

Report
Project: 135925-6.4.3

BARRETT LANDS - BLOCK 178 SERVICING BRIEF



Prepared for BARRETT CO-TENANCY
by IBI GROUP

November 2022

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1 INTRODUCTION

Barrett Lands Block 178 is located in the northern portion of the Leitrim Development Area (LDA) and is part of the Barrett Lands subdivision. IBI Group Professional Services Inc. (IBI Group) has been retained by Barrett Co-Tenancy to provide professional engineering services for Block 178. The subject site is approximately 1.28 ha and consists of 50 townhouse units. The site consists of freehold frontage onto an 8.5m and a 6.0m wide private lane. There will be a common elements agreement in place for the shared elements of the site.

Block 178 is bounded by Barrett Farm Drive to the North, Barrett Lands Phase 3 lands to the west, Cemetery lands to the south and a future commercial to the east. Refer to key plan below for block location.



The proposed servicing design conforms to current City of Ottawa and MECP design criteria, and no pre-consultation meetings were requested from the South Nation Conservation (SNC) or the Ontario Ministry of Environment, Conservation and Parks (MECP).

1.1 Guidelines and Standards

This evaluation takes into consideration the City of Ottawa Sewer Design Guidelines (OSDG) (October 2012), and the February 2014 Technical Bulletin ISDTB-2014-01, the September 2016 Technical Bulletin PIEDTB-2016-01, the June 2018 Technical Bulletin ISTB-2018-04, October 2019 Technical Bulletin 2019-01, and the July Technical Bulletin 2019-02.

It also considers the City of Ottawa Water Distribution Design Guidelines (OWDDG), and the 2010 Technical Bulletin 2010-02, the 2014 Technical Bulletin 2014-02, the 2018 Technical Bulletin 2018-02 and the 2020 Technical Bulletin 2020-02.

All specifications are as per current City of Ottawa standards and specifications, and Province of Ontario (OPSS/D) standards, specifications and drawings.

1.2 Pre-Consultation Meeting

The City of Ottawa hosted a virtual pre-consultation meeting on August 18th, 2021. Notes of the meeting are provided in **Appendix A**. There were no major engineering concerns flagged in this meeting. The City of Ottawa Servicing Study Checklist has also been included in **Appendix A**.

1.3 Environmental Issues

There are no environmental issues related to this site, as all environmental concerns were dealt with as part of the applicants Barrett Lands Phase 3 subdivision approval.

All existing watercourses or drainage features associated with this site have been addressed through SNCA permit number 2021-GLO-R234.

1.4 Geotechnical Concerns

Golder was retained by Barrett Co-Tenancy to review the grading plan to ensure that the recommendations with its original report for the subject area. There were no particular design concerns for this development.

2 WATER DISTRIBUTION

2.1 Existing Conditions

There is an existing 250mm watermain in Barrett Farm Drive in Barrett Lands Phase 3 to the north of the site, which is proposed to continue east on Barrett Farm Drive in Barrett Phase 3 to the northeast of the site. The proposed development was considered in the water model for the Barrett Phase 2 and 3 developments.

2.2 Design Criteria

2.2.1 Water Demands

Block 178 consists of 50 townhouse units. Per unit population density and consumption rates are taken from **Tables 4.1** and **4.2** of the Ottawa Design Guidelines – Water Distribution and are summarized as follows:

- Semi Detach/Townhouse 2.7 person per unit
- Average Day Demand 280 l/cap/day
- Peak Daily Demand 700 l/cap/day
- Peak Hour Demand 1,540 l/cap/day

A water demand calculation sheet is included in **Appendix B** and the total water demands are summarized as follows:

- Average Day 0.44 l/s
- Maximum Day 1.10 l/s
- Peak Hour 2.42 l/s

2.2.2 System Pressures

The 2010 City of Ottawa Water Distribution Guidelines states that the preferred practice for design of a new distribution system is to have normal operating pressures range between 345 kPa (50 psi) and 552 kPa (80 psi) under maximum daily flow conditions. Other pressure criteria identified in the guidelines are as follows:

- | | |
|------------------|--|
| Minimum Pressure | Minimum system pressure under peak hour demand conditions shall not be less than 276 kPa (40 psi). |
| Fire Flow | During the period of maximum day demand, the system pressure shall not be less than 140 kPa (20 psi) during a fire flow event. |
| Maximum Pressure | Maximum pressure at any point in the distribution system in unoccupied areas shall not exceed 689 kPa (100 psi). In accordance with the Ontario Building/Plumbing Code the maximum pressure should not exceed 552 kPa (80 psi) in occupied areas. Pressure reduction controls may be required for buildings where it is not possible/feasible to maintain the system pressure below 552 kPa. |

2.2.3 Fire Flow Rate

A Fire Underwriters Survey has been carried out on a representative block to determine the fire flow for the site. The calculations result in a fire flow of 12,000 l/min; a copy of the FUS calculation is included in **Appendix B**.

2.2.4 Boundary Conditions

The City of Ottawa has provided hydraulic boundary conditions two locations in Barrett Lands Phase 3. The City has provided existing condition and SUC Zone reconfiguration boundary conditions. The existing condition has the highest maximum HGL value and is used in the analysis to determine maximum pressure while the SUC Zone reconfiguration value has the lower values for peak hour and fire and is used in the analysis. A copy of the Boundary Condition is included in **Appendix B** and summarized as follows:

CRITERIA		HYDRAULIC HEAD	
		CONNECTION 1 Private Lane @ Barrett Farm Drive	CONNECTION 2 Private Lane @ Barrett Farm Drive
Existing Conditions	Max HGL (Basic Day)	154.6 m	154.6 m
	Peak Hour	144.7 m	144.7 m
	Max Day + Fire (12,000 l/m)	121.8 m	125.3 m
SUC Reconfiguration	Max HGL (Basic Day)	147.5 m	147.5 m
	Peak Hour	145.8m	145.8 m
	Max Day + Fire (12,000 l/m)	130.2 m	132.3 m

2.2.5 Hydraulic Model

A computer model for the Block 178 water distribution system has been developed using the InfoWater SA program. The model includes the boundary conditions provided by the City of Ottawa and a portion of Barrett Lands Phase 3 watermains.

2.3 Proposed Water Plan

2.3.1 Hydraulic Analysis

The hydraulic model was run under basic day conditions with the existing boundary condition to determine the maximum pressure for the site. The minimum pressure for the site is determined in the peak hour analysis using the SUC Zone reconfiguration boundary condition. There are two fire hydrants on this site, and they are represented by nodes S11-515 and S11-520 in the model; the model was run under the max day plus fire (12,000 l/min) SUC Zone Reconfiguration Boundary condition to determine the design fire flow at the hydrant locations. Results of the analysis for the Block 178 site are summarized in Section 2.3.2 and the water model schematic and model results are included in **Appendix B**.

2.3.2 Summary of Results

Results of the hydraulic analysis for Block 178 are summarized as follows:

SCENARIO	EXISTING	SUC
Basic Day Pressure (kPa)	507.6 – 513.48	439.98 – 451.74
Peak Hour Pressure (kPa)	409.63 – 424.27	423.32 – 435.08
Minimum Residual Pressure (kPa)	93.62	217.25

A comparison of the results and design criteria is summarized as follows:

- Maximum Pressure All nodes have basic day pressure below 552 kPa for existing conditions; therefore, pressure reducing control is not required for this site.
- Minimum Pressure All nodes exceed the minimum requirement of 276 kPa during peak hour conditions for the SUC Zone configuration.
- Fire Flow The model was run with a fire flow of 12,000 l/min under the SUC Zone Reconfiguration. The residual pressures at fire hydrant locations all exceed the minimum requirement of 140 kPa under the SUC Reconfiguration.

3 WASTEWATER

3.1 Existing Conditions

The Leitrim Pump Station is the wastewater outlet for all developed lands within the LDA, including the subject property. In 2002, the City constructed the station, associated forcemains and outlet sewers in Bank Street and Conroy Road. Sewage from the LDA outlets to the Conroy Road Trunk Sewer eventually discharging to a sewage treatment plant located near the Ottawa River. The Barrett Lands Phase 1 report prepared by IBI Group dated March 2017 confirmed that the existing 375mm sewer in Kelly Farm Drive has sufficient capacity for the Barrett Lands at Findlay Creek property inclusive of the proposed development.

3.1.1 Verification of Existing Sanitary Sewer Capacity

There is an existing 200mm sanitary sewer in Barrett Farm Drive, which connects to the 375 mm diameter sub-trunk sewer in Kelly Farm Drive. In the previous Barrett Lands Phase 3 report, the design for Block 178 was for 84 apartment units, with an allocated population of 159.6 people, a site area of 1.28 and a total flow of 2.26 L/s, see **Appendix C** for excerpts from the Phase 3 report.

For the subject development, it is proposed to build a total of 50 units – 14 townhomes and 36 back-to-back townhouse units and a future commercial site. The new total proposed population is 126.0 people, area 1.28 Ha and a total flow of 1.88L/s. This represents a total peaking flow decrease of **0.38L/s** when compared to the Phase 3 allocation. The decrease in flow on the existing system from the subject development is considered to have no negative impacts on downstream infrastructure.

3.2 Proposed Sewers

All on-site sewers have been designed to City of Ottawa and MOE design criteria which include but are not limited to the below listed criteria. A copy of the detailed sanitary tributary area plan 400 and the sanitary sewer design sheets are included in **Appendix C** illustrate the population densities and sewers which provide the necessary outlets.

3.2.1 Design Flow:

Average Residential Flow	-	280 l/cap/day
Peak Residential Factor	-	Harmon Formula
Infiltration Allowance	-	0.33 l/sec/Ha
Minimum Pipe Size	-	200mm diameter

3.2.2 Population Density:

Semi-Detached & Townhouse	-	2.7 person/unit
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4 SITE STORMWATER MANAGEMENT

4.1 Objective

The purpose of this evaluation is to prepare the dual drainage design, including the minor and major system, for the Block 178 development. The design includes the assignment of inlet control devices, on-site storage, maximum depth of surface ponding and hydraulic grade line analysis. The evaluation takes into consideration the City of Ottawa Sewer Design Guidelines (OSDG) (October 2012), the February 2014 Technical Bulletin ISDTB-2014-01, the September 2016 Technical Bulletin PIEDTB-2016-01 and the June 2018 Technical Bulletin ISTB-2018-04.

4.2 Existing Conditions

The subject development is tributary to the Barrett Farm Drive storm sewer, which was approved for construction for the Barrett Lands Phase 3 development. Subsequent to the approval of Phase 3, the stormwater management analysis for Barrett Lands Phase 3 included an updated to the subject site's tributary allocation into the Barrett Farm Drive storm sewer. As part of that approval, a 675mm diameter storm sewer was approved for the subject block. The subject block is referenced as "R11304" in the Barrett Lands Phase 3 design. A copy of the design sheet, and approved drainage area plan for Phase 3 have been included in **Appendix D**.

Additionally, the Barrett Lands Phase 3 stormwater management identified a minor system restriction for this site to be the 5-year modelled flow of **234 l/s**. An excerpt from the Phase 3 report has been included in **Appendix D**.

4.3 Design Criteria

The stormwater system was designed following the principles of dual drainage, making accommodations for both major and minor flow.

Some of the key criteria include the following:

- Design Storm 1:2-year return (Ottawa)
(It should be noted that the overall Barrett Lands Site utilized 1:5 year return storm for minor system release from the subject site, further details are provided in Section 4.4 and 4.5.2)
- Rational Method Sewer Sizing
- Initial Time of Concentration 10 minutes
- Runoff Coefficients
 - Front Yards C = 0.77
 - Rear Yards C = 0.52
- Pipe Velocities 0.80 m/s to 3.0 m/s
- Minimum Pipe Size 250 mm diameter
(200 mm CB Leads)

A sample calculation of run-off coefficients has been provided in Appendix D. The runoff coefficients used are based on the actual footprint in the site plan. Zoning setbacks do not apply to the site plan. The values calculated are lower than the values used, thus a conservative approach has been provided in this analysis.

4.4 System Concept

According to the Barrett Lands Phase 3 report prepared by IBI Group dated April 2022, the development of the adjacent downstream properties included the expected stormwater servicing needs of the subject property. The existing storm sewers constructed adjacent to the site were oversized to provide the needed capacity for minor storm runoff from the subject site. Minor storm runoff from the subject site will connect to the existing 675mm Ø sewer stub that connects to the existing 1050mm Ø trunk storm sewer in Barrett Farm Drive.

4.4.1 Dual Drainage Design

The dual drainage system proposed for the subject site will accommodate both major and minor stormwater runoff. Minor flow from the subject site will be conveyed through the storm sewer network and discharge into the existing 675mm Ø sewer stub that connects to the existing 1050mm Ø trunk storm sewer in Barrett Farm Drive.

The balance of the surface flow not captured by the minor system will be conveyed via the major system. Where possible, storage will be provided in surface sags or low points within the roadway. Storage will also be provided within oversized storm pipes. Once the maximum storage is utilized, the excess flow will cascade to the next downstream street sag. Based on Phase 3 information, the 100-year overflow allocation related to the subject development lands is 350 l/s and the 100 year + 20% stress test allocation is 476 l/s. Major flow from street segments will overflow to the major flow block connecting to adjacent Barrett Lands Phase 3 at Delphinium Crescent to the west and to Barrett Farm Drive to the North, once on-site surface ponds have reach capacity.

4.4.2 Proposed Minor System

Using the criteria identified in Section 4.3, the proposed on-site storm sewers were sized accordingly. A detailed storm sewer design sheet and the associated storm sewer drainage area plan is included in **Appendix D**. The general plan of services, depicting all on-site storm sewers can be found in **Appendix A**.

The owner of the site will be responsible for regular maintenance of the on-site sewers, catch basins and inlet control devices (ICDs). Maintenance includes but is not limited to the cost of regular cleaning of the structures and ICDs as necessary. The site owner will also be responsible for replacement of damaged or missing catch basin structures, grates or ICDs as needed.

4.5 Stormwater Management

4.5.1 Water Quality Control

The subject site is part of the larger development referred to as the Leitrim Development Area. The stormwater management strategy was outlined in the following reports:

- Addendum to Leitrim Development Area Stormwater Management Environmental Study Report and Pre-Design Volumes 1 and II (IBI Group, July 2005);
- Design Brief and Amendment to MOE Certificate of Approval Findlay Creek Village Stormwater Facility (IBI Group, July 2005);
- Final Serviceability Report Leitrim Development Area City of Ottawa (IBI Group, March 2007).
- 2016 Final Updated Serviceability Report (Class EA OPA76 Areas 8a, 9a and 9b) Leitrim Development Area (IBI Group, September 2016)

The subject site is part of the drainage area which ultimately discharges into the existing Findlay Creek Village Stormwater Facility. The Findlay Creek Village Stormwater Facility was constructed

in 2006 and provides water quality control to an Enhanced Level of Protection according to MOE Stormwater Management Planning and Design Guidelines (March 2003).

4.5.2 Water Quantity Control

The subject site will be limited to a maximum minor system release rate of **234 L/s** based on the Barrett Lands Phase 3 Servicing Brief, reference information is provided within **Appendix D**. This will be achieved through a combination of inlet control devices (ICD's) at inlet locations, surface storage where possible and underground storage in oversized storm pipes where required.

The restricted flows on the private residential site and future commercial site were calculated based on their respective areas. The following assumptions were used for the future commercial block: assume site storage of 12m³ and runoff coefficient of 0.8.

$$Q_{\text{restrictedPrivate}} = 234 \text{ L/s} \times (A_{\text{Private}} / (A_{\text{Private}} + A_{\text{Commercial}}))$$

$$Q_{\text{restrictedPrivate}} = 234 \text{ L/s} \times (1.06 \text{ ha} / (1.06 + 0.16) \text{ ha})$$

$$Q_{\text{restrictedPrivate}} = 203.31 \text{ L/s}$$

$$Q_{\text{restrictedCommercial}} = 30.19 \text{ L/s}$$

There are 2 small locations where water is left to discharge uncontrolled from the subject property. The uncontrolled release can be calculated as follows:

Uncontrolled Release, where $Q_{\text{uncontrolled}} = 2.78(C \times i_{100\text{yr}} \times A_{\text{uncontrolled}})$

C	=Runoff Coefficient	=0.80
T _c	=Time of Concentration	=10min
i _{100yr}	=100yr intensity (1735.688 / (T _c + 6.014) ^{0.820})	=178.56
A _{unc}	=Area uncontrolled	=0.06Ha

$$Q_{\text{uncontrolled}} = \mathbf{23.83 \text{ L/s}}$$

The Maximum allowable release rate from the site can be determined by subtracting the Uncontrolled release rate from the minor system restricted flow rate.

$$Q_{\text{max}} = Q_{\text{restricted}} - Q_{\text{uncontrolled}}$$

$$Q_{\text{max}} = 203.31 \text{ L/s} - 23.83 \text{ L/s}$$

$$Q_{\text{max}} = \mathbf{179.48 \text{ L/s}}$$

Surface flows in excess of the site's allowable release rate will be stored on site in strategic surface storage areas or oversized underground pipes and gradually released into the minor system to respect the site's allowable release rate. The maximum surface retention depth located within the developed areas will be limited to 300mm during a 1:100-year event as show on the ponding plan located in **Appendix D** and grading plans located in **Appendix E**. Overland flow routes will be provided in the grading to permit emergency overland flow.

The modified rational method was used to evaluate the on-site stormwater management. There are two uncontrolled areas on this site. The flows are calculated above. Therefore, the total restricted flow rate through the minor system will be the design flow rate of **179.48 l/s**. This will be achieved by the used of Inlet Control Devices (ICD's) placed in all on-site catchbasins. A summary of the ICD's, their corresponding storage requirements, storage availability, and associated drainage areas has been provided below.

DRAINAGE AREA	ICD RESTRICTED FLOW (L/s)	100 YEAR STORAGE REQUIRED (m ³)	SURFACE STORAGE PROVIDED (m ³)	SUB-SURFACE STORAGE PROVIDED (m ³)	100yr OVERFLOW (m ³)
S20A	15.00	18.68	20.59	0.00	0.00
S10	33.00	21.75	0.32	0.00	21.43*
S4	26.00	48.04	1.16	0.00	46.88
S20B	10.00	5.45	1.41	0.00	4.04
FUT COM	30.69	29.79	12.00	0.00	17.79
S5	25.00	78.31	1.32	0.00	76.99
S6	50.00	138.36	9.71	0.00	128.65
R6	20.00	163.78	4.38	6.98	152.42*
TOTAL	179.00				173.85

*Overflow only during major storm events, directed to Delphinium Crescent and Barrett Farm Drive with no negative impact on downstream storm sewer system

4.5.3 2 Year Ponding

A review of the 2-year ponding has been completed using the modified rational method. A minimum Tc of 10min has been used. Where volumes are calculated as a negative value, 0.0m3 has been shown. A summary of each drainage area has been provided below.

DRAINAGE AREA	Total 2-Year Ponding Volume (m3)	Comment
S4	0.0	-
S5	0.0	-
S6	0.75	Negligible volume of ponding during 2-year event
S10	0.0	-
S20A	0.22	Negligible volume of ponding during 2-year event
S20B	0.0	-
R6	8.98	This area is controlled at RYCB7, and there is 6.98m3 of sub- surface storage provided in this area. The required ponding is provided underground, not on the street. A 50% reduction to the release rate was considered for this area.

Based on the above, there will be no surface ponding in the 2-year event.

4.5.4 100 year + 20% Stress Test

A cursory review of the 100yr event + 20% has been performed using the modified rational method. The Peak flow from each area during a 100-year event has been increased by 20%. The calculations have been included in **Appendix D**.

A summary of the require storage volumes, and overflow balances is provided below.

DRAINAGE AREA	ICD RESTRICTED FLOW (L/s)	100yr20 STORAGE REQUIRED (m ³)	SURFACE STORAGE PROVIDED (m ³)	100yr20 OVERFLOW (m ³)
S20A	15.00	25.11	20.59	4.52
S10	33.00	34.18	0.32	33.86*
S4	26.00	69.58	1.16	68.42

FUT COM	30.69	40.90	12.00	28.90
S20B	10.00	7.26	1.41	5.85
S5	25.00	111.93	1.32	110.61
S6	50.00	195.06	9.71	185.35
R6	20.00	230.61	4.38	219.25*
TOTAL	179.00			253.11

*Overflow from S10 to Barrett Farm Drive, and from R6 to Delphinium Crescent.

DRAINAGE AREA	100yr20 OVERFLOW (m ³)	Time of Concentration	100yr20 OVERFLOW (l/s)	DEPTH (m)
S20A	4.52	15.00	5.02	0.02
S10	33.86*	9.00	62.70*	0.05
FUT COM	28.90	14.00	34.41	0.00
S4	68.42	14.00	81.45	0.05
S20B	5.85	6.00	16.24	0.03
S5	110.61	16.00	115.22	0.06
S6	185.35	18.00	171.62	0.05
R6	219.25*	26.00	140.55*	0.09
TOTAL	253.11		203.25	

As noted above, the overland flow from the rear yards (R6) is directed to Barrett Lands Phase 3 lands at Delphinium Crescent to the West. The volume of overflow is 253.11m³. Based on a Tc of 26minutes, this volume can be reverse calculated to 203.25 L/s. Channel cross section was used to determine the depth of flow for each area. Refer to calculation sheet in **Appendix D**.

The stress test overflow from S10 will follow the intended overflow route as identified in the Phase 3 grading design drawings. The volume of overflow is 33.86m³. Based on the Tc of 9minutes, this volume can be reverse calculated to 62.70 L/s. Channel cross section was used to determine the depth of flow for each area. Refer to calculation sheet in **Appendix D**.

4.5.5 Open-Channel Drainage Corridor

A summary of the various channel depths of flow is provided below. The cross-sections used for the calculations are shown on drawing 011 as section A-A and section B-B. A V-shape ditch was assumed for the open-channel drainage corridor, with a 22% side slope. Refer to calculation sheet in Appendix D.

POND ID	100yr OVERFLOW (L/s)	100yr20 OVERFLOW (L/s)	100yr Depth (m)	100yr20 Depth (m)
S4	55.81	81.45	0.13	0.14
S10	39.68	62.70	0.11	0.13
S20	0	5.02	0.00	0.04
S20A	11.22	16.24	0.07	0.08
S5	80.20	115.22	0.14	0.16
S6 (Open-Channel)	119.12	171.62	0.02	0.06
R6	97.70	140.55	0.09	0.10

4.6 Storm Hydraulic Grade Line

The Barrett Lands Phase 3 report indicates that the 100-year hydraulic grade line (HGL) in Bulkhead 11307N **93.68**, refer to **Appendix D** for the excerpt from the Barrett Lands Phase 3 HGL analysis. The HGL has been extended through the subject site have been calculated as follows:

LOCATION	MH #	USF ELEV (M)	STORM HGL (M)	FREEBOARD (M)
Unit 1-2	MH10	99.36	98.410	0.950
Unit 3	MH 9	99.53	98.460	1.070
Unit 4-18;23-27	MH 8	99.98	98.460	1.520
Unit 19-22;28-37	MH 5	100.43	99.020	1.410
Unit 38-46	MH 4	100.51	99.150	1.360
Unit 47-50	MH 2	100.65	99.340	1.310

All underside of footing elevations have been designed to provide a minimum of 300mm separation between the greater of governing pipe obvert or governing HGL. A copy of the storm HGL analysis for Block 178 is provided in **Appendix D**.

5 SOURCE CONTROLS

5.1 General

On site level or source control management of runoff will be provided to provide quality control for the subject lands. Such controls or mitigative measures are proposed for the development not only for final development but also during construction and build out. Some of these measures are:

- flat lot grading;
- split lot drainage;
- Roof-leaders to vegetated areas;
- vegetation planting; and
- groundwater recharge.

5.2 Lot Grading

There is an elevation difference of approximately 2m from southwest to northeast in Block 178. In accordance with local municipal standards, the parking lots will be graded northeast between 1.5% and 5.0%. Most landscaped area drainage will be directed into a swale drainage system and connects to the storm sewer system. Typically, swales will have slopes larger than 1.5% with subdrains. Copies of the grading plans have been included in **Appendix E**.

5.3 Roof Leaders

This development will consist of stacked homes and apartments. It is proposed that roof leaders from these units be constructed such that runoff is directed to grass areas adjacent to the units. This will promote water quality treatment through settling, absorption, filtration and infiltration and a slow-release rate to the conveyance network.

5.4 Vegetation

As with most subdivision agreements, the developer will be required to complete a vegetation and planting program. Vegetation throughout the development including planting along roadsides and within public parks provides opportunities to re-create lost natural habitat.

6 CONVEYANCE CONTROLS

6.1 General

Besides source controls, the development also proposes to use several conveyance control measures to improve runoff quality. These will include:

- flat vegetated swales;
- catchbasin and maintenance hole sumps; and
- pervious rear yard drainage.

6.2 Flat Vegetated Swales

The development will make use of relatively flat vegetated swales where possible to encourage infiltration and runoff treatment.

6.3 Catchbasins

All catchbasins within the development, either rear yard or street, will be constructed with minimum 600 mm deep sumps. These sumps trap pollutants, sand, grit and debris which can be mechanically removed prior to being flushed into the minor pipe system. Both rear yard and street catchbasins will be fabricated to OPSD 705.010 or 705.020. All storm sewer maintenance holes servicing local sewers less than 900 mm diameter shall be constructed with a 300 mm sump as per City standards.

6.4 Pervious Landscaped Area Drainage

Some of the landscaped area swales make use of a filter wrapped perforated drainage pipe constructed below the rear yard swale. This perforated system is designed to provide some ground water recharge and generally reduce both volumetric and pollutant loadings that enter the minor pipe system.

7 SEDIMENT AND EROSION CONTROL PLAN

7.1 General

During construction, existing stream and conveyance systems can be exposed to significant sediment loadings. Although construction is only a temporary situation, it is proposed to introduce a number of mitigative construction techniques to reduce unnecessary construction sediment loadings. These will include:

- groundwater in trench will be pumped into a filter mechanism prior to release to the environment;
- bulkhead barriers will be installed at the nearest downstream manhole in each sewer which connects to an existing downstream sewer;
- seepage barriers will be constructed in any temporary drainage ditches; and
- silt sacks will remain on open surface structure such as manholes and catchbasins until these structures are commissioned and put into use.

7.2 Trench Dewatering

During construction of municipal services, any trench dewatering using pumps will be discharged into a filter trap made up of geotextile filters and straw bales similar in design to the OPSD 219.240 Dewatering Trap. These will be constructed in a bowl shape with the fabric forming the bottom and the straw bales forming the sides. Any pumped groundwater will be filtered prior to release to the existing surface runoff. The contractor will inspect and maintain the filters as needed including sediment removal and disposal and material replacement as needed.

7.3 Bulkhead Barriers

At the first manhole constructed immediately upstream of an existing sewer, a ½ diameter bulkhead will be constructed over the lower half of the outletting sewer. This bulkhead will trap any sediment carrying flows, thus preventing any construction –related contamination of existing sewers. The bulkheads will be inspected and maintained including periodic sediment removal as needed.

7.4 Seepage Barriers

These barriers will consist of both the Light Duty Straw Bale Barrier as per OPSD 219.100 or the Light Duty Silt Fence Barrier as per OPSD 219.110 and will be installed in accordance with the sediment and erosion control drawing. The barriers are typically made of layers of straw bales or geotextile fabric staked in place. All seepage barriers will be inspected and maintained as needed.

7.5 Surface Structure Filters

All catchbasins, and to a lesser degree manholes, convey surface water to sewers. However, until the surrounding surface has been completed these structures will be covered to prevent sediment from entering the minor storm sewer system. Until rear yards are sodded or until streets are asphalted and curbed, all catchbasins and manholes will be equipped with geotextile filter socks. These will stay in place and be maintained during construction and build until it is appropriate to remove them.

7.6 Stockpile Management

During construction of any development similar to that being proposed both imported and native soils are stockpiled. Mitigative measures and proper management to prevent these materials entering the sewer systems is needed.

During construction of the deeper municipal services, water, sewers and service connections, imported granular bedding materials are temporarily stockpiled on site. These materials are however quickly used up and generally before any catchbasins are installed. Street catchbasins are installed at the time of roadway construction and rearyard catchbasins are usually installed after base course asphalt is placed.

Contamination of the environment as a result of stockpiling of imported construction materials is generally not a concern since these materials are quickly used and the mitigative measures stated previously, especially the use of filter fabric in catchbasins and manholes help to manage these concerns.

The roadway granular materials are not stockpiled on site. They are immediately placed in the roadway and have little opportunity of contamination. Lot grading sometimes generates stockpiles of native materials. However, this is only a temporary event since the materials are quickly moved off site.

The construction of this development will involve a substantial rock blasting, breaking and crushing operation. Given the existing topography, a substantial cut and fill operation is required in order to construction a development that meets City Standards. As part of this operation, materials will be manipulated onsite, and provided the sediment and erosion control measures are in place, are generally inconsequential to the surrounding environment.

8 ROADS AND NOISE ATTENUATION

Vehicular access to Block 178 is provided by two private entrances from Barrett Farm Drive.

There are no sidewalks or pathways proposed within the development. Pedestrian access to the site will be via the private roadway.

The site has been designed in order to provide curbside municipal waste disposal.

There are no bus routes proposed within Block 178.

There are no collector streets or nearby noise sources that would trigger an environmental noise assessment.

8.1 Aircraft Sound Levels

As stated in Section 2.1, the site is within the Airport Vicinity Development Zone (AVDZ), the limit of the AVCZ is shown on Figure 2. The site however is outside of the 25 NEF/NEP contour line so the building components and ventilation requirements of Part 6 Prescribed Measures for Aircraft Noise of the Guidelines do not apply. A warning clause is required for the residential units inside the AVDZ.

Warning clause for aircraft noise is as follows:

“Purchasers/tenants are advised that due to the proximity of the airport, noise from the airport and individual aircraft may at times interfere with outdoor or indoor activities”.

