
Queue Delay	0.0		0.2	0.0	
Total Delay	20.1		2.6	13.1	
LOS	20.1 C		2.0 A	13.1 B	
	20.1		2.6	13.1	
Approach LOS	20.1 C		2.0 A	13.1 B	
Approach LOS Queue Length 50th (m)	5.0		3.2	56.7	
Queue Length 95th (m)	13.5		4.7	91.2	
Internal Link Dist (m)	126.3		56.4	50.2	
Turn Bay Length (m)	120.3		30.4	30.2	
	366		1019	1061	
Base Capacity (vph) Starvation Cap Reductn			249	0	
	0			0	
Spillback Cap Reductn Storage Cap Reductn	0		0	0	
Reduced v/c Ratio	0.15		0.41	0.66	
	0.15		0.41	0.00	
Intersection Summary					
Cycle Length: 75					
Actuated Cycle Length: 75				0	
Offset: 7 (9%), Referenced	to phase 2	:NBTL ar	nd 6:SBT,	Start of 0	Green
Natural Cycle: 60					
Control Type: Pretimed					
Maximum v/c Ratio: 0.66					
Intersection Signal Delay: 1					ntersection LOS: B
Intersection Capacity Utiliza	ation 63.8%)		IC	CU Level of Service B
Analysis Period (min) 15					
Splits and Phases: 5: Bar	nk St & Ma	cl aren S	t		
-4	5, 6, 1110		-		
02 (R) 51s					
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Ø6 (R)					▼ Ø8

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Lane Group	EBT	NBT	SBL	SBT	
Lane Configurations	4	f)		ન	
Traffic Volume (vph)	102	443	19	374	
Future Volume (vph)	102	443	19	374	
Lane Group Flow (vph)	161	471	0	393	
Turn Type	NA	NA	Perm	NA	
Protected Phases	4	2		6	
Permitted Phases			6		
Minimum Split (s)	23.2	26.1	26.1	26.1	
Total Split (s)	25.0	50.0	50.0	50.0	
Total Split (%)	33.3%	66.7%	66.7%	66.7%	
Yellow Time (s)	3.3	3.3	3.3	3.3	
All-Red Time (s)	1.9	1.8	1.8	1.8	
Lost Time Adjust (s)	0.0	0.0		0.0	
Total Lost Time (s)	5.2	5.1		5.1	
Lead/Lag					
Lead-Lag Optimize?					
Act Effct Green (s)	19.8	44.9		44.9	
Actuated g/C Ratio	0.26	0.60		0.60	
v/c Ratio	0.36	0.45		0.38	
Control Delay	16.4	9.9		4.3	
Queue Delay	0.0	0.0		0.6	
Total Delay	16.4	9.9		4.8	
LOS	В	Α		Α	
Approach Delay	16.4	9.9		4.8	
Approach LOS	В	Α		A	
Queue Length 50th (m)	9.5	32.3		8.9	
Queue Length 95th (m)	19.6	51.7		12.9	
Internal Link Dist (m)	115.9	89.4		56.4	
Turn Bay Length (m)					
Base Capacity (vph)	450	1040		1030	
Starvation Cap Reductn	0	0		312	
Spillback Cap Reductn	0	0		0	
Storage Cap Reductn	0	0		0	
Reduced v/c Ratio	0.36	0.45		0.55	
	0.00	0.10		0.00	
Intersection Summary					
Cycle Length: 75					
Actuated Cycle Length: 75					_
Offset: 6 (8%), Referenced	to phase 2	:NBT and	l 6:SBTL,	Start of	Green
Natural Cycle: 50					
Control Type: Pretimed					
Maximum v/c Ratio: 0.45					
Intersection Signal Delay: 9					ntersection LOS: A
Intersection Capacity Utiliza	ation 61.4%	b		l	CU Level of Service B
Analysis Period (min) 15					
Splits and Phases: 6: Ba	nk St & Gil	mour St			
A	TIK OL & OII	illour ot			
Ø2 (R)					