9 SOILS

Golder Associates Ltd. was retained to prepare a geotechnical investigation for the proposed mixed use development for the Barrett Lands Phase 3. The objectives of the investigation were to prepare a report to:

- Determine the subsoil and groundwater conditions at the site by means of test pits and boreholes and;
- To provide geotechnical recommendations pertaining to design of the proposed development including construction considerations.

The geotechnical report 20442530-100 was prepared by Golder Associates Ltd. in February 2022. The report contains recommendations which include but are not limited to the following:

- The maximum permissible grade raise is 3.5m
- In areas where finished grade exceeds grade raise limits, geotechnical reviews are required
- Fill placed below the foundations to meet OPSS Granular 'A' or Granular 'B' Type II placed in 300 mm lifts compacted to 98% SPMDD.
- Fill for roads to be suitable native material in 300mm lifts compared to 95% SPMDD

Pavement Structure:

LOCAL ROAD	THICKNESS
Asphaltic Concrete	90mm
OPSS Granular A Base	150mm
OPSS Granular B Type II Subbase	400mm

- Pipe bedding and cover; bedding to be minimum 150 mm OPSS Granular 'A' up to spring line of pipe. Cover to be 300 mm OPSS A (PUC and concrete pipes) or sand for concrete pipes. Both bedding and cover to be placed in maximum 225 mm lifts compacted to 95% SPMDD.

In general the grading plan for Block 178 adheres to the grade raise constraints noted above. A copy of the grading plans is included in **Appendix E**. The site does not pose any significant grade raise; thus a grading plan review letter is not required for this development.

10 RECOMMENDATIONS

Water, wastewater and stormwater systems required to develop Barrett Lands Block 178 will be designed in accordance with MOE and City of Ottawa's current level of service requirements.

The use of lot level controls, conveyance controls and end of pipe controls outlined in the report will result in effective treatment of surface stormwater runoff from the site. Adherence to the proposed sediment and erosion control plan during construction will minimize harmful impacts on surface water.

Final detail design will be subject to governmental approval prior to construction, including but not limited to the following:

- Block 178 Commence Work Order: City of Ottawa
- ECA for Sewage Works: MOECP Transfer of Review by City of Ottawa
- Block 178 Watermain Approval: City of Ottawa
- Block 178 Commence Work Order (utilities): City of Ottawa

Report prepared by:



Demetrius Yannoulopoulos, P.Eng.
Director

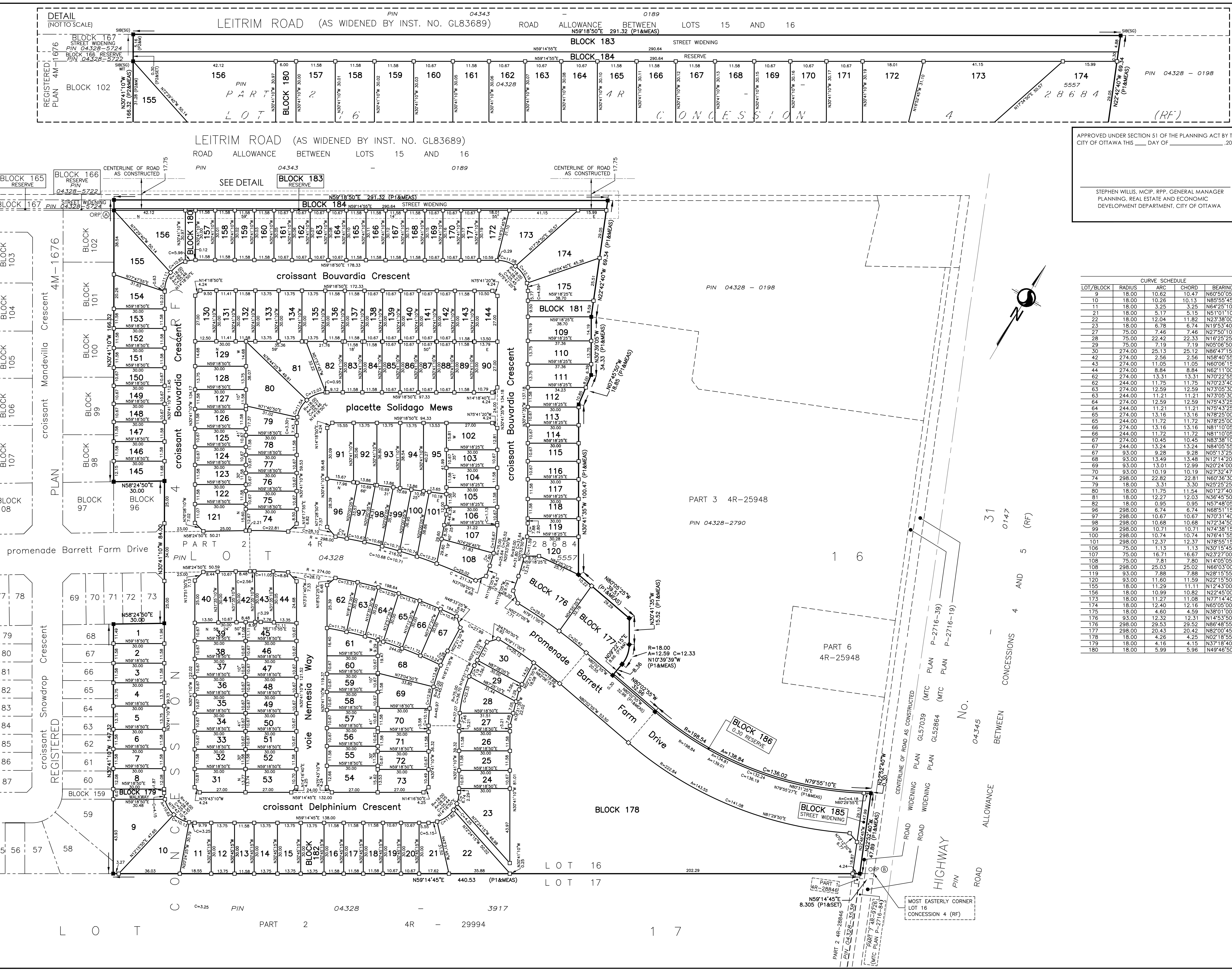
Ryan Magladry, C.E.T.
Project Manager

Anton Chetrar, P.Eng.
Civil Engineer

APPENDIX A

AOV Plan of Subdivision for Barrett Lands Phase 3
Site Plan for Barrett Lands Block 178
135925-001 - General Plan of Services
City of Ottawa Pre-Consultation Meeting Notes

March 16, 2022



PLAN 4M-

I CERTIFY THAT THIS PLAN IS REGISTERED IN THE LAND REGISTRY OFFICE FOR THE LAND TITLES DIVISION OF OTTAWA-CARLETON No.4 AT _____ O'CLOCK ON THE _____ DAY OF _____, 2022 AND ENTERED IN THE PARCEL REGISTER FOR PROPERTY IDENTIFIER _____ AND THE REQUIRED CONSENTS ARE REGISTERED AS PLAN DOCUMENT NUMBER OC-_____.

REPRESENTATIVE FOR LAND REGISTRAR

THIS PLAN COMPRISED OF ALL OF PIN 04328-5557

PLAN OF SUBDIVISION OF
**PART OF LOT 16
CONCESSION 4 (RIEADU FRONT)**
(GEOGRAPHIC TOWNSHIP OF GLOUCESTER)
CITY OF OTTAWA

Scale 1:1000

METRIC CONVERSION

DISTANCES AND COORDINATES SHOWN ON THIS PLAN ARE IN METRES AND CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048

GRID SCALE CONVERSION

DISTANCES ARE GROUND AND CAN BE CONVERTED TO GRID BY MULTIPLYING BY THE COMBINED SCALE FACTOR OF 0.999946.

BEARING NOTE

BEARINGS ARE GRID, DERIVED FROM CAN-NET VRS NETWORK GPS OBSERVATIONS ON NCC HORIZONTAL CONTROL MONUMENTS 19773035 AND 19680191, CENTRAL MERIDIAN, 76° 30' WEST LONGITUDE MTM ZONE 9, NAD83 (ORIGINAL).

19773035 N:5006060.42 E:324888.04
19680191 N:5033564.26 E:388064.94

OBSERVED REFERENCE POINTS DERIVED FROM THE CAN-NET VRS NETWORK GPS OBSERVATIONS ON NCC HORIZONTAL CONTROL MONUMENTS 19773035 AND 19680191, CENTRAL MERIDIAN, 76° 30' WEST LONGITUDE MTM ZONE 9, NAD83 (ORIGINAL). COORDINATES TO URBAN ACCURACY PER SEC. 14(2) OF O. REG. 216/10

ORP ID	NORTHING	EASTING
Ⓐ	5021294.73	374944.42
Ⓑ	5021182.72	375523.14

COORDINATES CANNOT, IN THEMSELVES, BE USED TO RE-ESTABLISH CORNERS OR BOUNDARIES SHOWN ON THIS PLAN.

LEGEND

□	DENOTES	FOUND MONUMENTS
■	SET MONUMENTS (B)	UNLESS OTHERWISE STATED
IB	IRON BAR	ROUND IRON BAR
SIB	STANDARD IRON BAR	SHORT STANDARD IRON BAR
SSB	CUT CROSS	CONCRETE PIN
CP	WITNESS	PROPERTY IDENTIFICATION NUMBER
WIT	PIN	MEASURED
PH	PROPORTIONED	ORIGIN UNKNOW
PROP	ORIGIN UNKNOW	STANTEC GEOMATICS LTD.
OU	STANTEC GEOMATICS LTD.	OBSERVED REFERENCE POINT
SO	OBSERVED REFERENCE POINT	PLAN 4R-28844
ORP	REGISTERED PLAN 4M-1676	
P1		
P2		

OWNER'S CERTIFICATE ALL OF PIN 04328-5557

THIS IS TO CERTIFY THAT:
1. LOTS 1 TO 175, BOTH INCLUSIVE, BLOCKS 176, 177 and 178, THE STREETS, NAMELY promenade Barrett Farm Drive, croissant Boulevard Crescent, croissant Delphinium Crescent, voie Nemesis Way and placette Soldado Mews WALKWAYS, NAMELY BLOCKS 179 and 180, ACCESS BLOCKS, NAMELY BLOCKS 181 and 182, RESERVES, NAMELY BLOCKS 183 and 186 and STREET WIDENINGS, NAMELY BLOCKS 184 and 185 HAVE BEEN LAID OUT IN ACCORDANCE WITH OUR INSTRUCTIONS.

2. THE STREETS AND STREET WIDENINGS AND LANES ARE HEREBY DEDICATED TO THE CITY OF OTTAWA AS PUBLIC HIGHWAYS.

DATE _____ CHRIS TAGGART, PRESIDENT
FINLAY GREEK PROPERTIES (NORTH) LTD.
TARTAN HOMES (NORTH LEITRIM) INC.
TARTAN LAND (NORTH LEITRIM) INC.
I HAVE THE AUTHORITY TO BIND THE CORPORATION

SURVEYOR'S CERTIFICATE
I CERTIFY THAT:
1. THIS SURVEY AND PLAN ARE CORRECT AND IN ACCORDANCE WITH THE SURVEYS ACT, THE SURVEYORS ACT AND THE LAND TITLES ACT AND THE REGULATIONS MADE UNDER THEM.
THE SURVEY WAS COMPLETED ON THE DAY OF _____, 2022

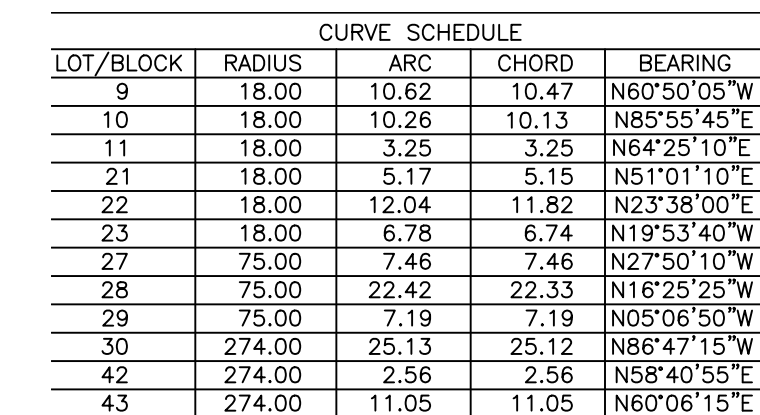
DATE _____ FRANCIS LAU
ONTARIO LAND SURVEYOR

Stantec Geomatics Ltd.
CANADA LAND SURVEYORS
ONTARIO LAND SURVEYORS
1331 CYDNE AVENUE, SUITE 300
OTTAWA, ONTARIO K2C 1G4
TEL: 613.722.4420 FAX: 613.722.2799
stantec.com

DRAWN: ME CHECKED: * P.M.: FL FIELD: * PROJECT No.: 161614242-132

APPROVED UNDER SECTION 51 OF THE PLANNING ACT BY THE CITY OF OTTAWA THIS _____ DAY OF _____, 20____.

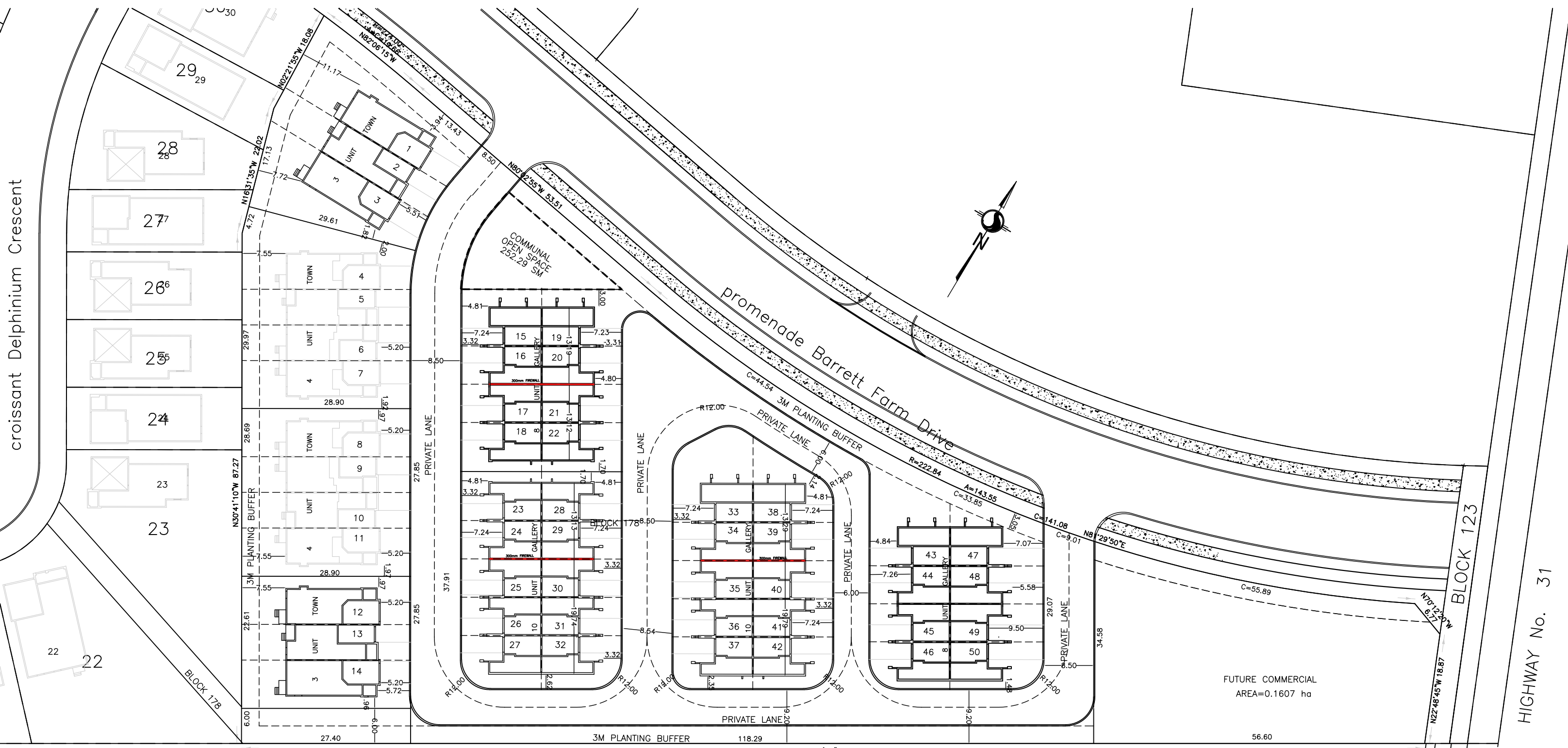
STEPHEN WILLIS, MCIP, RPP, GENERAL MANAGER
PLANNING, REAL ESTATE AND ECONOMIC
DEVELOPMENT DEPARTMENT, CITY OF OTTAWA



CURVE SCHEDULE

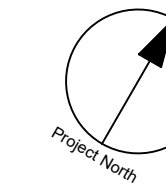
LOT/BLOCK	RADIUS	ARC	CHORD	BEARING
9	18.00	10.62	10.47	N60°50'05"W
10	18.00	10.28	10.13	N85°53'45"E
11	18.00	3.25	3.25	N64°25'10"E
21	18.00	5.17	5.15	N51°01'10"E
22	18.00	12.04	11.82	N23°38'00"E
23	18.00	6.78	6.74	N19°53'40"W
27	75.00	7.46	7.46	N27°50'10"W
28	75.00	22.42	22.33	N16°25'25"W
29	75.00	7.19	7.19	N05°06'50"W
30	274.00	25.13	25.12	N86°47'15"W
42	274.00	2.56	2.56	N58°43'55"E
43	274.00	11.05	11.05	N60°06'15"E
44	274.00	8.84	8.84	N62°11'00"E
62	274.00	13.31	13.31	N70°22'55"E
62	244.00	11.75	11.75	N70°23'40"E
63	274.00	12.59	12.59	N73°05'30"E
63	244.00	11.21	11.21	N73°05'30"E
64	274.00	12.59	12.59	N75°43'25"E
64	244.00	11.21	11.21	N75°43'25"E
65	274.00	13.16	13.16	N78°25'00"E
65	244.00	11.72	11.72	N78°25'00"E
66	274.00	13.16	13.16	N81°10'05"E
66	244.00	11.72	11.72	N81°10'05"E
67	274.00	10.45	10.45	N83°38'10"E
67	244.00	13.24	13.24	N84°05'55"E
68	93.00	9.28	9.28	N05°13'25"W
68	93.00	13.49	13.48	N12°14'20"W
69	93.00	13.01	12.99	N20°24'00"W
70	93.00	10.19	10.19	N27°32'47"W
74	298.00	22.82	22.81	N60°36'30"E
79	18.00	3.31	3.30	N25°25'25"W
80	18.00	11.75	11.54	N01°27'40"W
81	18.00	12.27	12.03	N36°45'50"E
82	18.00	0.95	0.95	N57°48'05"E
96	298.00	6.74	6.74	N68°51'15"E
97	298.00	10.67	10.67	N70°31'40"E
98	298.00	10.68	10.68	N72°34'50"E
99	298.00	10.71	10.71	N74°38'15"E
100	298.00	10.74	10.74	N76°41'55"E
101	298.00	12.37	12.37	N78°55'15"E
106	75.00	1.13	1.13	N30°15'45"W
107	75.00	16.71	16.67	N23°27'00"W
108	75.00	7.81	7.80	N14°05'05"W
108	298.00	25.03	25.02	N66°03'00"E
119	93.00	7.88	7.88	N28°15'55"W
120	93.00	11.60	11.59	N22°15'50"W
155	18.00	11.29	11.11	N12°43'00"W
156	18.00	10.99	10.82	N22°45'00"E
173	18.00	11.27	11.08	N77°14'40"E
174	18.00	12.40	12.16	N65°05'00"W
175	18.00	4.60	4.59	N38°01'00"W
176	93.00	12.32	12.31	N14°53'50"W
176	298.00	29.53	29.52	N86°48'55"W
177	298.00	20.43	20.42	N82°00'45"W
178	18.00	4.26	4.25	N02°18'55"W
179	18.00	4.16	4.15	N37°18'40"W
180	18.00	5.99	5.96	N49°46'50"E

croissant Delphinium Crescent



LOT 17
 CONCESSION 4 (RF)

DAPT 2 AD 20004



CLIENT

BARRETT CO-TENANCY

COPYRIGHT

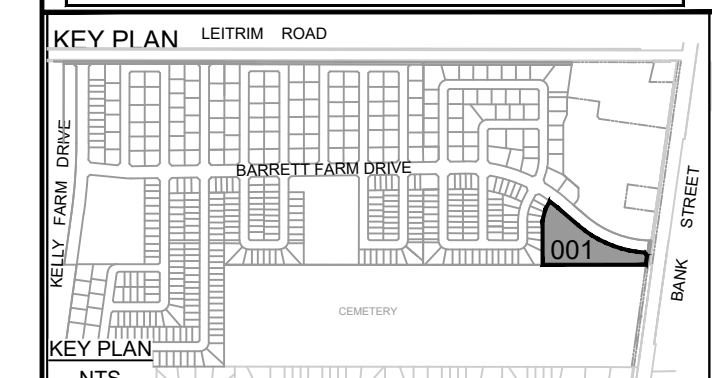
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ISSUES

No.	DESCRIPTION	DATE
1	SUBMISSION NO. 1 FOR CITY REVIEW	2022-05-10
2	SUBMISSION NO. 2 FOR CITY REVIEW	2022-11-25
3		
4		
5		
6		
7		
8		

SEE 010, 011, 012 FOR NOTES, LEGEND, CB TABLE, STREET SECTIONS AND DETAILS



CONSULTANTS

SEAL



IBI GROUP
Suite 400 - 333 Preston Street
Ottawa ON K1S 5N4 Canada
tel 613 225 1311 / 613 241 3300 fax 613 225 9868
ibigroup.com

PROJECT

BARRETT BLOCK 178

PROJECT NO:
135925

DRAWN BY:
M.M.

CHECKED BY:
A.C.

PROJECT MGR:
R.M.

APPROVED BY:
J.I.M.

SHEET TITLE

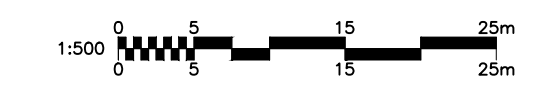
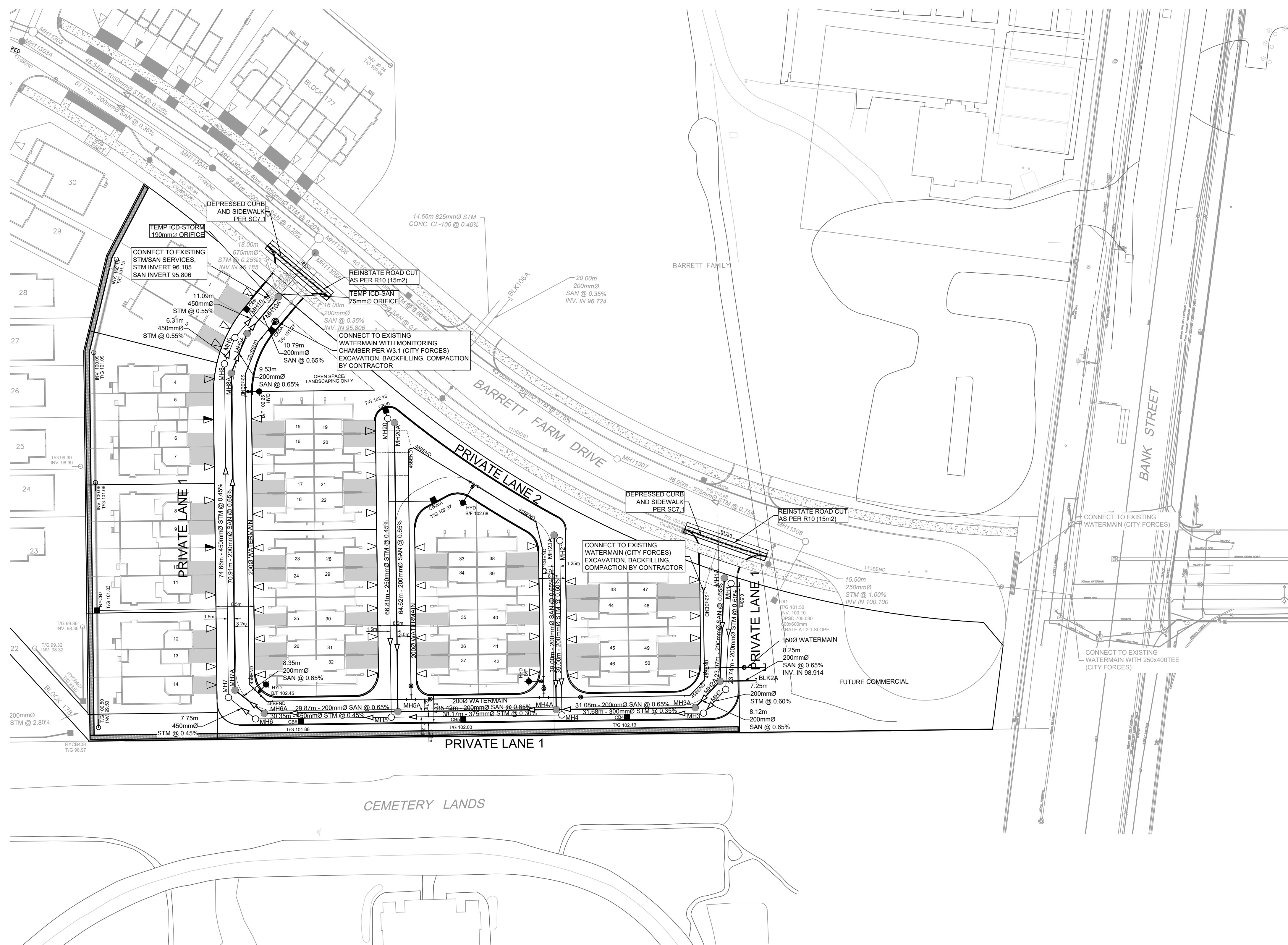
GENERAL PLAN

SHEET NUMBER

001

ISSUE

2



CITY FILE No. D07-12-22-0112
Last Saved: November 23, 2022, by rmmine
Plotted: Thursday, November 24, 2022 11:54:04 AM by Marian Milne

Blk 118, Bank Street at Barrett Farm

Meeting Summary Notes

July 27, 2021, Online Teams Meeting

Revised Aug 18, 2021

Attendees:

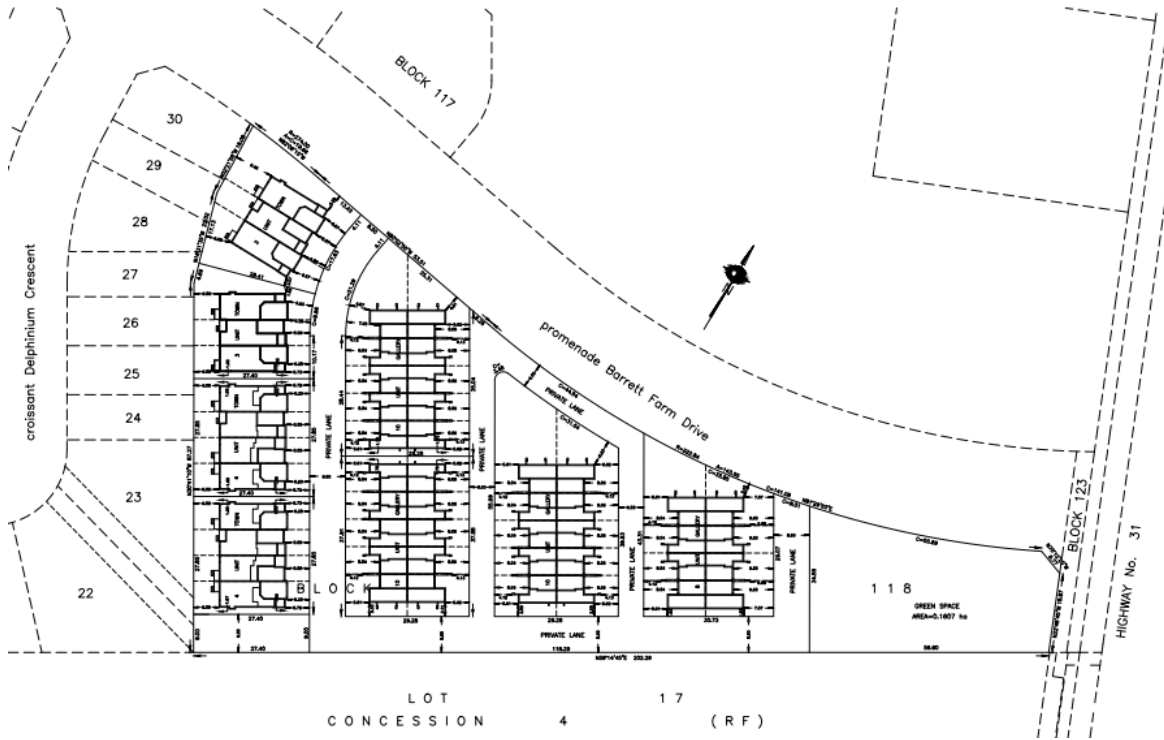
- Melissa Cote, Tartan
- Yvonne Mitchell, Planning Student, City of Ottawa
- Tracey Scaramozzino (File Lead, Planner, City of Ottawa)

Not in Attendance:

- Mark Young, Urban Design
- Burl Walker,
- John Sevigny
- Mark Richardson
- Matthew Hayley
- Mike Giampa
- James Holland, SNation

Issue of Discussion:

- Vacant site, within Plan of subdivision, Block 118
- PUD, 14 townhouse units, 38 back to back town;
- Density of 40units/net ha (only half of CDP recommendation of 80 units/ha). The Developer doesn't have a product that provides the recommended density – as they are no longer producing the Java product.
- Private laneways of 6m and 8.5m for servicing, utilities, and municipal garbage
- Site plan, Plan of Condo and Private Road Naming Applications are req'd
- The subdivision contemplated 90 units in this area and only 52 are being proposed. Therefore transportation/services should not be an issue



1. **Official Plan, Current:** General Urban Area
2. **Official Plan, Draft:** Suburban Transect, no overlays, no designations, Bank St in this area is a minor corridor
 - a. "Recognize this as suburban pattern, but to support the evolution to 15min n'hood"
3. **Leitrim CDP (from 2005):** Mixed Use (intended to be part of core retail along Bank st – smaller parcels to provide n'hood uses; larger retail is focused south at existing commercial plaza
 - a. The CDP is not being converted into a secondary plan in the new OP and will remain in effect.



4. Zoning Information: GM12 - General Mixed Use permits residential (apt, PUD, townhouse etc) and non-residential (animal hospital, bank, community centre etc) Subzone 12 permits additional non-residential uses such as bar, cinema, gas bar, theatre, sports arena.

5. Infrastructure/Servicing (John Sevigny):

- a. Servicing will be reviewed during Phase 3 of the subdivision, which as of July 26, hasn't been submitted.

6. Initial Planning Comments (Tracey Scaramozzino):

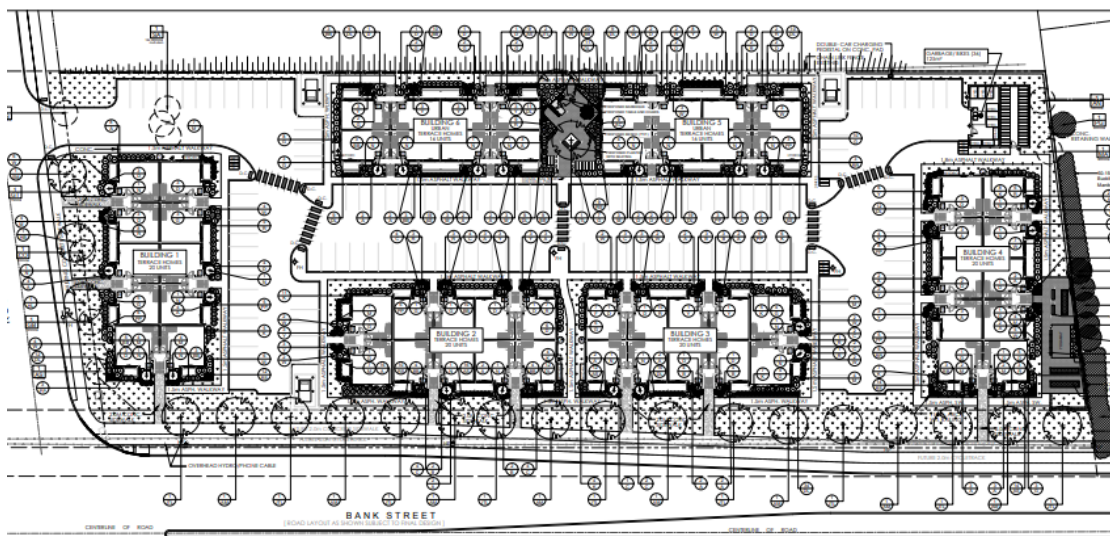
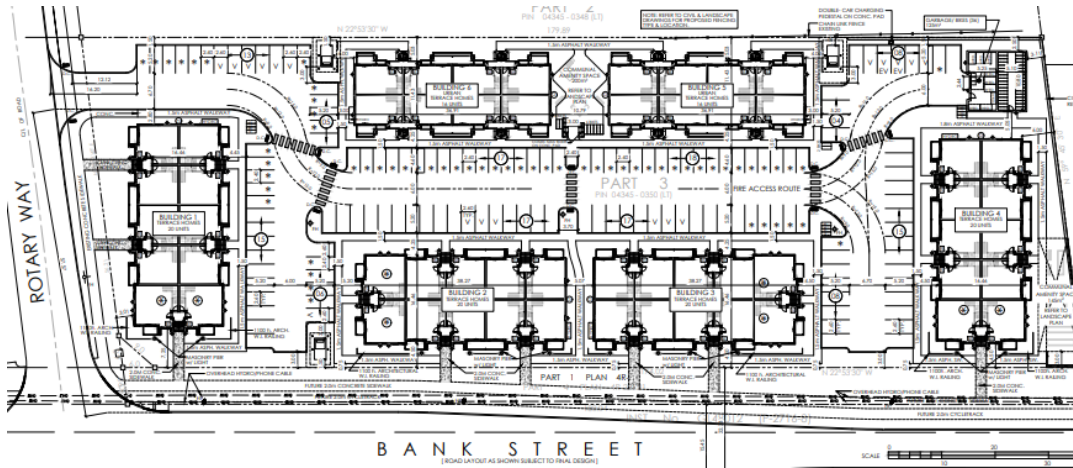
- a. Can density be increased as per the CDP (40u/ha is being provided; 80 u/ha was contemplated in the CDP).
- b. Possibly design open space along Bank St to have sitting area/plantings/soft surfaces, similar to POPS.
- c. Units in general should have higher floor-ceiling hts on ground floor to allow the conversion to commercial units over time.
- d. The 'empty' parcels along Barrett Farm should be nicely designed with trees and perennials and soft surfaces.
- e. The plan of subdivision does not provide guidance for development of this site.
- f. Within Airport Bird Hazard zone
- g. Follow up discussions between Tracey and Melissa:

May 28, 2021, from Melissa: I think there will be a lot of opportunity for trees and other soft landscaping along with a nice sitting area. I will wait to hear what Mark suggests and I'm following up with Tamarack regarding other product type suitable for possible conversion later on.

May 27, 2021 from Tracey: I was also thinking some more about the green spaces on your proposal and am wondering if the green areas fronting Barrett Farm Drive might be nice with a few trees and some perennials - soft surfaces to help with climate change and heat-island effect. The green location on the corner might be nice with a mixture of plantings, sitting areas and soft area - that may or may not be open to the general public like a POPS.....

- h. The almost-approved Glenview project on the east side of Bank St. at Rotary Way has back to back, stacked towns with the ability for future ground floor conversions to commercial (due to higher floor-ceiling hts) and were able to meet the req'd density for 'apts'.
- i. ~~Tracey sent Melissa an example of a design brief, and details from Glenview PUD across the street at Rotary Way that is near approval, — as per images below.~~

Document 7, Elevations showing potential business signage



7. Urban Design Comments (Mark Young):

1. A design brief is required. Please see attached terms of reference.
2. Early consideration needs to be given to the allowance of street trees, both public and private. The proposed private roadway width of 6.0 m combined with a 4.0 m front yard setback, may present a challenge in the provision of trees. This should be addressed.
3. Please provide direction regarding the proposed "Green Space". Is this intended to be public or private?
4. Is any visitor parking proposed? Lay-by parking should be considered for visitors within the private development.
5. What is the purpose of the 9.0 m block abutting the southern property line? Is this for servicing and a walkway? Please advise.

8. Parks (Burl Walker):

- a. Parks issues are being reviewed through the associated subdivision file.

9. Trees (Mark Richardson):

- a. Preserve and protect the healthy trees to create a visual buffer along southern property line.
- b. Tree permit is required prior to any tree removal on site
- c. Submit a TCR with application.

10. Environment (Matthew Hayley): (added August 18, 2021)

- a) They will need to have their TCR address butternut trees (or provide an EIS). Mark R will comment on tree conservation but I would point out that there is an excellent opportunity for tree retention along the southern property line.
- b) Landscaping - OP Section 4.9 has some policies addressing energy conservation through design - in part as ular for this area, I would recommend considering shading along the southern property line adjacent to that lane. This will combat urban heat island and to provide some screening from the adjacent use. Street trees are also important and should be provided.
- c) Integrated Environmental Review (IER) – if they are providing a planning rational the IER can be contained within that document as per the TOR for the Planning Rational, otherwise they should have an IER provided.

11. Conservation Authority (James Holland, South Nation):

- a. All issues are being reviewed through the associated subdivision file.

12. Transportation (Mike Giampa):

- a. Comments are outstanding at this time, likely dealt with during the plan of subdivision.

13. Waste Collection

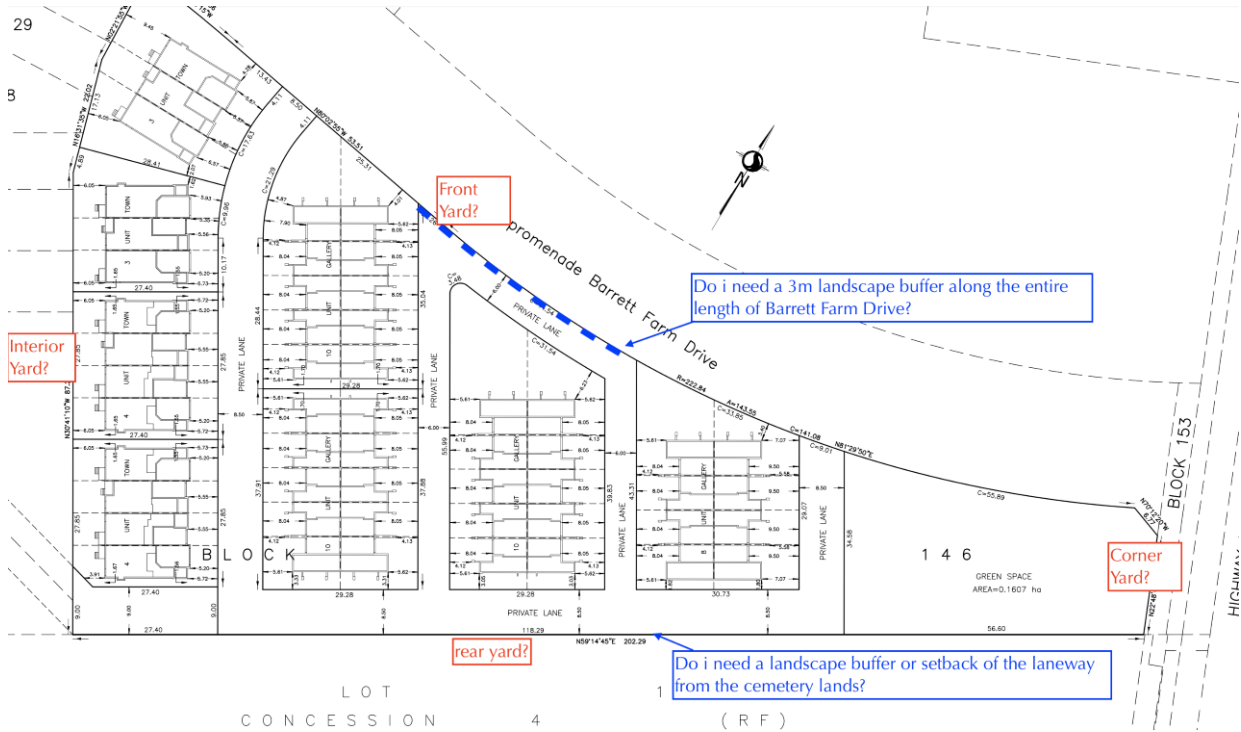
- a. Residential properties will receive City collection on the private streets.
- b. 6m ROW is acceptable for waste collection.

14. General Information

- a. Ensure that all plans and studies are prepared as per City guidelines – as available online...

<https://ottawa.ca/en/city-hall/planning-and-development/information-developers/development-application-review-process/development-application-submission/guide-preparing-studies-and-plans>

Response to August 1, 2021 Questions from Melissa:



1. Front yard? Corner yard? Rear yard? Side yard?

Response: Based on my interpretation of the definitions of each in the zoning bylaw:

- Front yard = Bank Street
- Corner yard = Barrett Farm Drive
- Rear yard = empty residential lot
- Side yard = cemetery lands

2. Do I need a landscape buffer along the entire length of Barret Farm Drive?

Response: Yes, but this buffer can be passed by driveways or roads. The full landscape buffer requirements under the [zoning \(GM12\)](#) are as follows:

(h) Minimum width of landscaped area	(i) abutting a street	3 m
	(ii) abutting a residential or institutional zone	3 m
	(iii) other cases	No minimum

Based on the above requirement for the GM zone, a 3m landscape buffer would be required around the entire site, as it borders on streets, residential and institutional zones.

3. Do I need a landscape buffer or setback of the laneway from the cemetery lands?

Response: As per previous question and Table 187 of the applicable [zoning \(GM12\)](#), a landscape buffer of 3m is required abutting an institutional zone. Regarding setbacks, the interior lot line setback would not apply from the lot line to the laneway but from the proposed townhomes as follows:

(d) Minimum interior side yard setbacks

(iii) For a residential use building

1. For a building equal or lower than 11m in height = 1.2m
2. For a building higher than 11m in height = 3m

APPENDIX B

Water Distribution Model

Boundary Conditions 3100 Leitrim Road

Provided Information

Scenario	Demand	
	L/min	L/s
Average Daily Demand	26	0.44
Maximum Daily Demand	66	1.10
Peak Hour	145	2.42
Fire Flow Demand #1	10,000	166.67
Fire Flow Demand #2	12,000	200.00
Fire Flow Demand #3	15,000	250.00

Location



Results – Existing Conditions

Connection 1 – Barrett Farm Drive

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	154.6	76.7
Peak Hour	144.7	62.5
Max Day plus Fire #1	122.0	28.3
Max Day plus Fire #2	118.1	22.8
Max Day plus Fire #3	110.6	12.1

Ground Elevation = 102.1 m

Connection 2 – Barrett Farm Drive

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	154.6	76.7
Peak Hour	144.7	62.5
Max Day plus Fire #1	123.3	27.7
Max Day plus Fire #2	119.8	22.8
Max Day plus Fire #3	113.1	13.3

Ground Elevation = 103.8 m

Results – SUC Zone Reconfiguration

Connection 1 – Barrett Farm Drive

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.5	66.6
Peak Hour	145.8	64.2
Max Day plus Fire #1	135.8	49.9
Max Day plus Fire #2*	130.2	41.9
Max Day plus Fire #3	124.5	33.8

Ground Elevation = 102.1 m

*Interpolated

Connection 2 – Barrett Farm Drive

Demand Scenario	Head (m)	Pressure ¹ (psi)
Maximum HGL	147.5	65.5
Peak Hour	145.8	63.0
Max Day plus Fire #1	137.2	50.8
Max Day plus Fire #2*	132.3	43.8
Max Day plus Fire #3	127.4	36.8

Ground Elevation = 103.8 m

*Interpolated

Disclaimer

The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.



IBI GROUP
 333 PRESTON STREET
 OTTAWA, ON
 K1S 5N4

WATERMAIN DEMAND CALCULATION SHEET

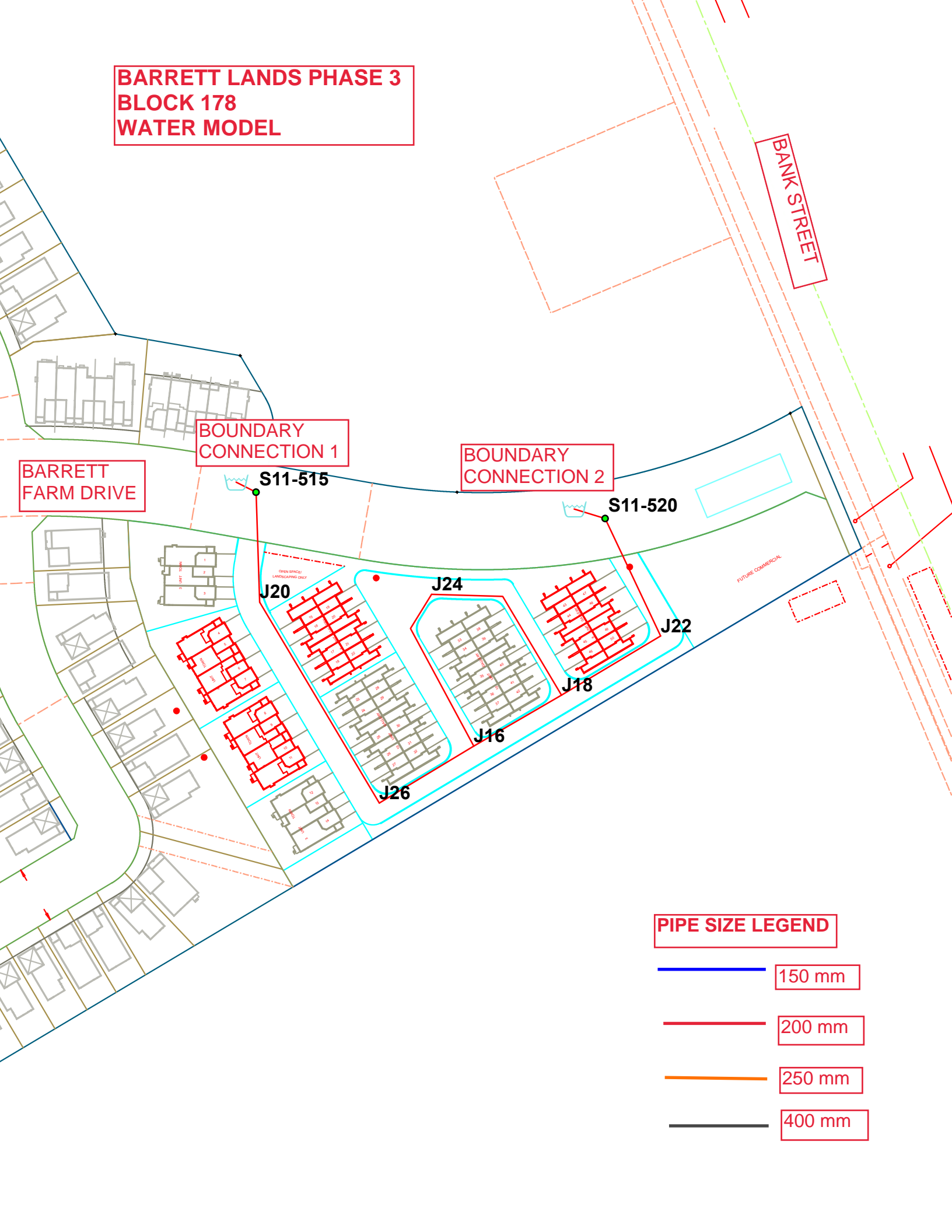
PROJECT : **BLOCK 178**
 LOCATION : **CITY OF OTTAWA**
 DEVELOPER : **TAGGART**

FILE: 135925
 DATE PRINTED: 22-Nov-22
 DESIGN: AC
 PAGE : 1 OF 1

NODE	RESIDENTIAL				NON-RESIDENTIAL			AVERAGE DAILY DEMAND (l/s)			MAXIMUM DAILY DEMAND (l/s)			MAXIMUM HOURLY DEMAND (l/s)			FIRE DEMAND (l/min)
	UNITS			POP'N	INDTRL (ha.)	COMM. (ha.)	INST. (ha.)	Res.	Non-res.	Total	Res.	Non-res.	Total	Res.	Non-res.	Total	
	SF	SD & TH	OTHER														
Findlay Creek																	
J20		13		35				0.11	0.00	0.11	0.28	0.00	0.28	0.63	0.00	0.63	
J26		10		27				0.09	0.00	0.09	0.22	0.00	0.22	0.48	0.00	0.48	
J16		8		22				0.07	0.00	0.07	0.18	0.00	0.18	0.39	0.00	0.39	
J18		8		22				0.07	0.00	0.07	0.18	0.00	0.18	0.39	0.00	0.39	
J22		7		19				0.06	0.00	0.06	0.15	0.00	0.15	0.34	0.00	0.34	
J24		4		11				0.04	0.00	0.04	0.09	0.00	0.09	0.19	0.00	0.19	
TOTALS		50		135						0.44			1.10			2.42	

ASSUMPTIONS					
RESIDENTIAL DENSITIES		AVG. DAILY DEMAND		MAX. HOURLY DEMAND	
- Single Family (SF)	3.4 p / p / u	- Residential	280 l / cap / day	- Residential	1,540 l / cap / day
- Semi Detached (SD) & Townhouse (TH)	2.7 p / p / u	- ICI	50,000 l / ha / day	- ICI	135,000 l / ha / day
		MAX. DAILY DEMAND		FIRE FLOW	
- Apartment (APT)	1.8 p / p / u	- Residential	700 l / cap / day	- SF, SD, TH & ST	10,000 l / min
- Other	66 u / p / ha	- ICI	75,000 l / ha / day	- ICI	13,000 l / min

**BARRETT LANDS PHASE 3
BLOCK 178
WATER MODEL**



BOUNDARY CONNECTION 1

BOUNDARY CONNECTION 2

BARRETT FARM DRIVE

BANK STREET

S11-515

S11-520

J20

J24

J22

J18

J16

J26

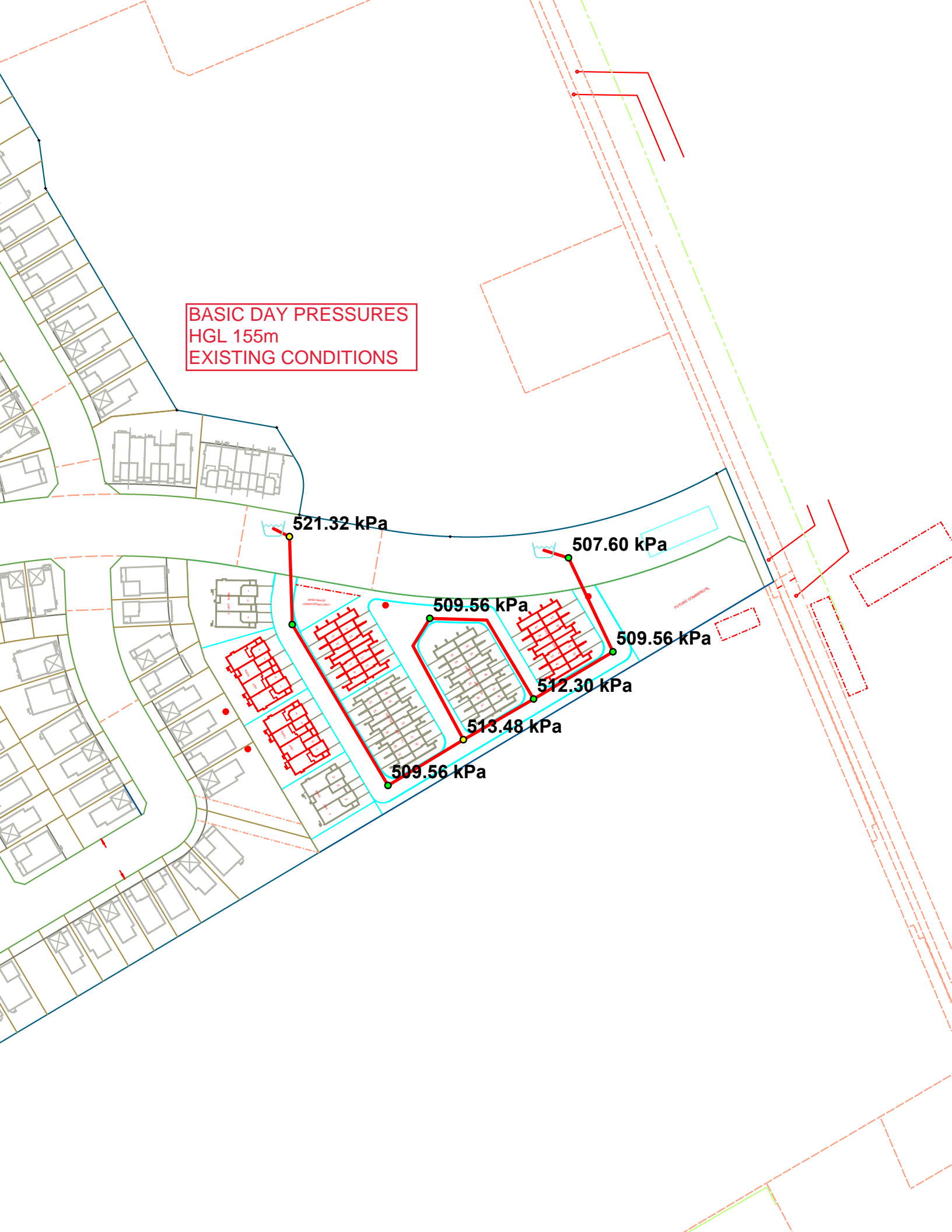
OPEN SPACE / LANDSCAPING ONLY

FUTURE COMMERCIAL

PIPE SIZE LEGEND

- 150 mm
- 200 mm
- 250 mm
- 400 mm

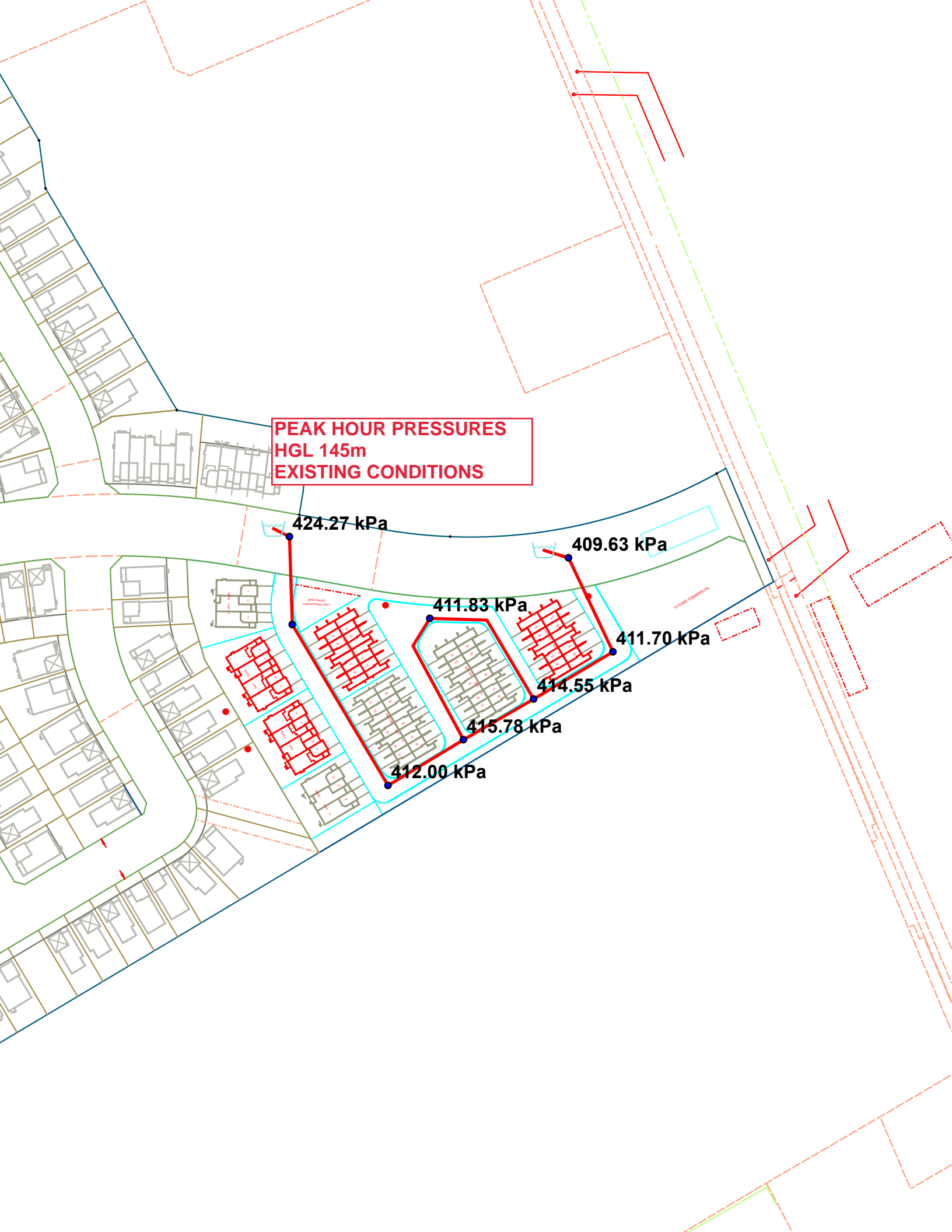
BASIC DAY PRESSURES
HGL 155m
EXISTING CONDITIONS



Basic Day HGL - Junction Report

		ID	Demand (L/s)	Elevation (m)	Head (m)	Pressure (kPa)	Water Age (hrs)
1	<input type="checkbox"/>	J16	0.07	102.20	154.60	513.48	0.00
2	<input type="checkbox"/>	J18	0.07	102.32	154.60	512.30	0.00
3	<input type="checkbox"/>	J20	0.11	102.60	154.60	509.56	0.00
4	<input type="checkbox"/>	J22	0.06	102.60	154.60	509.56	0.00
5	<input type="checkbox"/>	J24	0.04	102.60	154.60	509.56	0.00
6	<input type="checkbox"/>	J26	0.09	102.60	154.60	509.56	0.00
7	<input type="checkbox"/>	S11-515	0.00	101.40	154.60	521.32	0.00
8	<input type="checkbox"/>	S11-520	0.00	102.80	154.60	507.60	0.00