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Lane Group	EBT	SBL	
Lane Configurations	4	ň	
Traffic Volume (vph)	149	37	
Future Volume (vph)	149	37	
Lane Group Flow (vph)	180	37	
Sign Control	Free	Stop	
Intersection Summary			
Control Type: Unsignalize	d		
Intersection Capacity Utiliz	zation 20.1%		ICU Level of Service A

Analysis Period (min) 15

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Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		4			7		
Traffic Volume (veh/h)	31	149	0	0	37	0	
Future Volume (Veh/h)	31	149	0	0	37	0	
Sign Control		Free	Free		Stop		
Grade		0%	0%		0%		
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly flow rate (vph)	31	149	0	0	37	0	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type		None	None				
Median storage veh)							
Upstream signal (m)		67	140				
pX, platoon unblocked					0.98		
vC, conflicting volume	0				211	0	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	0				183	0	
tC, single (s)	4.1				6.4	6.2	
tC, 2 stage (s)							
tF (s)	2.2				3.5	3.3	
p0 queue free %	98				95	100	
cM capacity (veh/h)	1623				774	1085	
Direction, Lane #	EB 1	SB 1					
Volume Total	180	37					
Volume Left	31	37					
Volume Right	0	0					
cSH	1623	774					
Volume to Capacity	0.02	0.05					
Queue Length 95th (m)	0.4	1.1					
Control Delay (s)	1.4	9.9					
Lane LOS	Α	Α					
Approach Delay (s)	1.4	9.9					
Approach LOS		Α					
Intersection Summary							
Average Delay			2.8				
Intersection Capacity Utilization	on		20.1%	IC	U Level o	of Service	
Analysis Period (min)			15				

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Lane Group	EBT	SBT	Ø9	
Lane Configurations	î,	414		
Traffic Volume (vph)	84	1285		
Future Volume (vph)	84	1285		
ane Group Flow (vph)	159	1333		
Turn Type	NA	NA		
Protected Phases	4	6	9	
Permitted Phases	7	U	J	
Minimum Split (s)	20.1	26.1	5.0	
Total Split (s)	21.0	49.0	5.0	
Total Split (%)	28.0%	65.3%	7%	
Yellow Time (s)	3.3	3.3	2.0	
All-Red Time (s)	1.8	1.8	0.0	
			0.0	
Lost Time Adjust (s)	0.0	0.0		
Total Lost Time (s)	5.1	5.1		
Lead/Lag				
Lead-Lag Optimize?	45.0	40.0		
Act Effct Green (s)	15.9	43.9		
Actuated g/C Ratio	0.21	0.59		
v/c Ratio	0.42	0.67		
Control Delay	29.3	12.2		
Queue Delay	0.0	0.0		
Total Delay	29.3	12.2		
LOS	С	В		
Approach Delay	29.3	12.2		
Approach LOS	С	В		
Queue Length 50th (m)	15.8	58.3		
Queue Length 95th (m)	33.0	79.0		
Internal Link Dist (m)	159.1	135.3		
Turn Bay Length (m)				
Base Capacity (vph)	381	1987		
Starvation Cap Reductn	0	0		
Spillback Cap Reductn	0	0		
Storage Cap Reductn	0	0		
Reduced v/c Ratio	0.42	0.67		
Intersection Summary				
Cycle Length: 75				
Actuated Cycle Length: 75		0.00=	01 / 10	
Offset: 71 (95%), Reference	d to phase	6:SBTL,	Start of Gre	en
Natural Cycle: 60				
Control Type: Pretimed				
Maximum v/c Ratio: 0.67				
Intersection Signal Delay: 14				Intersection LOS: B
Intersection Capacity Utilizat	ion 63.3%)		ICU Level of Service B
Analysis Period (min) 15				
Splits and Phases: 8: O'C	onnor St 8	& Gilmour	St	
Ø9				
Ø6 (R)				
40 c				