**PEAK HOUR PRESSURES
HGL 145m
EXISTING CONDITIONS**



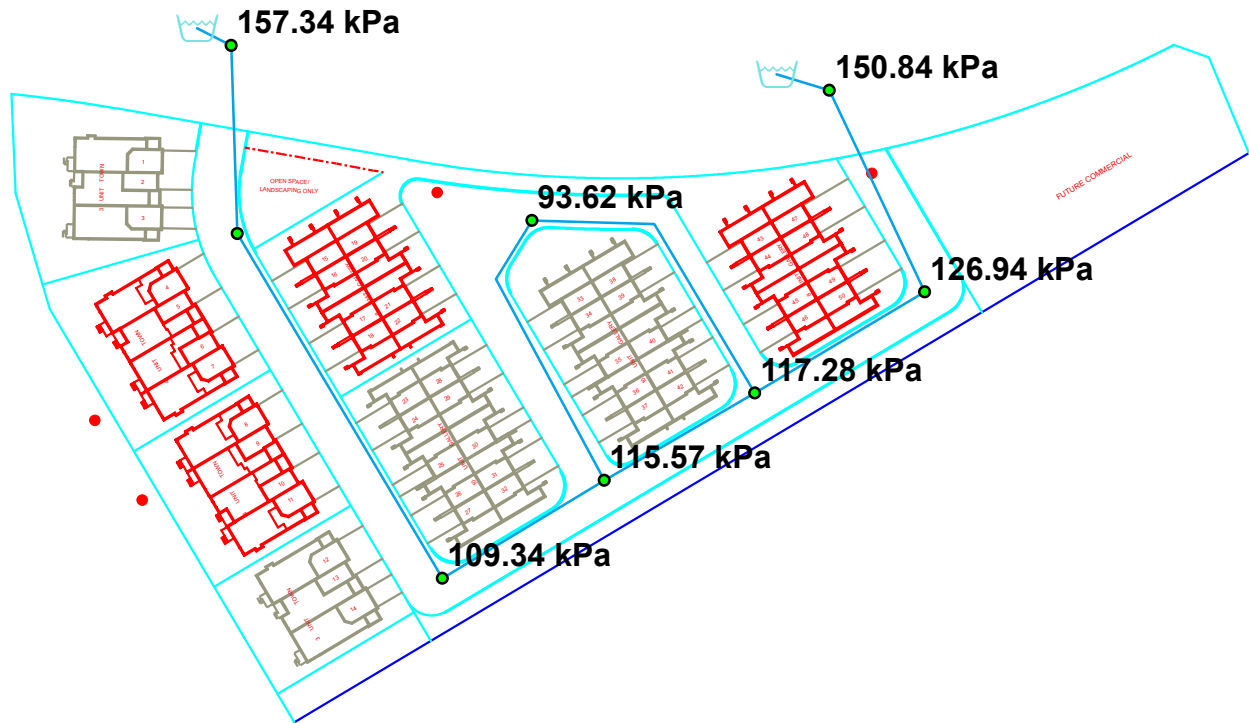
Peak Hour HGL - Junction Report

		ID	Demand (L/s)	Elevation (m)	Head (m)	Pressure (kPa)	Water Age (hrs)
1	<input type="checkbox"/>	J16	0.39	102.20	144.63	415.78	0.00
2	<input type="checkbox"/>	J18	0.39	102.32	144.62	414.55	0.00
3	<input type="checkbox"/>	J20	0.63	102.60	144.68	412.33	0.00
4	<input type="checkbox"/>	J22	0.40	102.60	144.61	411.70	0.00
5	<input type="checkbox"/>	J24	0.19	102.60	144.63	411.83	0.00
6	<input type="checkbox"/>	J26	0.48	102.60	144.64	412.00	0.00
7	<input type="checkbox"/>	S11-515	0.00	101.40	144.70	424.27	0.00
8	<input type="checkbox"/>	S11-520	0.00	102.80	144.60	409.63	0.00

Peak Hour HGL - Pipe Report

ID	From Node	To Node	Length (m)	Diameter (mm)	Roughness	Flow (L/s)	Velocity (m/s)	Headloss (m)	HL/1000 (m/k-m)	Status	Flow Reversal Count	Water Age (hrs)
1	P117 RES9000	S11-515	7.01	204.00	110.00	8.03	0.25	0.00	0.54	Open	0	0.00
2	P119 S11-515	J20	34.25	204.00	110.00	8.03	0.25	0.02	0.54	Open	0	0.00
3	P121 J16	J18	31.72	204.00	110.00	4.28	0.13	0.01	0.17	Open	0	0.00
4	P123 J18	J22	35.97	204.00	110.00	5.95	0.18	0.01	0.31	Open	0	0.00
5	P125 J16	J24	54.11	204.00	110.00	2.26	0.07	0.00	0.05	Open	0	0.00
6	P127 J20	J26	72.93	204.00	110.00	7.40	0.23	0.03	0.46	Open	0	0.00
7	P129 J22	S11-520	40.51	204.00	110.00	5.55	0.17	0.01	0.27	Open	0	0.00
8	P131 J24	J18	58.07	204.00	110.00	2.07	0.06	0.00	0.04	Open	0	0.00
9	P133 J26	J16	34.42	204.00	110.00	6.92	0.21	0.01	0.41	Open	0	0.00
10	P135 S11-520	RES9002	10.06	204.00	110.00	5.55	0.17	0.00	0.27	Open	0	0.00

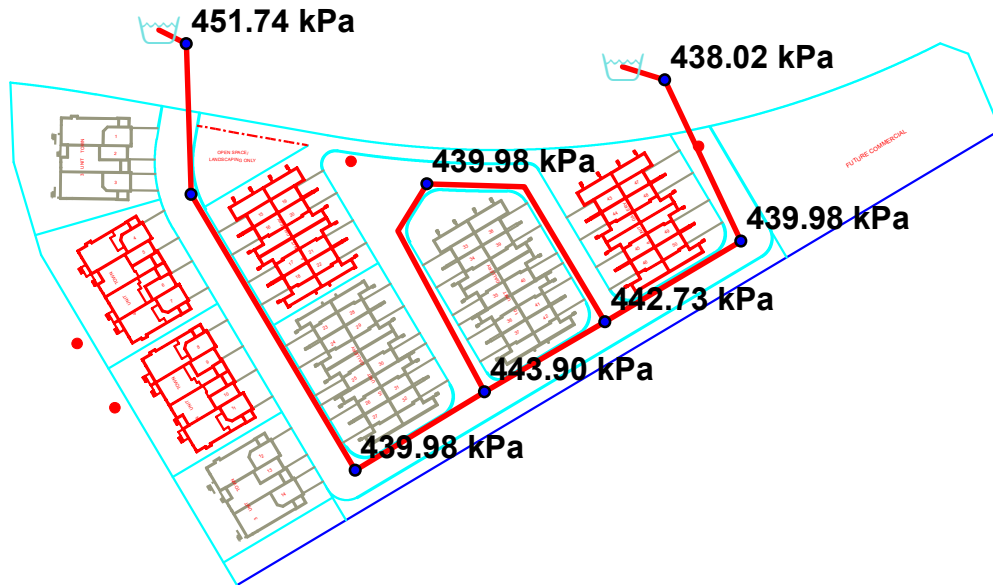
**MAX DAY + FIRE HGL 118m
RESIDUAL PRESSURES
EXISTING CONDITIONS**



Max Day + Fire HGL - Fireflow Design Report

	ID	Total Demand (L/s)	Hydrant Available Flow (L/s)	Critical Node ID for Design Run	Critical Node Pressure at Available Flow (kPa)	Critical Node Pressure at Fire Demand (kPa)	Critical Pressure for Design Run (kPa)	Hydrant Design Flow (L/s)	Hydrant Pressure at Design Flow (kPa)
1	<input type="checkbox"/> J16	166.85	114.10	J24	138.32	115.64	139.96	109.37	141.73
2	<input type="checkbox"/> J18	166.85	115.49	J24	137.89	116.26	139.96	109.40	142.14
3	<input type="checkbox"/> J20	166.95	116.16	J20	139.96	124.87	139.96	116.16	139.96
4	<input type="checkbox"/> J22	166.82	129.66	J22	139.96	126.94	139.96	129.66	139.96
5	<input type="checkbox"/> J24	166.76	86.88	J24	139.96	93.62	139.96	86.88	139.96
6	<input type="checkbox"/> J26	166.89	98.72	J26	139.96	109.34	139.96	98.72	139.96
7	<input type="checkbox"/> S11-515	166.67	314.80	J20	133.77	148.73	139.96	262.80	147.16
8	<input type="checkbox"/> S11-520	166.67	254.90	S11-520	139.96	150.84	139.96	254.90	139.96

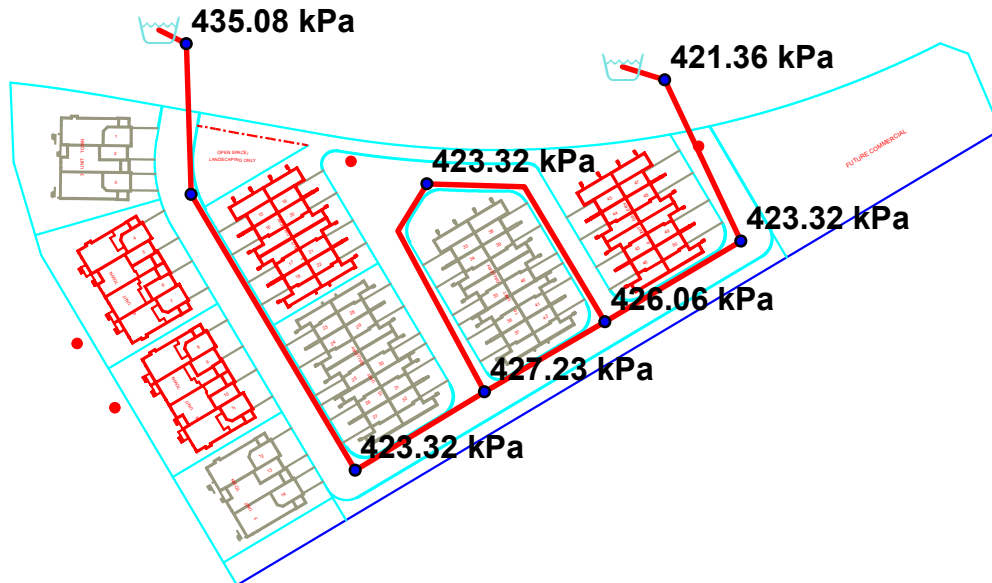
SUC ZONE RECONFIGURATION - BASIC DAY (MAX HGL) PRESSURES



SUC Zone Reconfiguration - Basic Day (MAX HGL) Pressure

		ID	Demand (L/s)	Elevation (m)	Head (m)	Pressure (kPa)	Water Age (hrs)
1	<input type="checkbox"/>	J16	0.07	102.20	147.50	443.90	0.00
2	<input type="checkbox"/>	J18	0.07	102.32	147.50	442.73	0.00
3	<input type="checkbox"/>	J20	0.11	102.60	147.50	439.98	0.00
4	<input type="checkbox"/>	J22	0.06	102.60	147.50	439.98	0.00
5	<input type="checkbox"/>	J24	0.04	102.60	147.50	439.98	0.00
6	<input type="checkbox"/>	J26	0.09	102.60	147.50	439.98	0.00
7	<input type="checkbox"/>	S11-515	0.00	101.40	147.50	451.74	0.00
8	<input type="checkbox"/>	S11-520	0.00	102.80	147.50	438.02	0.00

PEAK HOUR PRESSURES
HGL 146m



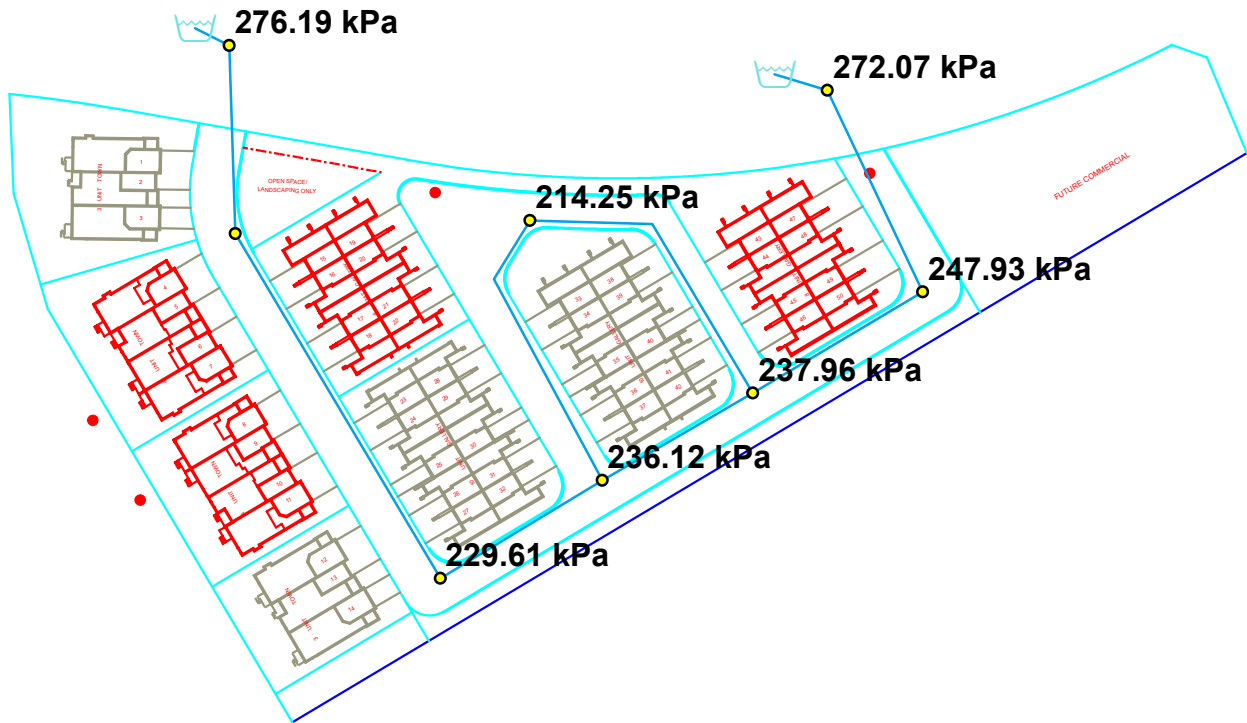
Peak Hour HGL - Junction Report

		ID	Demand (L/s)	Elevation (m)	Head (m)	Pressure (kPa)	Water Age (hrs)
1	<input type="checkbox"/>	J16	0.39	102.20	145.80	427.23	0.00
2	<input type="checkbox"/>	J18	0.39	102.32	145.80	426.06	0.00
3	<input type="checkbox"/>	J20	0.63	102.60	145.80	423.32	0.00
4	<input type="checkbox"/>	J22	0.40	102.60	145.80	423.32	0.00
5	<input type="checkbox"/>	J24	0.19	102.60	145.80	423.32	0.00
6	<input type="checkbox"/>	J26	0.48	102.60	145.80	423.32	0.00
7	<input type="checkbox"/>	S11-515	0.00	101.40	145.80	435.08	0.00
8	<input type="checkbox"/>	S11-520	0.00	102.80	145.80	421.36	0.00

Peak Hour HGL - Pipe Report

	ID	From Node	To Node	Length (m)	Diameter (mm)	Roughness	Flow (L/s)	Velocity (m/s)	Headloss (m)	HL/1000 (m/k-m)	Status	Flow Reversal Count	Water Age (hrs)
1	P117	RES9000	S11-515	7.01	204.00	110.00	1.27	0.04	0.00	0.02	Open	0	0.00
2	P119	S11-515	J20	34.25	204.00	110.00	1.27	0.04	0.00	0.02	Open	0	0.00
3	P121	J16	J18	31.72	204.00	110.00	-0.24	0.01	0.00	0.00	Open	0	0.00
4	P123	J18	J22	35.97	204.00	110.00	-0.81	0.02	0.00	0.01	Open	0	0.00
5	P125	J16	J24	54.11	204.00	110.00	0.01	0.00	0.00	0.00	Open	0	0.00
6	P127	J20	J26	72.93	204.00	110.00	0.64	0.02	0.00	0.00	Open	0	0.00
7	P129	J22	S11-520	40.51	204.00	110.00	-1.21	0.04	0.00	0.02	Open	0	0.00
8	P131	J24	J18	58.07	204.00	110.00	-0.18	0.01	0.00	0.00	Open	0	0.00
9	P133	J26	J16	34.42	204.00	110.00	0.16	0.01	0.00	0.00	Open	0	0.00
10	P135	S11-520	RES9002	10.06	204.00	110.00	-1.21	0.04	0.00	0.02	Open	0	0.00

**SUC ZONE RECONFIGURATION - Max Day + Fire 130m
Residual Pressures**



SUC ZONE RECONFIGURATION - Max Day + Fire HGL- Fireflow Design Report

	ID	Total Demand (L/s)	Hydrant Available Flow (L/s)	Critical Node ID for Design Run	Critical Node Pressure at Available Flow (kPa)	Critical Node Pressure at Fire Demand (kPa)	Critical Pressure for Design Run (kPa)	Hydrant Design Flow (L/s)	Hydrant Pressure at Design Flow (kPa)
1	J16	166.85	300.52	J16	139.96	236.12	139.96	300.52	139.96
2	J18	166.85	308.02	J18	139.96	237.96	139.96	308.02	139.96
3	J20	166.95	368.62	J20	139.96	244.41	139.96	368.62	139.96
4	J22	166.82	354.66	J22	139.96	247.93	139.96	354.66	139.96
5	J24	166.76	249.51	J24	139.96	214.25	139.96	249.51	139.96
6	J26	166.89	290.94	J26	139.96	229.61	139.96	290.94	139.96
7	S11-515	166.67	803.38	J20	150.93	268.09	139.96	803.38	139.96
8	S11-520	166.67	682.46	J22	163.39	273.38	139.96	682.46	139.96

APPENDIX C

Sanitary Sewer Design Sheet
135925-400 - Sanitary Drainage Plan
Barrett Lands Phase 3 Sanitary Design Sheet
Barrett Lands Phase 3 Sanitary Drainage Area Plan



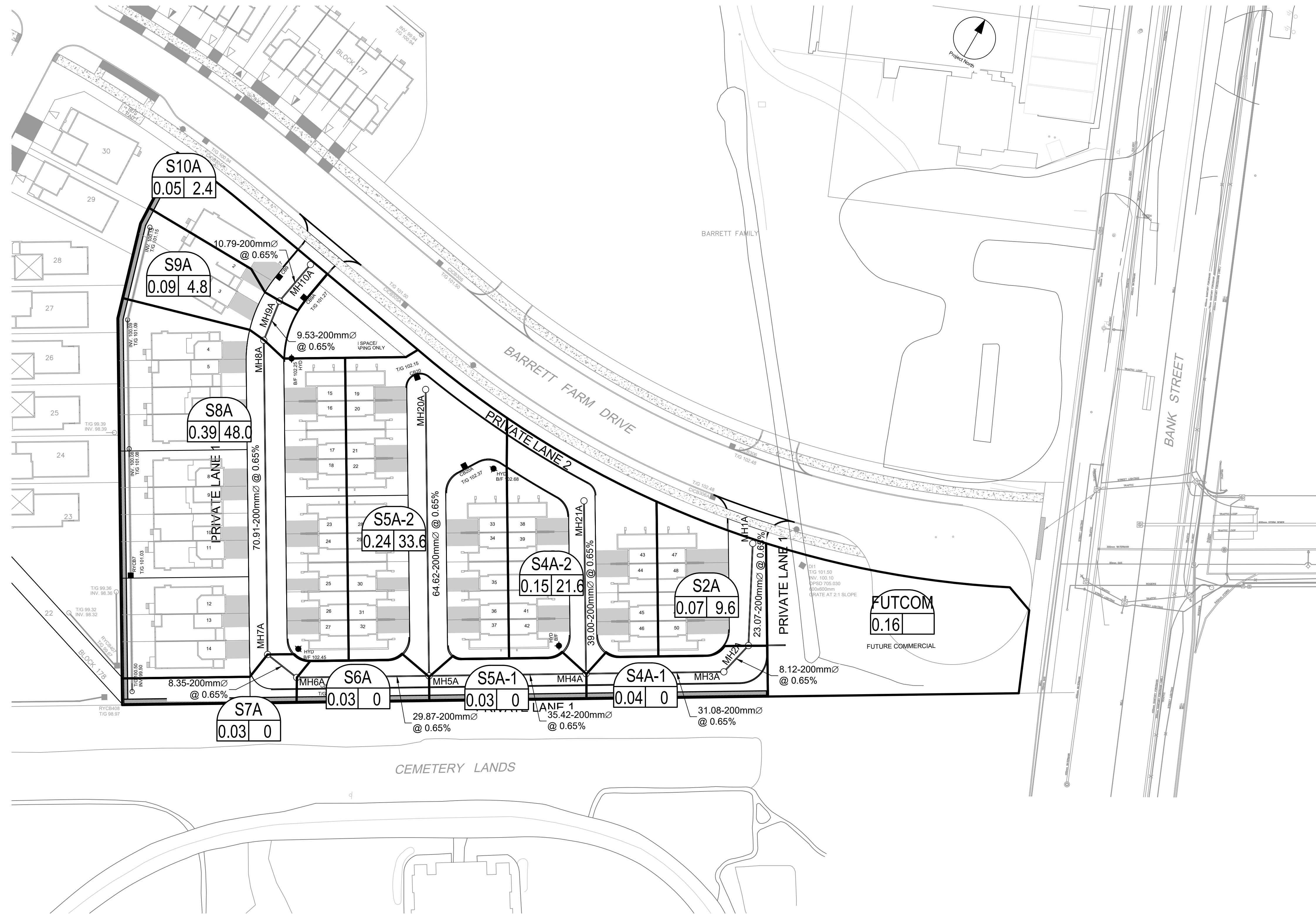
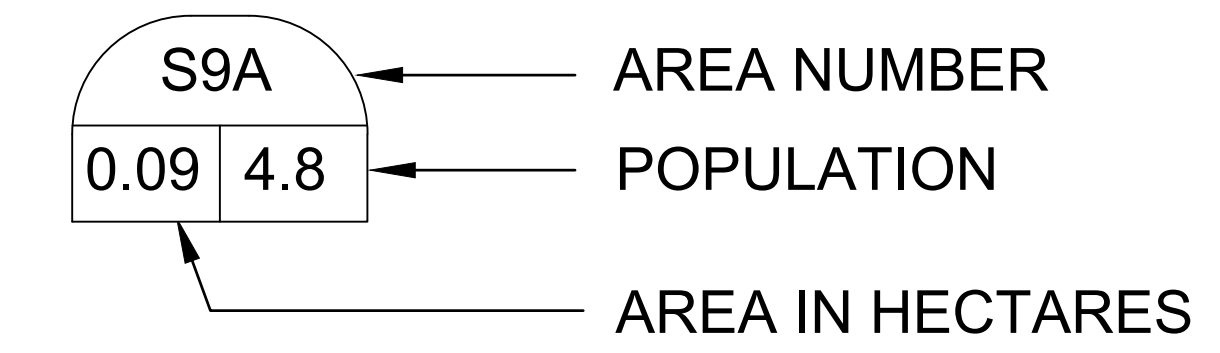
IBI GROUP
 400-333 Preston Street
 Ottawa, Ontario K1S 5N4 Canada
 tel 613 225 1311 fax 613 225 9868
 ibigroup.com

SANITARY SEWER DESIGN SHEET

Barrett Lands Block 178
 CITY OF OTTAWA
 Barrett Co-Tenancy

LOCATION				RESIDENTIAL										ICI AREAS						INFILTRATION ALLOWANCE			FIXED FLOW (L/s)		TOTAL FLOW	PROPOSED SEWER DESIGN										
STREET	AREA ID	FROM MH	TO MH	AREA w/ Units (Ha)	UNIT TYPES				AREA w/o Units (Ha)	POPULATION		RES PEAK FACTOR	PEAK FLOW (L/s)	INSTITUTIONAL		AREA (Ha)		INDUSTRIAL	ICI PEAK FACTOR	PEAK FLOW (L/s)	AREA (Ha)		FLOW	IND	CUM	(L/s)	IND	CUM	(L/s)	CAPACITY (L/s)	LENGTH (m)	DIA (mm)	SLOPE (%)	VELOCITY (full) (m/s)	AVAILABLE CAPACITY	
					SF	SD	TH	APT		IND	CUM			IND	CUM	IND	CUM				IND	CUM										L/s	(%)			
Private Lane No.1	S2A	MH1A	MH2A	0.07			4			9.6	9.6	3.73	0.12		0.00		0.00		1.00	0.00	0.07	0.07	0.02		0.00	0.14	27.59	23.07	200	0.65	0.851	27.45	99.50%			
Private Lane No.1	FUTCOM	Bikhd	MH2A	0.00						0.0	0.0	3.80	0.00		0.00	0.16		0.00	1.50	0.08	0.16	0.16	0.05		0.00	0.13	27.59	8.12	200	0.65	0.851	27.46	99.53%			
Private Lane No.1		MH2A	MH3A							0.0	9.6	3.73	0.12		0.00		0.16		1.50	0.08	0.00	0.07	0.02		0.00	0.22	27.59	8.12	200	0.65	0.851	27.37	99.21%			
Private Lane No.1		MH3A	MH4A						0.04	2.4	12.0	3.73	0.14		0.00		0.16		1.50	0.08	0.04	0.11	0.04		0.00	0.26	27.59	31.08	200	0.65	0.851	27.33	99.06%			
Private Lane No. 2	S4A-2	MH21A	MH4A	0.15			9			21.6	21.6	3.70	0.26		0.00		0.00		1.00	0.00	0.15	0.15	0.05		0.00	0.31	27.59	39.00	200	0.65	0.851	27.28	98.88%			
Private Lane No. 2	S5A-2	MH20A	MH5A	0.24			14			33.6	33.6	3.68	0.40		0.00		0.00		1.00	0.00	0.24	0.24	0.08		0.00	0.48	27.59	64.92	200	0.65	0.851	27.11	98.26%			
Private Lane No.1	S5A-1	MH4A	MH5A						0.03	1.8	35.4	3.67	0.42		0.00		0.16		1.50	0.08	0.03	0.29	0.10		0.00	0.59	27.59	35.42	200	0.65	0.851	26.99	97.84%			
Private Lane No.1	S6A	MH5A	MH6A						0.03	1.8	70.8	3.63	0.83		0.00		0.16		1.50	0.08	0.03	0.56	0.18		0.00	1.09	27.59	29.87	200	0.65	0.851	26.49	96.03%			
Private Lane No.1	S7A	MH6A	MH7A	0.03						0.0	70.8	3.63	0.83		0.00		0.16		1.50	0.08	0.03	0.59	0.19		0.00	1.10	27.59	8.35	200	0.65	0.851	26.48	96.00%			
Private Lane No.1	S8A	MH7A	MH8A	0.39			20			48.0	118.8	3.58	1.38		0.00		0.16		1.00	0.05	0.39	0.98	0.32		0.00	1.75	27.59	70.91	200	0.65	0.851	25.83	93.65%			
Private Lane No.1	S9A	MH8A	MH9A	0.09			2			4.8	123.6	3.57	1.43		0.00		0.16		1.00	0.05	0.09	1.07	0.35		0.00	1.84	27.59	9.53	200	0.65	0.851	25.75	93.34%			
Private Lane No.1	S10A	MH9A	MH10A	0.05			1			2.4	126.0	3.57	1.46		0.00		0.16		1.00	0.05	0.05	1.12	0.37		0.00	1.88	27.59	10.79	200	0.65	0.851	25.71	93.19%			
Block 178	11418A	MH10A	MH11305A							0.0	126.0	3.57	1.46		0.00		0.16		1.00	0.05	0.00	1.12	0.37		0.00	1.88	20.24	16.00	200	0.35	0.624	18.36	90.71%			
Design Parameters:				Notes:										Designed:						Revision			Date													
Residential				1. Manning's coefficient (n) = 0.013										AC						1. Submission No. 1 for City Review			2022-05-09													
ICI Areas				2. Demand (per capita): 280 L/day										RM						2. Submission No. 2 for City Review			2022-11-11													
SF 3.2 p/p/u				3. Infiltration allowance: 0.33 L/s/Ha																																
TH/SD 2.4 p/p/u				4. Residential Peaking Factor:																																
APT 1.9 p/p/u				Harmon Formula = $1 + (14 / (4 + (P / 1000)^{0.5}))^{0.8}$																																
Other 60 p/p/Ha				where K = 0.8 Correction Factor																																
				5. Commercial and Institutional Peak Factors based on total area, 1.5 if greater than 20%, otherwise 1.0										Dwg. Reference: 135925 - 400						File Reference: 135925.00			Date: 2022-05-09													
				MOE Chart																			Sheet No: 1 of 1													

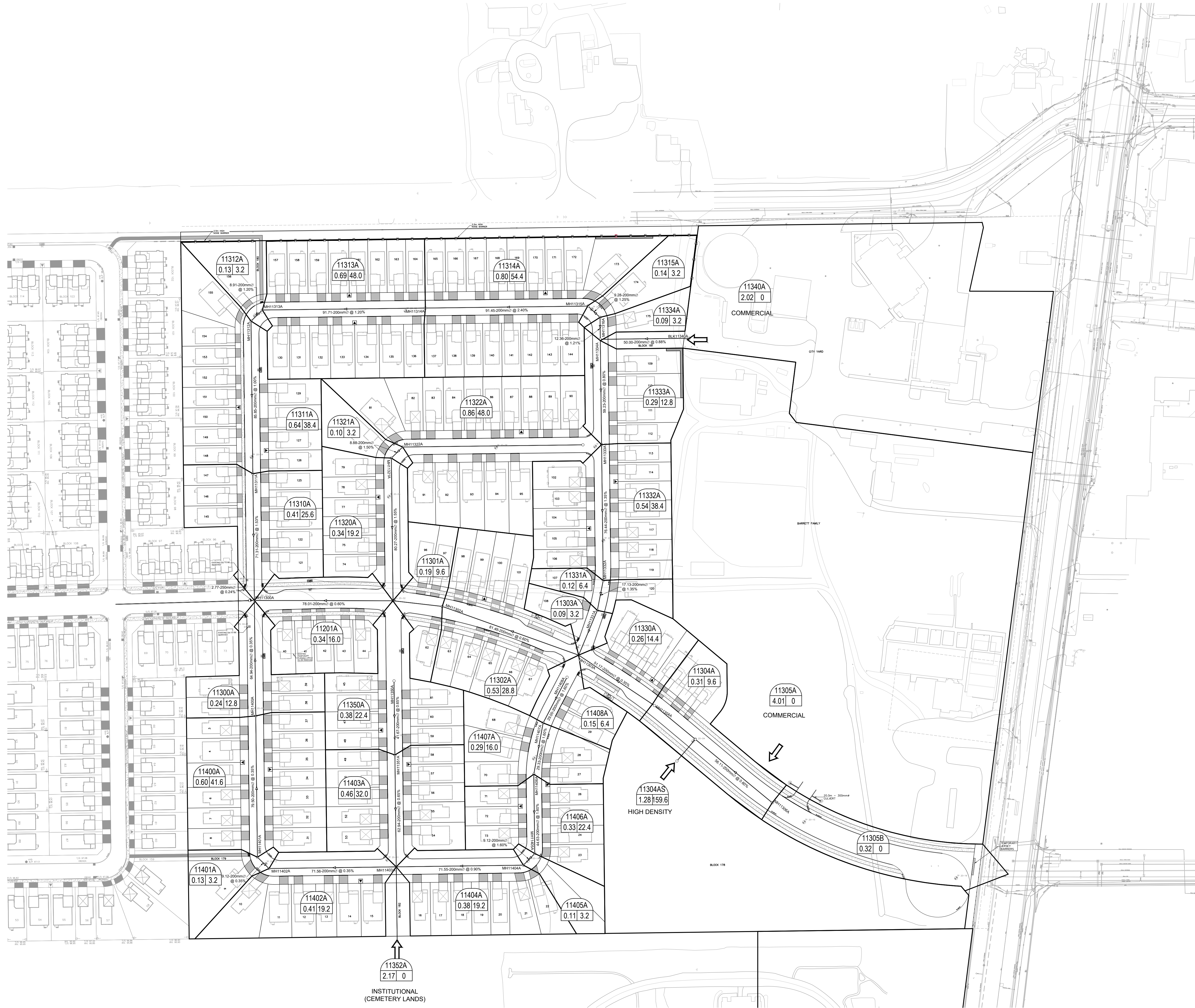
LEGEND :



CLIENT	
BARRETT CO-TENANCY	
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ISSUES	
No.	DESCRIPTION DATE
1	SUBMISSION NO. 1 FOR CITY REVIEW 2022-05-10
2	SUBMISSION NO. 2 FOR CITY REVIEW 2022-11-25
3	
4	
5	
6	
7	
8	
SEE 010, 011, 012 FOR NOTES, LEGEND, CB TABLE, STREET SECTIONS AND DETAILS.	
KEY PLAN LEITRIM ROAD	
KEY PLAN BARRETT FARM DRIVE	
KEY PLAN BANK STREET	
CONSULTANTS	
SEAL	
IBI GROUP Suite 400 - 333 Preston Street Ottawa ON K1S 5N4 Canada Tel: 613 225 1311 / 613 241 3300 Fax: 613 225 9868 ibigroup.com	
PROJECT BARRETT BLOCK 178	
PROJECT NO: 135925	
DRAWN BY: M.M.	CHECKED BY: A.C.
PROJECT MGR: R.M.	APPROVED BY: J.I.M.
SHEET TITLE SANITARY DRAINAGE AREA PLAN	
SHEET NUMBER 400	ISSUE 2

CITY PLAN No. D07-12-22-0112
 File Location: J:\135925_Barrett.ansd7.0_Production\7_03_Design\04_Civil\Sheet\400 SANITARY DRAINAGE AREA PLAN.dwg Last Saved: October 25, 2022, by ymine Printed: Thursday, November 24, 2022 11:56:07 AM by Marlon Milne

J:\3431-1-BarrettLands\3-Drainage\3-Drainage\Phase 3\402-Sanitary Drainage Planning Layout Name: 402-Sanitary Drainage Planning Layout Plot Size: A4 STAG2AB-HA/C1B Plot Scale: 1:50.8 Plotted At: 2022-02-25 10:50:18 Plotted At: 2022-02-25 10:50:18



LEGEND :

11408A	AREA NUMBER
0.15 6.4	RUNOFF COEFFICIENT
	AREA IN HECTARES

SEE 020, 021, 022 FOR NOTES, LEGEND, CB TABLE, STREET SECTIONS AND DETAILS



KEY PLAN

NTS		
14		
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10		
9		
8		
7		
6		
5		
4		
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2	SUBMISSION NO. 2 FOR CITY REVIEW	J.I.M. 2022.02.28
1	SUBMISSION NO. 1 FOR CITY REVIEW	J.I.M. 2021.11.10
No.	REVISIONS	By Date

BARRETT CO-TENANCY

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Project Title
BARRETT LANDS
 PHASE 3

Professional Engineer
 J. I. MOFFATT
 2022/02/28
 PROVINCE OF ONTARIO

Drawing Title
SANITARY DRAINAGE
AREA PLAN

Scale

 1:1000

Design	FHJF	Date	NOVEMBER 2021
Drawn	M.M.	Checked	J.I.M.
Project No.	34731	Drawing No.	420

APPENDIX D

Storm Sewer Design Sheet
135925-500 - Storm Drainage Plan
135925-600 - Ponding Plan
Barrett Lands Phase 3 Storm Design Sheet
Barrett Lands Phase 3 Storm Drainage Area Plan
Modified Rational Method on-site SWM calculations
On-site Underground Storage System
Storm HGL Calculations
Barrett Lands Phase 3 HGL Reference
Overflow Depth/Capacity Calculation
Temporary Orifice Sizing
Sample Runoff Coefficient Calculations
Minor system release rate (Barrett Lands Phase 3)



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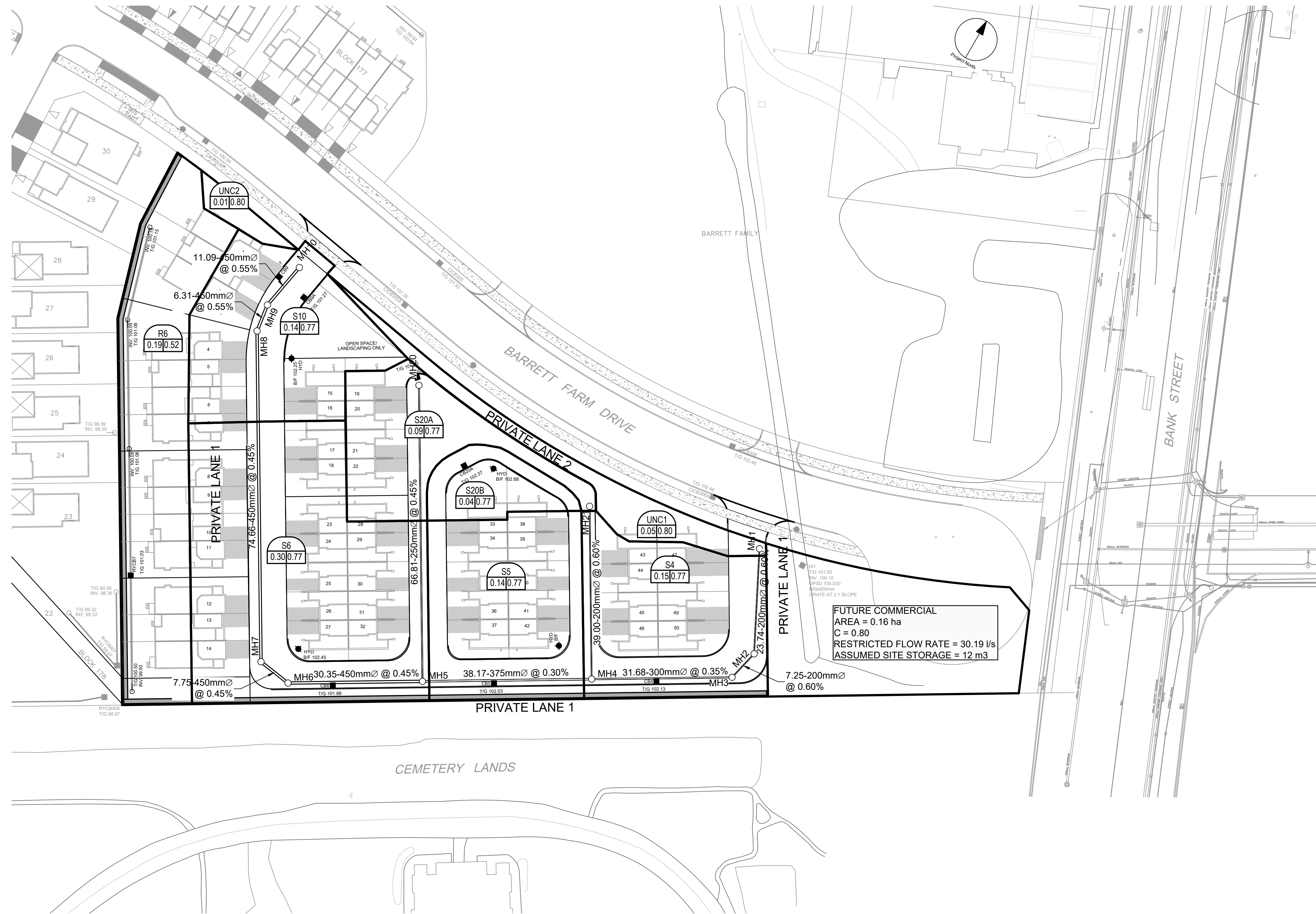
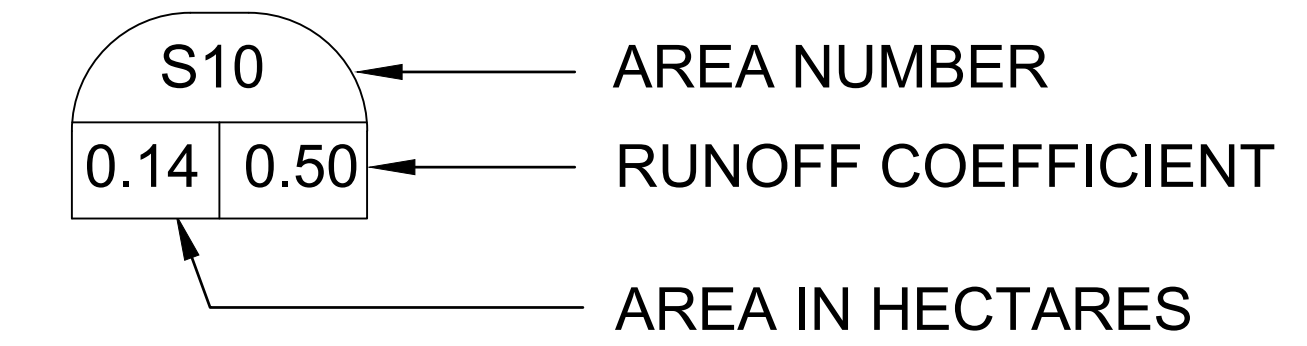
LEGEND
 Black text 2 year event curve design
 Blue text 5 year event curve design
 Green Text 100 year design curve

STORM SEWER DESIGN SHEET

Barrett Lands Block 178
 City of Ottawa
 Barrett Co-Tenancy

LOCATION				AREA (Ha)												RATIONAL DESIGN FLOW										SEWER DATA															
STREET	AREA ID	FROM	TO	C=	C=	C=	C=	C=	C=	C=	C=	C=	C=	IND	CUM	INLET	TIME	TOTAL	i (2)	i (5)	i (10)	i (100)	2yr PEAK	5yr PEAK	10yr PEAK	100yr PEAK	FIXED	DESIGN	CAPACITY	LENGTH	PIPE SIZE (mm)			SLOPE	VELOCITY	AVAIL CAP					
				0.20	0.30	0.42	0.50	0.52	0.72	0.73	0.77	0.85	1.00	2.78AC	2.78AC	(min)	IN PIPE	(min)	(mm/hr)	(mm/hr)	(mm/hr)	(mm/hr)	FLOW (L/s)	FLOW (L/s)	FLOW (L/s)	FLOW (L/s)	FLOW (L/s)	FLOW (L/s)	FLOW (L/s)	DIA	W	H	(%)	(m/s)	(L/s)	(%)					
Private Lane No.1		MH1	MH2											0.00	0.00	10.00	0.48	10.48	76.81					0.00			0.00	0.00	26.50	23.74	200				0.60	0.817	26.50	100.00%			
Private Lane No.1		MH2	MH3											0.00	0.00	10.48	0.15	10.63	74.99					0.00			0.00	0.00	26.50	7.25	200				0.60	0.817	26.50	100.00%			
Private Lane No.1	S4	MH3	MH4											0.32	0.32	10.63	0.65	11.28	74.46					23.91			26.00	26.00	59.68	31.68	300				0.35	0.818	33.68	56.44%			
Private Lane No. 2		MH21	MH4											0.00	0.00	10.00	0.80	10.80	76.81					0.00			0.00	0.00	26.50	39.00	200				0.60	0.817	26.50	100.00%			
Private Lane No. 2	S20A, S20B	MH20	MH5											0.28	0.28	10.00	1.36	11.36	76.81					21.37			25.00	25.00	41.62	66.81	250				0.45	0.821	16.62	39.93%			
Private Lane No.1	S5	MH4	MH5											0.30	0.62	11.28	0.72	12.00	72.22					44.84			51.00	51.00	100.18	38.17	375				0.30	0.879	49.18	49.09%			
Private Lane No.1	S6	MH5	MH6											0.64	1.54	12.00	0.42	12.42	69.89					107.72			126.00	126.00	199.52	30.35	450				0.45	1.215	73.52	36.85%			
Private Lane No.1	R6	MH6	MH7											0.27	1.82	12.42	0.11	12.52	68.62					124.61			146.00	146.00	199.52	7.75	450				0.45	1.215	53.52	26.83%			
Private Lane No.1		MH7	MH8											0.00	1.82	12.52	1.02	13.55	68.30					146.00			146.00	146.00	199.52	74.66	450				0.45	1.215	53.52	26.83%			
Private Lane No.1		MH8	MH9											0.00	1.82	13.55	0.08	13.63	65.42					118.80			146.00	146.00	220.58	6.31	450				0.55	1.344	74.58	33.81%			
Private Lane No.1	S10	MH9	MH10											0.30	2.12	13.63	0.14	13.76	65.21					137.96			179.00	179.00	220.58	11.09	450				0.55	1.344	41.58	18.85%			
Private Lane No.1		MH10	MH11305A											0.00	2.12	13.76	0.19	13.95	64.85					137.19			179.00	179.00	518.80	16.00	675				0.35	1.404	339.80	65.50%			
Definitions: Q = 2.78CIA, where: Q = Peak Flow in Litres per Second (L/s) A = Area in Hectares (Ha) i = Rainfall intensity in millimeters per hour (mm/hr) [i = 732.951 / (TC+6.199)^0.810] 2 YEAR [i = 998.071 / (TC+6.053)^0.814] 5 YEAR [i = 1174.184 / (TC+6.014)^0.816] 10 YEAR [i = 1735.688 / (TC+6.014)^0.820] 100 YEAR				Notes: 1. Mannings coefficient (n) = 0.013 0.024												Designed: AC Checked: RM Dwg. Reference: 135925-500										No. 1. 2.		Revision Submission No.1 for City Review Submission No.2 for City Review						Date 2022-05-09 2022-11-11		File Reference: 135925.00		Date: 2022-05-09		Sheet No: 1 of 1	

LEGEND :



CLIENT

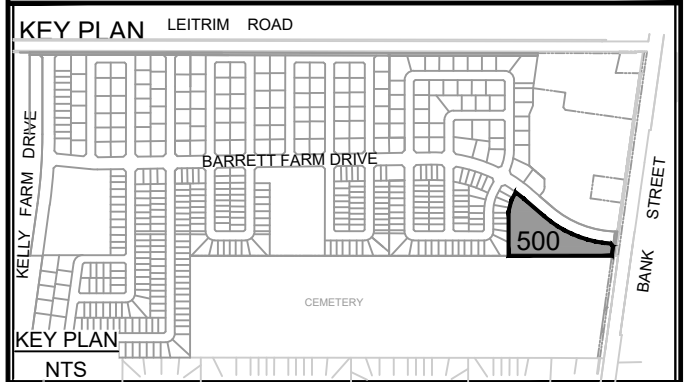
**BARRETT
CO-TENANCY**

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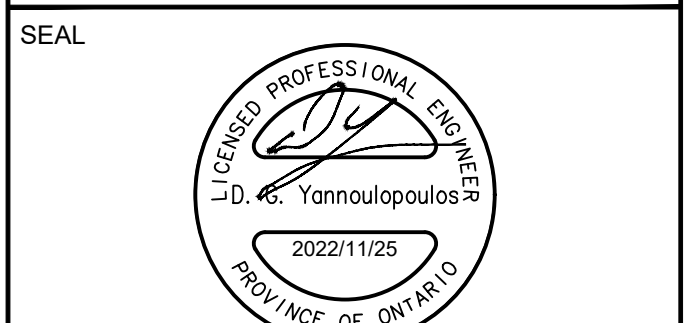
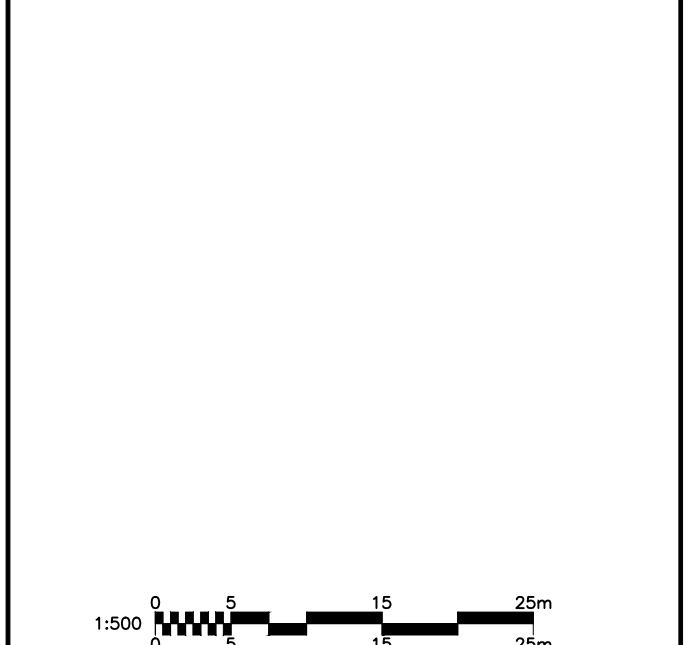
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ISSUES		
No.	DESCRIPTION	DATE
1	SUBMISSION NO. 1 FOR CITY REVIEW	2022-05-10
2	SUBMISSION NO. 2 FOR CITY REVIEW	2022-11-25
3		
4		
5		
6		
7		
8		

SEE 010, 011, 012 FOR NOTES, LEGEND, CB TABLE, STREET SECTIONS AND DETAILS



CONSULTANTS



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PROJECT
BARRETT BLOCK 178

PROJECT NO:
135925

DRAWN BY: M.M.	CHECKED BY: A.C.
PROJECT MGR: R.M.	APPROVED BY: J.I.M.

SHEET TITLE
STORM DRAINAGE AREA PLAN

SHEET NUMBER 500	ISSUE 2
-----------------------------------	--------------------------

CITY FILE No. D07-12-22-0112



IBI Group
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 Ottawa, Ontario
 K1S 5N4

Run-off Coefficients

PROJECT: Barrett Block 146
 DATE: 2022-04-22
 CLIENT: Barrett Co-Tennancy
 FILE: 135925.6.4

			S2&R3								
			BACK TO BACK			TOWNS - REAR			TOWNS - FRONT		
			GRASS	ROOF	ASPHALT	GRASS	ROOF	ASPHALT	GRASS	ROOF	ASPHALT
			255.00	1121.00		492.00	400.00		380.00	1601.00	
TOTAL (m ²)			255.00	1121.00		492.00	400.00		380.00	1601.00	
			1376.00			892.00			1981.00		

Runoff Coefficient (C) :			0.2	0.9	0.9	0.2	0.9	0.9	0.2	0.9	0.9
Ave. Runoff Coefficient (C):			0.77			0.51			0.77		

Runoff Coefficient Used(C):			0.77			0.52			0.77		
-----------------------------	--	--	------	--	--	------	--	--	------	--	--



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LEGEND
 Black text 2 year event curve design
 Blue text 5 year event curve design (Barrett Farm Drive)
 Green text 100 year design curve

STORM SEWER DESIGN SHEET

Barrett Lands Phase 3
 City of Ottawa
 Barrett Co-Tenancy

STREET	LOCATION			AREA (Ha)												RATIONAL DESIGN FLOW										SEWER DATA													
	AREA ID	FROM	TO	C=	C=	C=	C=	C=	C=	C=	C=	C=	C=	IND	CUM	INLET	TIME	TOTAL	i (2)	i (5)	i (10)	i (100)	2yr PEAK	5yr PEAK	10yr PEAK	100yr PEAK	FIXED	DESIGN	CAPACITY	LENGTH	PIPE SIZE (mm)			SLOPE (%)	VELOCITY (m/s)	AVAIL CAP (2yr)			
				0.20	0.30	0.42	0.57	0.68	0.72	0.73	0.78	0.80	1.00	2.78AC	2.78AC	(min)	IN PIPE	(min)	(mm/hr)	(mm/hr)	(mm/hr)	(mm/hr)	FLOW (L/s)	FLOW (L/s)	FLOW (L/s)	FLOW (L/s)	FLOW (L/s)	FLOW (L/s)	DIA	W	H								
Temporary Barrett Farm Drive	Area 9 S11306	DI1 MH11307	MH11307 MH11306			0.24									0.28	0.28	12.00	0.21	12.21	69.89	94.70	110.96	162.13				45.43		45.43	62.04	15.50	250				1.00	1.224	16.61	26.77%
Barrett Family Temporary Temporary	R11305 Culvert	DI2 Ditch	MH11306 MH11306 Ditch										4.01		8.92	8.92	12.00	0.14	12.14	69.89	94.70	110.96	162.13	623.33				623.33	821.24	14.66	750				0.50	1.801	197.92	24.10%	
Barrett Farm Drive Barrett Farm Drive	S11305	MH11306 MH11306	MH11305 MH11305											0.09	0.00	8.92	12.14	0.43	12.57	69.47	94.12	110.28	161.14	619.59				698.95	1,159.96	54.50	825				0.60	2.102	461.01	39.74%	
Barrett Farm Drive	R11304	BLK11305	MH11305									1.28		2.85	2.85	12.00	0.25	12.25	69.89	94.70	110.96	162.13	198.97						198.97	438.47	18.00	675				0.25	1.187	239.50	54.62%
Barrett Farm Drive Barrett Farm Drive	S11305	MH11305 MH11305	MH11304 MH11304												0.00	11.76	12.57	0.36	12.92	68.18	92.34	108.18	158.06	802.08				820.82	1,274.02	30.40	1050				0.20	1.425	453.20	35.57%	
Barrett Farm Drive Barrett Farm Drive	S11304A	MH11304 MH11304	MH11303 MH11303												0.00	11.76	12.92	0.51	13.43	67.15	90.93	106.52	155.62	789.98				917.90	1,424.40	48.54	1050				0.25	1.594	506.50	35.56%	
Bouvardia Crescent Block 120	R11334 R11340	MH11316 BLK11340	MH11334 MH11334										1.98		0.05	0.05	10.00	0.17	10.17	76.81	104.19	122.14	178.56	3.65				3.65	62.04	12.72	250				1.00	1.224	58.39	94.11%	
Bouvardia Crescent	S11334, R11333	MH11334	MH11333												0.67	5.12	12.45	0.54	12.98	68.54	92.84	108.77	158.92	350.80				350.80	535.93	59.22	600				0.70	1.836	185.14	34.54%	
Bouvardia Crescent	R11332, S11333	MH11333	MH11332												0.59	5.71	12.98	0.51	13.49	66.98	90.70	106.25	155.22	382.36				382.36	744.26	78.11	600				1.35	2.550	361.90	48.63%	
Bouvardia Crescent	R11331, S11331	MH11332	MH11331												0.95	6.66	13.49	0.11	13.60	65.57	88.76	103.98	151.88	436.87				436.87	744.26	16.85	600				1.35	2.550	307.39	41.30%	
Bouvardia Crescent		MH11331	MH11303												0.00	6.66	13.60	0.16	13.77	65.27	88.36	103.50	151.18	434.90				434.90	744.26	25.07	600				1.35	2.550	309.36	41.57%	
Barrett Farm Drive Barrett Farm Drive	S11303A, S11303B	MH11303 MH11303	MH11302 MH11302												0.00	18.43	13.77	0.57	14.33	64.84	87.76	102.80	150.16	1,194.83				1,360.93	2,206.67	83.97	1050				0.60	2.469	845.73	38.33%	
Barrett Farm Drive Barrett Farm Drive		MH11302 MH11302	MH11301 MH11301												0.00	18.43	14.33	0.20	14.54	63.38	85.77	100.46	146.72	1,168.04				1,330.37	2,206.67	30.00	1050				0.60	2.469	876.29	39.71%	
Solidago Mews Solidago Mews Solidago Mews	R11323, S11323	MH11323 MH11322 MH11321	MH11322 MH11321 MH11301												0.91	0.91	10.00	0.87	10.87	76.81	104.19	122.14	178.56	70.08				70.08	146.19	104.38	300				2.10	2.004	76.12	52.07%	
Barrett Farm Drive Barrett Farm Drive	R11301, S11301A, S11301B	MH11301 MH11301	MH11300 MH11300												0.00	20.92	14.54	0.53	15.07	62.88	85.08	99.65	145.53	1,315.81				1,569.35	2,206.67	79.16	1050				0.60	2.469	637.32	28.88%	
Bouvardia Crescent Bouvardia Crescent Bouvardia Crescent Bouvardia Crescent Bouvardia Crescent Bouvardia Crescent	R11315 S11314 R11313A, R11313B, S11313 R11313C R11311, S11312, S11311B S11311, S11311A	MH11316 MH11315 MH11314 MH11313 MH11313 MH11312 MH11312 MH11311	MH11315 MH11314 MH11313 MH11312 MH11311 MH11300												0.22	0.22	10.00	0.11	10.11	76.81	104.19	122.14	178.56	17.04				17.04	69.36	9.12	250				1.25	1.369	52.32	75.43%	
Delphinium Crescent Delphinium Crescent Delphinium Crescent Delphinium Crescent Delphinium Crescent	S11407 R11406 S11404	MH11408 MH11407 MH11406 MH11405 MH11404	MH11407 MH11406 MH11405 MH11404 MH11403												0.00	0.00	10.00	0.27	10.27	76.81	104.19	122.14	178.56	0.00				0.00	78.47	25.01	250				1.60	1.549	78.47	100.00%	
Nemesia Way Nemesia Way	S11350, R11350A, R11350B S11351	MH11350 MH11351	MH11351 MH11403												0.92	0.92	10.00	0.61	10.61	76.81	104.19	122.14	178.56	70.97				70.97	141.68	45.20	375				0.60	1.243	70.71	49.91%	
Delphinium Crescent Delphinium Crescent Delphinium Crescent Delphinium Crescent	S11401A, R11402 S11401B, R11401 S11400, R11400A, R11400B	MH11403 MH11402 MH11401 MH11400	MH11402 MH11401 MH11400 MH11300												0.91	4.70	11.91	0.93	12.85	70.17	95.07	111.40	162.78	330.05				330.05	580.71	71.43	750				0.25	1.273	250.66	43.16%	
Barrett Farm Drive Barrett Farm Drive		MH11300 MH11300	EX Blkh EX Blkh												0.00	31.57	15.07	0.02	15.09	61.60	83.33	97.58	142.50	1,944.49				2,192.79	4,658.21	2.51	1650				0.24	2.110	2465.42	52.93%	

Definitions:
 Q = 2.78CIA, where:
 Q = Peak Flow in Litres per Second (L/s)
 A = Area in Hectares (Ha)
 i = Rainfall intensity in millimeters per hour (mm/hr)
 [i = 732.951 / (TC+6.199)^0.810]
 [i = 998.071 / (TC+6.053)^0.814]
 [i = 1174.184 / (TC+6.014)^0.816]
 [i = 1735.688 / (TC+6.014)^0.820]

2 YEAR
5 YEAR
10 YEAR
100 YEAR

Notes:
 1. Mannings coefficient (n) =
 0.013
 0.024

Designed: AC

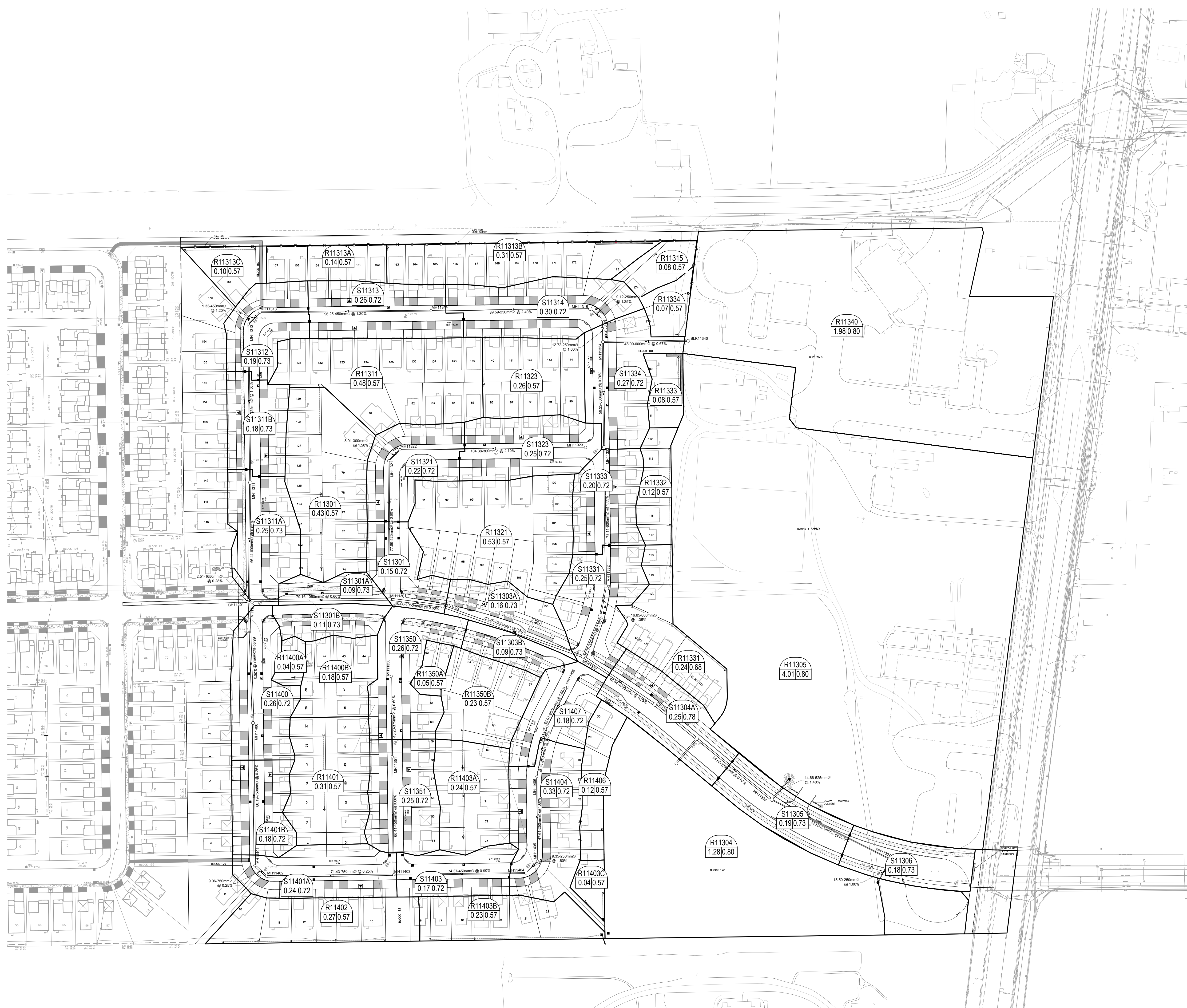
Checked: JIM

Dwg. Reference: 34731-520

No.	Revision	Date
1.	Submission No.1 for City Review	2021-11-10
2.	Submission No.2 for City Review	2022-02-24
3.	Submission No.3 for City Review	2022-04-06

File Reference: 34731-5.7
Date: 2021-11-10
Sheet No: 1 of 1

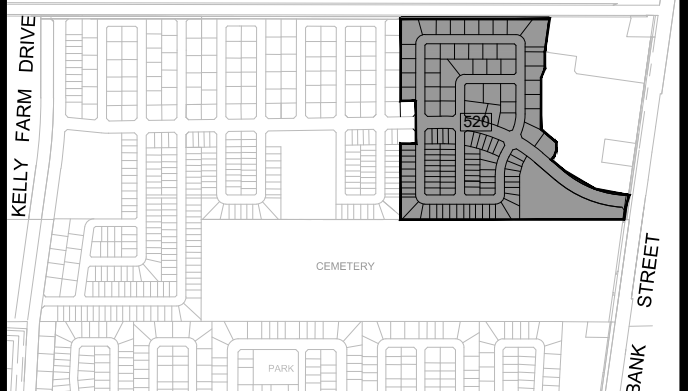
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LEGEND :

S11407 → AREA NUMBER
0.18 0.69 → RUNOFF COEFFICIENT
→ AREA IN HECTARES

SEE 020, 021, 022 FOR NOTES, LEGEND, CB TABLE, STREET SECTIONS AND DETAILS



KEY PLAN

NTS			
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5			
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2	SUBMISSION NO. 2 FOR CITY REVIEW	J.I.M. 2022.02.28	
1	SUBMISSION NO. 1 FOR CITY REVIEW	J.I.M. 2021.11.10	
No.	REVISIONS	By	Date

BARRETT CO-TENANCY

IBI GROUP
400 - 333 Preston Street
Ottawa ON K1S 5N4 Canada
tel 613 225 1311 fax 613 225 9868
ibigroup.com

BARRETT LANDS PHASE 3

LICENSÉ PROFESSIONNEL
J. I. MOFFATT
2022/02/28
PROVINCE OF ONTARIO

STORM DRAINAGE AREA PLAN

Scale: 1:1000

Design	FHJF	Date	NOVEMBER 2021
Drawn	M.M.	Checked	J.I.M.
Project No.	34731	Drawing No.	520

D07-16-17-0002PH3



IBI GROUP
 400-333 Preston Street
 Ottawa, Ontario K1S 5N4 Canada
 tel 613 225 1311 fax 613 225 9868
 ibigroup.com

PROJECT: Barrett Block 178
DATE: 2022-11-11
FILE: 135925-6.4.4
REV #: 2
DESIGNED BY: AC
CHECKED BY: RM

STORMWATER MANAGEMENT

Formulas and Descriptions

$i_{2yr} = 1.2 \text{ year Intensity} = 732.951 / (T_c + 6.199)^{0.810}$
 $i_{5yr} = 1.5 \text{ year Intensity} = 998.071 / (T_c + 6.053)^{0.814}$
 $i_{100yr} = 1:100 \text{ year Intensity} = 1735.688 / (T_c + 6.014)^{0.820}$
 $T_c = \text{Time of Concentration (min)}$
 $C = \text{Average Runoff Coefficient}$
 $A = \text{Area (Ha)}$
 $Q = \text{Flow} = 2.78CiA \text{ (L/s)}$

Maximum Allowable Release Rate

Restricted Flowrate (based on modelled flow from Phase 3)

EXT	234.000 l/s	From Barrett Phase 3
Private Residential	203.31	1.060 ha
Future Commercial	30.69	0.160 ha

$Q_{restricted} = 203.31 \text{ L/s}$

Uncontrolled Release ($Q_{uncontrolled} = 2.78 \cdot C \cdot i_{100yr} \cdot A_{uncontrolled}$)

C =	0.8
$T_c =$	10 min
$i_{100yr} =$	178.56 mm/hr
$A_{uncontrolled} =$	0.06 Ha

$Q_{uncontrolled} = 23.83 \text{ L/s}$

Maximum Allowable Release Rate ($Q_{max\ allowable} = Q_{restricted} - Q_{uncontrolled}$)

$Q_{max\ allowable} = 179.48 \text{ L/s}$

MODIFIED RATIONAL METHOD (100-Year, 100-Year +20% & 2-Year Ponding)

Drainage Area S20A								
Area (Ha)	0.090							
C =	1.00							
Restricted Flow Q_r (L/s) = 15.00								
100-Year Ponding						100Yr +20%		
T_c Variable (min)	i_{100yr} (mm/hour)	Peak Flow $Q_p = 2.78 \cdot C \cdot i_{100yr} \cdot A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)	100YRQp 20% (L/s)	$Q_p - Q_r$ (L/s)	Volume 100+20 (m^3)
5	242.70	60.72	15.00	45.72	13.72	42.90	27.90	25.11
10	178.56	44.68	15.00	29.68	17.81			
15	142.89	35.75	15.00	20.75	18.68			
20	119.95	30.01	15.00	15.01	18.01			
25	103.85	25.98	15.00	10.98	16.47			

Storage (m^3)					100+20		
Overflow	Required	Surface	Sub-surface	Balance	Overflow	Required	Balance
0.00	18.68	20.59	0	0.00	0.00	25.11	4.52
				L/s = 0.00	L/s = 5.02		
				overflows to: S10			

OVERFLOW SUMMARY TABLE			
Area ID	Overflow to	100year	100year+20%
S10	Barrett Farm Drive	39.68	62.70
R6	Delphinium Cres.	97.70	140.55
Total		137.38	203.25
Barrett Phase 3 allowance		350	476

Drainage Area S20A					
Area (Ha)	0.090				
C =	0.80				
Restricted Flow Q_r (L/s) = 15.00					
2-Year Ponding					
T_c Variable (min)	i_{2yr} (mm/hour)	Peak Flow $Q_p = 2.78 \cdot C \cdot i_{2yr} \cdot A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 2yr (m^3)
8	85.46	17.11	15.00	2.11	1.01
9	80.87	16.19	15.00	1.19	0.64
10	76.81	15.37	15.00	0.37	0.22
11	73.17	14.65	15.00	-0.35	-0.23
12	69.89	13.99	15.00	-1.01	-0.73

Storage (m^3)				
Overflow	Required	Surface	Sub-surface	Balance
0.00	0.22	20.59	0	0.00
overflows to: S10				

Drainage Area		S10						
Area (Ha)	0.140	Restricted Flow Q_r (L/s)= 33.00						
C =	1.00							
100-Year Ponding						100Yr +20%		
T_c Variable (min)	i_{100yr} (mm/hour)	Peak Flow $Q_p = 2.78x Ci_{100yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)	100YRQp 20% (L/s)	$Q_p - Q_r$ (L/s)	Volume 100+20 (m^3)
-1	462.72	180.09	33.00	147.09	-8.83	87.92	54.92	29.66
4	262.41	102.13	33.00	69.13	16.59			
9	188.25	73.27	33.00	40.27	21.75			
14	148.72	57.88	33.00	24.88	20.90			
19	123.87	48.21	33.00	15.21	17.34			
Storage (m^3)						100+20		
Overflow	Required	Surface	Sub-surface	Balance	Overflow	Required	Balance	
0.00	21.75	0.32	0	21.43	4.52	34.18	33.86	
				L/s = 39.68	L/s = 62.70			
overflows to: Barrett Farm Drive								

Drainage Area		S10						
Area (Ha)	0.140	Restricted Flow Q_r (L/s)= 33.00						
C =	0.80							
2-Year Ponding						100Yr +20%		
T_c Variable (min)	i_{2yr} (mm/hour)	Peak Flow $Q_p = 2.78x Ci_{2yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 2yr (m^3)	100YRQp 20% (L/s)	$Q_p - Q_r$ (L/s)	Volume 100+20 (m^3)
8	85.46	26.61	33.00	-6.39	-3.07	87.92	54.92	29.66
9	80.87	25.18	33.00	-7.82	-4.22			
10	76.81	23.91	33.00	-9.09	-5.45			
11	73.17	22.78	33.00	-10.22	-6.74			
12	69.89	21.76	33.00	-11.24	-8.09			
Storage (m^3)						100+20		
Overflow	Required	Surface	Sub-surface	Balance	Overflow	Required	Balance	
0.00	-5.45	0.32	0	0.00	4.52	34.18	33.86	
overflows to: Barrett Farm Drive								

Drainage Area		FUTCOM						
Area (Ha)	0.160	Restricted Flow Q_r (L/s)= 30.69						
C =	1.00							
100-Year Ponding						100Yr +20%		
T_c Variable (min)	i_{100yr} (mm/hour)	Peak Flow $Q_p = 2.78x Ci_{100yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)	100YRQp 20% (L/s)	$Q_p - Q_r$ (L/s)	Volume 100+20 (m^3)
4	262.41	116.72	30.69	86.03	20.65	79.38	48.69	40.90
9	188.25	83.74	30.69	53.05	28.65			
14	148.72	66.15	30.69	35.46	29.79			
19	123.87	55.10	30.69	24.41	27.83			
24	106.68	47.45	30.69	16.76	24.14			
Storage (m^3)						100+20		
Overflow	Required	Surface	Sub-surface	Balance	Overflow	Required	Balance	
0.00	29.79	12.00	0	17.79	0.00	40.90	28.90	
				L/s = 21.18	L/s = 34.41			
overflows to: S4								

Drainage Area		FUTCOM						
Area (Ha)	0.160	Restricted Flow Q_r (L/s)= 30.69						
C =	0.80							
2-Year Ponding						100Yr +20%		
T_c Variable (min)	i_{2yr} (mm/hour)	Peak Flow $Q_p = 2.78x Ci_{2yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 2yr (m^3)	100YRQp 20% (L/s)	$Q_p - Q_r$ (L/s)	Volume 100+20 (m^3)
8	85.46	30.41	30.69	-0.28	-0.13	79.38	48.69	40.90
9	80.87	28.78	30.69	-1.91	-1.03			
10	76.81	27.33	30.69	-3.36	-2.01			
11	73.17	26.04	30.69	-4.65	-3.07			
12	69.89	24.87	30.69	-5.82	-4.19			
Storage (m^3)						100+20		
Overflow	Required	Surface	Sub-surface	Balance	Overflow	Required	Balance	
0.00	-2.01	12.00	0	0.00	0.00	40.90	28.90	
overflows to: S4								

Drainage Area		S4						
Area (Ha)	0.150	Restricted Flow Q_r (L/s)= 26.00						
C =	1.00							
100-Year Ponding						100Yr +20%		
T_c Variable (min)	i_{100yr} (mm/hour)	Peak Flow $Q_p = 2.78x Ci_{100yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)	100YRQp 20% (L/s)	$Q_p - Q_r$ (L/s)	Volume 100+20 (m^3)
4	262.41	109.42	26.00	83.42	20.02	74.42	48.42	40.67
9	188.25	78.50	26.00	52.50	28.35			
14	148.72	62.02	26.00	36.02	30.25			
19	123.87	51.65	26.00	25.65	29.24			
24	106.68	44.48	26.00	18.48	26.62			
Storage (m^3)						100+20		
Overflow	Required	Surface	Sub-surface	Balance	Overflow	Required	Balance	
17.79	48.04	1.16	0	46.88	28.90	69.58	68.42	
				L/s = 55.81	L/s = 81.45			
overflows to: S5								

Drainage Area		S4						
Area (Ha)	0.150	Restricted Flow Q_r (L/s)= 26.00						
C =	0.80							
2-Year Ponding						100Yr +20%		
T_c Variable (min)	i_{2yr} (mm/hour)	Peak Flow $Q_p = 2.78x Ci_{2yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 2yr (m^3)	100YRQp 20% (L/s)	$Q_p - Q_r$ (L/s)	Volume 100+20 (m^3)
8	85.46	28.51	26.00	2.51	1.20	74.42	48.42	40.67
9	80.87	26.98	26.00	0.98	0.53			
10	76.81	25.62	26.00	-0.38	-0.23			
11	73.17	24.41	26.00	-1.59	-1.05			
12	69.89	23.32	26.00	-2.68	-1.93			
Storage (m^3)						100+20		
Overflow	Required	Surface	Sub-surface	Balance	Overflow	Required	Balance	
0.00	-0.23	1.16	0	0.00	28.90	69.58	68.42	
overflows to: S5								

Drainage Area		S20B						
Area (Ha)	0.040	Restricted Flow Q_r (L/s)= 10.00						
C =	1.00							
100-Year Ponding						100Yr +20%		
T_c Variable (min)	i_{100yr} (mm/hour)	Peak Flow $Q_p = 2.78x Ci_{100yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)	100YRQp 20% (L/s)	$Q_p - Q_r$ (L/s)	Volume 100+20 (m^3)
-4	977.56	108.70	10.00	98.70	-23.69	30.16	20.16	7.26
1	351.38	39.07	10.00	29.07	1.74			
6	226.01	25.13	10.00	15.13	5.45			
11	169.91	18.89	10.00	8.89	5.87			
16	137.55	15.30	10.00	5.30	5.08			
Storage (m^3)						100+20		
Overflow	Required	Surface	Sub-surface	Balance	Overflow	Required	Balance	
0.00	5.45	1.41	0	4.04	0.00	7.26	5.85	
				L/s = 11.22	L/s = 16.24			
overflows to: S5								

Drainage Area		S20B						
Area (Ha)	0.040	Restricted Flow Q_r (L/s)= 10.00						
C =	0.80							
2-Year Ponding						100Yr +20%		
T_c Variable (min)	i_{2yr} (mm/hour)	Peak Flow $Q_p = 2.78x Ci_{2yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 2yr (m^3)	100YRQp 20% (L/s)	$Q_p - Q_r$ (L/s)	Volume 100+20 (m^3)
8	85.46	7.60	10.00	-2.40	-1.15	30.16	20.16	7.26
9	80.87	7.19	10.00	-2.81	-1.51			
10	76.81	6.83	10.00	-3.17	-1.90			
11	73.17	6.51	10.00	-3.49	-2.30			
12	69.89	6.22	10.00	-3.78	-2.72			
Storage (m^3)						100+20		
Overflow	Required	Surface	Sub-surface	Balance	Overflow	Required	Balance	
0.00	-1.90	1.41	0	0.00	0.00	7.26	5.85	
overflows to: S5								

Drainage Area		S5						
Area (Ha)	0.140	Restricted Flow Q_r (L/s)= 25.00						
C =	1.00							
100-Year Ponding						100Yr +20%		
T_c Variable (min)	i_{100yr} (mm/hour)	Peak Flow $Q_p = 2.78 \times C i_{100yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)	100YRQp 20% (L/s)	$Q_p - Q_r$ (L/s)	Volume 100+20 (m^3)
6	226.01	87.96	25.00	62.96	22.67			
11	169.91	66.13	25.00	41.13	27.14			
16	137.55	53.53	25.00	28.53	27.39	64.24	39.24	37.67
21	116.30	45.26	25.00	20.26	25.53			
26	101.18	39.38	25.00	14.38	22.43			
Storage (m^3)						100+20		
Overflow	Required	Surface	Sub-surface	Balance	Overflow	Required	Balance	
50.92	78.31	1.32	0	76.99	74.26	111.93	110.61	
				L/s = 80.20				
				overflows to: S6				

Drainage Area		S5						
Area (Ha)	0.140	Restricted Flow Q_r (L/s)= 25.00						
C =	0.80							
2-Year Ponding						100Yr +20%		
T_c Variable (min)	i_{2yr} (mm/hour)	Peak Flow $Q_p = 2.78 \times C i_{2yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 2yr (m^3)	100YRQp 20% (L/s)	$Q_p - Q_r$ (L/s)	Volume 100+20 (m^3)
8	85.46	26.61	25.00	1.61	0.77			
9	80.87	25.18	25.00	0.18	0.10			
10	76.81	23.91	25.00	-1.09	-0.65			
11	73.17	22.78	25.00	-2.22	-1.46			
12	69.89	21.76	25.00	-3.24	-2.33			
Storage (m^3)						100+20		
Overflow	Required	Surface	Sub-surface	Balance	Overflow	Required	Balance	
0.00	-0.65	1.32	0	0.00	74.26	111.93	110.61	
				L/s = 80.20				
				overflows to: S6				

Drainage Area		S6						
Area (Ha)	0.300	Restricted Flow Q_r (L/s)= 50.00						
C =	1.00							
100-Year Ponding						100Yr +20%		
T_c Variable (min)	i_{100yr} (mm/hour)	Peak Flow $Q_p = 2.78 \times C i_{100yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)	100YRQp 20% (L/s)	$Q_p - Q_r$ (L/s)	Volume 100+20 (m^3)
8	199.20	166.13	50.00	116.13	55.74			
13	155.11	129.36	50.00	79.36	61.90			
18	128.08	106.82	50.00	56.82	61.37	128.19	78.19	84.44
23	109.68	91.47	50.00	41.47	57.23			
28	96.27	80.29	50.00	30.29	50.89			
Storage (m^3)						100+20		
Overflow	Required	Surface	Sub-surface	Balance	Overflow	Required	Balance	
76.99	138.36	9.71	0	128.65	110.61	195.06	185.35	
				L/s = 119.12				
				overflows to: R6				

Drainage Area		S6						
Area (Ha)	0.300	Restricted Flow Q_r (L/s)= 50.00						
C =	0.80							
2-Year Ponding						100Yr +20%		
T_c Variable (min)	i_{2yr} (mm/hour)	Peak Flow $Q_p = 2.78 \times C i_{2yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 2yr (m^3)	100YRQp 20% (L/s)	$Q_p - Q_r$ (L/s)	Volume 100+20 (m^3)
8	85.46	57.02	50.00	7.02	3.37			
9	80.87	53.96	50.00	3.96	2.14			
10	76.81	51.24	50.00	1.24	0.75			
11	73.17	48.82	50.00	-1.18	-0.78			
12	69.89	46.63	50.00	-3.37	-2.42			
Storage (m^3)						100+20		
Overflow	Required	Surface	Sub-surface	Balance	Overflow	Required	Balance	
0.00	0.75	9.71	0	0.00	110.61	195.06	185.35	
				L/s = 119.12				
				overflows to: R6				

Drainage Area		R6						
Area (Ha)	0.170	Restricted Flow Q_r (L/s)= 20.00						
C =	0.68	50% Restricted Flow Q_r (L/s)= 10.00						
100-Year Ponding						100Yr +20%		
T_c Variable (min)	i_{100yr} (mm/hour)	Peak Flow $Q_p = 2.78 \times C i_{100yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 100yr (m^3)	100YRQp 20% (L/s)	$Q_p - Q_r$ (L/s)	Volume 100+20 (m^3)
16	137.55	44.20	10.00	34.20	32.84			
21	116.30	37.37	10.00	27.37	34.49			
26	101.18	32.52	10.00	22.52	35.12	39.02	29.02	45.27
31	89.83	28.87	10.00	18.87	35.09			
36	80.96	26.02	10.00	16.02	34.60			
Storage (m^3)						100+20		
Overflow	Required	Surface	Sub-surface	Balance	Overflow	Required	Balance	
128.65	163.78	4.38	6.98	152.42	185.35	230.61	219.25	
				L/s = 97.70				
				overflows to: Delphinium Cres.				

Drainage Area		R6						
Area (Ha)	0.170	Restricted Flow Q_r (L/s)= 10.00						
C =	0.68							
2-Year Ponding						100Yr +20%		
T_c Variable (min)	i_{2yr} (mm/hour)	Peak Flow $Q_p = 2.78 \times C i_{2yr} A$ (L/s)	Q_r (L/s)	$Q_p - Q_r$ (L/s)	Volume 2yr (m^3)	100YRQp 20% (L/s)	$Q_p - Q_r$ (L/s)	Volume 100+20 (m^3)
11	73.17	23.51	10.00	13.51	8.92			
12	69.89	22.46	10.00	12.46	8.97			
13	66.93	21.51	10.00	11.51	8.98			
14	64.23	20.64	10.00	10.64	8.94			
15	61.77	19.85	10.00	9.85	8.87			
Storage (m^3)						100+20		
Overflow	Required	Surface	Sub-surface	Balance	Overflow	Required	Balance	
0.00	8.98	4.38	6.98	0.00	185.35	230.61	219.25	
				L/s = 97.70				
				overflows to: Delphinium Cres.				



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PROJECT: Barrett Block 178
DATE: 2022-10-27
FILE: 135925-6.4.4
REV #: 2
DESIGNED BY: AC
CHECKED BY: RM

UNDERGROUND STORAGE CALCULATIONS - BARRETT BLOCK 178

Pipe Storage		Area R6				
From	To	Length	Diameter	X-sec Area	Volume	
ECB1	RYCB7	26.07	250	0.049	1.28	
RYCB7	ECB2	28.46	250	0.049	1.40	
ECB2	ECB3	29.13	250	0.049	1.43	
ECB3	ECB4	20.50	250	0.049	1.01	
Total					5.11	

Structure Storage		Area				
	Base	Top	Height	diameter	X-sec Area	Volume
ECB1	98.480	99.48	1.00	300	0.071	0.07
ECB2	100.080	101.08	1.00	300	0.071	0.07
ECB3	100.090	101.09	1.00	300	0.071	0.07
ECB4	100.150	101.15	1.00	300	0.071	0.07
RYCB7	99.630	101.03	1.40	1200	1.131	1.58
Total						1.87

TOTAL AREA 6.98



STORM HYDRAULIC GRADE LINE DESIGN SHEET
 PROJECT TITLE
 CITY OF OTTAWA
 DEVELOPPER

JOB #: 135925 - 6.04
 DATE: 2022-11-11
 DESIGN: AC
 CHECKED: RM
 REV #: 2

FRICITION LOSS				MANNING FORMULA - FLOWING FULL						
	FROM MH	TO MH	PIPE ID	DIA (m)	Area (m2)	Perim. (m)	Slope (%)	Hyd.R. (m)	Vel. (m/s)	Q (l/s)
Block 178	10	9		0.45	0.16	1.41	0.550	0.11	1.33	211.34
INVERT ELEVATION (m)	98.030	98.091		HYDRAULIC SLOPE = 17.82 %						
OBVERT ELEVATION (m)	98.480	98.541		DESIGN FLOW TO FULL FLOW RATIO (Q) = 0.847						
DIAMETER (mm)				DESIGN FLOW DEPTH = 0.315						
LENGTH (m)				450						
FLOW (l/s)				11.1						
HGL (m) ***	96.430	96.474	0.044	<div style="border: 1px solid black; padding: 5px;"> Head loss in manhole simplified method p. 71 (MWDM) fig1.7.1, Kratio = 0.75 for 45 bends K_L=0.75 Velocity = Flow / Area = 1.13 m/s HL = K_L * V²/ 2g </div>						
MANHOLE COEF K= 0.75	LOSS (m)	0.048								
TOTAL HGL (m)		98.406								
MAX. SURCHARGE (mm)		-135								

FRICITION LOSS				MANNING FORMULA - FLOWING FULL						
	FROM MH	TO MH	PIPE ID	DIA (m)	Area (m2)	Perim. (m)	Slope (%)	Hyd.R. (m)	Vel. (m/s)	Q (l/s)
Block 178	9	8		0.45	0.16	1.41	0.550	0.11	1.34	212.23
INVERT ELEVATION (m)	98.121	98.156		HYDRAULIC SLOPE = 0.92 %						
OBVERT ELEVATION (m)	98.571	98.606		DESIGN FLOW TO FULL FLOW RATIO (Q) = 0.749						
DIAMETER (mm)				DESIGN FLOW DEPTH = 0.288						
LENGTH (m)				450						
FLOW (l/s)				6.3						
HGL (m) ***	98.406	98.426	0.020	<div style="border: 1px solid black; padding: 5px;"> Head loss in manhole simplified method p. 71 (MWDM) fig1.7.1, Kratio = 0.75 for 45 bends K_L=0.75 Velocity = Flow / Area = 1.00 m/s HL = K_L * V²/ 2g </div>						
MANHOLE COEF K= 0.75	LOSS (m)	0.038								
TOTAL HGL (m)		98.464								
MAX. SURCHARGE (mm)		-142								

FRICITION LOSS				MANNING FORMULA - FLOWING FULL						
	FROM MH	TO MH	PIPE ID	DIA (m)	Area (m2)	Perim. (m)	Slope (%)	Hyd.R. (m)	Vel. (m/s)	Q (l/s)
Block 178	8	7		0.45	0.16	1.41	0.450	0.11	1.20	191.17
INVERT ELEVATION (m)	98.186	98.522		HYDRAULIC SLOPE = 0.49 %						
OBVERT ELEVATION (m)	98.636	98.972		DESIGN FLOW TO FULL FLOW RATIO (Q) = 0.832						
DIAMETER (mm)				DESIGN FLOW DEPTH = 0.311						
LENGTH (m)				450						
FLOW (l/s)				74.7						
HGL (m) ***	98.464	98.696	0.232	<div style="border: 1px solid black; padding: 5px;"> Head loss in manhole simplified method p. 71 (MWDM) straight through K_L=0.05 Velocity = Flow / Area = 1.00 m/s HL = K_L * V²/ 2g </div>						
MANHOLE COEF K= 0.05	LOSS (m)	0.003								
TOTAL HGL (m)		98.833								
MAX. SURCHARGE (mm)		-139								

FRICITION LOSS				MANNING FORMULA - FLOWING FULL						
	FROM MH	TO MH	PIPE ID	DIA (m)	Area (m2)	Perim. (m)	Slope (%)	Hyd.R. (m)	Vel. (m/s)	Q (l/s)
Block 178	7	6		0.45	0.16	1.41	0.450	0.11	1.20	191.50
INVERT ELEVATION (m)	98.552	98.587		HYDRAULIC SLOPE = 0.26 %						
OBVERT ELEVATION (m)	99.002	99.037		DESIGN FLOW TO FULL FLOW RATIO (Q) = 0.658						
DIAMETER (mm)				DESIGN FLOW DEPTH = 0.266						
LENGTH (m)				450						
FLOW (l/s)				7.8						
HGL (m) ***	98.833	98.848	0.015	<div style="border: 1px solid black; padding: 5px;"> Head loss in manhole simplified method p. 71 (MWDM) straight through K_L=0.05 Velocity = Flow / Area = 0.79 m/s HL = K_L * V²/ 2g </div>						
MANHOLE COEF K= 0.05	LOSS (m)	0.002								
TOTAL HGL (m)		98.853								
MAX. SURCHARGE (mm)		-185								



STORM HYDRAULIC GRADE LINE DESIGN SHEET
 PROJECT TITLE
 CITY OF OTTAWA
 DEVELOPPER

JOB #: 135925 - 6.04
 DATE: 2022-11-11
 DESIGN: AC
 CHECKED: RM
 REV #: 2

FRICTION LOSS				MANNING FORMULA - FLOWING FULL							
	FROM MH	TO MH	PIPE ID	DIA (m)	Area (m ²)	Perim. (m)	Slope (%)	Hyd.R. (m)	Vel. (m/s)	Q (l/s)	
Block 178	6	5		0.45	0.16	1.41	0.450	0.11	1.20	190.76	
INVERT ELEVATION (m)	98.617	98.753		HYDRAULIC SLOPE = 0.55 %							
OBVERT ELEVATION (m)	99.067	99.203		DESIGN FLOW TO FULL FLOW RATIO (Q _d) = 0.661							
DIAMETER (mm)				450	DESIGN FLOW DEPTH = 0.266						
LENGTH (m)				30.4							
FLOW (l/s)				126.00							
HGL (m) ***	98.853	98.912	0.059	<div style="border: 1px solid black; padding: 5px;"> Head loss in manhole simplified method p. 71 (MWDM) straight through K_L=0.05 Velocity = Flow / Area = 0.79 m/s HL = K_L * V²/ 2g </div>							
MANHOLE COEF K= 0.05	LOSS (m)	0.002									
TOTAL HGL (m)		99.019									
MAX. SURCHARGE (mm)		-185									

FRICTION LOSS				MANNING FORMULA - FLOWING FULL							
	FROM MH	TO MH	PIPE ID	DIA (m)	Area (m ²)	Perim. (m)	Slope (%)	Hyd.R. (m)	Vel. (m/s)	Q (l/s)	
Block 178	5	4		0.375	0.11	1.18	0.300	0.09	0.87	96.19	
INVERT ELEVATION (m)	98.848	98.963		HYDRAULIC SLOPE = 0.36 %							
OBVERT ELEVATION (m)	99.223	99.338		DESIGN FLOW TO FULL FLOW RATIO (Q _d) = 0.530							
DIAMETER (mm)				375	DESIGN FLOW DEPTH = 0.191						
LENGTH (m)				38.2							
FLOW (l/s)				51.00							
HGL (m) ***	99.019	99.051	0.032	<div style="border: 1px solid black; padding: 5px;"> Head loss in manhole simplified method p. 71 (MWDM) straight through K_L=0.05 Velocity = Flow / Area = 0.46 m/s HL = K_L * V²/ 2g </div>							
MANHOLE COEF K= 0.05	LOSS (m)	0.001									
TOTAL HGL (m)		99.154									
MAX. SURCHARGE (mm)		-184									

FRICTION LOSS				MANNING FORMULA - FLOWING FULL							
	FROM MH	TO MH	PIPE ID	DIA (m)	Area (m ²)	Perim. (m)	Slope (%)	Hyd.R. (m)	Vel. (m/s)	Q (l/s)	
Block 178	5	20		0.25	0.05	0.79	0.450	0.06	0.81	39.90	
INVERT ELEVATION (m)	99.013	99.314		HYDRAULIC SLOPE = 0.66 %							
OBVERT ELEVATION (m)	99.263	99.564		DESIGN FLOW TO FULL FLOW RATIO (Q _d) = 0.627							
DIAMETER (mm)				250	DESIGN FLOW DEPTH = 0.143						
LENGTH (m)				66.8							
FLOW (l/s)				25.00							
HGL (m) ***	99.019	99.137	0.118	<div style="border: 1px solid black; padding: 5px;"> Head loss in manhole simplified method p. 71 (MWDM) straight through K_L=0.05 Velocity = Flow / Area = 0.51 m/s HL = K_L * V²/ 2g </div>							
MANHOLE COEF K= 0.05	LOSS (m)	0.001									
TOTAL HGL (m)		99.457									
MAX. SURCHARGE (mm)		-108									

FRICTION LOSS				MANNING FORMULA - FLOWING FULL							
	FROM MH	TO MH	PIPE ID	DIA (m)	Area (m ²)	Perim. (m)	Slope (%)	Hyd.R. (m)	Vel. (m/s)	Q (l/s)	
Block 178	4	21		0.2	0.03	0.63	0.600	0.05	0.81	25.39	
INVERT ELEVATION (m)	99.198	99.432		HYDRAULIC SLOPE = 0.00 %							
OBVERT ELEVATION (m)	99.398	99.632		DESIGN FLOW TO FULL FLOW RATIO (Q _d) = 0.000							
DIAMETER (mm)				200	DESIGN FLOW DEPTH = 0.002						
LENGTH (m)				39.0							
FLOW (l/s)				0.00							
HGL (m) ***	99.457	99.457	0.000	<div style="border: 1px solid black; padding: 5px;"> Head loss in manhole simplified method p. 71 (MWDM) straight through K_L=0.05 Velocity = Flow / Area = 0.00 m/s HL = K_L * V²/ 2g </div>							
MANHOLE COEF K= 0.05	LOSS (m)	0.000									
TOTAL HGL (m)		99.457									
MAX. SURCHARGE (mm)		-176									



STORM HYDRAULIC GRADE LINE DESIGN SHEET
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 DEVELOPPER

JOB #: 135925 - 6.04
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 CHECKED: RM
 REV #: 2

FRICTION LOSS		FROM MH	TO MH	PIPE ID	MANNING FORMULA - FLOWING FULL						
Block 178		4	3		DIA (m)	Area (m ²)	Perim. (m)	Slope (%)	Hyd.R. (m)	Vel. (m/s)	Q (l/s)
INVERT ELEVATION (m)		99.058	99.168		0.3	0.07	0.94	0.350	0.08	0.81	56.95
OBVERT ELEVATION (m)		99.358	99.468		HYDRAULIC SLOPE = 0.49 %						
DIAMETER (mm)				300	DESIGN FLOW TO FULL FLOW RATIO (Q _d) = 0.457						
LENGTH (m)				31.7	DESIGN FLOW DEPTH = 0.141						
FLOW (l/s)				26.00	<div style="border: 1px solid black; padding: 5px;"> Head loss in manhole simplified method p. 71 (MWDM) straight through K_L=0.05 Velocity = Flow / Area = 0.37 m/s HL = K_L * V²/ 2g </div>						
HGL (m) ***		99.154	99.177	0.023							
MANHOLE COEF K=	0.05	LOSS (m)	0.000								
TOTAL HGL (m)				99.309							
MAX. SURCHARGE (mm)				-159							

FRICTION LOSS		FROM MH	TO MH	PIPE ID	MANNING FORMULA - FLOWING FULL						
Block 178		3	2		DIA (m)	Area (m ²)	Perim. (m)	Slope (%)	Hyd.R. (m)	Vel. (m/s)	Q (l/s)
INVERT ELEVATION (m)		99.298	99.342		0.2	0.03	0.63	0.600	0.05	0.81	25.54
OBVERT ELEVATION (m)		99.498	99.542		HYDRAULIC SLOPE = 0.48 %						
DIAMETER (mm)				200	DESIGN FLOW TO FULL FLOW RATIO (Q _d) = 0.000						
LENGTH (m)				7.3	DESIGN FLOW DEPTH = 0.002						
FLOW (l/s)				0.00	<div style="border: 1px solid black; padding: 5px;"> Head loss in manhole simplified method p. 71 (MWDM) straight through K_L=0.05 Velocity = Flow / Area = 0.00 m/s HL = K_L * V²/ 2g </div>						
HGL (m) ***		99.309	99.309	0.000							
MANHOLE COEF K=	0.05	LOSS (m)	0.000								
TOTAL HGL (m)				99.344							
MAX. SURCHARGE (mm)				-198							

FRICTION LOSS		FROM MH	TO MH	PIPE ID	MANNING FORMULA - FLOWING FULL						
Block 178		2	1		DIA (m)	Area (m ²)	Perim. (m)	Slope (%)	Hyd.R. (m)	Vel. (m/s)	Q (l/s)
INVERT ELEVATION (m)		99.372	99.514		0.2	0.03	0.63	0.600	0.05	0.81	25.35
OBVERT ELEVATION (m)		99.572	99.714		HYDRAULIC SLOPE = 0.72 %						
DIAMETER (mm)				200	DESIGN FLOW TO FULL FLOW RATIO (Q _d) = 0.000						
LENGTH (m)				23.7	DESIGN FLOW DEPTH = 0.002						
FLOW (l/s)				0.00	<div style="border: 1px solid black; padding: 5px;"> Head loss in manhole simplified method p. 71 (MWDM) straight through K_L=0.05 Velocity = Flow / Area = 0.00 m/s HL = K_L * V²/ 2g </div>						
HGL (m) ***		99.344	99.344	0.000							
MANHOLE COEF K=	0.05	LOSS (m)	0.000								
TOTAL HGL (m)				99.516							
MAX. SURCHARGE (mm)				-198							

Table 5.10 Storm Hydraulic Grade Line - Local Sewers within Barrett Lands Phase 3 for the 100 Year 3 Hour Chicago and 100 Year 3 Hour Chicago increased by 20% Storm Events

XPSWMM NODE	USF (M)	STORM HYDRAULIC GRADE LINE ⁽¹⁾							
		100 YEAR 3 HOUR CHICAGO				100 YEAR 3 HOUR CHICAGO + 20%			
		RARE EVENT SANITARY FLOW		ANNUAL EVENT SANITARY FLOW		RARE EVENT SANITARY FLOW		ANNUAL EVENT SANITARY FLOW	
		HGL (M)*	USF-HGL (M)	HGL (M)*	USF-HGL (M)	HGL (M)*	USF-HGL (M)	HGL (M)*	USF-HGL (M)
MH11300	95.63	93.91	1.72	93.91	1.72	93.96	1.67	93.96	1.67
MH11301	96.58	94.96	1.62	94.96	1.62	94.96	1.62	94.96	1.62
MH11302	97.98	95.70	2.28	95.70	2.28	95.71	2.27	95.71	2.27
MH11303	98.67	96.23	2.44	96.23	2.44	96.23	2.44	96.23	2.44
MH11304	99.23	96.36	2.87	96.36	2.87	96.36	2.87	96.36	2.87
MH11305	99.28	96.43	2.85	96.43	2.85	96.43	2.85	96.43	2.85
MH11311	96.38	94.90	1.48	94.90	1.48	94.90	1.48	94.90	1.48
MH11312	97.28	95.85	1.43	95.85	1.43	95.85	1.43	95.85	1.43
MH11313	97.48	96.13	1.35	96.13	1.35	96.13	1.35	96.13	1.35
MH11314	98.63	97.27	1.36	97.27	1.36	97.27	1.36	97.27	1.36
MH11315	100.93	99.51	1.42	99.51	1.42	99.51	1.42	99.51	1.42
MH11316	100.93	99.60	1.33	99.60	1.33	99.60	1.33	99.60	1.33
MH11321	97.63	96.11	1.52	96.11	1.52	96.11	1.52	96.11	1.52
MH11322	97.88	96.61	1.27	96.61	1.27	96.61	1.27	96.61	1.27
MH11323	100.08	98.80	1.28	98.80	1.28	98.80	1.28	98.80	1.28
MH11331	98.93	97.60	1.33	97.60	1.33	97.60	1.33	97.60	1.33
MH11332	98.93	97.73	1.21	97.73	1.21	97.73	1.21	97.73	1.21
MH11333	100.38	98.71	1.67	98.71	1.67	98.71	1.67	98.71	1.67
MH11334	100.88	99.24	1.64	99.24	1.64	99.24	1.64	99.24	1.64
MH11350	96.48	95.77	0.71	95.77	0.71	95.77	0.71	95.77	0.71
MH11351	96.63	95.42	1.21	95.42	1.21	95.42	1.21	95.42	1.21
MH11400	95.43	94.56	0.87	94.56	0.87	94.59	0.84	94.59	0.84
MH11401	96.03	94.85	1.18	94.85	1.18	94.89	1.14	94.89	1.14
MH11402	95.78	94.89	0.89	94.89	0.89	94.94	0.84	94.94	0.84
MH11403	96.01	95.06	0.95	95.06	0.95	95.11	0.90	95.11	0.90
MH11404	96.73	95.62	1.11	95.62	1.11	95.63	1.10	95.63	1.10
MH11405	97.13	96.03	1.10	96.03	1.10	96.03	1.10	96.03	1.10
MH11406	97.68	96.77	0.91	96.77	0.91	96.77	0.91	96.77	0.91
MH11407	98.08	97.23	0.85	97.23	0.85	97.23	0.85	97.23	0.85
MH11408	98.73	97.49	1.24	97.49	1.24	97.49	1.24	97.49	1.24

(1) HGL results were obtained from the XPSWMM models entitled 34738-202002-RARE-3CHI100.out, 34738-202002-ANN-3CHI100.out, 34738-202002-RARE-3CHI120.out or 34738-202002-ANN-3CHI120.out and enclosed as part of the digital submission.

Ditch S6		Length = 17.26 m	
New Ditch Section Required 1:100 yr. flow = 119.12 l/s 0.119 Cu m/sec			
From Seelye use n =	0.013 (Channels)		area= 0.01
choose: slope S =	12.17 %	Up Stream Ditch btm= 102.10	wpr= 0.92
	Ditch Bottom 0.00 metres	Dn Stream Ditch Blm = 100.00	
	Ditch slopes 22.00 :1	Difference = 2.10	
	Water depth 0.621 metres (depth needed to carry 0.13 Cu. M/sec)	Top Bank = 100.15	
Check Ditch Capacity (Q)	Q = 0.012 Cu M/Sec	and Velocity = 1.29 M/s	Free Board = 0.13

Ditch S6		Length = 17.26 m	
New Ditch Section Required 1:100 yr. +20% flow = 171.62 l/s 0.172 Cu m/sec			
From Seelye use n =	0.013 (Channels)		area= 0.07
choose: slope S =	12.17 %	Up Stream Ditch btm= 102.10	wpr= 2.46
	Ditch Bottom 0.00 metres	Dn Stream Ditch Blm = 100.00	
	Ditch slopes 22.00 :1	Difference = 2.10	
	Water depth 0.666 metres (depth needed to carry 0.13 Cu. M/sec)	Top Bank = 100.15	
Check Ditch Capacity (Q)	Q = 0.172 Cu M/Sec	and Velocity = 2.48 M/s	Free Board = 0.09

Ditch S10		Length = 38.00 m	
New Ditch Section Required 1:100 yr. flow = 39.68 l/s 0.040 Cu m/sec			
From Seelye use n =	0.013 (Channels)		area= 0.04
choose: slope S =	0.89 %	Up Stream Ditch btm= 101.44	wpr= 0.71
	Ditch Bottom 0.00 metres	Dn Stream Ditch Blm = 101.10	
	Ditch slopes 3.00 :1	Difference = 0.34	
	Water depth 0.113 metres (depth needed to carry 0.13 Cu. M/sec)	Top Bank = 101.25	
Check Ditch Capacity (Q)	Q = 0.040 Cu M/Sec	and Velocity = 1.03 M/s	Free Board = 0.04

Ditch S10		Length = 38.00 m	
New Ditch Section Required 1:100 yr. +20% flow = 62.7 l/s 0.063 Cu m/sec			
From Seelye use n =	0.013 (Channels)		area= 0.05
choose: slope S =	0.89 %	Up Stream Ditch btm= 101.44	wpr= 0.85
	Ditch Bottom 0.00 metres	Dn Stream Ditch Blm = 101.10	
	Ditch slopes 3.00 :1	Difference = 0.34	
	Water depth 0.135 metres (depth needed to carry 0.13 Cu. M/sec)	Top Bank = 101.25	
Check Ditch Capacity (Q)	Q = 0.064 Cu M/Sec	and Velocity = 1.16 M/s	Free Board = 0.02

Ditch S20		Length = 26.64 m	
New Ditch Section Required 1:100 yr. flow = 0 l/s 0.000 Cu m/sec			
From Seelye use n =	0.013 (Channels)		area= 0.00
choose: slope S =	3.72 %	Up Stream Ditch btm= 102.39	wpr= 0.00
	Ditch Bottom 0.00 metres	Dn Stream Ditch Blm = 101.40	
	Ditch slopes 3.00 :1	Difference = 0.99	
	Water depth 0.600 metres (depth needed to carry 0.13 Cu. M/sec)	Top Bank = 101.55	
Check Ditch Capacity (Q)	Q = 0.000 Cu M/Sec	and Velocity = 0.02 M/s	Free Board = 0.15

Ditch S20		Length = 26.64 m	
New Ditch Section Required 1:100 yr. +20% flow = 6.02 l/s 0.005 Cu m/sec			
From Seelye use n =	0.013 (Channels)		area= 0.00
choose: slope S =	3.72 %	Up Stream Ditch btm= 102.39	wpr= 0.25
	Ditch Bottom 0.00 metres	Dn Stream Ditch Blm = 101.40	
	Ditch slopes 3.00 :1	Difference = 0.99	
	Water depth 0.629 metres (depth needed to carry 0.13 Cu. M/sec)	Top Bank = 101.55	
Check Ditch Capacity (Q)	Q = 0.005 Cu M/Sec	and Velocity = 1.04 M/s	Free Board = 0.11

Ditch S20B		Length = 37.00 m	
New Ditch Section Required 1:100 yr. flow = 11.22 l/s 0.011 Cu m/sec			
From Seelye use n =	0.013 (Channels)		area= 0.01
choose: slope S =	0.86 %	Up Stream Ditch btm= 102.47	wpr= 0.44
	Ditch Bottom 0.00 metres	Dn Stream Ditch Blm = 102.15	
	Ditch slopes 3.00 :1	Difference = 0.32	
	Water depth 0.670 metres (depth needed to carry 0.13 Cu. M/sec)	Top Bank = 102.3	
Check Ditch Capacity (Q)	Q = 0.011 Cu M/Sec	and Velocity = 0.74 M/s	Free Board = 0.08

Ditch S20B		Length = 37.00 m	
New Ditch Section Required 1:100 yr. +20% flow = 16.24 l/s 0.016 Cu m/sec			
From Seelye use n =	0.013 (Channels)		area= 0.02
choose: slope S =	0.86 %	Up Stream Ditch btm= 102.47	wpr= 0.51
	Ditch Bottom 0.00 metres	Dn Stream Ditch Blm = 102.15	
	Ditch slopes 3.00 :1	Difference = 0.32	
	Water depth 0.681 metres (depth needed to carry 0.13 Cu. M/sec)	Top Bank = 102.3	
Check Ditch Capacity (Q)	Q = 0.016 Cu M/Sec	and Velocity = 0.81 M/s	Free Board = 0.07

Ditch S5		Length = 21.30 m	
New Ditch Section Required 1:100 yr. flow = 80.20 l/s 0.080 Cu m/sec			
From Seelye use n =	0.013 (Channels)		area= 0.06
choose: slope S =	1.20 %	Up Stream Ditch btm= 102.23	wpr= 0.88
	Ditch Bottom 0.00 metres	Dn Stream Ditch Blm = 102.15	
	Ditch slopes 3.00 :1	Difference = 0.08	
	Water depth 0.139 metres (depth needed to carry 0.13 Cu. M/sec)	Top Bank = 102.3	
Check Ditch Capacity (Q)	Q = 0.080 Cu M/Sec	and Velocity = 1.38 M/s	Free Board = 0.01

Ditch S5		Length = 21.30 m	
New Ditch Section Required 1:100 yr. +20% flow = 115.22 l/s 0.115 Cu m/sec			
From Seelye use n =	0.013 (Channels)		area= 0.08
choose: slope S =	1.20 %	Up Stream Ditch btm= 102.23	wpr= 1.01
	Ditch Bottom 0.00 metres	Dn Stream Ditch Blm = 102.15	
	Ditch slopes 3.00 :1	Difference = 0.08	
	Water depth 0.160 metres (depth needed to carry 0.13 Cu. M/sec)	Top Bank = 102.3	
Check Ditch Capacity (Q)	Q = 0.116 Cu M/Sec	and Velocity = 1.51 M/s	Free Board = -0.01

Ditch S4		Length = 20.10 m	
New Ditch Section Required 1:100 yr. flow = 55.81 l/s 0.056 Cu m/sec			
From Seelye use n =	0.013 (Channels)		area= 0.05
choose: slope S =	1.00 %	Up Stream Ditch btm= 102.47	wpr= 0.86
	Ditch Bottom 0.00 metres	Dn Stream Ditch Blm = 102.23	
	Ditch slopes 3.00 :1	Difference = 0.24	
	Water depth 0.126 metres (depth needed to carry 0.13 Cu. M/sec)	Top Bank = 102.3	
Check Ditch Capacity (Q)	Q = 0.056 Cu M/Sec	and Velocity = 1.18 M/s	Free Board = -0.06

Ditch S4		Length = 20.10 m	
New Ditch Section Required 1:100 yr. +20% flow = 81.45 l/s 0.081 Cu m/sec			
From Seelye use n =	0.013 (Channels)		area= 0.06
choose: slope S =	1.00 %	Up Stream Ditch btm= 102.47	wpr= 0.92
	Ditch Bottom 0.00 metres	Dn Stream Ditch Blm = 102.23	
	Ditch slopes 3.00 :1	Difference = 0.24	
	Water depth 0.145 metres (depth needed to carry 0.13 Cu. M/sec)	Top Bank = 102.3	
Check Ditch Capacity (Q)	Q = 0.081 Cu M/Sec	and Velocity = 1.29 M/s	Free Board = -0.08

Ditch R6		Length = 116.00 m	
New Ditch Section Required 1:100 yr. flow = 97.7 l/s 0.098 Cu m/sec			
From Seelye use n =	0.013 (Channels)		area= 0.12
choose: slope S =	0.70 %	Up Stream Ditch btm= 101.34	wpr= 2.70
	Ditch Bottom 0.00 metres	Dn Stream Ditch Blm = 100.81	
	Ditch slopes 15.00 :1	Difference = 0.53	
	Water depth 0.690 metres (depth needed to carry 0.13 Cu. M/sec)	Top Bank = 102.3	
Check Ditch Capacity (Q)	Q = 0.098 Cu M/Sec	and Velocity = 0.81 M/s	Free Board = 1.40

Ditch R6		Length = 116.00 m	
New Ditch Section Required 1:100 yr. +20% flow = 140.55 l/s 0.141 Cu m/sec			
From Seelye use n =	0.013 (Channels)		area= 0.16
choose: slope S =	0.70 %	Up Stream Ditch btm= 101.34	wpr= 3.10
	Ditch Bottom 0.00 metres	Dn Stream Ditch Blm = 100.81	
	Ditch slopes 15.00 :1	Difference = 0.53	
	Water depth 0.183 metres (depth needed to carry 0.13 Cu. M/sec)	Top Bank = 102.3	
Check Ditch Capacity (Q)	Q = 0.142 Cu M/Sec	and Velocity = 0.89 M/s	Free Board = 1.39

Q = A*(1.49)*R^{2/3}*S^{1/2} where: A = cross sectional area in Sq. m



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PROJECT: BARRETT BLOCK 178
DATE: 2022-10-27
FILE: 135925.6.04
REV #: 2
DESIGNED BY: Anton Chetrar
CHECKED BY: Ryan Magladry

TEMPORARY ICD ORIFICE SIZING

Orifice coefficients	
Cv =	0.60
Cv =	0.65

	Invert (m)	Diameter (mm)	Centre ICD (m)	Max. Pond Elevation (m)	Hydraulic Slope (m)	Target Flow (l/s)	Theoretical		Recommended	
							Orifice (m)	Actual Flow (l/s)	Orifice (m)	Actual Flow (l/s)
SANITARY MH	95.810	200	95.910	101.40	2.000	1.99	0.0230	1.99	0.075	21.14
STORM MH	96.180	675	96.518	101.40	2.000	136.00	0.1905	136.40	0.190	135.68

* minimum orifice size to be 0.075m



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PROJECT: BARRETT BLOCK 178
DATE: 2022-10-27
FILE: 135925.6.04
REV #: 1
DESIGNED BY: Anton Chetrar
CHECKED BY: Ryan Magladry

ICD ORIFICE SIZING

Orifice coefficients	
Cv =	0.60
Cv =	0.65

	Invert (m)	Diameter (mm)	Centre ICD (m)	Max. Pond Elevation (m)	Hydraulic Slope (m)	Target Flow (l/s)	Theoretical		Recommended	
							Orifice (m)	Actual Flow (l/s)	Orifice (m)	Actual Flow (l/s)
CB4	100.730	200	100.830	102.23	1.400	26.00	0.0910	26.04	0.090	25.47
CB5	100.630	200	100.730	102.13	1.400	25.00	0.0892	25.02	0.089	24.91
CB6	100.480	200	100.580	102.10	1.520	50.00	0.1236	50.06	0.123	49.57
CB9	99.870	200	99.970	101.33	1.360	16.50	0.0730	16.50	0.075	17.43
CB9A	99.870	200	99.970	101.33	1.360	16.50	0.0730	16.50	0.075	17.43
CB20	100.750	200	100.850	102.39	1.540	15.00	0.0675	15.00	0.075	18.55
CB20A	100.970	200	101.070	102.47	1.400	10.00	0.0564	10.00	0.075	17.69
RYCB7	99.850	250	99.975	101.40	1.425	20.00	0.0794	20.00	0.079	19.80

* minimum orifice size to be 0.075m

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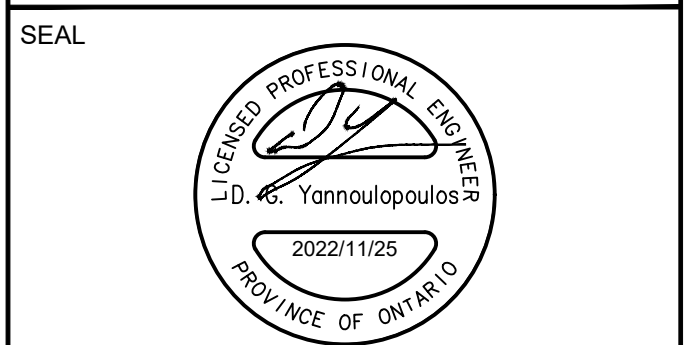
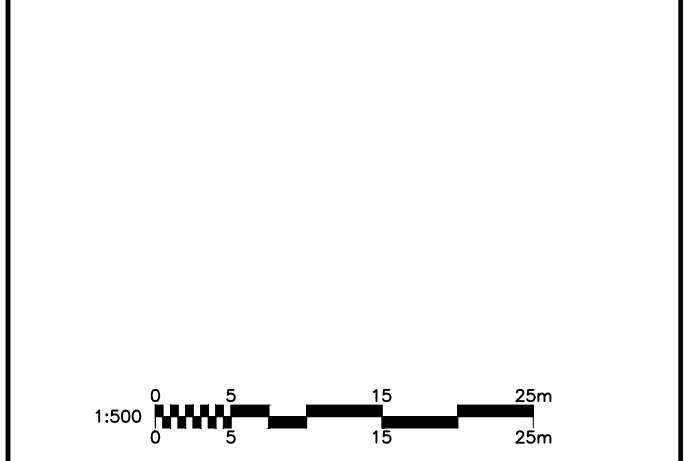
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SEE 010, 011, 012 FOR NOTES, LEGEND, CB TABLE, STREET SECTIONS AND DETAILS.



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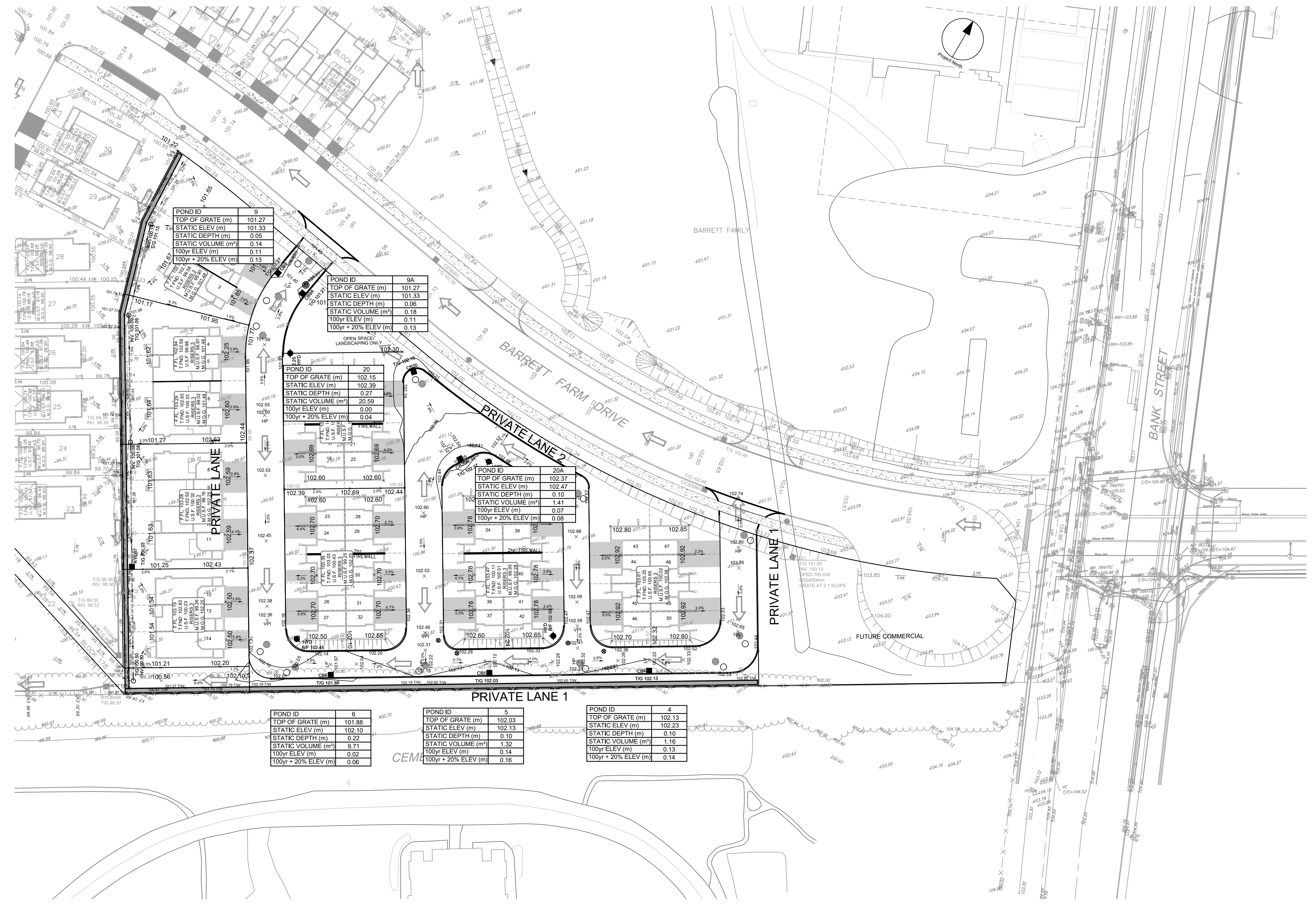
PROJECT
BARRETT BLOCK 178

PROJECT NO:
135925

DRAWN BY: M.M.	CHECKED BY: A.C.
PROJECT MGR: R.M.	APPROVED BY: J.I.M.

SHEET TITLE
PONDING PLAN

SHEET NUMBER 600	ISSUE 2
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POND ID	9
TOP OF GRATE (m)	101.27
STATIC ELEV (m)	101.33
STATIC DEPTH (m)	0.06
STATIC VOLUME (m³)	0.14
100yr ELEV (m)	0.11
100yr + 20% ELEV (m)	0.13

POND ID	9A
TOP OF GRATE (m)	101.27
STATIC ELEV (m)	101.33
STATIC DEPTH (m)	0.06
STATIC VOLUME (m³)	0.18
100yr ELEV (m)	0.11
100yr + 20% ELEV (m)	0.13

POND ID	20
TOP OF GRATE (m)	102.15
STATIC ELEV (m)	102.39
STATIC DEPTH (m)	0.27
STATIC VOLUME (m³)	20.59
100yr ELEV (m)	0.00
100yr + 20% ELEV (m)	0.04

POND ID	20A
TOP OF GRATE (m)	102.37
STATIC ELEV (m)	102.47
STATIC DEPTH (m)	0.10
STATIC VOLUME (m³)	1.41
100yr ELEV (m)	0.07
100yr + 20% ELEV (m)	0.08

POND ID	6
TOP OF GRATE (m)	101.88
STATIC ELEV (m)	102.10
STATIC DEPTH (m)	0.22
STATIC VOLUME (m³)	9.71
100yr ELEV (m)	0.02
100yr + 20% ELEV (m)	0.06

POND ID	5
TOP OF GRATE (m)	102.03
STATIC ELEV (m)	102.13
STATIC DEPTH (m)	0.10
STATIC VOLUME (m³)	1.32
100yr ELEV (m)	0.14
100yr + 20% ELEV (m)	0.16

POND ID	4
TOP OF GRATE (m)	102.13
STATIC ELEV (m)	102.23
STATIC DEPTH (m)	0.10
STATIC VOLUME (m³)	1.16
100yr ELEV (m)	0.13
100yr + 20% ELEV (m)	0.14

CITY FILE No. D07-12-22-0112
Last Saved: May 8, 2022, by rmline
Plotted: Thursday, November 24, 2022 11:57:00 AM by Marian Milne

DRAINAGE AREA ID	CONTINUOUS /SAG ⁽¹⁾⁽²⁾	ROAD TYPE	MINOR SYSTEM DESIGN TARGET (BASED ON ROAD TYPE)		MINOR SYSTEM RE-STRICITION (L/S)	NOTES ⁽³⁾
			MINOR SYSTEM DESIGN STORM	GENERATED FLOW ON INDIVIDUAL SEGMENT SIMULATED (L/S)		
R11315	Rear Yard	Rear Yard	5	12	17	
R11321	Rear Yard	Rear Yard	5	73	80	
R11323	Rear Yard	Rear Yard	5	36	40	
R11331	Rear Yard	Rear Yard	5	36	40	
R11332	Rear Yard	Rear Yard	5	17	17	
R11334	Rear Yard	Rear Yard	5	10	17	
R11333	Rear Yard	Rear Yard	5	11	16	
R11350A	Rear Yard	Rear Yard	5	7	17	
R11350B	Rear Yard	Rear Yard	5	31	40	
R11400A	Rear Yard	Rear Yard	5	6	6	
R11400B	Rear Yard	Rear Yard	5	25	80	
R11401	Rear Yard	Rear Yard	5	44		
R11402	Rear Yard	Rear Yard	5	38	45	
R11403A	Rear Yard	Rear Yard	5	34	40	
R11403B	Rear Yard	Rear Yard	5	33	40	
R11403C	Rear Yard	Rear Yard	5	6		
R11406	Rear Yard	Rear Yard	5	19	22	
Total restricted flow (ICD flow) – Rear Yard Segments (L/s)					756	
External Areas						
R11304	Rear Yard	Rear Yard	2	234	234	
R11305	Rear Yard	Rear Yard	2	581	581	
R11340	Rear Yard	Rear Yard	2	287	287	

(1) Capture on continuous grade is limited to capacity of grate.

(2) The minor flow restriction has been increased in sags to allow full capture of overflow from upstream segments on continuous grade during the design storm event without ponding.

(3) Where CB lead restricts flow, lead diameter is specified in the CB Data Table.

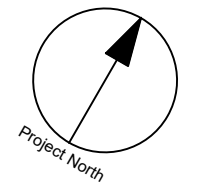
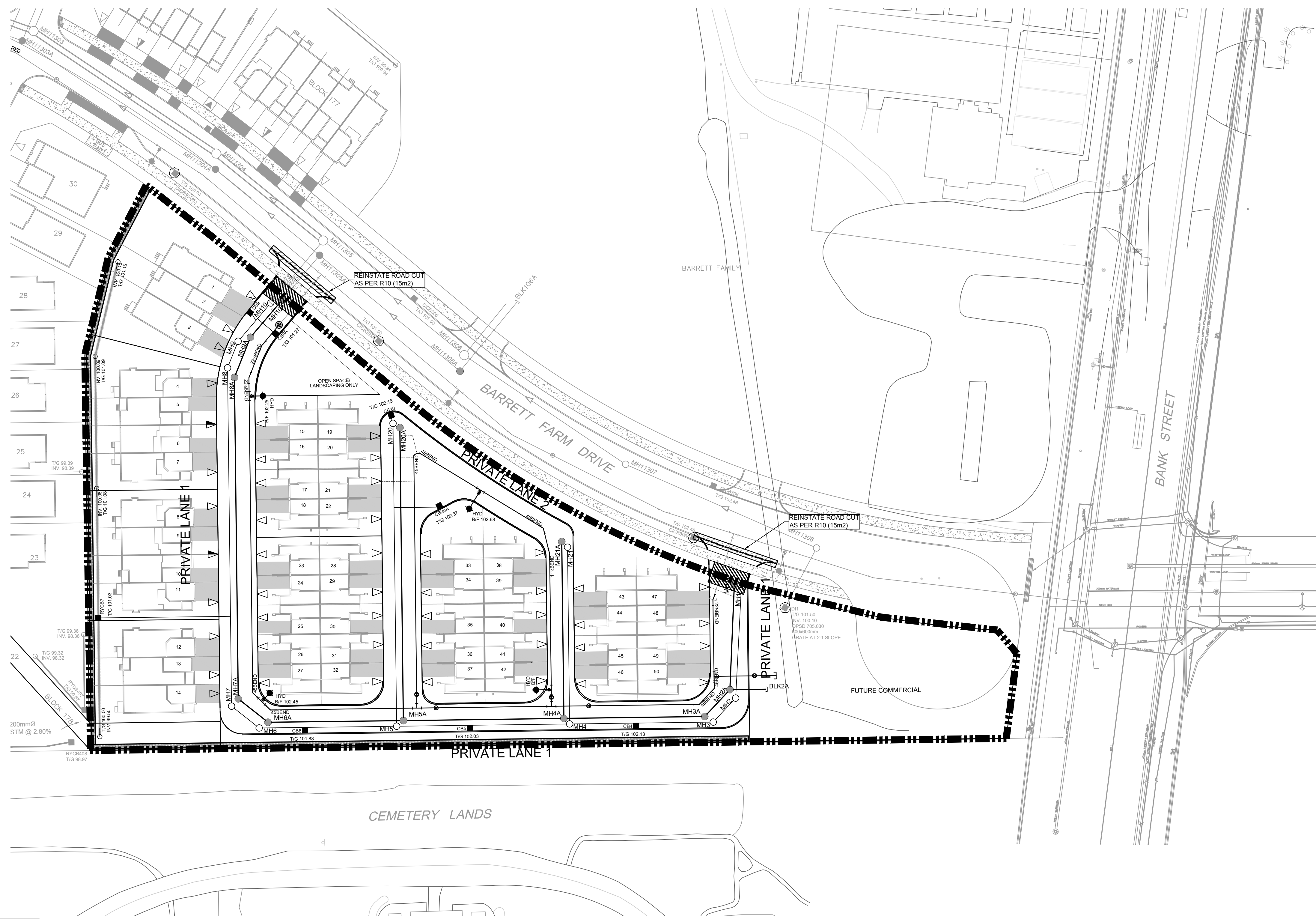
The storage available on-site and its maximum depth and the results of the DDSWMM evaluation for the subject site are presented in **Table 5.4**. The ponding plan for the subject site is presented on **Drawing 34731-620**. The DDSWMM output files are presented in **Appendix E**.

Table 5.4 Summary of On-Site Storage during the Target Minor System Design Storm

DRAINAGE AREA ID	MINOR SYSTEM DESIGN STORM	AVAILABLE STATIC STORAGE (CU-M)	TOTAL STORAGE USED (CU-M)	OVERFLOW (L/S)
S11304A	5	5.67	0	0
S11304B	5	6.17	0	0
S11401A	2	38.98	0	0
S11401B	2	7.14	0	0
S11400	2	20.04	0	0
S11311A	2	6.22	0	0

APPENDIX E

135925-900 - Erosion and Sediment Control Plan
135925-200 - Grading Plan



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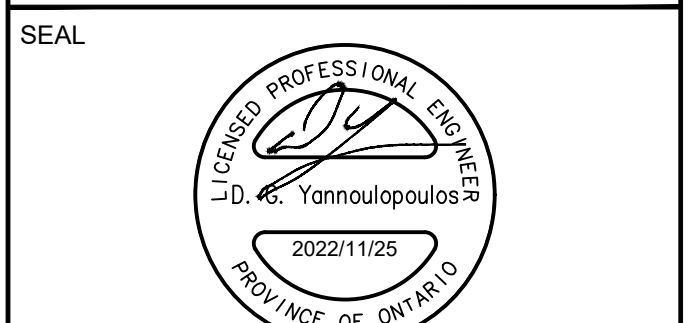
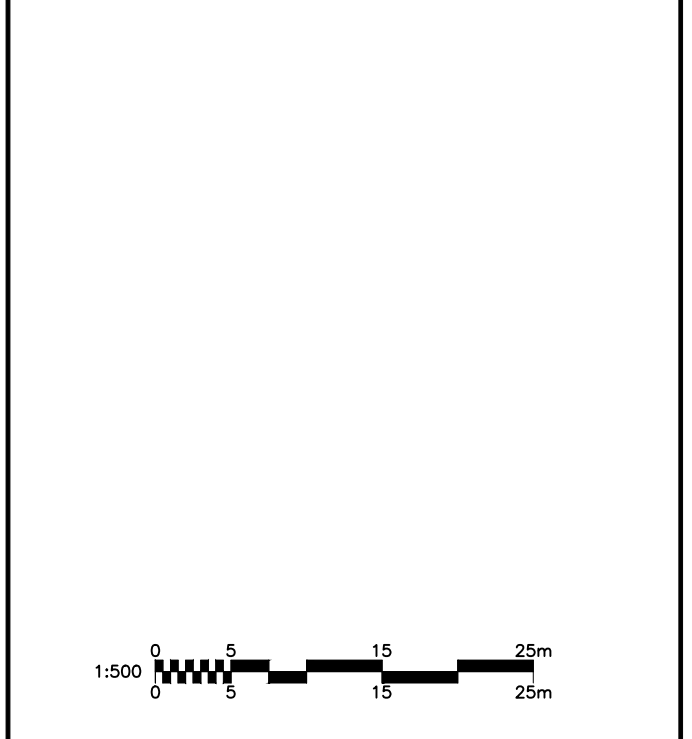
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PROJECT
BARRETT BLOCK 178

PROJECT NO:
135925

DRAWN BY:
M.M.

CHECKED BY:
A.C.

PROJECT MGR:
R.M.

APPROVED BY:
J.I.M.

SHEET TITLE
SEDIMENT AND EROSION CONTROL PLAN

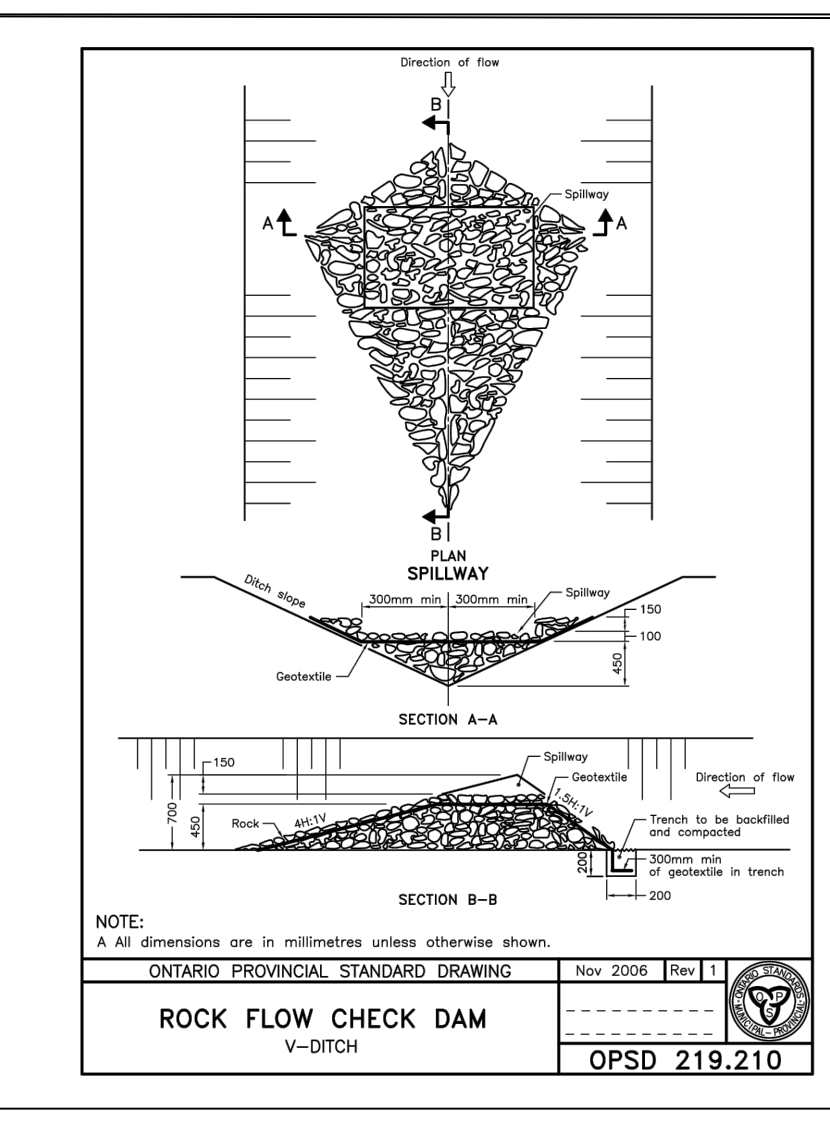
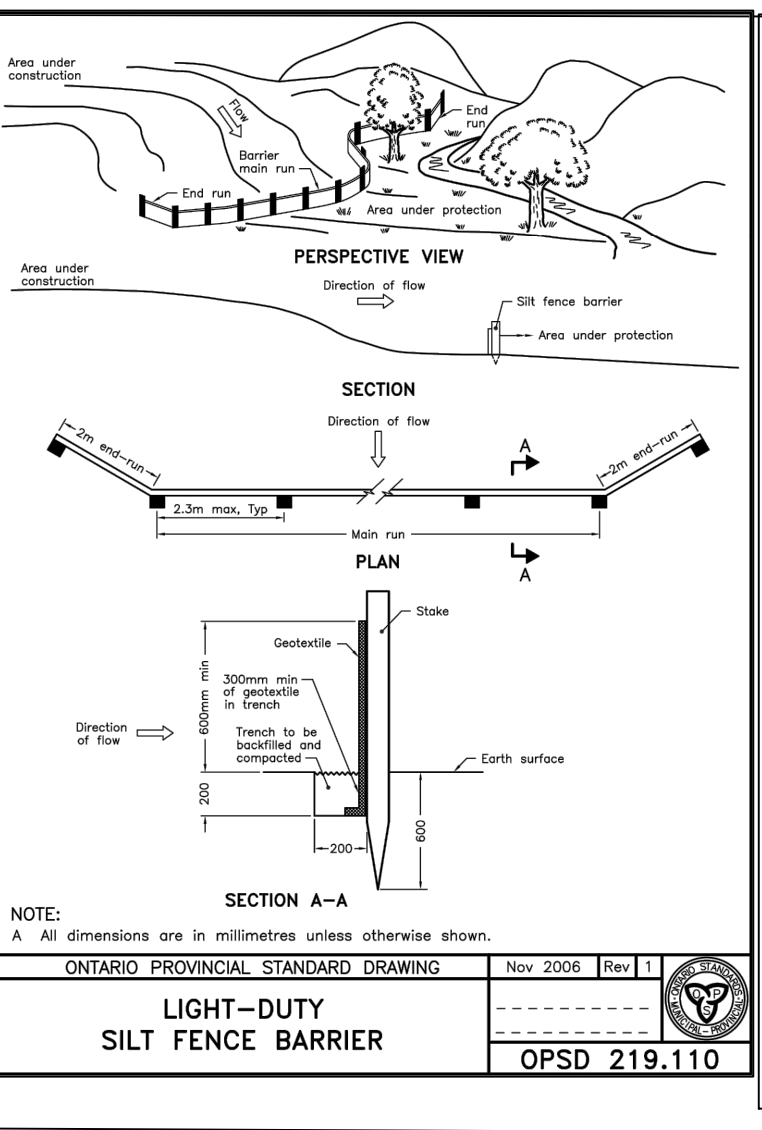
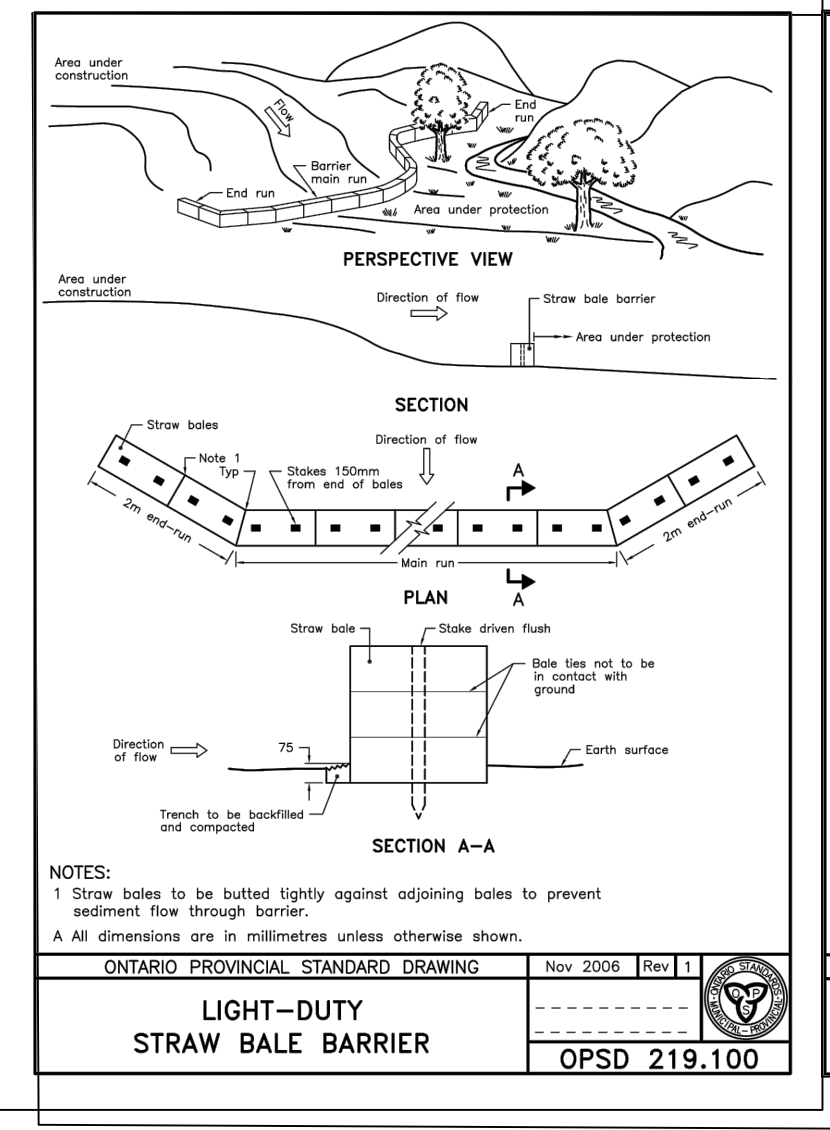
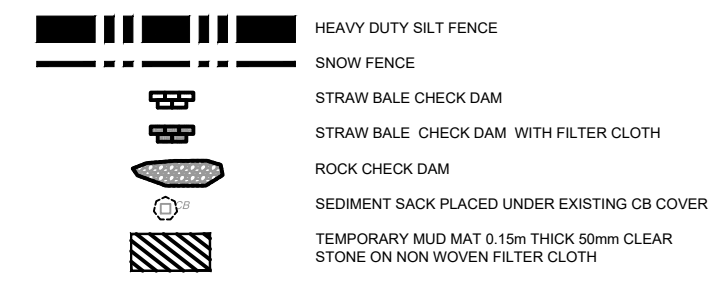
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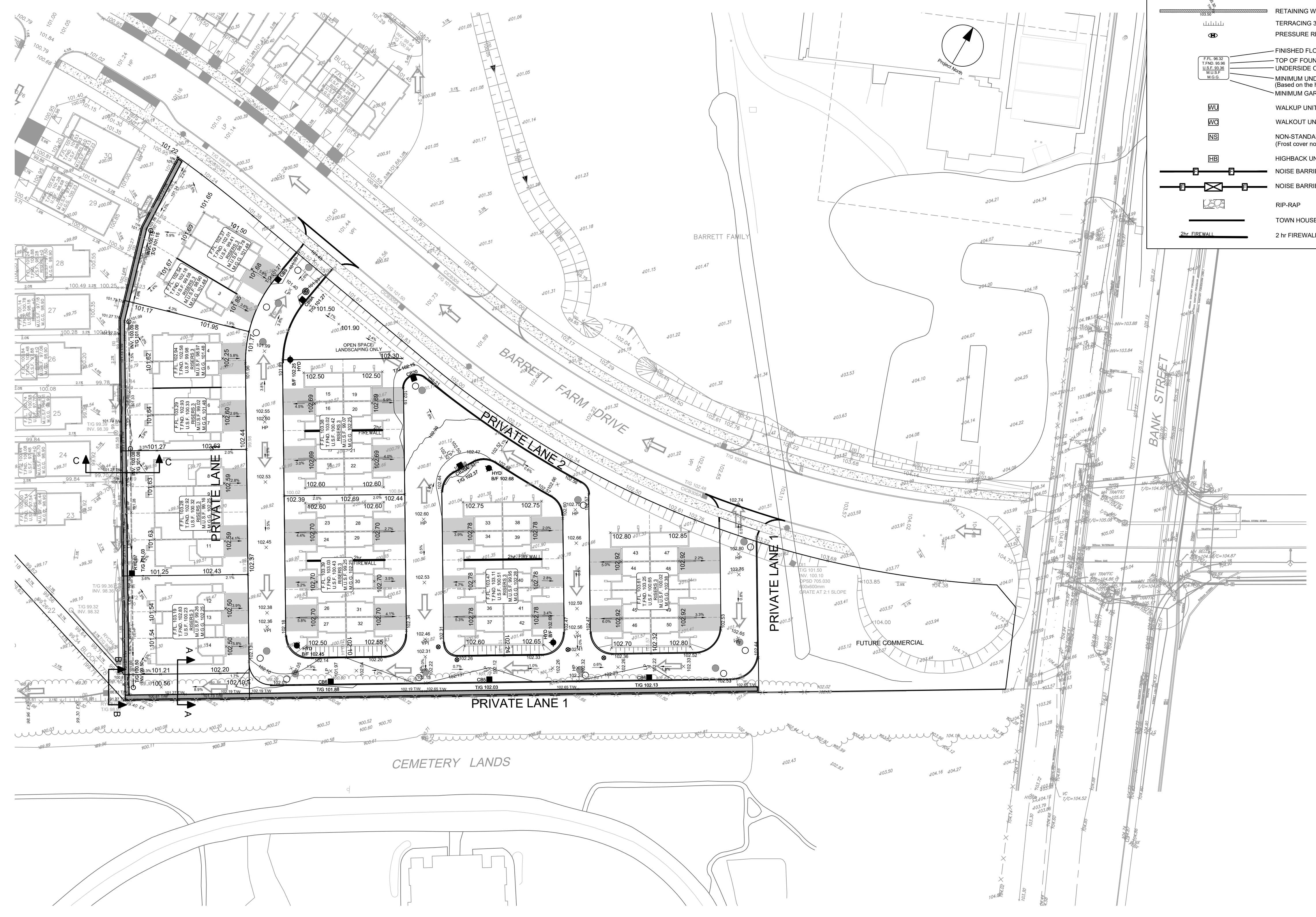
ISSUE
2

NOTES:

- SILT FENCE TO BE ERECTED PRIOR TO EARTH WORKS BEING COMMENCED. SILT FENCE TO BE MAINTAINED UNTIL VEGETATION IS ESTABLISHED OR UNTIL START OF SUBSEQUENT PHASE.
- STRAW BALE SEDIMENT TRAPS TO BE CONSTRUCTED IN EXISTING ROAD SIDE DITCHES. TRAPS TO REMAIN AND BE MAINTAINED UNTIL VEGETATION IS ESTABLISHED.
- SILT SACK TO BE PLACED AND MAINTAINED UNDER COVER OF ALL CATCHBASINS. GEOTEXTILE SILT SACK IN STREET C&B TO REMAIN UNTIL ALL CURBS ARE CONSTRUCTED. GEOTEXTILE FABRIC IN RYCBs TO REMAIN UNTIL VEGETATION IS ESTABLISHED. ALL CATCHBASINS TO BE REGULARLY INSPECTED AND CLEANED, AS NECESSARY, UNTIL SOD AND CURBS ARE CONSTRUCTED.
- CONTRACTOR TO PROVIDE DETAILS ON LOCATION(S) AND DESIGN OF DEWATERING TRAP(S) PRIOR TO COMMENCING WORK. CONTRACTOR ALSO RESPONSIBLE FOR MAINTAINING TRAP(S) AND ADJUSTING SIZE(S) IF DEEMED REQUIRED BY THE ENGINEER DURING CONSTRUCTION.
- CONTRACTOR TO PROTECT EXISTING CATCHBASINS WITH FILTER CLOTH UNDER THE COVERS TO TRAP SEDIMENTATION. REFER TO IDENTIFIED STRUCTURES.
- IF DEEMED NECESSARY BY CITY INSPECTOR, LOCATION OF MUD MATS TO BE DETERMINED IN THE FIELD.
- CONTRACTOR IS RESPONSIBLE FOR THE INSTALLATION, INSPECTION, MAINTENANCE AND REMOVAL OF ALL CONTROL MEASURES.
- THIS PLAN IS A "LIVING DOCUMENT" WHICH MAY BE REVISED IN THE EVENT THE CONTROL MEASURES ARE INSUFFICIENT.

SEDIMENT EROSION LEGEND





GRADING LEGEND

- PROPOSED DITCH CW FLOW DIRECTION AND SLOPE
- SLOPE CW FLOW DIRECTION
- MAJOR OVERLAND FLOW ROUTE
- PROPOSED SPOT GRADE
- PROPOSED SWALE GRADE
- PROPOSED SWALE HIGH POINT GRADE
- LOT CORNER GRADE CW EXISTING GRADE
- FULL STATIC PONDING GRADE
- RETAINING WALL CW TOP OF WALL AND GRASS GRADE
- TERRACING 3:1 MAXIMUM UNLESS NOTED OTHERWISE
- PRESSURE REDUCING VALVE
- FINISHED FLOOR ELEVATION
- TOP OF FOUNDATION ELEVATION
- UNDERSIDE OF FOOTING ELEVATION
- MINIMUM UNDERSIDE OF FOOTING (Based on the higher of the sewer overbays, or hydraulic grade line)
- MINIMUM GARAGE GRADE
- WALKUP UNIT
- WALKOUT UNIT
- NON-STANDARD FOUNDATION (Frost cover not provided for standard unit)
- HIGHBACK UNIT (1.5m frost cover on footings)
- NOISE BARRIER LOCATION
- NOISE BARRIER GATE
- RIP-RAP
- TOWN HOUSE SPLITS
- 2 hr FIREWALL

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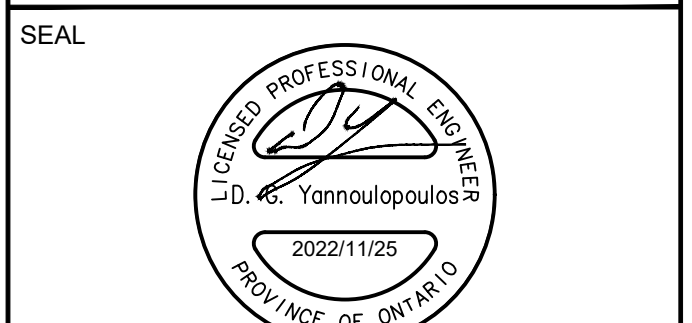
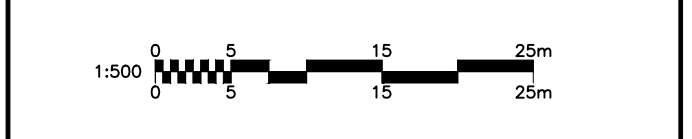
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PROJECT
BARRETT BLOCK 178

PROJECT NO:
135925

DRAWN BY: M.M.	CHECKED BY: A.C.
PROJECT MGR: R.M.	APPROVED BY: J.I.M.

SHEET TITLE
GRADING PLAN

SHEET NUMBER 200	ISSUE 2
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File Location: \\135925_Barrett\ndr\0_Production\7_03_Design\04_Civil\Sheet\200 GRADING PLAN.dwg Last Saved: November 2, 2022, by rmlme Printed: Thursday, November 24, 2022, 11:55:49 AM by Marim Milne
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 CITY PLAN No. 18826